

RESEARCH ARTICLE

Numerical modelling of the spatial variation of sediment transport using wave climate schematisation method - a case study of west coast of Sri Lanka

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Abstract: This study quantifies the variations in wave characteristics and the resulting variations in potential longshore sediment transport rate along the coastline between Mount Lavinia and Negombo, Sri Lanka. Over the last 25 years, this coastal belt has been subjected to dramatic interventions due to the influence of rapid socio-economic development in the country such as construction of the Colombo South Harbor jetty, ongoing Colombo Port City Project and mega sand dredging off Negombo coast. For the wave transformation, SWAN (Simulating Waves Nearshore) numerical model was applied, forced by offshore wave/wind. The Delft3D-FLOW model was used to estimate the longshore sediment transport rates and related morphodynamics using input reduction and morphological acceleration techniques. Results of the alongshore sediment transport capacity computations clearly indicate the variable characteristics of different parts of the study zone. The annual alongshore sediment transport capacity computed in the study area oriented northward, complying very well with the observations. The coastal belt between Mount Lavinia and Colombo, the wave climate, and subsequently the annual alongshore transport reached the highest values indicating a relative dynamic environment and thereafter decreased with a strong gradient northward. The explanation for these negative steep gradients and the environmental forcing/human interventions that govern the regional sediment transport are discussed in this paper.

Keywords: ERA Interim, morphodynamics, morphological acceleration, sediment transport, SWAN.

INTRODUCTION

The ever-increasing economic and environmental considerations of coastal zones have provoked further studies of the variety of coastal processes such as coastal erosion, deposition and sediment transportation. Development within the coastal areas has increased interest in management of coastal erosion and restoration of coastal capacity to accommodate short- and long-term changes induced by human activities, extreme events and sea level rise. Coastal erosion problem becomes worse whenever the countermeasures (i.e. hard or soft structural options) are inappropriately applied, improperly designed, built, or maintained and if the effects on adjacent shores are not carefully evaluated. Often erosion is addressed locally at specific places or at regional or jurisdictional boundaries instead of at system boundaries that reflect natural processes. Human activities along the coast (land reclamation, port development, shrimp farming) and offshore (dredging, sand mining) in combination with these natural forces often exacerbate coastal erosion in many places and jeopardise opportunities for the coasts to fulfil their socio-economic and ecological roles in the long term at a reasonable societal cost. Moreover, the Western and Southern provinces are linked by two of the country's busiest highways and railway lines, touching the coast along most of their length (Garcin *et al.*, 2008).

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This particular coastal stretch has been identified as one of the highly dynamic coastal regions due to its facing that receives the dominant southwest swell and monsoonal wind and wave conditions (Ranasinghe *et al.*, 2011, Jayathilaka, 2015). The Colombo Port City Development Project and the off-shore sand mining site off Negombo are also located in this coastal stretch. Moreover, Sri Lanka's west coast is the island's front door as it is the most developed, populous and most busiest region in the country, whereas contribution of the coastal sector implied the increasing economic activity in the gross domestic product (GDP) (CC&CRMD, 2004).

In 1992, a study on the littoral sediment transport along the southwest coast of Sri Lanka was conducted as a part of the master plan of Coast Conservation Department (CCD), performed under the CCD-German Agency for Technical Cooperation (GTZ) coastal conservation project. CCD-GTZ used five different sediment transport formulas [CERC, Bijker, Van Rijn, Bailard and Engelund-

Hansen (E-H)]. In general, the results predicted that the relations of net transport calculated with both CERC and Bailard formulas for the entire coastal stretch from Hikkaduwa to Colombo are close to one million cubic metres per year, whereas Bijker and E-H showed figures of two million cubic metres per year or more (Fittschen *et al.*, 1992). According to CCD-GTZ study, the long shore transport trend along the southwest coast observed nearly a constant pattern for all different formulas.

According to the CC & CRMD report in 2004, the transport gradient along the southwest coast has been balanced by the sand supply from the rivers. Furthermore, the high deficit in the littoral budget due to sand mining in the Maha Oya causes the severe erosion north of Negombo. Studies conducted by the Central Environmental Authority (CEA) and the CCD in 1992–1997 estimated a total sand transport of 400,000 m³/year and the annual volume of sand mined from Kelani River to be 800,000 m³/year. Based on data from previous

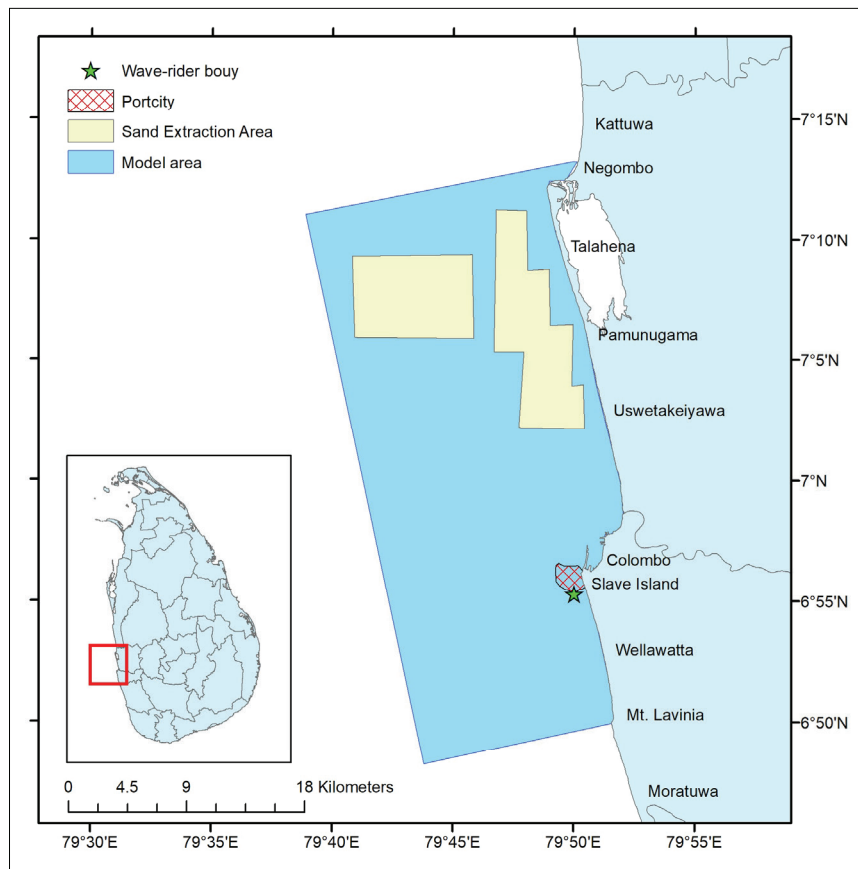


Figure 1: Map showing the study area along with the wave-rider buoy locations (green star) in Colombo Port

studies on sand supply to the coast and the Coastal Resources Management Project-1999, the estimated sand supply to the coast is 100,000 m³/year representing a significant decline. A study conducted in 1999 (CC & CRMD, 2004) estimated that the sediment outflow from the Kelani River would further decline by 40 % in the next 12 years. A study conducted by the University of Moratuwa, Sri Lanka (Ansaf, 2012) estimated coastal transport rates of the various coastal segments between Galle to Colombo using MIKE21 modelling (developed by the Danish Hydraulics Institute). The results of MIKE21 showed the northward net transport rate along the coastline from Galle to Colombo.

The main objectives of the present study were to (1) obtain a better understanding of near shore wave climate and (2) quantify the alongshore sediment transport rates along the west coast from Mount Lavinia to Negombo (Figure 1).

METHODOLOGY

Model description and settings

Numerical simulations were carried out by means of the process-based model Delft3D to obtain state-of-the-art estimates of the annual longshore sediment transport rates. The Delft3D software developed by the Deltares, Netherlands is a world leading 3D modelling suite to investigate hydrodynamics, sediment transport and morphology and water quality for fluvial, estuarine and coastal environments. The applications of Delft3D have proven its capabilities on many places around the world, like The Netherlands, USA, Hong Kong, Singapore, Australia, Venice, UAE, etc. In the context of Sri Lanka, Delft3D applications are hardly found except for a few collaboration studies of foreign experts (Jayathilaka, 2015; Duong *et al.*, 2016).

Delft3D combines a short-wave driver (SWAN), a 2DH flow module, a sediment transport model (Van Rijn & Boer, 2006), and a bed level update scheme that solves the 2D sediment continuity equation (Hydraulics, 1999). In particular, the hydrodynamic and sediment transport module Delft3D-FLOW, and the wave module Delft3D-WAVE were used (Hydraulics, 2006; Lesser, 2009; Giardino *et al.*, 2010). The Delft3D-FLOW and Delft3D-WAVE exchange information by means of online coupling.

We start from a bathymetry, given on a detailed two-dimensional grid (in case of area models) or one dimension

(in case of coastline or coastal profile models). Wave and current fields, which usually interact together, were predicted by the given boundary conditions for waves and currents, these processes determine the sediment transport. The sediment transport gradients lead to bottom changes, which then feedback into the bathymetry, the currents and waves and the sediment transports etc. Delft3D wave parameters and coefficients such as depth-induced breaking, non-linear triad interactions and bottom friction were applied and checked on the wave runs. Further, different processes such as wind and wave growth, white-capping, quadruplet's interaction and refraction were activated and de-activated to understand their effect on the results of Delft3D.

Sediment transport formula

The sediment transport and morphology module of Delft3D supports both bed load and suspended load transport of non-cohesive sediments and suspended load of cohesive sediments (Lesser *et al.*, 2004). Sediment transport algorithms, predominantly based on the formulations of Van Rijn (1993), were added to the Delft3D-FLOW hydrodynamic solver which is widely used, well tested, and well suited to modelling the three-dimensional hydrodynamics of coastal regions (Hydraulics, 2006). The settling velocity of a non-cohesive ('sand') sediment fraction was computed following the method of Van Rijn (1993).

The suspended load transport can be determined by depth-integration of the product of sand concentration and fluid velocity from the top of the bed load layer (at about 0.01 m above the bed) to the water surface. Herein, the net (averaged over the wave period) total sediment transport is obtained as the sum of net bed-load (q_b) and net suspended load (q_s) transport rates, as follows:

$$q_{tot} = q_b + q_s \quad \dots(1)$$

The net bed-load transport rate in conditions with uniform bed material was obtained by time-averaging (over the wave period T) of the instantaneous transport rate using a bed-load transport formula (quasi-steady approach), as follows:

$$q_b = \left(\frac{1}{T}\right) \cdot \int q_{b,t} dt \quad \dots(2)$$

$$q_b = \gamma \cdot \rho_s \cdot d_{50} \cdot D_*^{-0.3} \left[\tau_{b,cw} / \rho \right]^{0.5} \left[(\tau_{b,cw} - \tau_{b,cr}) / \tau_{b,cr} \right]^\eta \quad \dots(3)$$

in which, $\hat{\tau}_{b,cw}$: instantaneous grain-related bed-shear stress due to both currents and waves, $\hat{\tau}_{b,cw} = 0.5 \rho \hat{f}_{cw} (U_{\delta,cw})^2$, $U_{\delta,cw}$: instantaneous velocity due to currents and waves at edge of wave boundary layer, \hat{f}_{cw} : grain friction coefficient due to currents and waves, $\hat{f}_{cw} = \alpha \beta \hat{f}_c + (1 - \alpha) \hat{f}_w$, \hat{f}_c : current-related grain friction coefficient, \hat{f}_w : wave-related grain friction coefficient, α : coefficient related to relative strength of wave and current motion, β : wave-current-interaction coefficient, $\tau_{b,cr}$: critical bed-shear stress according to Shields, ρ_s : sediment density, ρ : fluid density, d_{50} : particle size, D_* : dimensionless particle size, γ : coefficient = 0.5, η : exponent = 1.

The net time-averaged depth-integrated suspended sand transport is defined as the sum of the net current-related ($q_{s,c}$) and the net wave-related ($q_{s,w}$) transport components, as follows (Van Rijn 2013):

$$q_s = q_{s,c} + q_{s,w} = \int (v \cdot c) dz + \int \langle (V - v)(C - c) \rangle dz \quad \dots(4)$$

in which: $q_{s,c}$: time-averaged current-related suspended sediment transport rate, $q_{s,w}$: time-averaged wave-related suspended sediment transport rate, v : time-averaged velocity, V : instantaneous velocity vector, C : instantaneous concentration, c : time-averaged concentration and $\langle \rangle$: averaging over time, \int : the integral from the top of bed-load layer to the water surface.

Model area, domain and bathymetry

In order to achieve the resolution needed we applied an overall model and nesting to a detailed model. The large-scale wave grid with lowest resolution (Figure 2 a, grid in blue color) was forced with measured schematised time series of wave heights, periods and directions of the Era-Interim at offshore boundary. The model output of the large-scale wave grid will then be used as the boundary conditions of the smaller hydrodynamic grid with higher resolution (Figure 2 b, grid in green colour). As a grid type, structured mesh grids were constructed. As is standard practice, wave domains (overall model) were created larger than flow domains (nested detail model) to avoid any wave shadowing effects at the lateral boundaries. The flow model domain covers an area of approximately 50 km \times 20 km alongshore and cross-shore, respectively. The depth was extended up to a maximum 30 m water depth. The cross-shore resolution of the computational flow grid increases from about 500 m offshore to 25 m near the coast; the alongshore resolution is 150 m. The seaward model boundary of the computation grid of the wave model is about 50 km from the coast. The computation grid of the wave model is on average four times coarser than the grid of flow model with a cross-shore resolution increase from 2000 m offshore to 200 m near the coast and an alongshore resolution is 1500 m.

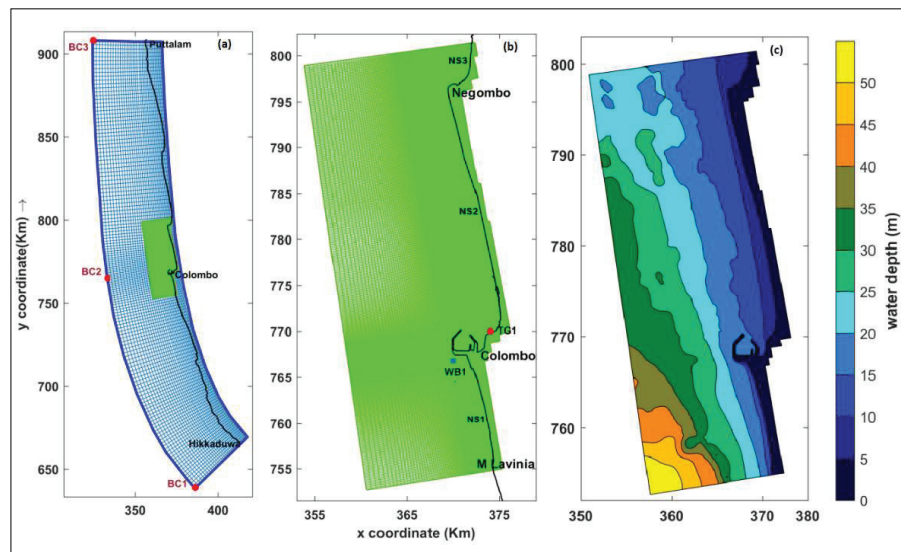


Figure 2: (a) Computational grid for wave model (blue lines), BC1-3 (red dots) was used to derive boundary conditions for offshore. (b) Curvilinear structured nesting grid for flow model (green lines). WB1 is wave measurements (blue dot) that was used to calibrate the wave model. TG1 is tide gauge data at Mutwal, Colombo. NS1-3 are points of nearshore wave extraction. (c) Bed elevations of hydrodynamic flow model (Colour bar indicates elevation: positive- water in meters w.r.t. MSL).

Bathymetry data was divided into offshore and near-shore where near-shore data was divided into two parts: bathymetric and topographic data. For offshore bathymetry, GEBCO, 30 arc second grid resolution data was used. GEBCO was largely generated by combining quality-controlled ship depth soundings with interpolation between sounding points guided by satellite-derived gravity data. Nearshore bathymetric data was generated by integrating existing bathymetric data from various sources such as sounding data from the Hydrographic Department of the National Aquatic Resources Research and Development Agency (NARA), CCD and large scale nautical charts.

Wave climate schematisation

ERA-Interim of the European Centre for Medium-Range Weather Forecasts (ECMWF) were used for the wave boundary of the model. Era interim gives 6 hourly intervals of data over a $0.75^\circ \times 0.75^\circ$ grid resolution. ERA-Interim records between 1979 and 2016 were chosen from three off-shore locations (BC1 to BC3) to derive time-varying and space-varying wave boundary conditions as seen in Figure 2 (a). The off-shore wave boundary points were chosen as sufficient distances from the coast to reduce the effect of shadowing. Usually, significant wave height and mean wave direction are considered into two dimensional classes. In order to schematise scenarios, a location has to be defined as a reference location (in this case BC2) and the occurrences of all other locations (BC1 and BC3) are coupled to the reference location on the basis of simultaneous occurrence. The number of records fall in each class is determined as the average

value of the scenario, hence the occurrence probability of the class for defined offshore point over the given period.

Morphologically insignificant waves (i.e., the wave heights less than 0.5 m) and offshore directed waves were removed from the data. Those events represented 39 % of the total wave climate and the remaining 61 % of data was applied to schematise the wave records of 37 years to 16 wave conditions using the concept of the wave energy flux method (EFM). Benedet *et al.* (2016) provided a detailed description on EFM, showing the better performance in their analysis comparing to other schematisation methods such as Opti, Fixed bins and CERC. In EFM, the energy flux of each wave in the recodes was calculated using the following equation (Benedet *et al.*, 2016).

$$E_f = (\rho \cdot g \cdot H_s^2 / 8) \cdot C_g \quad \dots(5)$$

In which ρ is the water density, g is the gravitational acceleration, H_s is the significant wave height and C_g is the group wave celerity in deep water. The directional and wave height bins are determined in a way that the summation of the energy flux of different waves within each bin is the same. For each bin, the representative wave height is then calculated so that the energy flux of the representative wave is equal to the average of the energy flux of all the waves in that bin, following equation (6).

$$H_s^{Rep} = \sqrt{8 \cdot \bar{E}_f / \rho \cdot g \cdot C_g} \quad \dots(6)$$

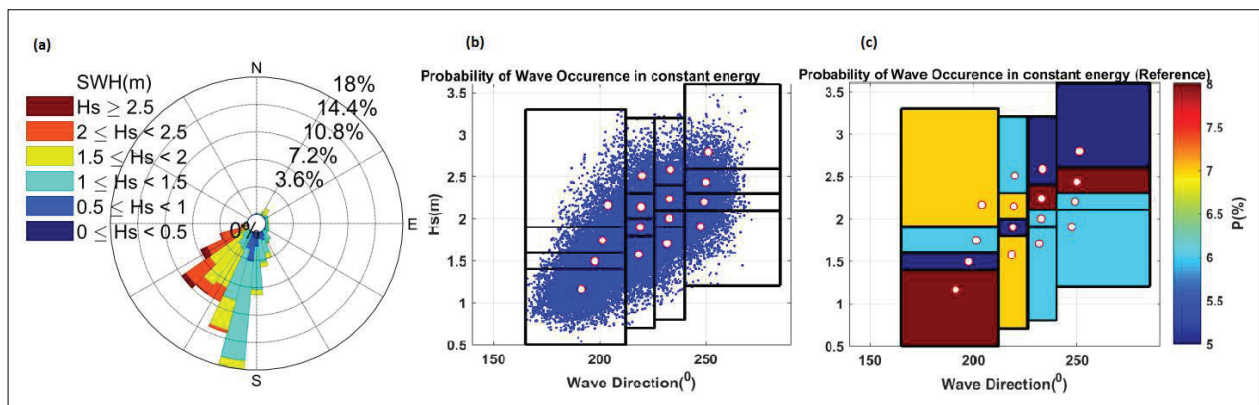


Figure 3: (a) Overview of offshore wave climate at reference location (BC2) off Colombo between 1979 and 2016, ERA-Interim data from ECMWF; (b) representative wave conditions (black lines: boarder of the bins, blue dots: individual waves, red dots: representative wave conditions); (c) probability of wave occurrence in constant energy, the output of 16 representative waves using EFM

Table 1: List of the main numerical parameters used for numerical simulations

Delft3D wave		
Parameter	Description	Value
Diffraction	1600 (kg/m ³)	Activated
Wind growth	1.0	Radiation stress
White-capping	0.25 (m)	Activated
		Alpha: 1
		Gamma 0.73
Quadruplets	Non-linear triad interactions	Activated
		Alpha: 0.1
		Beta: 2.2
Refraction	Bottom friction	Activated
		Type: JONSWAP
		Coefficient: 0.2 (m ² s ⁻³)

In EFM, for example, if the total wave climate is divided into 4 directions and 4 heights (16 bins) each bin represents 1/16 of the total energy (E/16). Figure 3 shows the procedure of the wave schematisation, and the results of 16 schematised wave conditions that were computed are presented in Table 1.

The MORFAC approach

The morphological acceleration factor (MORFAC) concept was applied in this study. The MORFAC approach multiplies the bed levels computed after each hydrodynamic time step by a factor (MORFAC) to enable much faster computation (Roelvink, 2006; Ranasinghe, 2011). The significantly up-scaled new bathymetry is then used in the next hydrodynamic step. During a morphological simulation, each of the selected wave conditions is simulated for the duration of one morphological tide (i.e. 12 hours and 25 minutes = 745 mins) in order to account for the random phasing between waves and tides that occurs in nature. For each wave condition the morphological acceleration factor applied will depend on the percentage occurrence (days per year) of that particular wave condition. The morphological acceleration factors applied to each wave condition are indicated in Table 2.

Boundary conditions

The model boundaries are limited to three open (North, West and South) and one closed boundary (East). Due to the limited extension of the model in the cross-shore directions (North and South), a uniform value of the water level gradient was applied, the so called

Neumann boundary. The sea boundary was forced by a harmonic water level with the schematised wave conditions (Table 1). One representative tide, known as ‘morphological tide’, was selected to minimise the computational time, holding the best representation of the net transport during a spring-neap tidal cycle (Hydraulics, 1999; Walstra *et al.*, 2013).

Tidal constants (tidal amplitude and phase angle) from different stations around Sri Lanka by Wijerathne & Pattiaratchi (2003) were applied for the Neumann and sea boundaries in the study. The sea level variation in the study areas is mainly due to the semi diurnal tides (M2), which is considered as the main tidal constituent in Sri Lanka. The spring tide range recorded in the Bay of Bengal is around 2.4 m while it is about 0.6 m in Colombo. It is between 0.40 – 0.60 m for mixed semi-diurnal and spring tidal range (2M2+2S2) (Wijerathne & Pattiaratchi, 2003)

Model validation

Wave measurements from 25th of August to 28th of September in 2005 were used as a validation period for the model. This period is in the Southwest (SW) monsoon season in Sri Lanka, which was characterised by relative high wave activity with a storm event having a significant wave height above 3 m (Figure 4). Most of the severe erosion damages in the South west part of Sri Lanka were recorded during the SW monsoon period. The most frequent waves come from the South west, approaching the coast with an angle of about 220^o–250^o with respect to the North (Jayathilaka, 2015). Figure 4 shows the comparison between wave characteristics from measured wave data and ERA-Interim of ECMWF (European Center for Medium-Range Weather Forecasts) forced SWAN model results at WB1, a point near port of Colombo, Sri Lanka. The significant wave height, mean wave direction and wave periods computed by the SWAN wave model were compared to the measured wave data at WB1.

Overall, the model validation shows a moderate performance. The coefficient of determination computed for significant wave height gives a good correlation to measured data with a lower root mean squared error, although the peak of the storm on the 5th of September 2005 is slightly underestimated. The model reproduced significant wave heights rather well. Although there are some discrepancies in wave direction and mean wave periods, the order of magnitude and the overall pattern of the time series match. The main reason for the discrepancies can be attributed to the effect of offshore

Table 2: The morphological acceleration factors applied to each wave condition.

Condition	Hs (m)	Tp (s)	Direction (0)	Occurrence (days)	Morfac
1	1.16	9.76	191.46	72.61	143.62
2	1.50	9.82	197.68	26.06	51.54
3	1.74	9.95	201.41	26.11	51.65
4	2.16	10.52	203.90	20.21	39.98
5	1.57	8.41	218.47	35.48	70.18
6	1.90	8.96	219.12	18.44	36.48
7	2.14	9.47	219.52	19.39	38.36
8	2.51	10.21	219.96	11.40	22.56
9	1.70	8.03	231.65	25.51	50.47
10	2.00	8.57	232.65	18.14	35.88
11	2.23	8.96	232.76	21.24	42.01
12	2.58	9.45	233.21	10.35	20.47
13	1.90	7.84	247.43	20.57	40.69
14	2.20	8.27	249.13	15.00	29.67
15	2.43	8.54	249.95	16.34	32.32
16	2.79	8.77	251.26	8.16	16.15

Note: In the last column, the morphological acceleration factors for the different wave conditions are shown.

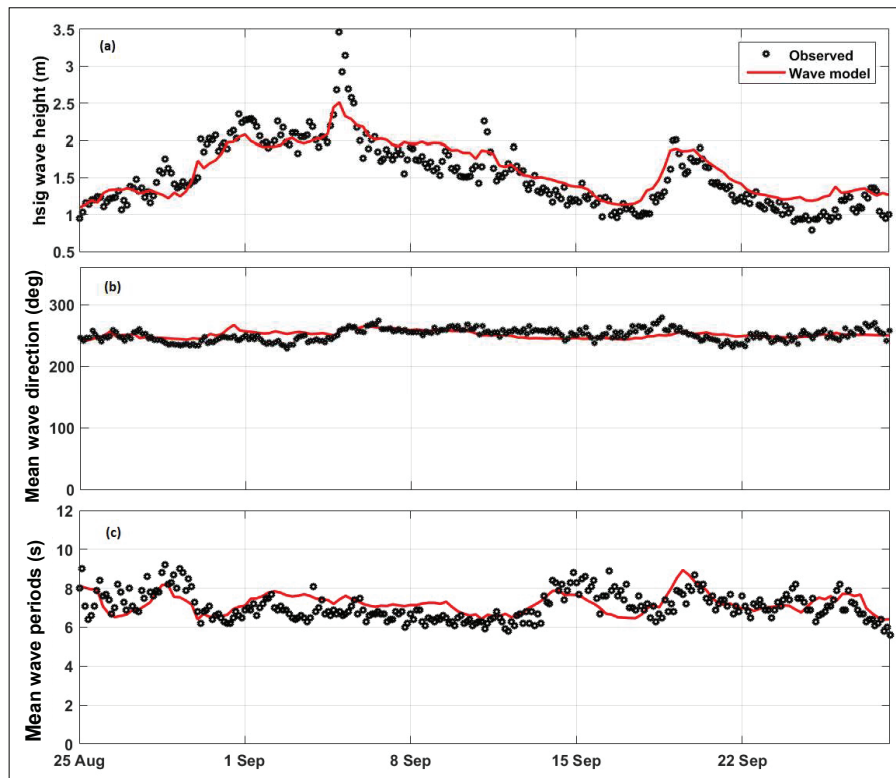


Figure 4: Comparison between wave characteristics from measured wave data and ERA-Interim of ECMWF forced SWAN model results at point WBI (near Colombo port at 19 m depth). (a) Significant wave height; (b) mean wave direction; (c) mean wave periods.

wind field and nearshore bathymetry. Due to poor wind field resolution, we applied interpolated coarser wind field into the wave model which is unlikely to correspond with the wind field for the entire study area. Indeed, the inaccuracies in the variable wind field can affect the wind set up (Elias *et al.*, 2001). Due to the presence of the reef area from Mount Lavinia to Colombo, a JONSWAP coefficient of 0.2 was used to represent the higher bottom friction in reef areas. Default JONSWAP coefficient (0.064) was used for the area between Colombo to Negombo.

RESULTS AND DISCUSSION

Wind/wave analysis

The northern Indian Ocean is characterised by bi-annually reversing monsoon winds resulting from the seasonal differential heating and cooling of the continental land mass and the ocean. The SW monsoon generally operates between June and October, and the NE monsoon operates from December through April (Tomczak & Godfrey, 2003). The transition periods are termed the first inter-monsoon (May) and the second inter-monsoon (November). We studied 37 years (from 1979 to 2016) of ERA-Interim data to schematise the wind/wave climate for boundary conditions.

According to the wind climate in the study area, wind is coming from the NE direction from November to March with an angle of 40° – 60° while it changes to the SW direction with angle of 220° – 240° from May to September. During SW monsoon, the wind speed varies between 7–11 m/s while it varies between 1×5 m/s during the NE monsoon. The total number of wind-wave events studied here is 54053 during 1979 and 2015. The wind analysis for extreme events (wind speeds greater than 12 m/s) in this study found 43 and 65 events at BC2 and BC3, respectively. More than 95 % of such events occurred during the SW monsoon period.

Analysis of wave climate indicates that the significant wave height varies between 0.5 m to 3 m having most probable wave heights around 1.5 m. The distribution of wave direction is mostly from 210° – 250° (SW) and from 30° – 80° (NE). The total number of wave events greater than 3 m recorded at offshore point of BC2 was 149 and no such extreme wave events were found at BC3. The SW monsoon experiences the highest probability of occurrence of higher wave events than the

NE monsoon.

Wave transformation (SWAN)

The annual wave climate has been schematised to 16 wave conditions accompanied by the relative morphological acceleration factors that represent the net transport occurring in one-year period (Table 2). The wave chronology plays a significant role in process based modelling (Lesser *et al.*, 2009). In this study, the schematised wave climates were repeated for reducing the ‘chronology effect’ (Lesser *et al.*, 2009). Locally, wave data were extracted for representative location depths ranging between 10 m and 15 m (always larger than local closure depth). The decrease in wave heights and gradual change in wave direction are observed along the coastline from NS1 to NS3 (northward). The distribution of near-shore wave direction at NS1 is mostly directed from 230° to 250° whereas it is varying between 260° and 290° at NS3.

The local wave climates depend on the shoreline orientation and steepness of the cross-shore profile due to wave shielding, which is clearly visible in wave transformation in Figure 5. Just north of Negombo (section P30-35, Figure 7) the coast is shielded from waves approaching the southern directions due to the sudden change in coastline. Moreover, the effect of swell wave decreases and the influence of the onset of NE monsoon is visible in the northward of the coast line. Therefore, relatively small values of wave heights observed in this section are particularly due to the attenuation of wave’s heights over the shallow and mild slope in the area and further shielding from the tip of Negombo. The sector is experiencing rather small values of the average wave heights, ranging from 0.50 m to 1.2 m at NS3. The wave analysis for point NS3 indicates that for waves over 1 m height the dominant direction is South. The average significant wave height in the section is 0.8 m, the lowest from the entire studied zone.

The Colombo-Negombo section (P12-30, Figure 7) is a 30 km long straight coastline with 250° to shore normal. The waves approaching from the SW directions dominate the wave climate. This sector is sheltered against the southern wave directions by a 2 km long breakwater arm of the Colombo port, which would increase the wave shadow, extending it northwards. As a consequence, wave conditions in this area (probably up to Uswatakeiyawa) would become calmer. This region is identified as a severely eroding coastline (CC & CRMD, 2004; Jayathilaka, 2015). This sector is

experiencing average wave heights ranging from 0.80 m to 1.8 m at NS2 (Figure 3). The average significant wave height in the section is 1.2 m and is gradually decreasing northwards.

Mount Lavinia-Colombo is experiencing rather higher values of the average wave heights, ranging from 1.0 m to 2.7 m at NS1, reaching the average wave height of 1.8 m which is the highest from the entire studied zone, as this sector is exposed to all wave directions. The distribution of wave directions in this section shows the same major direction as for the Colombo-Negombo sector. Particularly, waves over 1.5 m height are predominant from the SW direction, which already suggests that the dominant alongshore current is oriented towards the North. This sector is sheltered against the weak northern waves by the breakwater arm of the Colombo port making the section more calm during the NE monsoon.

Sediment transport and morphological evolution

The calibrated wave model was then forced with the sediment transport and morphology module of Delft3D-Flow, which supports both bed load and suspended load transport of non-cohesive sediments and suspended load

of cohesive sediments.

The numerical results of depth average velocity (m/s) show northward orientation. It is observed that with the increase of off-shore wave heights and direction (shore normal angle) the depth average velocity is increasing. The flow velocity outside the surf zone decreases towards the off-shore (it suddenly decreases to compensate for the increased water depth or continuity of mass). In the breaker zone, within 5 m contour line, the flow velocity is significant and became maximum as the waves break. As a result, the lose energy and momentum are transferred to the alongshore current. Mount Lavinia-Colombo is experiencing a rather higher value of depth average (d.a.) velocity, which is the highest from the entire studied zone as this sector is exposed to higher wave conditions (Figure 6 G-H). The d.a. velocity around Colombo port showed a rather complex behaviour due to wave shielding and sand accumulation around the area (south to breakwater), at the downstream end, it takes few behaviour distance to pick up the current again. On the other hand, the leeside of the Colombo port faces smaller wave heights over the shallow milder slope of the basin, further weakening the down drift current. Thereafter, d.a. velocities increased to 0.3 m/s showing rather constant values from Pegasus reef (P14) to the tip of Negombo (P29) where the coastline changes suddenly. As shown

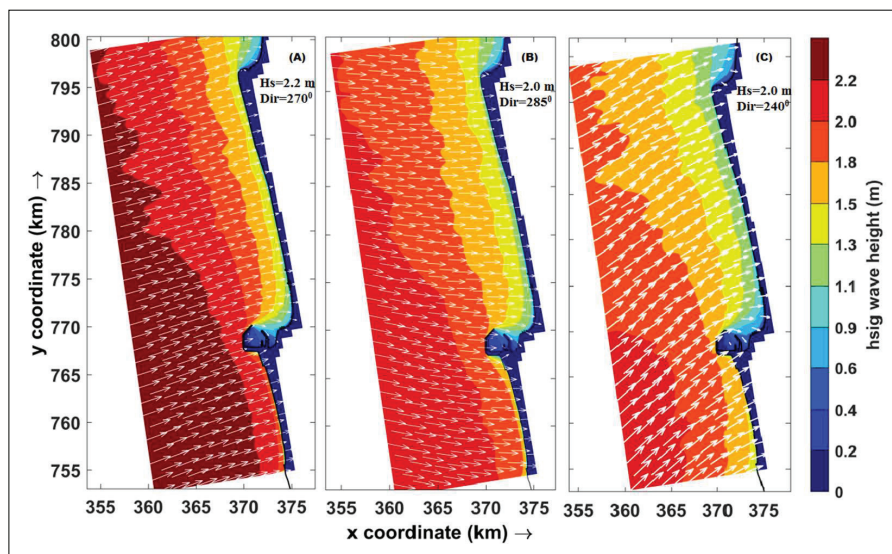


Figure 5: Numerical results of significant wave height and its spatial distribution for different offshore wave conditions. The arrows show the direction of the wave and the colour indicates the wave heights. For off-shore boundary, (A) $H_s = 2.2$ m, $Dir = 270^\circ$; (B) $H_s = 2.0$ m, $Dir = 285^\circ$; (C) $H_s = 2.0$ m, $Dir = 240^\circ$

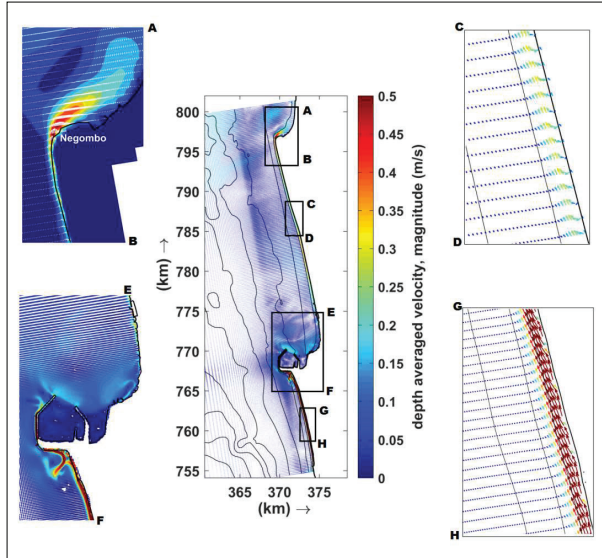


Figure 6: Numerical results of depth average velocity (m/s) and their spatial distribution for the offshore wave height of 2.2 m and direction of 240° degree. The arrows show the direction of the current and the colour indicates the magnitude of the current. (A-B) P28-35 (C-D) P22-36 (E-F) P08-15 (G-H) P03-06

in Figure 7, the breakwater structure has a strong effect on current pattern and thereby on the local morphology. Around the convex shape of Negombo coastline, a complex pattern in d.a. velocity is observed (Figure 6 A-B). There is a strong longshore current that is diverted away from the coast and dissipate in magnitudes at the downstream side (between P30 and P35).

Figure 7 shows the computed net yearly sand transport rates through 35 cross-shore transects between Mount Lavinia and Wallaweediya. The transects run from approximately the -16 to the +3-meter MSL contour in order to integrate all transport magnitude within the closure depth which is computed by the Hallermeier equation (Hallermeier 1980, 1983). The letters P1-35 indicate the cross-shore transects used to compute the net annual sediment transport shown in Figure 7 b. Positive values of transport indicate the northward transport while negative values give southward transport. This transport was then used to derive sediment transport gradient (Figure 7 c). Positive values indicate the increase of transport (erosion) while negative values indicate a decrease of transport (accretion).

In overall, the net transport rates through the cross-shore transect are directed Northward. A review of

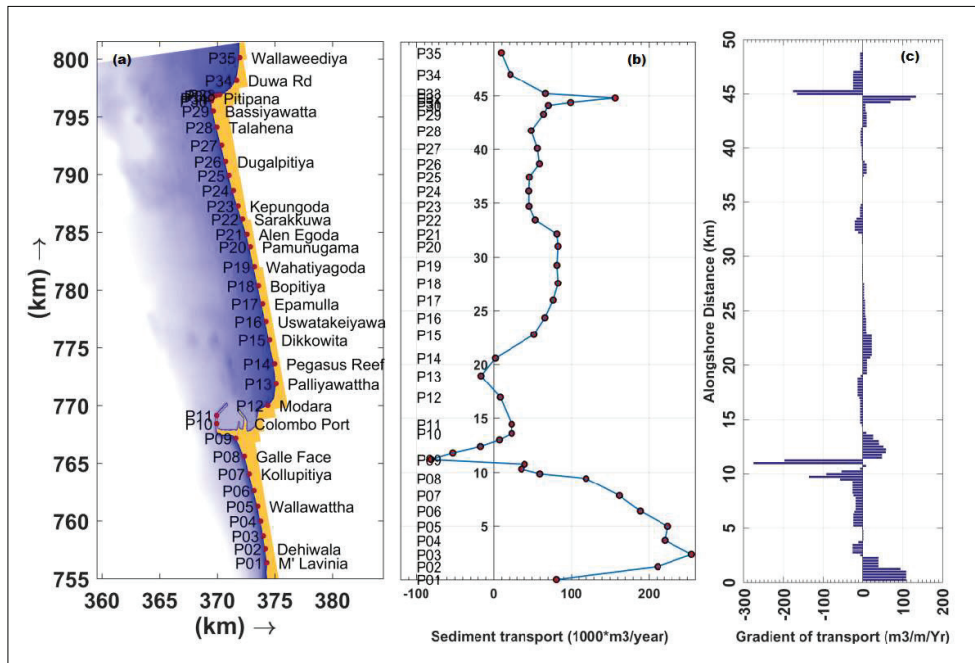


Figure 7: (a) Overview of study area indicating points in which to calculate sediment transports; (b) net alongshore sediment transport through the cross-shore transects up to 25 m depth contour (P01-25); (c) sediment transport gradient (m³/m/year)

several publications and technical reports about sediment transport along the SW coast of Sri Lanka indicates northward transport (CC&CRMD, 2004; Laknath & Sasaki, 2011; Ansaf, 2012; Jayathilaka, 2015). The net sediment transport capacity computed between Mount Lavinia (P1) and Colombo (P8) shows the maximum value of the transport capacity for the whole studied area reached as 255,000 m³/year (P3) and then gradually decreased to 90,000 m³/year near Galle Face (P8). In this sector the wave climate and subsequently, the gross alongshore transport reach the highest values indicating a relative dynamic environment. The two important factors contributing to this are the full exposure of the southern side of the island to swell and wind waves during the SW monsoon and the rather steep slope of the beach compared to Colombo (P10)-Negombo (P35) coastline (Jayathilaka, 2015).

The transport rates in Mount Lavinia - Colombo are in general less than the transport rates shown by previous studies (Ansaf, 2012; Jayathilaka, 2015). According to CCD-GTZ Coast Conservation Project in 1992, the estimated alongshore transport rate in this section varies between 460,000 m³/year to 1,180,000 m³/year. The separate studies of MIKE21 and UNIBEST ST showed same magnitudes of the northward net transport rate along the same coastline (Anfas, 2012; Jayathilaka, 2015). The main reason for these higher values of transport rates was not including the presence of reefs and the beach rock laying parallel to the Mount Lavinia - Colombo coastline. The greater width of non-mobile topology, which decreases the sediment transport due to roughness plays a significant role on calibrating behaviour sediment transport to actual sediment transport. In the present study, we have introduced spatial varying roughness parameter (JONSWAP coefficient) to estimate coastal transport rates for the various coastal segments between Mount Lavinia-Colombo and Colombo-Negombo as discussed in model validation.

The southern breakwater arm of the Colombo South Port (2 km long) shows the strong effect on transport gradient (Figure 7 c). The shoreline upstream of the structure accretes rapidly and the coast downward shows rapid erosion. The seaward end of the breakwater will be in about 18 m depth of water, which is far deeper than the depth of closure where most of the sediment transport occurs. Therefore, a significant proportion of sand transport is captured by the breakwater arm in which it accumulates against the breakwater. Over time the area of accumulation will extend both seawards and southwards along the coast where the ongoing Port City Development Project is located. The behaviour of the

down drift side of the breakwater depends on the process of building of the longshore current. The slow building of the longshore current leads the long area of extends for erosion. Figure 7c shows positive values in sediment gradients between Modara (P10) to Uswatakeiyawa (P16). This region is identified as a severely eroding coastline (CC & CRMD, 2004; Bureau, 2005; Jayathilaka, 2015; Wijayawardane *et al.*, 2013). In the Master Plan for Coast Erosion Management Summary, the erosion rate was identified as 2.5 m/year for 70 % of the coastline from Palliyawatte (P13) to Uswetikeiyawa (P16). Decrease in sand supply from the Kaleni river located between P12 and P13 is said to accelerate the down drift erosion north of the river mouth (CC & CRMD, 2004). A nodal point, the transport direction changed and building is observed behind Pegasus Reef. Thereafter, 55 000 m³/year rate of transport shown in Dikkowita (P15) and to reached 90 000 m³/year near Pamunugama (P20) with a milder gradient along the straight coastline. Then the coastline bends slightly extending to Bassiyawattha (P29) and afterwards the coastline exhibits a convex form. In the area between Bassiyawattha (P29) and Pitipana (P33) the transport is increasing with a steep gradient, suggesting intense erosion (Figure 7c). The convex shape of the coastline shields the down drift side of Pitipana from wave action, preventing much longshore drift where waves are not strong enough to move the sediment along the coast.

CONCLUSION

The main contribution of this work is the quantitative determination of a nearshore wave climate, the alongshore sediment transport along 55 km long coastal belt between Mount Lavinia and Negombo. Delft3D-FLOW, a process-based model, modeled together with input reduction and morphological acceleration techniques has been used to estimate the long shore sediment transport rates and related morphodynamics. The importance of nearshore wave transformation is highlighted in the conclusion. In data poor environment, this is achieved by transforming offshore wave time series (schematised) to nearshore locations (SWAN model). The final reduced set of forcing conditions (16 conditions) is supposed to be sufficient enough to capture the morphological change to its full extent. However, certain low probability, high-energy conditions responsible for alongshore sediment transport can also be included for better model results. The simulated wave climate based on wind records proved to be reliable despite the lack of precise data. The numerical results can very well be related to the accuracy of bathymetry or measured wave buoy data

or both. Default parameter settings and coefficients in Delft3D such as depth induced breaking, non-linear triad interactions and bottom friction applied during simulation, show quite unrealistic results and have to tune accordingly. Increased coverage and accuracy of wave measurements would probably provide the greatest improvements in future studies. Therefore, at least one wave buoy should deploy between Colombo and Negombo where the shoreline orientation changes abruptly.

The wave model is calibrated and reproduces nearshore waves well. Analysis of model results indicated that the Southwest monsoon has greater influences having the highest probability of occurrence of higher wave events on the near-shore wave climate, thus sediment transport in the study area. More than 95 % of the extreme events occurred during the SW monsoon periods. However, the effect of swell wave decreases and the influence of onset of NE monsoon are visible when it goes to the northward of the coastline. Mount Lavinia-Colombo coastline shows the highest wave heights from the entire studied area, thus rather higher value of d.a. velocity than in Colombo-Negombo coastline. Numerical results of the alongshore sediment transport gradients clearly indicate the potential erosion/accretion prone areas, emphasising the importance of detailed coastal morphological studies and quantitative risk assessments at vulnerable coastal areas along the coast of Mount Lavinia-Negombo. The study shows that the Colombo-Negombo coastline is naturally subjected to erosion due to its coastal orientation and wave actions. To compensate the erosion rate, the importance of a steady sand supply from the Kaleni river is highlighted. The reduction of sand supply from the Kaleni river due to extensive sand mining is a contributory factor that accelerates the coastal erosion in that area. In addition, the breakwater of Colombo port has a partial effect on changing the local erosion/accretion rates in the area extending to adjacent coastal areas. Sand nourishment in the down drift area is a nature- friendly method to restore the regional sediment budget in the area. This requires the complete stop of sand mining in rivers and sea-sand dredging within the closure depth.

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