STRUCTURAL NOTES:

1. FOUNDATION:
   A. FOUNDATION SYSTEM CONSISTS OF WALL CONTINUOUS AND PAD FOOTINGS DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF IN ACCORDANCE WITH SOIL REPORT BY ARDAMAN & ASSOCIATES, INC., DATED 04-13-2022. CONTRACTOR TO OBTAIN A COPY OF THE GEOTECHNICAL REPORT AND FOLLOW THE INSTRUCTIONS INDICATED.
   B. CONCRETE WALL FOOTINGS AND PADS SHALL NOT RECEIVE SUPERIMPOSED LOADS UNTIL 48 HOUR OR MORE AFTER THE CONCRETE IS PLACED.
   C. EXCAVATIONS FOR MONOLITHIC FOOTINGS AND FOUNDATIONS, WHICH ARE TO SERVE AS FORMS, SHALL BE THOROUGHLY WET PRIOR TO PLACING CONCRETE.
   D. AS A MINIMUM, BOTTOM OF MONOLITHIC FOOTING SHALL BE 8" MINIMUM BELOW GRADE AND TOP OF STEM WALL FOOTING AND ISOLATED FOOTING AS WELL. IF ELEVATION OF FOOTING SHOWN IN DETAILS PUT THE FOOTING HIGHER THAN 8" BELOW GRADE, THE 8" RULE SHALL GOVERN OVER THE ELEVATION SHOWN.

2. SLABS ON FILL:
   FILL AND BACKFILL TO BE COMPACTED TO 95% OF MAXIMUM DRY DENSITY FOR ALL LAYERS AS VERIFIED BY FIELD DENSITY TEST FOR ALL LAYERS. TESTS SHALL BE MADE IN ACCORDANCE WITH METHODS OF TEST FOR MOISTURE DENSITY RELATIONS OF SOILS, ASTM D 1557-12 MODIFIED. COMPACTION LAYERS NOT TO EXCEED 12". BACKFILL MATERIAL TO BE APPROVED BY SOIL ENGINEER. SLABS TO BE PLACED CONTINUOUSLY. PROVIDE CONTRACTION JOINTS, MAXIMUM AREA BETWEEN JOINTS LIMITED TO 225 SF, AND/OR 12 FEET IN ANY DIRECTION. COORDINATE WITH ARCHITECTURE FOR LOCATION OF SAW CUTS AT EXPOSED AREAS. SAW CUT FOR THE JOINTS TO BE A MINIMUM OF t/4 INCH DEEP AND 1/8 INCH WIDE. PROVIDE VAPOR BARRIER BELOW ALL SLABS ON FILL (6 MIL.). WELDED WIRE FABRIC REINFORCEMENT FOR SLAB ON GRADE SHALL BE ADEQUATELY CHAIRED FOR ELEVATION, WET-LIFTING SHALL NO BE PERMITTED.

3. CONCRETE:
   ALL CONCRETE TO ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH AS SHOWN BELOW IN 28 DAYS. AGGREGATES TO BE CLEAN WELL GRADED, MAXIMUM SIZE 3/4".
   CONCRETE SLUMP= 3" MIN. TO 5" MAX.
   VERTICAL CONCRETE DROP NOT TO EXCEED 9'-0".

FOUNDATIONS AND SLAB ON GRADE	= 3000 PSI
COLUMNS, TIE COLUMNS AND BEAMS, U.N.O	= 4000 PSI
SLABS	= 4000 PSI
ALL OTHER MEMBERS	= 3000 PSI

4. CONCRETE COVER:

	MINIMUM CLEAR COVER (IN.)
A. SLAB ON GRADE AND FOOTINGS	3
B. WALLS	3/4
C. COLUMNS	1 1/2
D. BEAMS	1 1/2
E. WALLS AND COLUMNS EXPOSED TO EARTH OR WEATHER	2
F. SLABS	
TOP BARS	3/4
BOTTOM BARS	3/4
G. ANY CONCRETE CAST AGAINST EARTH	3

5. REINFORCING STEEL:
   A. TO BE DEFORMED BARS CONFORMING TO ASTM A615/ASTM A615M-12, GRADE 60.
   B. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
   C. ALL TOP REINFORCEMENT SHALL TERMINATE WITH STANDARD HOOKS AT DISCONTINUOUS ENDS.
   D. ALL BOTTOM BARS SHALL BEAR A MINIMUM OF 6" OVER SUPPORTS UNLESS OTHERWISE NOTED.
   E. REINFORCING STEEL TO BE DETAILED AND FABRICATED IN ACCORDANCE WITH "MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCING CONCRETE STRUCTURES", AND THE ACI BUILDING CODE 318-14.
   F. WHEN REQUIRED, WELDABLE REINFORCING TO BE A706.
   G. FOR FOUNDATION SPLICES IN REINFORCING BARS SHALL BE NOT LESS THAN 36 BAR DIAMETERS AND REINFORCEMENT SHALL BE CONTINUOUS AROUND ALL CORNERS AND CHANGES IN DIRECTION. CONTINUITY SHALL BE PROVIDED AT CORNERS OR CHANGES IN DIRECTION BY BENDING THE LONGITUDINAL STEEL AROUND THE CORNER 48 BAR DIAMETERS OR BY ADDING MATCHING REINFORCING STEEL, WHICH SHALL EXTEND 48 BAR DIAMETERS FROM EACH CORNER OR CHANGE IN DIRECTION. WHEN THREE OR MORE BARS ARE REQUIRED, THE BARS SHALL BE HELD IN PLACE AND ALIGNED BY TRANSVERSE BARS SPACED NOT MORE THAN 4 FEET (1219 MM) APART.
      FOR ALL OTHER SPLICES SEE SPECIFIC SECTION OR DETAIL AND TENSION LAP SPLICE SCHEDULE.
   H. ALL REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI-315-11.

6. MASONRY:
   A. ALL CONCRETE BLOCK MASONRY WALLS, TO COMPLY WITH FLORIDA BUILDING CODE REQUIREMENTS AND THE ACI STANDARD CALLED "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES ACI 530.1 /ASCE 6/TMS402-16/TMS-602-16.
   B. CONCRETE BLOCK UNITS TO BE TYPE II-NONMOISTURE CONTROLLED, CONFORMING TO ASTM C90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI, (AVERAGE OF THREE).
      PRISM STRENGTH OF CMU WALLS F'm=1500 PSI.
   C. MORTAR SHALL CONFORM TO ASTM C 270-12A:
      MORTAR TYPE "M", WITH A MINIMUM AVERAGE STRENGTH OF 2500 PSI. SHALL BE USED FOR WALLS BELOW GRADE.
      MORTAR TYPE "M" (2500 PSI) OR TYPE "S" (1800 PSI) SHALL BE USED FOR ALL OTHER MASONRY WALLS.
   D. ALL CMU WALLS SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 9 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY OTHER COURSE. EXTEND REINFORCING A MINIMUM OF 4 INCHES INTO TIE COLUMNS.
   E. ALL VERTICAL REINFORCEMENT, AS SPECIFIED ON PLANS, SHALL BE PLACED IN FULLY GROUTED CELLS. DOWELS SHALL BE PROVIDED TO ENSURE CONTINUITY OF REINFORCEMENT AT THE STRUCTURE ABOVE AND BELOW. ALL LAPS SHALL BE SPLICED MINIMUM 48 BAR DIAMETERS. ADDITIONAL VERTICAL WALL REINFORCEMENT SHALL BE PROVIDED AT ALL WALL ENDS, CORNERS, INTERSECTIONS AND AT ALL WALL OPENINGS NOT BOUND BY CONCRETE COLUMNS. PROVIDE CLEANOUT HOLES IN REINFORCED WALL CELLS AT BOTTOM OF EACH POUR. CLEAN CELLS FREE OF MORTAR AND DEBRIS. TIE VERTICAL REINFORCEMENT. MAXIMUM LIFT OF EACH POUR SHALL NOT EXCEED 4 FEET. MAXIMUM VERTICAL DROP FOR GROUTING SHALL NOT EXCEED 10 FEET. VERTICAL REINFORCEMENT WITHIN TERMINATING WALLS SHALL EXTEND TO 2" BELOW TOP OF CONCRETE CAP/TIE BEAM ABOVE.
   F. PROVIDE INTERLOCKING BLOCK CONSTRUCTION AT ALL INTERSECTIONS AND CORNERS, PROVIDE CONTINUOUS JOINT REINFORCING AROUND CORNERS AND AT INTERSECTIONS.
   G. CONCRETE TIE COLUMNS AND TIE BEAMS SHALL BE CAST AGAINST ERECTED MASONRY WALLS. HORIZONTAL JOINT REINFORCEMENT SHALL EXTEND A MINIMUM OF 4 INCHES INTO TIE COLUMNS.
   H. PROVIDE GALVANIZED DOVETAIL ANCHORS AT ALTERNATE JOINTS AT ALL MASONRY WALLS ABUTTING CONCRETE COLUMNS.
   I. REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER ACI 530.1 FILLING OF CELLS SHALL BE DONE IN FOUR FOOT LIFTS WITH A MAXIMUM POUR OF 9 FEET. USE MECHANICAL VIBRATION TO ACHIEVE GROUT-FILLED. GROUT SHALL CONFORM TO ASTM C476-02. SLUMP SHALL BETWEEN 8" AND 11". PEAROCK PUMP MIX CAN BE USED AS AN ALTERNATIVE TO GROUT.

7. WIND LOADS AND DESIGN STANDARDS:
   STRUCTURE HAS BEEN DESIGNED TO RESIST LATERAL LOADS IN ACCORDANCE WITH THE REQUIREMENTS OF ASCE 7-16:
   WIND VELOCITY = 150 MPH
   EXPOSURE C, ENCLOSED BUILDING.

8. STRUCTURAL STEEL:
   A. STRUCTURAL STEEL HAS BEEN DESIGNED IN ACCORDANCE WITH THE FBC AND AISC SPECIFICATIONS.
   B. STRUCTURAL STEEL SHALL MEET THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE ON THE DRAWINGS:

TYPE	ASTM	GRADE	fy
WIDE FLANGE (W SHAPE)	A36	-	36KSI
C, S&M SHAPES	A36	-	36KSI
ANGLES	A36	-	36KSI
SQUARE & RECTANGULAR HSS	A500	B	46KSI
ROUND HSS	A500	B	42KSI
STEEL PIPE	A53	-	35KSI
STRUCTURAL BOLTS	ASTM1554	36	36KSI
ANCHOR BOLTS-THREADED RODS	F1554	36	36KSI
PLATES	A36		36KSI
THREADED RODS	A36		36KSI

   ALL STRUCTURAL STEEL THAT IS EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED.
   C. ALL CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED BY THE FABRICATOR. SHOP DRAWINGS AND CONNECTION CALCULATIONS SHALL BE SUBMITTED BEARING THE SEAL OF AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
   D. BEAM SHEAR CONNECTIONS DESIGN SHALL BE BASED ON BEARING TYPE BOLTED CONNECTIONS WITH BOLTS "SNUG TIGHT" PER AISC.

9. WELDING:
   CONNECTIONS SHOWN ARE BASED ON WELD MADE WITH E70XX ELECTRODES. WELDING TO BE DONE BY CERTIFIED WELDERS HOLDING CURRENT WELDING CERTIFICATES, AND MUST PRESENT SAME AT JOB SITE AT ALL TIMES. ALL WELDING PER PLANS AND PER GUIDELINES OF THE AMERICAN WELDING SOCIETY.

10. DETAILS AND SECTIONS:
    ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL, AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, UNLESS A DIFFERENT DETAIL, OR SECTION IS SHOWN.

11. SHORING AND RESHORING:
    CONTRACTOR WILL PROVIDE SHORING AND RESHORING DRAWINGS, ALONG WITH RE-SHORING SCHEDULE FOR ALL ELEVATED STRUCTURAL COMPONENTS, INCLUDING BUT NOT LIMITED TO BEAMS, SLABS, AND PRECAST ELEMENTS. DRAWINGS TO BE SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL STRUCTURAL ENGINEER, FOR SUBMITTAL TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD, AND THE THRESHOLD INSPECTOR (IF APPLICABLE). NO ELEVATED WORK MAY PROCEED WITHOUT THIS SUBMITTAL.
    CONTRACTOR TO ALSO RETAIN SAME ENGINEER TO PROVIDE FIELD INSPECTIONS, ALONG WITH REPORTS, THAT ALL OF THE SHORING AND RE-SHORING IS COMPLETED AS PER HIS DRAWINGS AND SPECIFICATIONS.
    SHORING SCHEDULE TO PROVIDE TIME THAT SHORES MAY BE REMOVED IN ORDER TO REMOVE FORMWORK, AND SPECIFIC SEQUENCE OF THE REMOVAL OF THE SHORING AS IT PROGRESSES THRU THE HEIGHT OF THE BUILDING.
    SHORING DRAWINGS TO ALSO CONTAIN DETAIL OF CONNECTION OF HEADERS AT THE TOP OF THE SHORES.

12. GENERAL:
    A. IN CASE OF DISCREPANCY BETWEEN THESE NOTES AND PLAN NOTES OF INFORMATION IN THE PLANS, SECTION OR DETAILS, THE MOST STRINGENT REQUIREMENT SHALL BE APPLIED.
    B. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, SPECIFICATIONS, OR OTHER DOCUMENTS, THE CONTRACTOR MUST NOTIFY THE ARCHITECT AND ENGINEER, IN WRITING, OF THE SAME PRIOR TO PROCEEDING WITH THE WORK IN QUESTION. IN THE EVENT THAT THE CONTRACTOR FAILS TO GIVE NOTICE, OR PROVIDE SUFFICIENT TIME FOR A RESPONSE, CONTRACTOR IS RESPONSIBLE FOR THE RESULTS OF SUCH ERRORS OR OMISSIONS, AND FOR ALL COSTS FOR RECTIFYING SAME AND FOR ANY DELAYS OR ANY OTHER COSTS INCURRED BY SAME.
    C. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS OF EXISTING STRUCTURES AFFECTING THE NEW CONSTRUCTION PRIOR TO COMMENCING THE WORK. ANY VARIATIONS IN ACTUAL FIELD CONDITIONS/DIMENSIONS FROM THOSE SHOWN IN THE PERMITTED CONTRACT DRAWINGS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR DETERMINING THE NEED FOR RE-DESIGN PRIOR TO CONTRACTOR'S SUBMITTAL OF SHOP DRAWINGS FOR REVIEW.
    D. CONTRACTOR SHALL COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS, VERIFY ALL FIELD DIMENSIONS PRIOR TO INSTALLATION, AND VERIFY THAT PROPOSED DIMENSIONS AND FIELD CONDITIONS AGREE WITH THIS PROPOSED PLAN. USE OF THIS DOCUMENT CONSTITUTES ACCEPTANCE OF THE PROPOSED SYSTEM LAYOUT, COMPONENTS SELECTED, AND INSTALLATION. THESE DRAWINGS ARE NOT INTENDED TO BE USED AS FABRICATION OR SHOP DRAWINGS.

13. WOOD MEMBERS:
    ALL STRUCTURAL WOOD MEMBERS OTHER THAN TRUSSES TO BE SOUTHERN PINE GRADE NO. 2 UNLESS OTHERWISE NOTED.
    ALL WOOD MEMBERS TO BE FREE OF ALL IMPERFECTIONS AS: SPLITS, CHECKS, OR EXCESSIVE KNOTS. UNSATISFACTORY MATERIALS TO BE REPLACED AT NO COST TO OWNER. ALL WOOD MEMBERS EXPOSED TO THE WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY TO BE PRESSURE TREATED. MOISTURE CONTENT SHALL BE 19% OR LESS.
    ALL BOLTED CONNECTIONS TO BE COMPLETED WITH A325 GALVANIZED STEEL BOLTS WITH WASHERS AT EACH END. FABRICATION, ERECTION, AND CONNECTIONS TO BE AS PER RECOMMENDATIONS OF THE A.I.T.C. (AMERICAN INSTITUTE OF TIMBER CONSTRUCTION), LATEST EDITION.
    PLYWOOD SHEATHING CLASSIFICATION SHALL BE "C" OR BETTER AND THE EXPOSURE SHALL BE APA RATED EXTERIOR.
    INSTALL PANELS WITH STRENGTH DIRECTION PERPENDICULAR TO TRUSSES OR JOISTS.
    PRESSURE-TREATED WOOD SHALL BE USED WHERE IN CONTACT WITH CONCRETE OR EXPOSED TO WEATHER.

14. FASTENERS:
    ALL BOLTS SHALL BE HOT DIPPED GALVANIZED, OR STAINLESS STEEL & MEET THE REQUIREMENTS OF ASTM A325. WASHERS SHALL BE USED BETWEEN WOOD & BOLT HEAD & BETWEEN WOOD & NUT. WHERE GENERIC FASTENERS ARE LABELED IN DETAILS, CAPACITIES SHALL BE EQUAL TO OR GREATER THAN HILTI KWIK BOLT III OR RED HEAD THRU BOLTS. EMBEDMENT DEPTHS SPECIFIED HEREIN ARE DEPTHS INTO SOLID SUBSTRATE AND DO NOT INCLUDE THICKNESS OF STUCCO OR OTHER FINISHES.

15. SUBMITTALS & SHOP DRAWINGS:
    NO SHOP DRAWING SHALL BE SUBMITTED FOR ARCHITECT/ENGINEER'S REVIEW UNTIL AFTER THEY HAVE BEEN REVIEWED AND NOTED FOR CONSTRUCTION METHOD, DIMENSIONING, AND OTHER TRADE REQUIREMENTS BY THE CONTRACTOR, AND STAMPED WITH THE CONTRACTOR'S APPROVAL SEAL. SIGNED AND SEALED CALCULATIONS SHALL BE INCLUDED OF APPLICABLE,
    A. REINFORCING STEEL SHOP DRAWINGS.
    B. STRUCTURAL STEEL SHOP DRAWINGS. CONNECTIONS CALCULATIONS, IF ANY, SHALL BE SIGNED AND SEALED.
    C. SIGNED AND SEALED SHORING AND RE-SHORING SHOP DRAWINGS.
    D. SIGNED AND SEALED SHOP DRAWINGS FOR WINDOWS, DOORS AND STOREFRONTS.
    E. ANCHORING DETAILS FOR WINDOWS AND DOOR BUCKS NOT SHOWN ON THE DRAWINGS SHALL BE DESIGNED AND DETAIL BY MANUFACTURER'S SPECIALTY ENGINEER AND SUBMITTED TO THE E.O.R. FOR APPROVAL. SUBMIT SIGNED AND SEAL SHOP DRAWINGS.
    F. SIGNED AND SEALED SHOP DRAWINGS FOR STEEL LADDER.
    G. SIGNED AND SEALED SHOP DRAWINGS FOR STEEL STAIRS.
    H. SIGNED AND SEALED SHOP DRAWINGS FOR ALL RAILINGS.
    J. SIGNED AND SEALED SHOP DRAWINGS FOR PRE-FABRICATED CONCRETE ELEMENTS.
    K. SIGNED AND SEALED SHOP DRAWINGS FOR OPEN WEB STEEL JOISTS.
    L. SIGNED AND SEALED SHOP DRAWINGS FOR WOOD TRUSSES.

16. CODES:
    ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES AND ORDINANCES, INCLUDING:

- FLORIDA BUILDING CODE 2020 7TH EDITION.
- ASCE 7-16, LOADS.
- ACI 318-14, REINFORCED CONCRETE.
- AISC MANUAL OF STEEL CONSTRUCTION, AISC360-16.
- ACI 530.1 (TMS402-2016, TMS-602-2016) MASONRY.
- NDS FOR WOOD CONSTRUCTION, NDS-18, 2018.
- ALUMINUM DESIGN MANUAL ADMI-2015.

17. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE FOLLOWING INSTALLATION OF ALL COMPONENTS AS INDICATED ON THE DRAWINGS. IT SHALL BE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE METHOD AND SEQUENCE OF ERECTION PROCEDURES (INCLUDING IMPLEMENTATION OF TEMPORARY SHORING, BRACING, ETC.) IF A SEQUENCE IS SPECIFIED IN THE ARCHITECTURAL OR STRUCTURAL DRAWINGS, CONTRACTOR SHALL VERIFY THE FEASIBILITY OF IT, AND INFORM THE ENGINEER OR ARCHITECT OF ANY DISCREPANCY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY THROUGHOUT THE PERIOD OF CONSTRUCTION. THIS STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, PLUMBING MECHANICAL AND ELECTRICAL DRAWINGS TO COORDINATE LOCATION OF DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, REGLETS, BOLT SETTINGS, SLEEVES, DIMENSIONS, ETC.

INDEX:

LEGEND:

ADDN'L	-ADDITIONAL	LSV	-LONG LEG VERTICAL
ARCH	-ARCHITECTURAL	LP	-LOW POINT
BLDG	-BUILDING	LW	-LONG WAY
BM	-BEAM	LWC	-LIGHT WEIGHT CONCRETE
CIP	-CAST IN PLACE	LWIC	-LIGHT WEIGHT INSULATED CONCRETE
CJ	-CONTROL JOINT	MAX	-MAXIMUM
CL	-CENTERLINE	MECH	-MECHANICAL
CLR	-CLEAR	MIN	-MINIMUM
CMU	-CONCRETE MASONRY UNIT	NTS	-NOT TO SCALE
COL	-COLUMN	NS	-NEAR SIDE
CTR	-CENTER	O/C	-ON CENTER
DBL	-DOUBLE	OD	-OUTSIDE DIAMETER
DET	-DETAIL	OPP	-OPPOSITE
DN	-DOWN	PL	-PLATE
DWG	-DRAWING	PLF	-POUNDS PER LINEAR FOOT
EA	-EACH	PLWD	-PLYWOOD
EE	-EACH END	PSF	-POUNDS PER SQUARE FOOT
EF	-EACH FACE	PSI	-POUNDS PER INCH
EJ	-EXPANSION JOINT	REINF	-REINFORCING
EL	-ELEVATION	REQ'D	-REQUIRED
EOR	-ENG. OF RECORD	REV	-REVISED/REVISION
EW	-EACH WAY	RM	-ROOM
EX	-EXISTING	SER	-STRUCT. ENG. OF RECORD
EXP	-EXPANSION	SF	-SQUARE FOOT
EWF	-EXISTING WALL FOOTING	SIM	-SIMILAR
FIN	-FINISH	SL	-SLOPE
FLR	-FLOOR	STD	-STANDARD
FND	-FOUNDATION	SW	-SHEARWALL/SHORT WAY
FS	-FAR SIDE	STL	-STEEL
FT	-FOOT	STRUCT	-STRUCTURAL
FTG	-FOOTING	TB	-TIE BEAM
F.V.	-FIELD VERIFY	TC	-TIE COLUMN
GA	-GAGE	TOC	-TOP OF CONCRETE
GALV	-GALVANIZED	T	-TOP
HORIZ	-HORIZONTAL	TEMP	-TEMPERATURE
HP	-HIGH POINT	TYP	-TYPICAL
ID	-INSIDE DIAMETER	UNO	-UNLESS NOTED OTHERWISE
IJ	-ISOLATION JOINT	UON	-UNLESS OTHERWISE NOTED
INFO	-INFORMATION	VERT	-VERTICAL
INT	-INTERIOR	W/	-WITH
LSV	-LONG SIDE VERTICAL	WHS	-WELDED HEADED STUD
LLH	-LONG LEG HORIZONTAL	WWF	-WELDED WIRE FABRIC

TERMITE PROTECTION:

PROVIDE SOIL TREATMENT PROTECTION AGAINST SUBTERRANEAN TERMITES FOR ALL SLAB CUTS IN COMPLIANCE W/ FBC SECTION 1816. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

SITE PREPARATION:

1.- EXCAVATION SHALL REMOVE ALL GRASS, WEEDS, ROOTS, AND ANY DEBRIS.
2.- EXISTING SOFT SILT AND ORGANIC SOIL LAYER SHALL BE REMOVED AND REPLACED WITH CLEAN FILL, SEE NOTE 7.
3.- SAND AND LIME ROCK SOIL CAN BE STOCKEDPILED AND USED AS BACKFILL.
4.- ONCE THE ORGANIC LAYER HAS BEEN REMOVED, THE DEMUCKED SURFACE SHALL BE COMPACTED.
5.- FILL MATERIAL SHALL BE PLACED IN LIFTS NOT EXCEEDING 12 INCHES IN LOOSE THICKNESS.
6.- EACH LIFT SHALL BE THOROUGHLY COMPACTED WITH VIBRATORY COMPACTION EQUIPMENT.
7.- FILL SHALL CONSIST OF CLEAN SAND, LIME STONE OR GRAVEL. FILL MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 3" AND NO MORE THAN 10 % PASSING THE No. 200 SIEVE.

SOIL STATEMENT:

FOUNDATION SYSTEM HAS BEEN DESIGNED USING A LOAD-BEARING CAPACITY OF 2000 PSF, BASED ON THE GEOTECHNICAL REPORT BY ARDAMAN & ASSOCIATES, INC., DATED 04-13-2022. CONTRACTOR TO OBTAIN A COPY OF THE GEOTECHNICAL REPORT AND FOLLOW THE INSTRUCTIONS INDICATED IN THE REPORT.

REQUIREMENTS FOR CONDUITS AND PIPES EMBEDDED IN CONCRETE:

1-CONDUITS AND PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE UNLESS EFFECTIVELY COATED OR COVERED.
2-THE OUTSIDE DIAMETER OF CONDUITS AND PIPES SHALL NOT BE LARGER THAN 1/3 THE OVERALL THICKNESS OF SLAB, BEAM OR WALL IN WHICH THEY ARE EMBEDDED.
3-THE NOMINAL INSIDE DIAMETER IF THE CONDUITS AND PIPES SHALL NOT BE GREATER THAN 2".
4-STEEL AND IRON PIPES AND CONDUITS SHALL NOT BE THINNER THAN STANDARD SCHEDULE 40, AND CAN BE UNCOATED OR GALVANIZED. PIPES AND CONDUITS OF OTHER MATERIAL SHALL BE DESIGNED TO RESIST PRESSURE AND TEMPERATURE WHICH THEY WILL BE SUBJECTED.
5-CONDUITS AND PIPES SHALL NOT BE SPACED CLOSER THAN 4 TIMES THE OUTSIDE DIAMETER OR WIDTH (CLEAR SPACING).
6-IN SOLID SLABS PIPING AND CONDUITS SHALL BE PLACED BETWEEN TOP AND BOTTOM REINFORCEMENT.
7-CONCRETE COVER FOR PIPES, CONDUITS AND FITTINGS SHALL BE NO LESS THAN 1½" FOR CONCRETE EXPOSED TO EARTH OR WEATHER AND NO LESS THAN ¾" FOR CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND.
8-PIPING AND CONDUIT SHALL BE SO FABRICATED AND INSTALLED THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS PROPER LOCATION WILL NOT BE REQUIRED.

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PARTIAL FOUNDATION / GROUND FLOOR PLAN
SCALE 1/8"=1'-0"

PLAN NOTES:
1.– NEW 6" CONCRETE SLAB OVER WELL COMPACTED FILL REINFORCED WITH 6x6 W2.1 xW2.1 WELDED WIRE FABRIC AT MID DEPTH, TYPICAL U.O.N. REFER TO DETAIL A/S–3.1 FOR TYPICAL CONCRETE SLAB CONTROL JOINT (C.J.).
2.– TOP OF FOOTING ELEVATION: SEE SECTIONS.
3.– PROVIDE 1 #5 IN GROUTED CELLS AT EACH SIDE OF ALL WALL OPENINGS OF WIDTH EQUAL OR LESS THAN 5'–11".
PROVIDE 2 #5 IN GROUTED CELLS AT EACH SIDE OF ALL WALL OPENING OF WIDTH BETWEEN 6'–0" AND 7–11" U.N.O.
PROVIDE 1 #5 IN GROUTED CELLS AT ALL WALL INTERCEPTIONS.
4.– PROVIDE STANDARD HOOK FOR ALL TOP BARS AT DISCONTINUOUS END.
5.– TE –– DENOTES A THICKENED EDGE SEE TYPICAL DETAIL B/S–3.1.
6.– 2 #4 x 36" LONG @ 12" AT UPPER THIRD OF THE SLAB THICKNESS, TYP.
7.– SAW CUT EXISTING SLAB ON GRADE AND WALL FOOTING AS REQUIRED TO POUR THE NEW FOOTING, AVOID ANY DAMAGE TO THE REINFORCING OF THE EXISTING FOOTING.
8.– FOR WATER PROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.
9.– SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS. ALL DIMENSIONS SHOWN HERE SHALL BE VERIFIED WITH ARCHITECTURAL DRAWINGS. IN CASE OF DISCREPANCY ARCHITECTURAL DRAWINGS SHALL CONTROL.

SUPERIMPOSED LOADS		
WAREHOUSE	DEAD	10 PSF
	LIVE	250 PSF

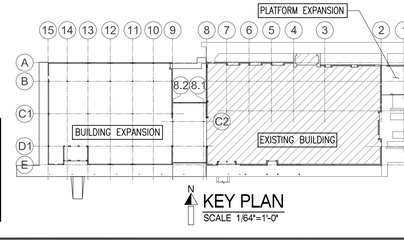
SOIL STATEMENT:
FOUNDATION SYSTEM HAS BEEN DESIGNED USING A LOAD BEARING CAPACITY OF 2000 PSF, BASED ON THE GEOTECHNICAL REPORT BY ARDAMAN & ASSOCIATES, INC., DATED 04–13–2022. CONTRACTOR TO OBTAIN A COPY OF THE GEOTECHNICAL REPORT AND FOLLOW THE INSTRUCTIONS INDICATED IN THE REPORT.

CONCRETE MASONRY NOTE:
ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90 "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS", WITH A NET AREA COMPRESSIVE STRENGTH OF MASONRY OF 1900 PSI.
ALL MASONRY WALL SHALL BE LOAD BEARING IN THIS LEVEL AND REINFORCED W/ #5 @ 24", U.O.N.

LEGEND:
- EF — EXISTING WALL FOOTING (F.V.)
- EWF–24 — EXISTING WALL FOOTING (F.V.)
- ECC — EXISTING CONCRETE COLUMN (F.V.)
- AC — NEW ALUMINUM COLUMN
- EXISTING 8" CMU WALL
- NEW LOAD–BEARING REINFORCED CMU WALL
- EXISTING CONC. LOAD BEARING COLUMN OR TIE COLUMN
- NEW CONC. LOAD BEARING COLUMN OR TIE COLUMN
- DENOTES COLUMN STARTING ABOVE THIS LEVEL

IMPACT WALL NOTES:
CRASH/IMPACT WALLS ARE REQUIRED WHEREVER WALLS ARE EXPOSED TO VEHICULAR TRAFFIC. SEE SECTIONS WHERE APPLICABLE. CONTRACTOR TO SUBMIT RFI TO CLARIFY, IF SECTION IS NOT PROVIDED IN THIS DRAWING SET THROUGH AN AREA EXPOSED TO VEHICULAR TRAFFIC. CRASH/IMPACT WALLS WILL BE "BOND BEAM" WALLS, SEE SECTIONS.
"BOND BEAM" WALL: MUST HAVE A 2'–8" HIGH BOND BEAM IMPACT WALL. SEE SECTIONS FOR REINFORCING.

REFERENCE NOTE:
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S–1.0.
- FOR CONCRETE STRENGTH @ 28 DAYS SEE SHEET S–1.0.
- FOR CONCRETE COLUMNS/TIE COLUMNS SCHEDULES SEE SHEET S–2.1.
- FOR CONCRETE BEAM SCHEDULES SEE SHEET S–2.1.
- FOR STRUCTURAL TYPICAL DETAILS SEE SHEETS S–3.1, S–3.2 & S–3.3.
- FOR WIND PRESSURES SEE SHEET S–5.1.



KEY PLAN
SCALE 1/64"=1'-0"

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STRUCTURAL DESIGN
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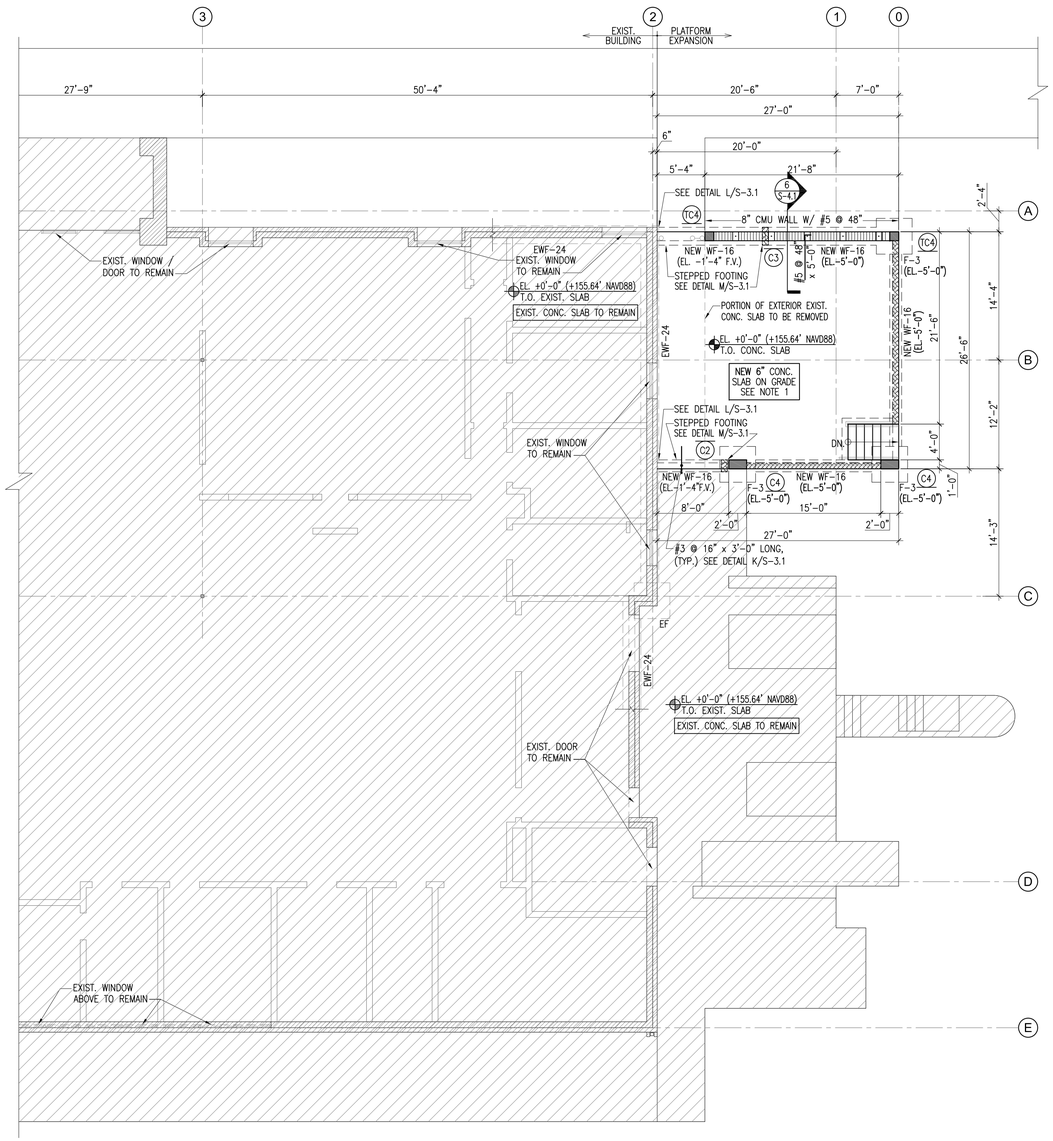
BUILDING DESIGN STANDARDS, 10/1/2019



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PARTIAL FOUNDATION / GROUND FLOOR PLAN

SCALE 1/8"=1'-0"

PLAN NOTES:

1.– NEW 6" CONCRETE SLAB OVER WELL COMPACTED FILL REINFORCED WITH 6x6 W2.1 xW2.1 WELDED WIRE FABRIC AT MID DEPTH, TYPICAL U.O.N.
REFER TO DETAIL A/S–3.1 FOR TYPICAL CONCRETE SLAB CONTROL JOINT (C.J.).
2.– TOP OF FOOTING ELEVATION: SEE SECTIONS.
3.– PROVIDE 1 #5 IN GROUTED CELLS AT EACH SIDE OF ALL WALL OPENINGS OF WIDTH EQUAL OR LESS THAN 5'–11".
PROVIDE 2 #5 IN GROUTED CELLS AT EACH SIDE OF ALL WALL OPENING OF WIDTH BETWEEN 6'–0" AND 7–11" U.N.O.
PROVIDE 1 #5 IN GROUTED CELLS AT ALL WALL INTERCEPTIONS.
4.– PROVIDE STANDARD HOOK FOR ALL TOP BARS AT DISCONTINUOUS END.
5.– FOR WATER PROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.
6.– SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS. ALL DIMENSIONS SHOWN HERE SHALL BE VERIFIED WITH ARCHITECTURAL DRAWINGS. IN CASE OF DISCREPANCY ARCHITECTURAL DRAWINGS SHALL CONTROL.

SUPERIMPOSED LOADS		
WAREHOUSE	DEAD	10 PSF
	LIVE	250 PSF

SOIL STATEMENT:
FOUNDATION SYSTEM HAS BEEN DESIGNED USING A LOAD BEARING CAPACITY OF 2000 PSF, BASED ON THE GEOTECHNICAL REPORT BY ARDAMAN & ASSOCIATES, INC., DATED 04–13–2022. CONTRACTOR TO OBTAIN A COPY OF THE GEOTECHNICAL REPORT AND FOLLOW THE INSTRUCTIONS INDICATED IN THE REPORT.

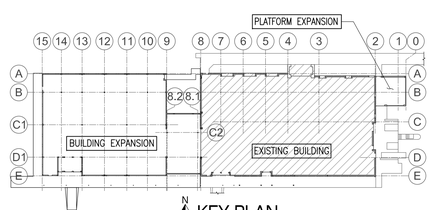
CONCRETE MASONRY NOTE:
ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90 "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS", WITH A NET AREA COMPRESSIVE STRENGTH OF MASONRY OF 1900 PSI.
ALL MASONRY WALL SHALL BE LOAD BEARING IN THIS LEVEL AND REINFORCED W/ #5 @ 24", U.O.N.

IMPACT WALL NOTES:
CRASH/IMPACT WALLS ARE REQUIRED WHEREVER WALLS ARE EXPOSED TO VEHICULAR TRAFFIC. SEE SECTIONS WHERE APPLICABLE. CONTRACTOR TO SUBMIT RFI TO CLARIFY, IF SECTION IS NOT PROVIDED IN THIS DRAWING SET THROUGH AN AREA EXPOSED TO VEHICULAR TRAFFIC. CRASH/IMPACT WALLS WILL BE "BOND BEAM" WALLS, SEE SECTIONS.
"BOND BEAM" WALL: MUST HAVE A 2'–8" HIGH BOND BEAM IMPACT WALL. SEE SECTIONS FOR REINFORCING.

LEGEND:

EF	EXISTING WALL FOOTING (F.V.)
EWF–24	EXISTING WALL FOOTING (F.V.)
	EXISTING 8" CMU WALL
	NEW LOAD–BEARING REINFORCED CMU WALL
	NEW 8" CONC. WALL BELOW OR CONC. COLUMN/TIE COLUMN BELOW
	NEW CONC. LOAD BEARING COLUMN OR TIE COLUMN
	DENOTES COLUMN STARTING ABOVE THIS LEVEL

REFERENCE NOTE:
– FOR GENERAL STRUCTURAL NOTES SEE SHEET S–1.0.
– FOR CONCRETE STRENGTH @ 28 DAYS SEE SHEET S–1.0.
– FOR CONCRETE COLUMNS/TIE COLUMNS SCHEDULES SEE SHEET S–2.1.
– FOR CONCRETE BEAM SCHEDULES SEE SHEET S–2.1.
– FOR STRUCTURAL TYPICAL DETAILS SEE SHEETS S–3.1, S–3.2 & S–3.3.
– FOR WIND PRESSURES SEE SHEET S–5.1.



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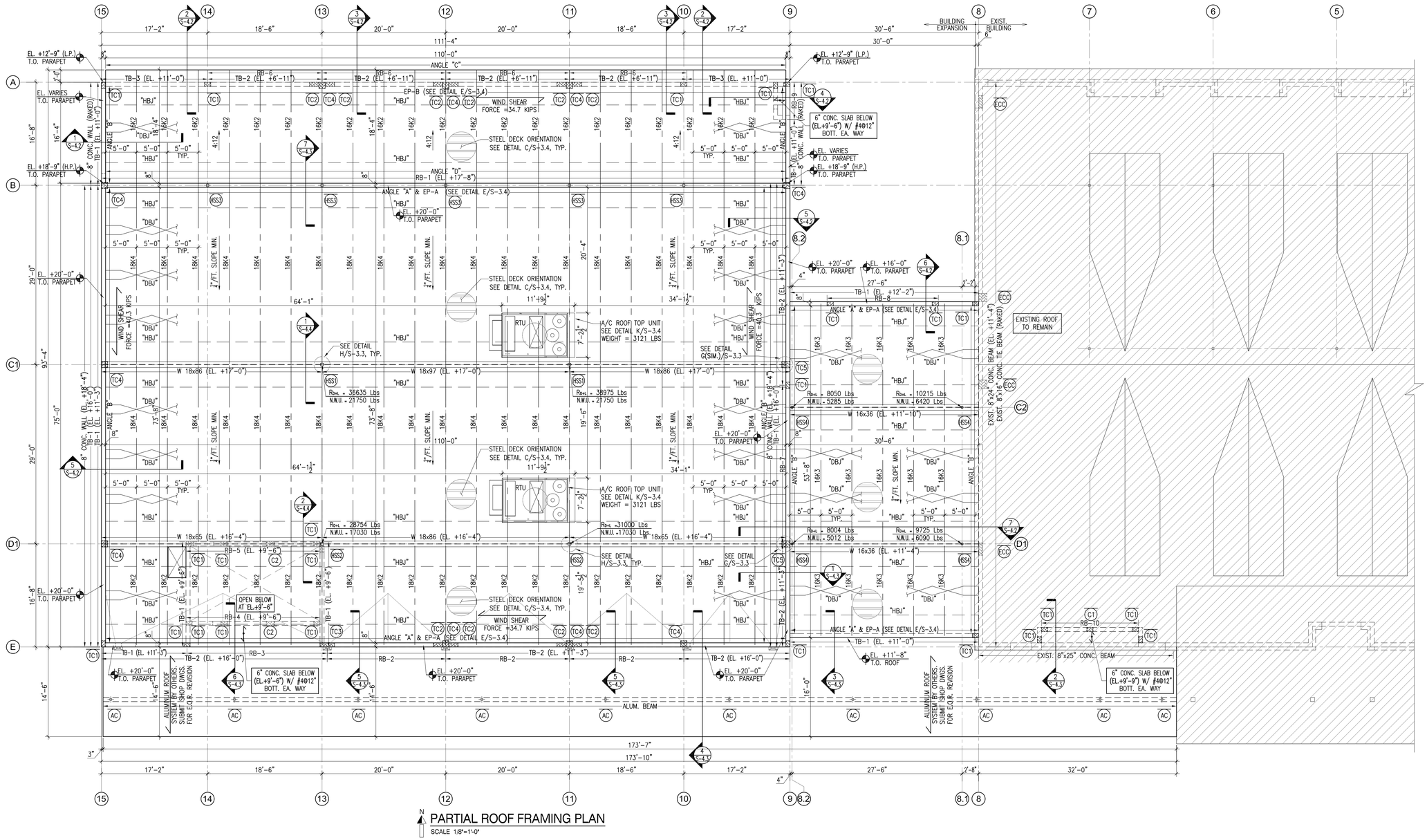


PARTIAL ROOF FRAMING PLAN

SCALE 1/8"=1'-0"

PLAN NOTES:

1.–ROOF ASSEMBLY:
- A.) STEEL DECK SHALL CONFORM TO THE CURRENT AISC "SPECIFICATIONS FOR THE DESIGN OF LIGHT GAUGE COLD FORMED STEEL STRUCTURAL MEMBERS" AND SDI. "DESIGN MANUAL FOR FLOORS AND DECKS".
- B.) RIGID INSULATION ON A 1.5B GALV. METAL DECK 18 GA. SUPPORTED ON STEEL JOISTS SPACED AS SHOWN ON PLAN. DECKS TO BE IN SHEET LENGTHS TO SPAN THREE (3) OR MORE SUPPORTS FOR ALL ZONES. METAL DECK SHALL BE GRADE A, Fy=33 K.S.I. MINIMUM. THICK=0.0474 IN. MIN. I=0.292 IN4. MIN. Sp=0.327 IN3.
- C.) ZONE A. METAL DECK TO BE CONNECTED TO STEEL SUPPORTS: ⅜" ø PUDDLE WELDS. WELDING PATTERN: 36/7
  - SIDE LAPS: 1½" LONG ARC-SEAM WELD FOR 2" LONG FILLET WELD SPACED AT 6" O.C.
  - PROVIDE ⅜" ø PUDDLE WELDS @ 6" AT EDGE SUPPORT OVER ANGLES, TYP. U.N.O.

  ZONE B. METAL DECK TO BE CONNECTED TO STEEL SUPPORTS: ⅜" ø PUDDLE WELDS. WELDING PATTERN: 36/7
  - SIDE LAPS: 1½" LONG ARC-SEAM WELD OR 2" LONG FILLET WELD SPACED AT 15" O.C.
  - PROVIDE ⅜" ø PUDDLE WELDS @ 6" AT EDGE SUPPORT OVER ANGLES, TYP. U.N.O.
- D.) DECKS SHALL BE CAPABLE OF SUPPORTING DESIGN LOADS NOTED IN PLAN.
- E.) NO LOAD SHALL BE HUNG FROM STEEL DECK. USE ADDITIONAL STEEL FRAMING BETWEEN STEEL SUPPORTS TO SUPPORT CEILINGS, EQUIPMENT, ACCESSORIES, ETC.

2.–BRIDGING: A SINGLE LINE OF BOTTOM CHORD BRIDGING MUST BE PROVIDED NEAR THE FIRST BOTTOM CHORD PANEL POINT (UPLIFT BRIDGING) AT EACH END OF THE JOISTS, FOR ALL JOISTS (SEE DETAIL B/S–3.4). PROVIDE BOLTED DIAGONAL (CROSS) BRIDGING CONNECTED AT JOIST AS PER S.J.I. (SEE DETAIL A/S–3.4). BRIDGING SIZES: FOR K SERIES JOIST USE: L 1"x1"x ¼"

3.–STEEL ANGLES:
- ANGLE "A": L 4"x4"x ¼" CONT. WELDED TO STEEL JOISTS WITH ¼" FILLET WELD, AND FASTENED TO CONC. W/ (1) ⅝" ø @ 18" HILTI KB3 ANCHOR, EMBEDMENT=4¼" (TYP.).
- ANGLE "B": L 4"x4"x ¼" CONT. FASTENED TO CONC. W/ (1) ⅝" ø @ 18" HILTI KB3 ANCHOR, EMBEDMENT=4¼" (TYP.).
- ANGLE "C": L 4"x4"x ¼" CONT. BENT PLATE WELDED TO STEEL JOISTS WITH ¼" FILLET WELD (TYP.).
- ANGLE "D": L 4"x4"x ¼" CONT. BENT PLATE WELDED TO STEEL JOISTS WITH ¼" FILLET WELD, AND FASTENED TO CONC. W/ (1) ⅝" ø @ 18" HILTI KB3 ANCHOR, EMBEDMENT=4¼".

ANGLES A, B, C & D SHALL BE CONTINUOUS ALONG THE ENTIRE PERIMETER. SEE DETAIL F/S–3.4 FOR SPLICE DETAIL.

4.–ALL STEEL JOISTS SUPPORTED ON TOP OF CONCRETE BEAMS/TIE BEAMS SHALL BE WELDED TO: FOR K SERIES JOISTS: 6"x8"x ⅜" STEEL PLATE WITH (2) ½" ø x 0'–5" LONG STUDS WELDED TO PLATE EMBEDDED IN CONCRETE, SEE DETAIL E/S–3.4.

5.–COORDINATE SIZE AND LOCATION OF ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. SEE DETAIL K/S–3.4.

6.–DESIGN LOADS GRAVITY. BUILDING ROOF: SDL=30 PSF LL=20 PSF.

7.–STEEL AND JOISTS DESIGN METHOD: ASD.

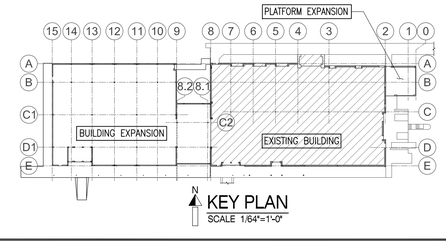
8.–FOR WATER PROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.

9.–SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS. ALL DIMENSIONS SHOWN HERE SHALL BE VERIFIED WITH ARCHITECTURAL DRAWINGS. IN CASE OF DISCREPANCY ARCHITECTURAL DRAWINGS SHALL CONTROL.

LEGEND:

⊠	CONC. COLUMN OR TIE COLUMN BELOW
AC	NEW ALUMINUM COLUMN
○	DENOTES COLUMN BELOW
"DBJ"	DIAGONAL BRIDGING SEE DETAIL A/S–3.4, TYP.
"HBJ"	HORIZONTAL BRIDGING SEE DETAIL B/S–3.4, TYP.

REFERENCE NOTE:
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S–1.0.
- FOR CONCRETE STRENGTH @ 28 DAYS SEE SHEET S–1.0.
- FOR CONCRETE COLUMNS/TIE COLUMNS SCHEDULES SEE SHEET S–2.1.
- FOR CONCRETE BEAM SCHEDULES SEE SHEET S–2.1.
- FOR STRUCTURAL TYPICAL DETAILS SEE SHEETS S–3.1, S–3.2, S–3.3 & S–3.4.
- FOR WIND PRESSURES SEE SHEET S–5.1.



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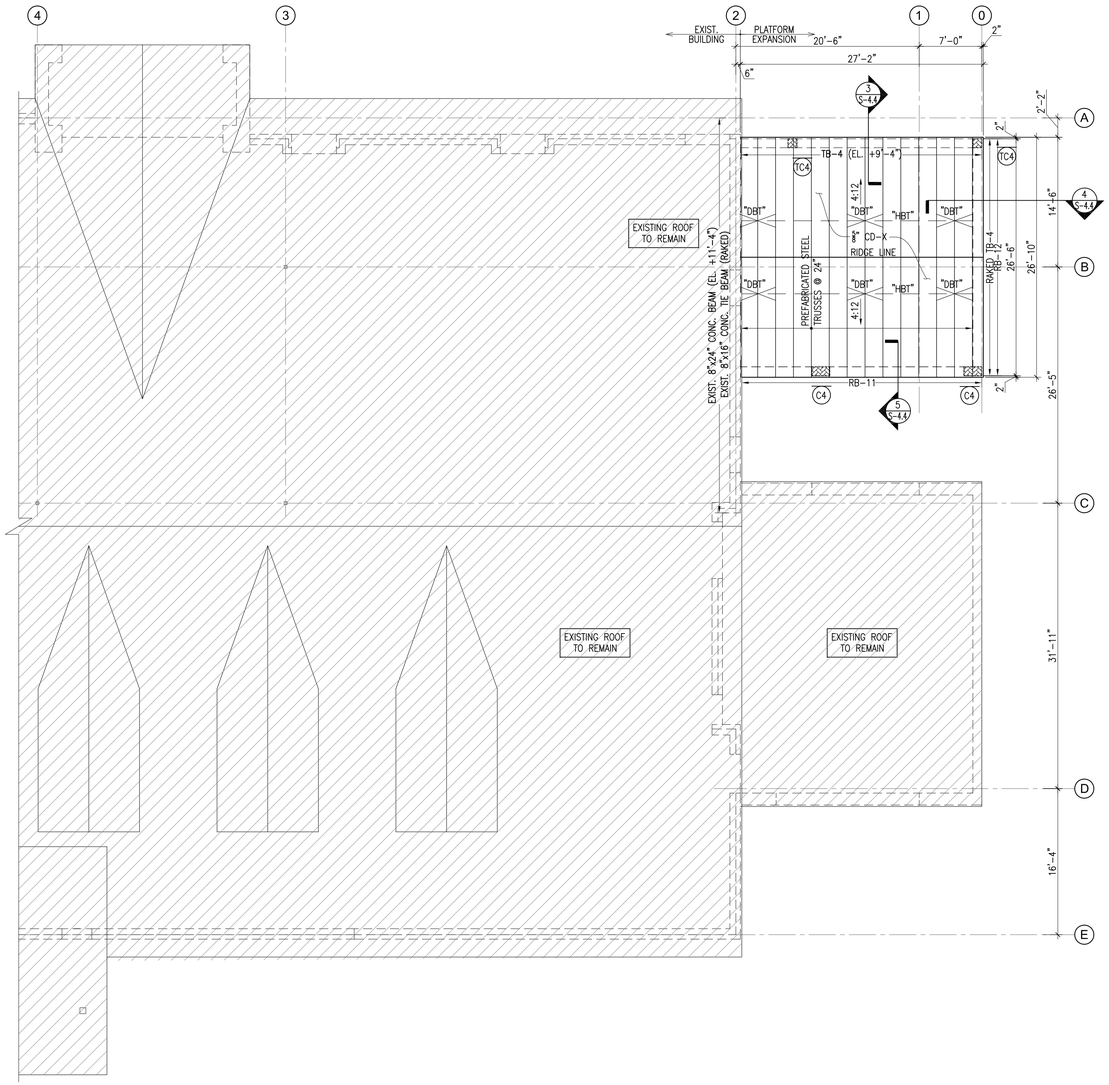


PARTIAL ROOF FRAMING PLAN

SCALE 1/8"=1'-0"

PLAN NOTES:

1.– COLD-FORMED METAL TRUSSES: ⅝" PLYWOOD SHEATHING FASTENED TO AN STEEL METAL DECK AND OVER A PRE-ENGINEERED AND PRE-FABRICATED COLD-FORMED METAL TRUSSES SPACED AT 24".
SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY FLORIDA REGISTERED PROFESSIONAL ENGINEER.
METAL DECK AND FASTENERS AS PER DETAIL G/S-3.4. PLYWOOD SHEATHING AS PER DETAIL H/S-3.4.
ROOFING INSTALLER SHALL PROVIDE SHOP DRAWINGS FOR APPROVAL, INCLUDING TYPE OF SCREWS WITH N.O.A.
2.– SCREWING REQUIREMENTS: SIZE & TYPE: USE #12-24x2½" WOOD TO METAL TEK SCREWS. SCREWS SHALL BE CORROSION RESISTANT.
SCREW SPACING: 4" O.C. AT PANEL EDGES.
6" O.C. AT INTERMEDIATE SUPPORTS.
3.– TRUSS DESIGN LOAD: SDL: 30 PSF LL: 20 PSF.
UPLIFT IN ACCORDANCE WITH THE FLORIDA BUILDING CODE AND ASCE 7-16.
METAL TRUSSES END CONNECTIONS SHALL BE WELDED TO AN EMBEDDED STEEL PLATE AS SHOWN IN ROOF SECTIONS.
4.– BRACING:
A. HORIZONTAL BRACING BOTTOM CHORD: PROVIDE ONE ROW OF HORIZONTAL BRACING AT PANEL POINTS AS SHOWN EVERY 10'-0" OF SPAN OF TRUSS.
B. DIAGONAL BRACING: PROVIDE CROSS BRACING, AS SHOWN, AT EACH END OF EACH ROW OF PERMANENT HORIZONTAL BRACING, AND AT NOT MORE THAN 20'-0" SPACING IN DIRECTION OF HORIZONTAL BRACING.
5.– STEEL AND TRUSSES DESIGN METHOD: ASD.
6.– FOR WATER PROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.
7.– SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS. ALL DIMENSIONS SHOWN HERE SHALL BE VERIFIED WITH ARCHITECTURAL DRAWINGS. IN CASE OF DISCREPANCY ARCHITECTURAL DRAWINGS SHALL CONTROL.

NOTES TO TRUSS DESIGNER MANUFACTURER:

– TRUSS CONCENTRATED LOAD REQUIREMENT: ANY SINGLE PANEL POINT OF THE LOWER CHORD OF ROOF TRUSSES SHALL BE CAPABLE OF SAFELY CARRYING A SUSPENDED, CONCENTRATED LOAD OF NOT LESS THAN 200 POUNDS (8896 N) IN ADDITION TO 10 PSF DEAD LOAD AS PRESCRIBED IN FBC 2319.17.2.1.3.
– TRUSSES SURFACE DESIGN LOAD:
DL = 15 PSF TOP CHORD AND 10 PSF BOTTOM CHORD
LL = 20 PSF TOP CHORD
– TEMPORARY BRACING SHALL ALWAYS BE REQUIRED DURING THE ERECTION OF ROOF TRUSSES. THE PROVISIONS FOR TEMPORARY BRACING SHOWN IN HIB-91 SHALL BE USED.
– PROVIDE VERTICAL TUBULAR MEMBER OR 2 CHANNEL WELDED TOGETHER TO ALLOW FOR SIDE PLATE CONNECTIONS. SEE ROOF SECTIONS.

TRUSS REACTION SCHEDULE

MARK	DL REACTION (Lbs)			LL REACTION (Lbs)			NET UPLIFT REACTION (Lbs)		
	LEFT	INTER	RIGHT	LEFT	INTER	RIGHT	LEFT	INTER	RIGHT
T-1	805	–	825	537	–	537	1261	–	1261

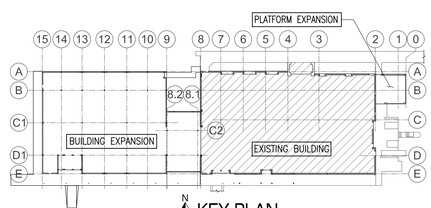
NOTE:
REFERENCE FOR LEFT AND RIGHT SUPPORT SHALL BE TAKEN WHEN THE OBSERVER INSIDE THE BUILDING IS FACING WEST OR FACING SOUTH.

LEGEND:

	CONC. COLUMN OR TIE COLUMN BELOW
	DENOTES COLUMN BELOW
"DBT"	DIAGONAL BRACING SEE DETAIL G/S-3.4, TYP.
"HBT"	HORIZONTAL BRACING SEE DETAIL H/S-3.4, TYP.
⅝" CD-X	⅝" CD-X PLYWOOD OVER PREFABRICATED STEEL TRUSSES @ 24". SEE DETAIL J/S-3.4

REFERENCE NOTE:

– FOR GENERAL STRUCTURAL NOTES SEE SHEET S-1.0.
– FOR CONCRETE STRENGTH @ 28 DAYS SEE SHEET S-1.0.
– FOR CONCRETE COLUMNS/TIE COLUMNS SCHEDULES SEE SHEET S-2.1.
– FOR CONCRETE BEAM SCHEDULES SEE SHEET S-2.1.
– FOR STRUCTURAL TYPICAL DETAILS SEE SHEETS S-3.1, S-3.2, S-3.3 & S-3.4.
– FOR WIND PRESSURES SEE SHEET S-5.1.



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FOOTING SCHEDULE

B=BOTTOM
T=TOP

MARK	SIZE		REINFORCEMENT		REMARKS
	DIMENSION	THICKNESS (IN)	LONG BARS	SHORT BARS	
F-1	3'-0"x3'-0"	12	4 #4 T&B	4 #4 T&B	
F-2	3'-6"x3'-6"	12	5 #4 T&B	5 #4 T&B	
F-3	4'-0"x4'-0"	12	6 #4 T&B	6 #4 T&B	
F-4	5'-0"x5'-0"	12	5 #5 T&B	5 #5 T&B	
F-5	5'-6"x5'-6"	12	6 #5 T&B	6 #5 T&B	
F-6	6'-6"x6'-6"	13	6 #5 T&B	6 #5 T&B	
F-7	6'-6"x6'-6"	16	8 #5 T&B	8 #5 T&B	
F-8	7'-0"x7'-0"	14	7 #5 T&B	7 #5 T&B	
F-9	4'-0"x4'-0"	12	6 #4 T&B	6 #4 T&B	(SEE DETAIL ON S-3.5)
F-10	7'-0"x10'-0"	14	#5 @ 12" T&B	#5 @ 12" T&B	(SEE DETAIL ON S-3.5)
F-11	4'-0"x4'-0"	24	———	———	ESTIMATED SIZE (SEE PLAN)

WALL FOOTING SCHEDULE

MARK	SIZE (WxDxL)	REINFORCING BARS		REMARKS
		TRANSVERSE	LONGITUDINAL	
WF-16	16"x12"x CONT.	#3 @ 36"	2 #5 CONT.	
WF-24	24"x12"x CONT.	#4 @ 12"	3 #5 CONT.	

STEEL COLUMN SCHEDULE

MARK	TYPE	BASE PLATE	CAP PLATE	HEADED STUDS				REMARKS
				NUMBER		SIZE ø / LENGTH "		
				BASE	CAP	BASE	CAP	
HSS1	HSS 6"x6"x¼"	12"x12"x¾"	8"x12"x½"	4	–	⅞"øx0'-8"	–	
HSS2	HSS 6"x6"x¼"	12"x12"x⅞"	8"x12"x½"	4	–	⅞"øx0'-8"	–	
HSS3	HSS 5"x5"x¼"	12"x12"x⅞"	8"x12"x⅞"	4	2	⅞"øx0'-8"	⅞"øx0'-8"	
HSS4	HSS 4"x4"x¼"	12"x12"x½"	6"x10"x½"	4	–	⅝"øx0'-8"	–	

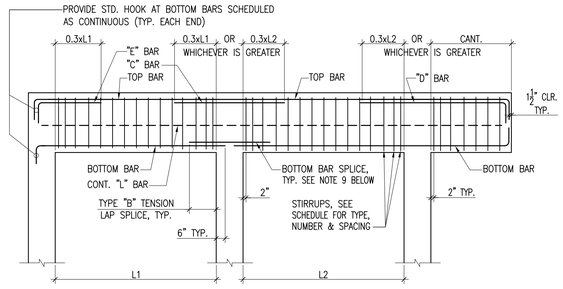
NOTES: 1. PROVIDE TWO COATS OF ANTI-CORROSIVE PAINT FOR ALL STEEL COLUMNS AND PLATES.
2. SIZE IS REFERRED TO THE OUTSIDE DIMENSION OF THE MEMBER.

BEAM / TIE BEAM SCHEDULE

B- BOTTOM
T- TOP

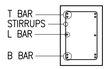
EE- EACH END
EC- EACH CORNER
EB- EACH BENT

MARK	TOP OF BEAM	SIZE (IN.) WxD	REINFORCING						STIRRUPS		REMARKS
			B	T	C	E	D	L	No.	SPACING	
RB-1	+17'-8"	8x24	2 #6	2 #6				1 #5 E.F.	3	@ 9"	
RB-2	+16'-0"	8x32	2 #6	2 #6					3	@ 11"	
RB-3	+11'-3"	8x27	2 #8	2 #8					3	@ 11"	
RB-4	+9'-6"	8x26	2 #6	2 #5					3	@ 11"	
RB-5	+9'-6"	8x12	2 #5	2 #5					3	@ 12"	
RB-6	+11'-0"	8x24	2 #5	2 #5					3	@ 10"	
RB-7	+11'-3"	8x27	4 #7*	2 #7					3	@ 11"	* IN TWO LAYERS
RB-8	+8'-8"	8x18	2 #7	2 #6					3	@ 7"	
RB-9	+11'-0"	8x24	2 #5	2 #5					3	@ 12"	
RB-10	+8'-3"	8x12	2 #5	2 #5					3	@ 12"	
RB-11	+9'-4"	12x24	2 #7	2 #7					3	@ 10"	
RB-12	+10'-10"	12x18	3 #8	3 #8					3	@ 6"	
TB-1	SEE PLAN	8x12 MIN.	2 #5	2 #5					3	4 @ 12" EE. EB. EC. BALANCED @ 36"	
TB-2	SEE PLAN	8x16	2 #5	2 #5				1 #5 E.F.	3	@ 12"	
TB-3	+11'-0"	8x24	2 #6	2 #6					3		
TB-4	+9'-4"	12x12	2 #5	2 #5					3		



TYPICAL BEAM BAR PLACING DIAGRAM

N.T.S.



BEAM SECTION

N.T.S.

NOTES:

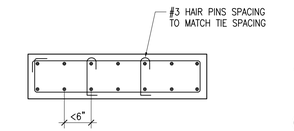
1.- "C" BARS ARE TOP BARS AT NON-CONTINUOUS ENDS.
2.- "E" BARS ARE TOP BARS OVER RIGHT INTERIOR SUPPORTS.
3.- TOP BARS CALLED FOR AS CONTINUOUS, WHEN SPLICED, SHALL BE SPLICED IN THE MIDDLE THIRD OF THE SPAN. THE SPLICE SHALL BE CLASS "B" TENSION LAP SPLICE.
4.- CONCRETE BEAM DEPTH, AS SCHEDULED, IS MINIMUM. IT CAN BE INCREASED AS REQUIRED TO MATCH BLOCK COURSING, TO FOLLOW A SLOPE, TO FORM ARCHES, OR TO BE USED AS A LINTEL OR TO ACCOMMODATE WINDOW & DOOR OPENINGS. REFER TO ARCH. DWGS. FOR OPENING SIZES. PROVIDE 2 #5 BOTTOM (ADD'L.) AND #3 @ 12" STIRRUPS IF INCREASE IN DEPTH EXCEEDS 4". EXTEND DROP PORTION OF TIE BEAM MINIMUM 8" BEYOND FACE OF OPENING.
5.- SEE ARCHED BEAM DETAIL FOR REQUIRED ADDITIONAL REINFORCING.
6.- ALL BARS OF BEAMS SHALL BE EXTENDED TO FAR FACE OF T.C., T.B. OR BEAM AT INTERCEPTIONS. TOP BARS SHALL BE PROVIDED WITH A STANDARD HOOK.
7.- BARS IN TWO OR MORE LAYERS: BARS IN UPPER LAYER SHALL BE PLACED DIRECTLY ABOVE BARS IN THE BOTTOM LAYER WITH AT LEAST 1 INCH CLEAR VERTICALLY BETWEEN LAYERS.
8.- E.E. (EACH END) FOR STIRRUPS REINFORCEMENTS IS DEFINED AS EACH COLUMNS OR WALL SUPPORT.
9.- PROVIDE ½ OF THE AREA OF THE BOTTOM REINFORCING OF THE ADJACENT SPAN WITH THE HIGHEST AREA OF BOTTOM REINFORCING, BUT NOT LESS THAN TWO BARS. BAR SIZE NO LESS THAN A #5 BAR. WHERE ADJACENT BEAMS ARE DIFFERENT IN DEPTH, PLACE THE SPLICE BAR AT THE LEVEL OF THE BOTTOM BARS OF THE SHALLOWEST BEAM. THE SPLICE SHALL BE CLASS "B" TENSION LAP SPLICE.

TIE - COLUMN & CONCRETE COLUMN SCHEDULE

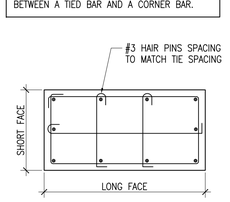
MARK	REINFORCING			REMARKS
	SIZE	VERTICAL	TIES	
TC1	8"x12"	4 #5	#3 @ 8"	
TC2	8"x18"	6 #5	#3 @ 8" W/ HAIRPIN	
TC3	8"x28"	8 #5	#3 @ 8" W/ HAIRPIN	
TC4	12"x12"	8 #5	#3 @ 10" W/ HAIRPIN	
TC5	12"x16"	8 #5	#3 @ 10"	
C1	8"x12"	4 #5	#3 @ 8"	
C2	8"x16"	6 #5	#3 @ 8"	
C3	8"x24"	8 #5	#3 @ 8"	
C4	12"x24"	10 #5	#3 @ 8"	
FC1	1 #5 IN 1 GROUT-FILLED CELLS			
FC2	2 #5 IN 2 GROUT-FILLED CELLS			

NOTE: SEE COLUMN BAR ARRANGEMENT DIAGRAM THIS PAGE FOR ALL COLUMNS.

TIE-COLUMN & COLUMN BAR ARRANGEMENT DIAGRAMS



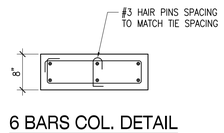
HAIR PIN NOTE OF REINFORCEMENT: PLACE HAIR PINS WHEN THE SPACING BETWEEN ADJACENT BARS IS MORE THAN 6", WHEN THE SPACING IS LESS THAN 6" ONE UNTIED BAR MAY BE LOCATED BETWEEN EACH TIED BAR OR BETWEEN A TIED BAR AND A CORNER BAR.



10 BARS
4L: 4 BARS ALONG LONG FACE, EA SIDE.
3S: 3 BARS ALONG SHORT FACE, EA SIDE.

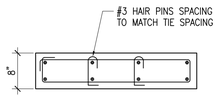
COL. DETAIL

SCALE 3/4"=1'-0"



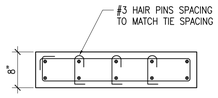
6 BARS COL. DETAIL

SCALE 3/4"=1'-0"



8 BARS COL. DETAIL

SCALE 3/4"=1'-0"



10 BARS COL. DETAIL

SCALE 3/4"=1'-0"

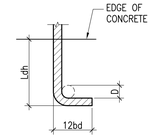
TENSION LAP SPLICE

BAR SIZE	SPLICE LENGTH (INCHES) 3000 PSI		SPLICE LENGTH (INCHES) 4000 PSI	
	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
# 3	28	22	24	19
# 4	37	29	32	25
# 5	47	36	40	31
# 6	56	43	48	37
# 7	81	63	70	54
# 8	93	72	80	62

MIN. TENSION EMBEDMENT LENGTH (Ldh) FOR STANDARD HOOKS

BAR SIZE	f'c (psi) 3000	f'c (psi) 4000
# 3	6"	6"
# 4	7"	7"
# 5	9"	9"
# 6	10"	10"
# 7	12"	12"
# 8	14"	14"

NOTES:
-SIDE CONC COVER >= 2½"
-D:
D=6db FOR #3 TO #8
D=8db FOR #9 TO #11
-db: BAR DIAMETER BAR



STANDARD 90° END HOOKS

TENSION EMBEDMENT TYPICAL DETAIL

N.T.S.

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S-2.1

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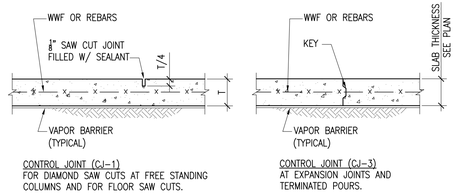
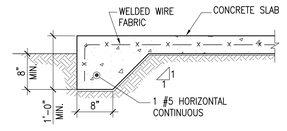


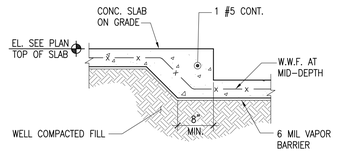
A S-3.1 CONTROL JOINT TYPICAL DETAILS

SCALE 3/4 "=1'-0"



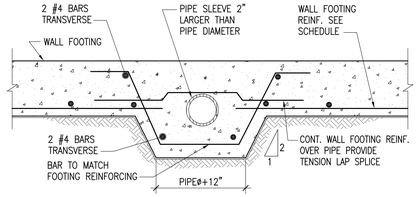
B S-3.1 TYPICAL SECTION THRU CONCRETE THICKENED EDGE

SCALE 3/4 "=1'-0"



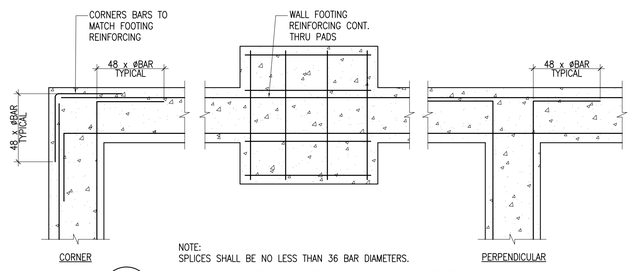
C S-3.1 STEP IN SLAB TYPICAL DETAIL

SCALE 3/4 "=1'-0"



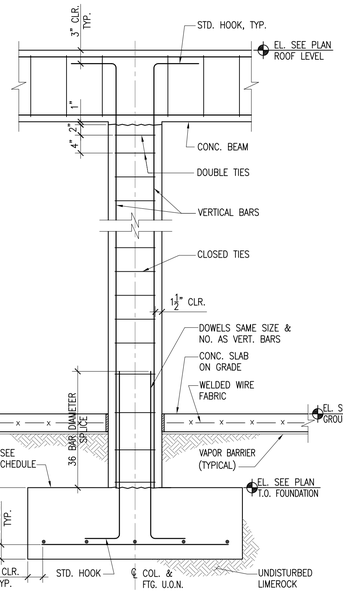
D S-3.1 PIPE SLEEVE THRU WALL FOOTING TYPICAL DETAIL

SCALE 3/4 "=1'-0"



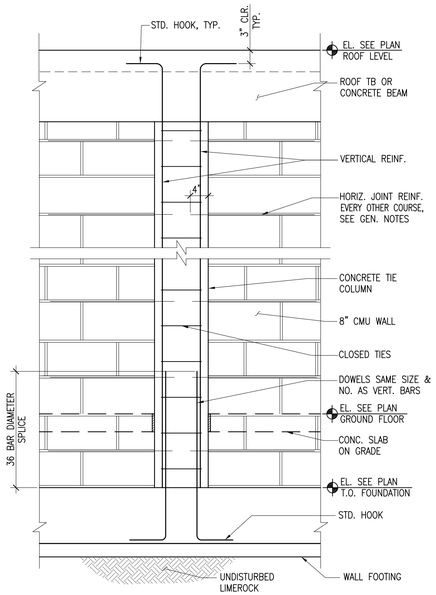
E S-3.1 FOOTING INTERSECTION TYPICAL DETAIL

SCALE 3/8 "=1'-0"



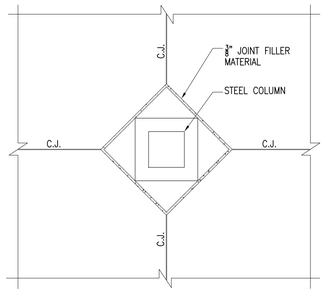
F S-3.1 TYPICAL COLUMN-FOOTING DETAIL

SCALE N.T.S



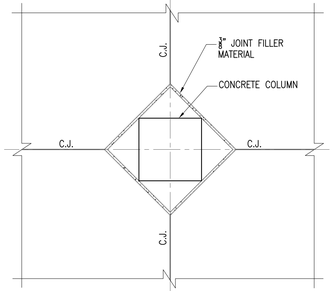
G S-3.1 TYPICAL TIE COLUMN / WALL FOOTING DETAIL

SCALE N.T.S



H S-3.1 CONTROL JOINT AROUND STEEL COLUMN TYPICAL DETAIL

SCALE N.T.S.



I S-3.1 CONTROL JOINT AROUND CONCRETE COLUMN TYPICAL DETAIL

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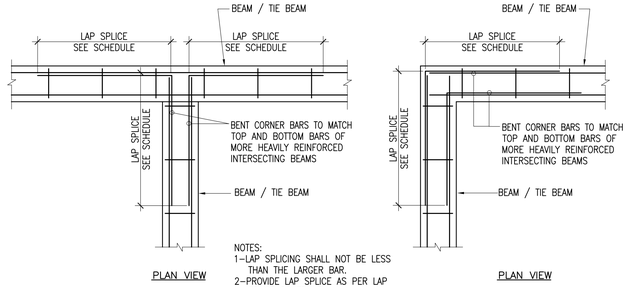
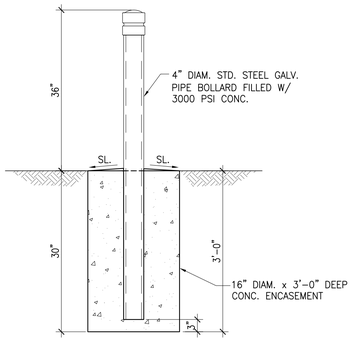


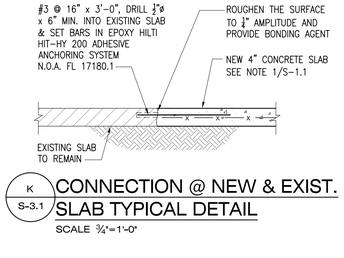
J S-3.1 BEAM CORNER BARS AND INTERSECTION DETAILS

SCALE ¾ "= 1'-0"



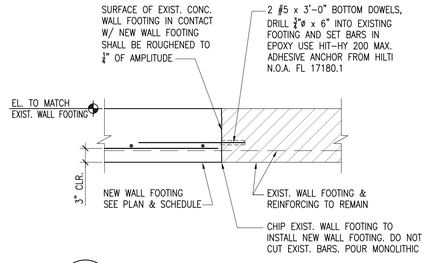
N S-3.1 BOLLARD FOOTING TYP. DETAIL

SCALE 3/4 "=1'-0"



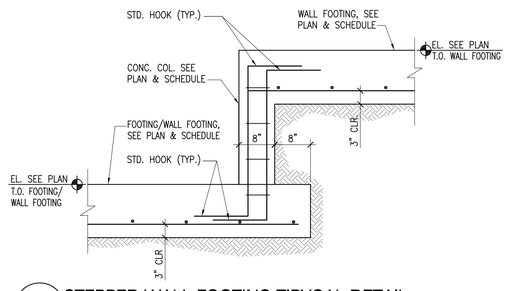
K S-3.1 CONNECTION @ NEW & EXIST. SLAB TYPICAL DETAIL

SCALE ¾"=1'-0"



L S-3.1 WALL FOOTING CONNECTION TYPICAL DETAIL

SCALE N.T.S.



M S-3.1 STEPPED WALL FOOTING TIPYCAL DETAIL

SCALE 3/4"=1'-0"

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S-3.1

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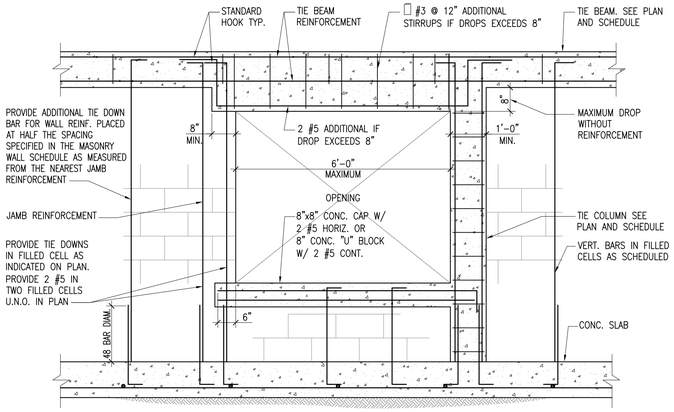
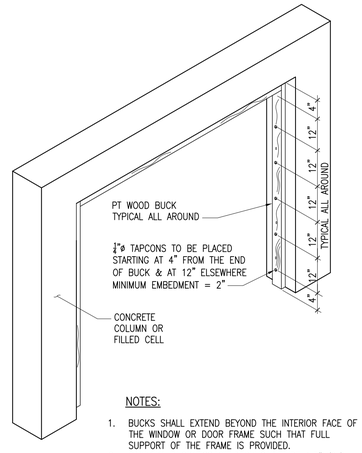


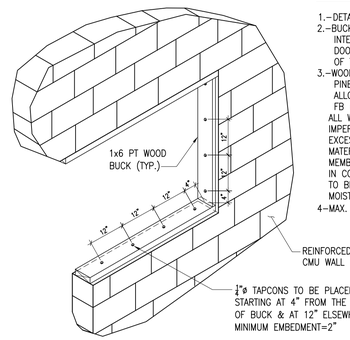
NOTE:
THIS IS A TYPICAL DETAIL SHOWING IN THE LEFT SIDE A MASONRY JAMB CONDITION AND IN THE RIGHT SIDE A TIE COLUMN CONDITION. REFER TO FLOOR PLAN TO DETERMINE WHICH OF THE TWO CONDITIONS APPLY TO EACH PARTICULAR OPENING.

A S-3.2 TYPICAL LINTEL / SILL / JAMB DETAIL
N.T.S. DROPPED TIE BEAM OPTION SIMILAR FOR DOOR OPENINGS



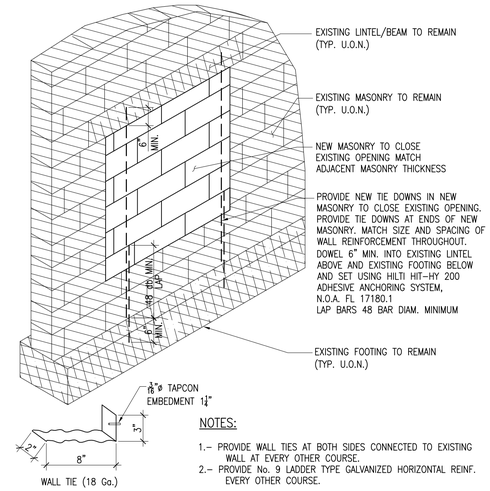
NOTES:
1. BUCKS SHALL EXTEND BEYOND THE INTERIOR FACE OF THE WINDOW OR DOOR FRAME SUCH THAT FULL SUPPORT OF THE FRAME IS PROVIDED.
2. MAXIMUM THICKNESS OF BUCK SHALL BE 1.5" (2x).

B S-3.2 DOOR BUCK CONNECTION DETAIL
N.T.S. IF REQUIRED



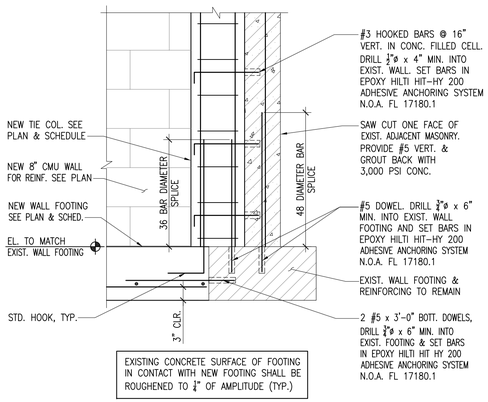
NOTES:
1.–DETAIL ALSO VALID FOR DOOR BUCKS.
2.–BUCKS SHALL EXTEND BEYOND THE INTERIOR FACE OF THE WINDOW OR DOOR FRAME SUCH THAT FULL SUPPORT OF THE FRAME IS PROVIDED.
3.–WOOD BUCK MEMBERS TO BE SOUTHERN PINE GRADE No. 2 MIN, WITH A MINIMUM ALLOWABLE BENDING STRESS OF FB = 1250 PSI.
ALL WOOD MEMBERS TO BE FREE OF ALL IMPERFECTIONS AS: SPLITS, CHECKS, OR EXCESSIVE KNOTS. UNSATISFACTORY MATERIALS TO BE REPLACED. ALL WOOD MEMBERS EXPOSED TO THE WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY TO BE PRESSURE TREATED.
MOISTURE CONTENT SHALL BE 19% OR LESS.
4–MAX. THICKNESS OF BUCK SHALL BE 1.5" (2x).

C S-3.2 WINDOW BUCK DETAIL
N.T.S. IF REQUIRED

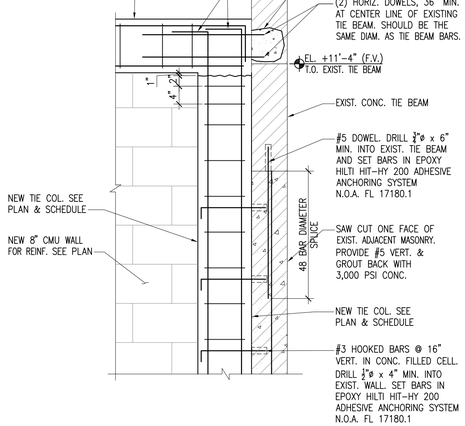


NOTES:
1.– PROVIDE WALL TIES AT BOTH SIDES CONNECTED TO EXISTING WALL AT EVERY OTHER COURSE.
2.– PROVIDE No. 9 LADDER TYPE GALVANIZED HORIZONTAL REINF. EVERY OTHER COURSE.

D S-3.2 CLOSE EXISTING WINDOW OPENING DETAIL
N.T.S.



E S-3.2 STARTER COLUMN DETAIL
SCALE N.T.S



F S-3.2 STARTER COLUMN DETAIL
SCALE N.T.S

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Project: 21–23
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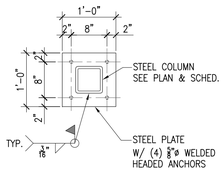
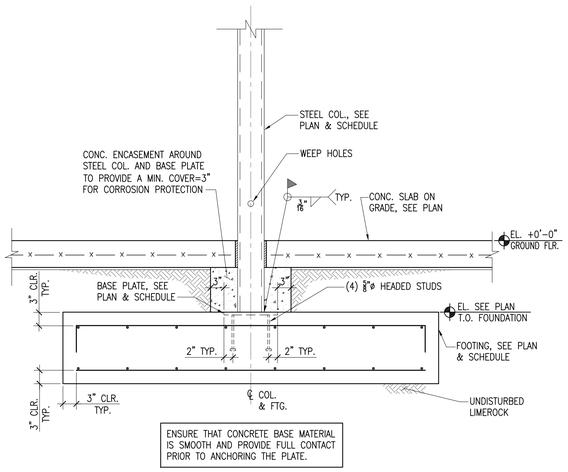
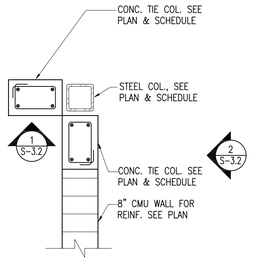


BASE PLATE PLAN VIEW

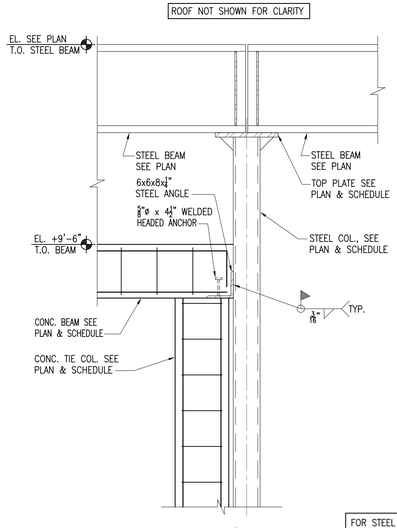


E S-3.3 STEEL COLUMN-FOOTING TYPICAL DETAIL

SCALE N.T.S.

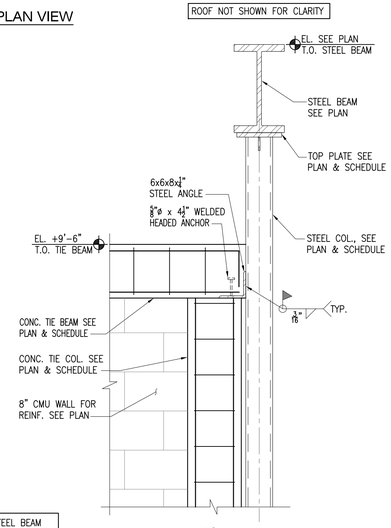


PLAN VIEW



ELEVATION 1

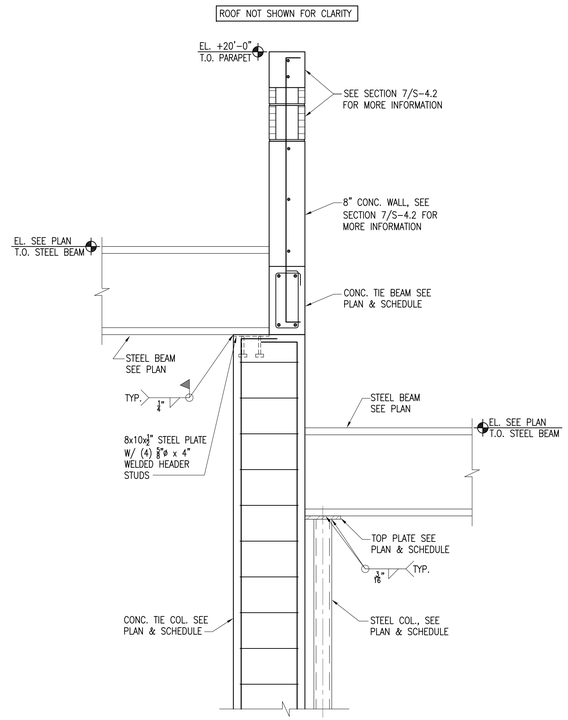
FOR STEEL COL. / STEEL BEAM CONNECTION, SEE DETAIL H/S-3.3



ELEVATION 2

F S-3.3 STEEL COLUMN/CONCRETE COLUMNS/BEAM CONNECTION DETAIL

SCALE N.T.S.



G S-3.3 STEEL COLUMN/CONCRETE COLUMN CONNECTION DETAIL

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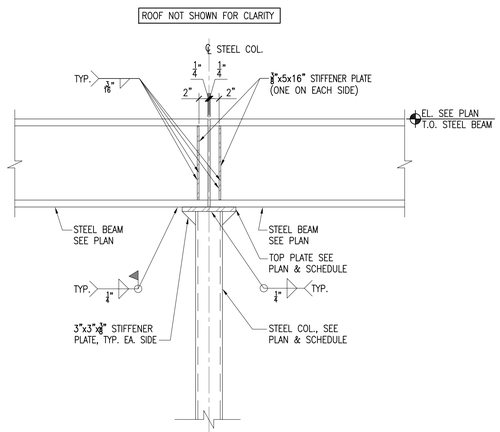

H S-3.3 STEEL COLUMN/STEEL BEAM CONNECTION DETAIL

SCALE N.T.S.

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S-3.3

Revisions:

Date: 06/16/2022

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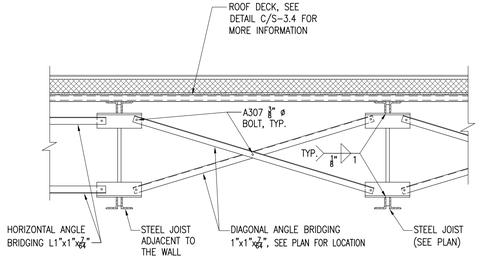
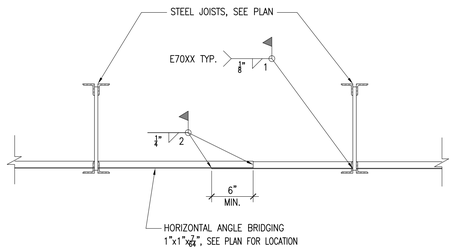


A S-3.4 DIAGONAL ANGLE BRIDGING FOR STEEL JOISTS TYPICAL DETAIL

N.T.S.



B S-3.4 HORIZONTAL ANGLE BRIDGING FOR STEEL JOISTS TYPICAL DETAIL

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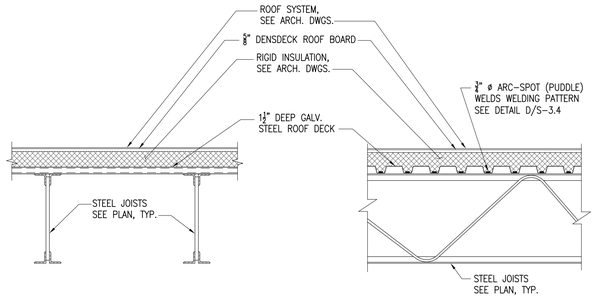

C S-3.4 ROOF DECK FOR STEEL JOISTS TYPICAL DETAIL

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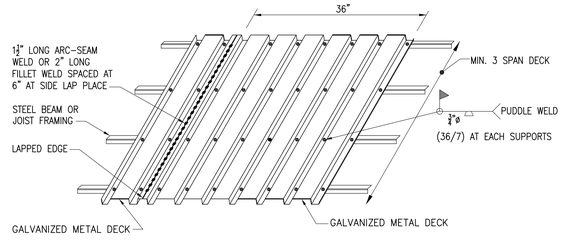

D S-3.4 DECK ATTACHMENT TO STEEL JOISTS TYPICAL DETAIL

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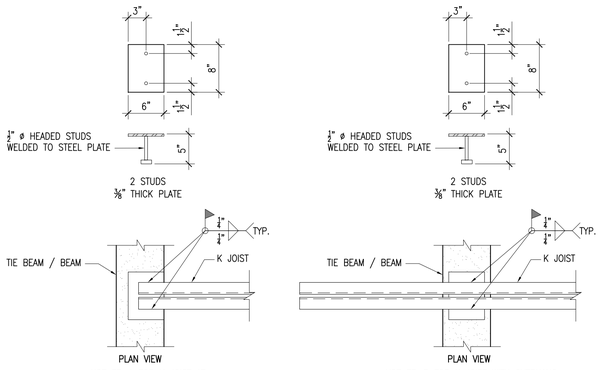
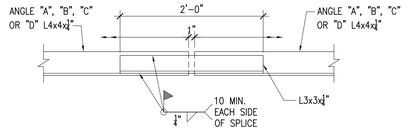


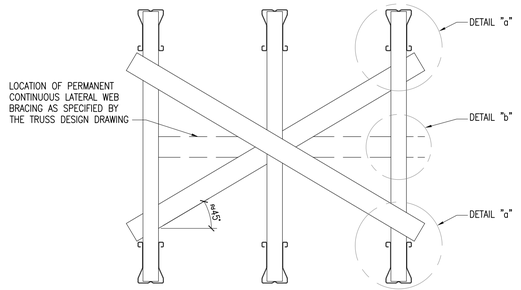
E S-3.4 EMBEDDED PLATE

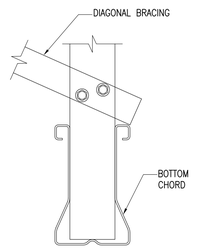
SCALE 1"=1'-0"

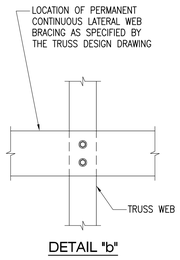


F S-3.4 SPLICE FOR ROOF CHORD ANGLE "A", "B", "C" OR "D" TYPICAL DETAIL

SCALE 1"=1'-0"







G S-3.4 PERMANENT LATERAL BRACING OF STEEL TRUSSES TYPICAL DETAIL

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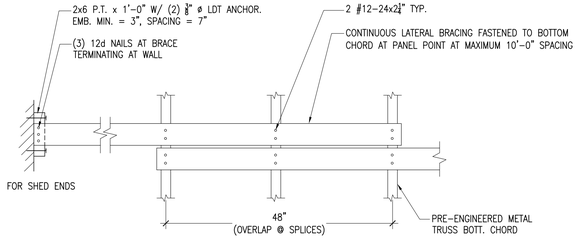

H S-3.4 PERMANENT HORIZONTAL LATERAL BRACING OF STEEL TRUSSES TYPICAL DETAIL

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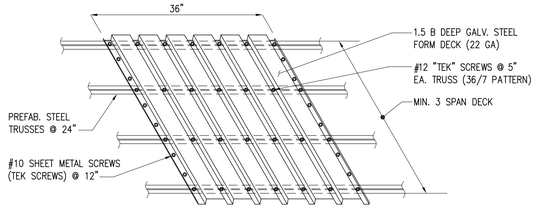

I S-3.4 DECK ATTACHMENT TO STEEL TRUSSES TYPICAL DETAIL

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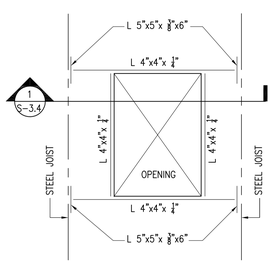
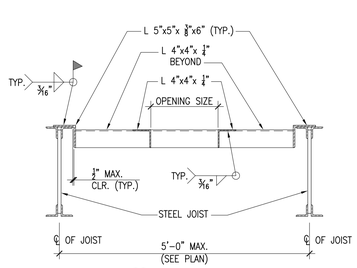
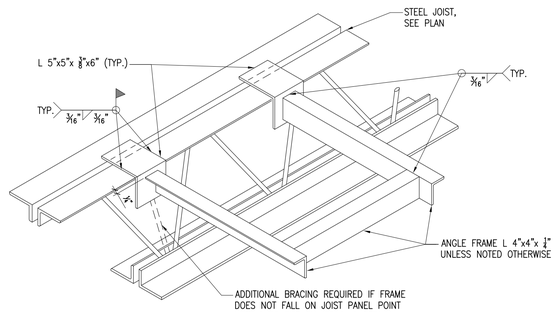


PLAN VIEW



1 S-3.4 SECTION



REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR EXACT SIZE AND LOCATION OF OPENINGS.
USE THIS DETAIL FOR ANY OPENING LARGER THAN 12" IN ANY DIRECTION OR EQUIPMENT WEIGHING IN EXCESS OF 50 POUNDS.

J S-3.4 OPENING AT ROOF TYPICAL DETAIL

N.T.S.

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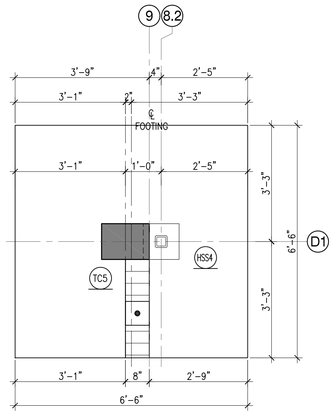
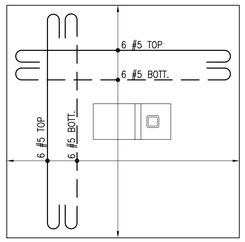


F-6 REFERENCE PLAN

SCALE $\frac{3}{4}$"=1'-0"



F-6 REINFORCING PLAN

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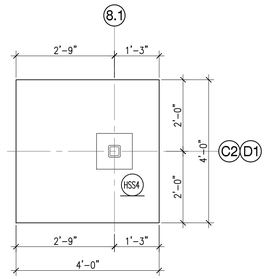

F-9 REFERENCE PLAN

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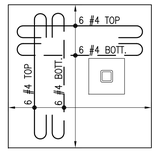

F-9 REINFORCING PLAN

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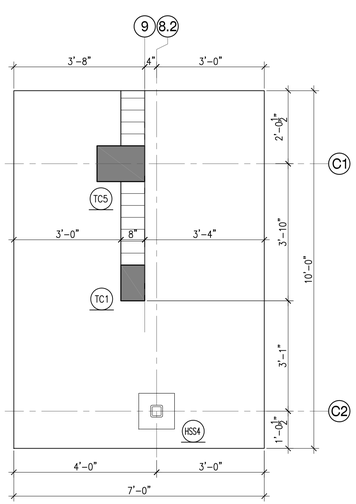

F-10 REFERENCE PLAN

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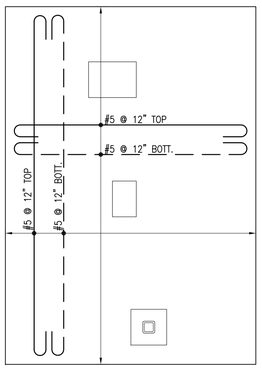

F-10 REINFORCING PLAN

SCALE $\frac{3}{4}$"=1'-0"

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Scale: NOTED Date: 06/16/2022
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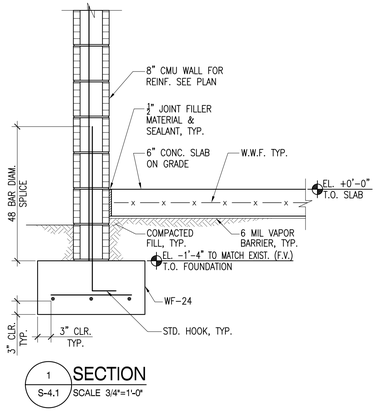
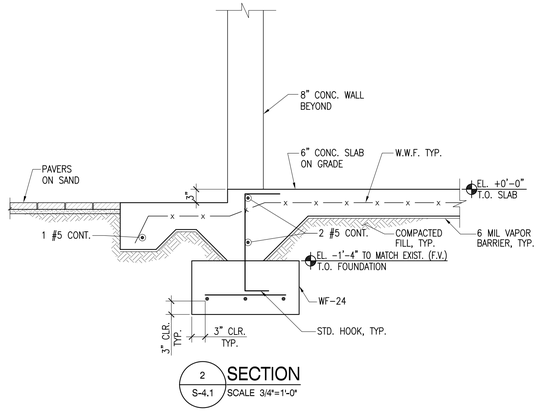


1 SECTION S-4.1 SCALE 3/4"=1'-0"



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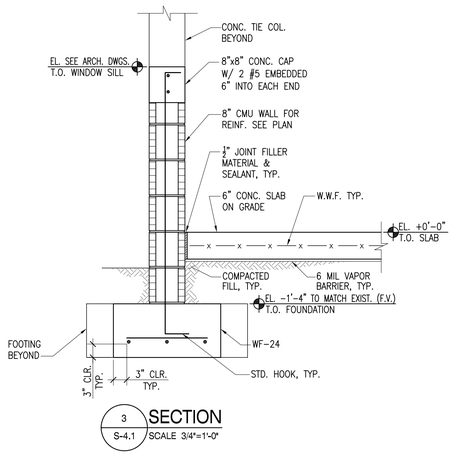

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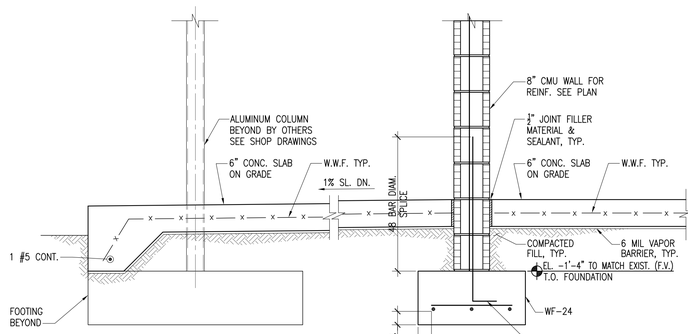
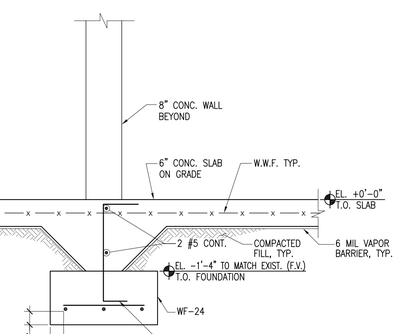
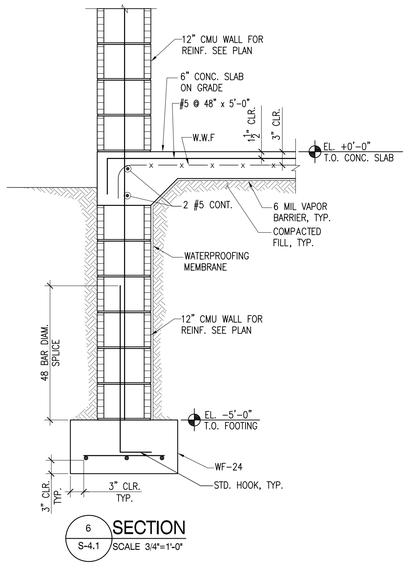


4 SECTION S-4.1 SCALE 3/4"=1'-0"



5 SECTION S-4.1 SCALE 3/4"=1'-0"



6 SECTION S-4.1 SCALE 3/4"=1'-0"

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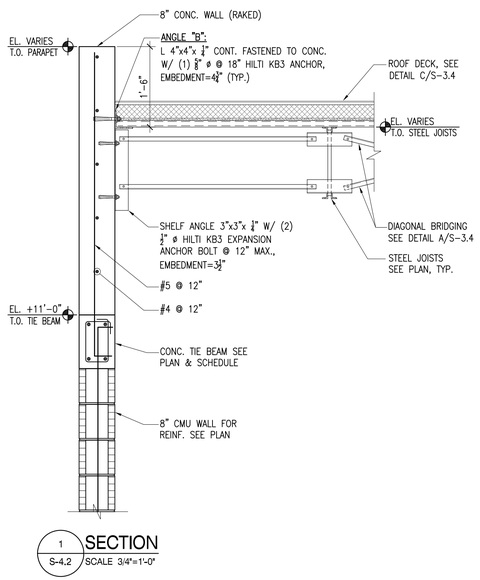

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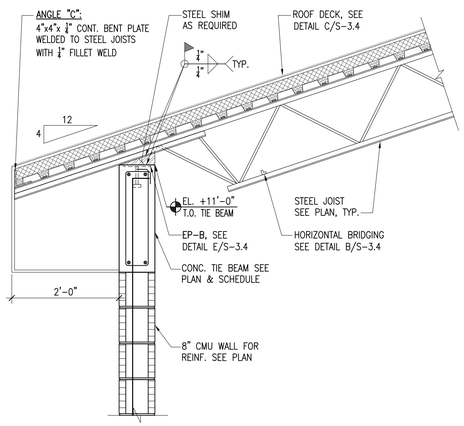

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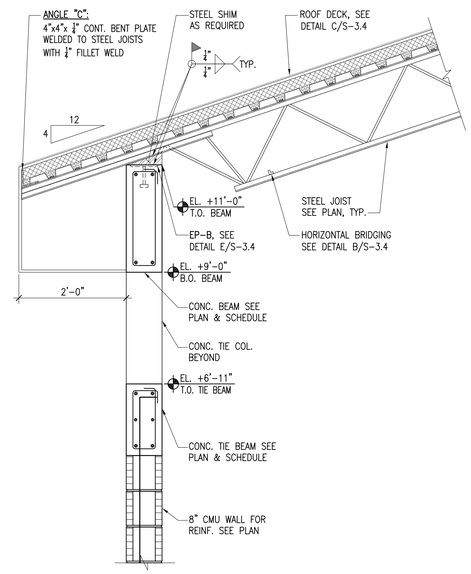

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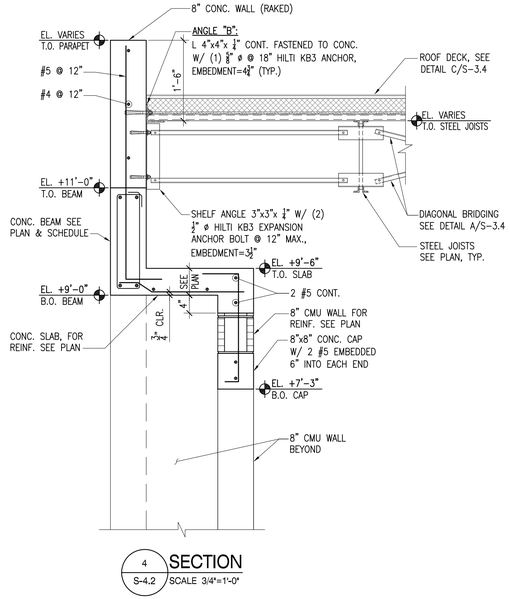

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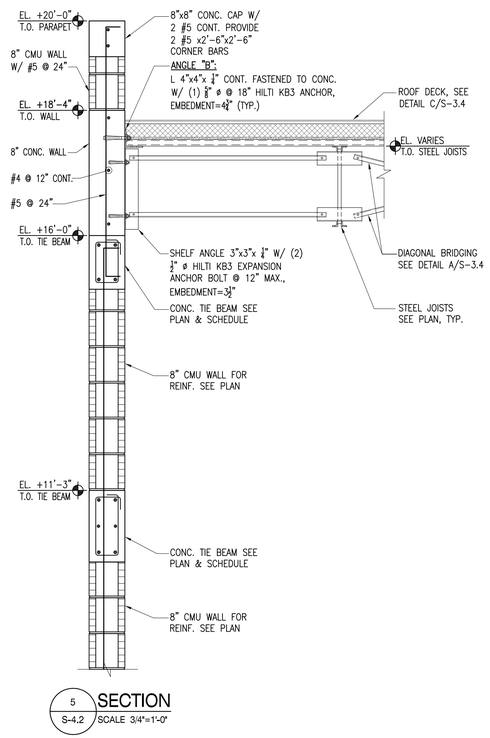

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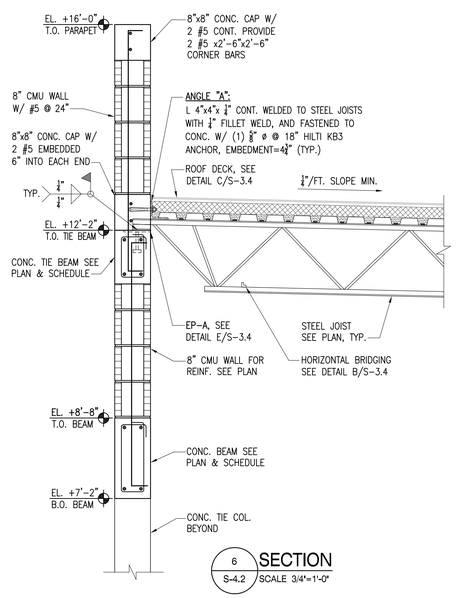

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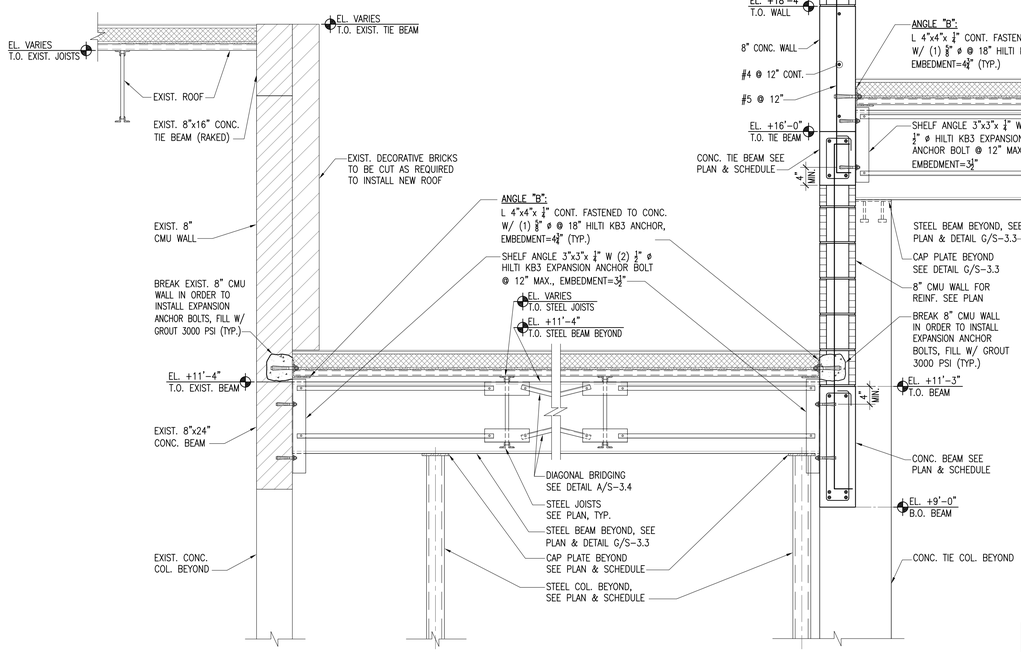

7 S-4.2 SECTION
SCALE 3/4"=1'-0"

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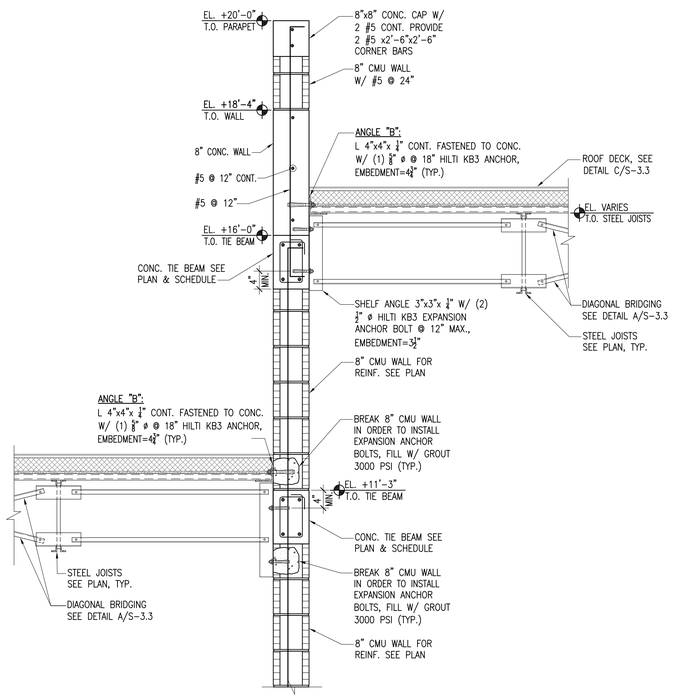

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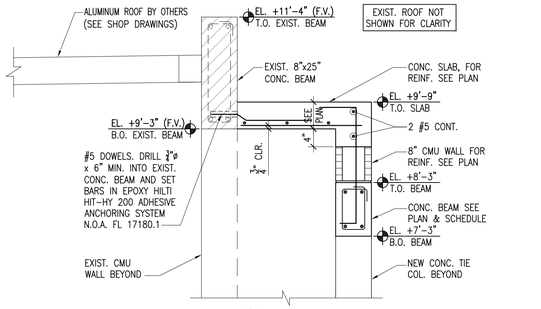

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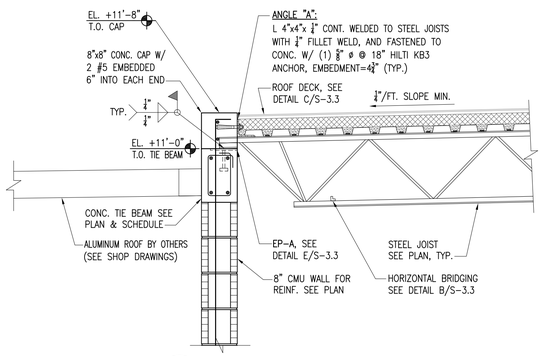

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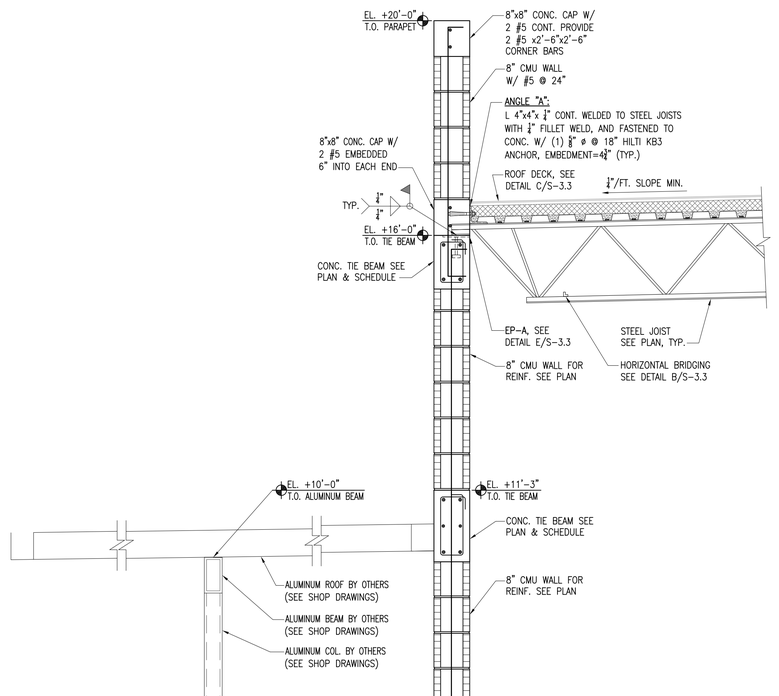

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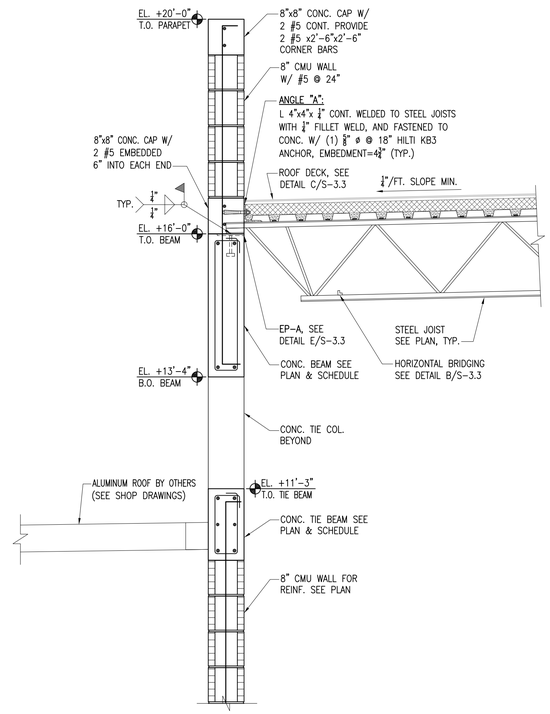

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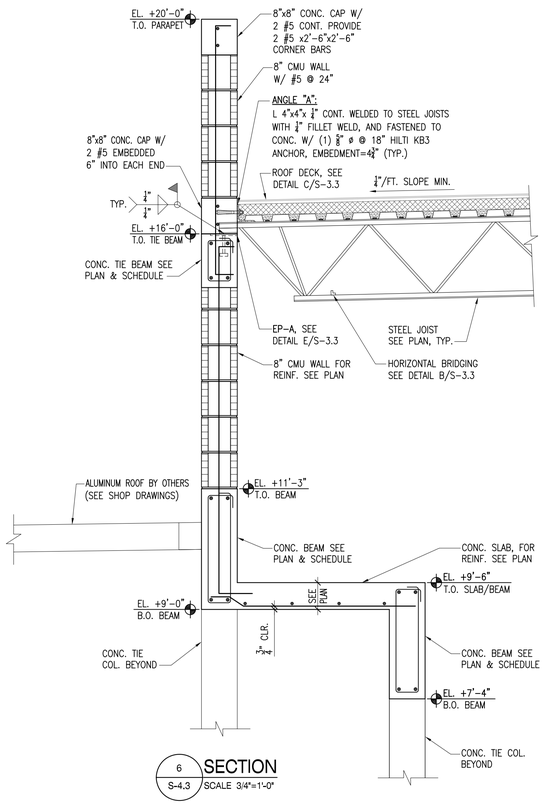

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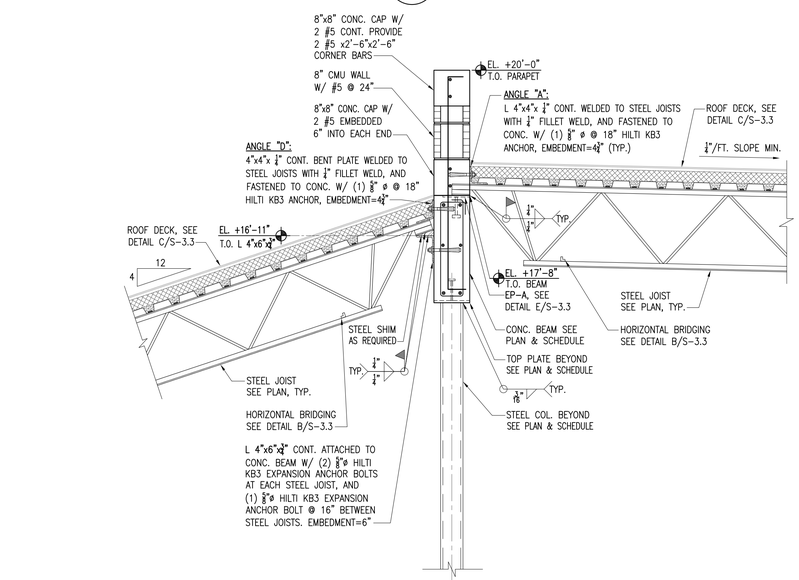

7 SECTION
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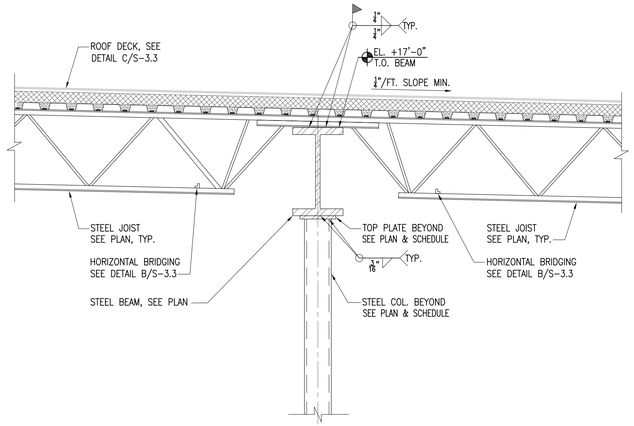
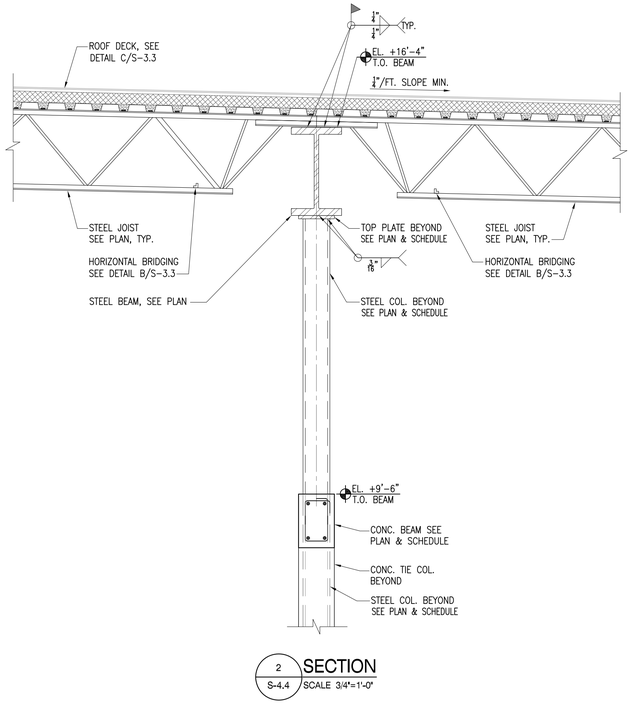


1 S-4.4 SECTION
SCALE 3/4"=1'-0"



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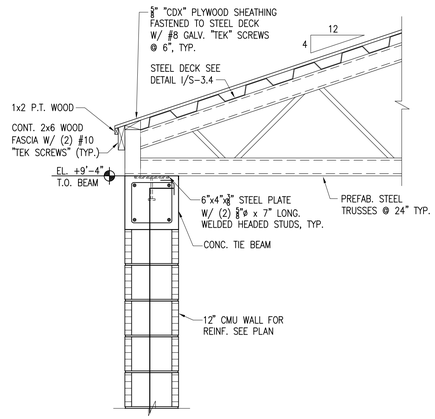

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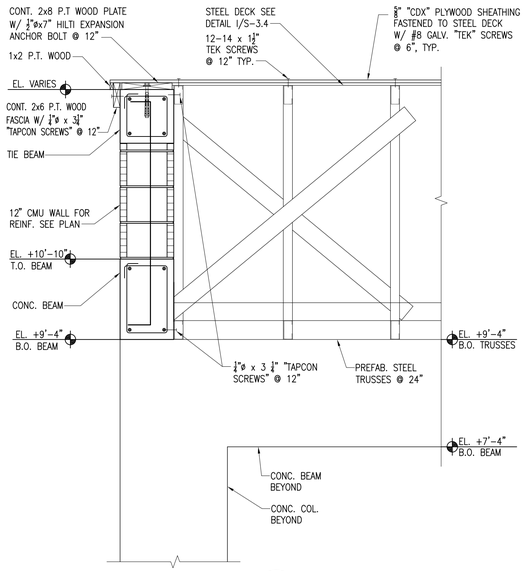

4 S-4.4 SECTION
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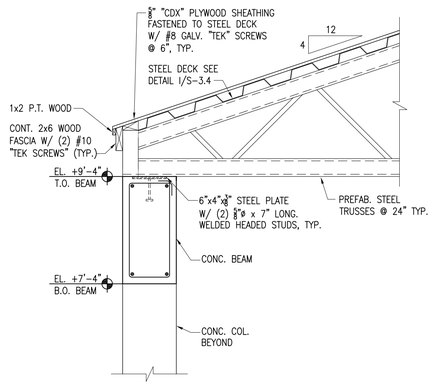

5 S-4.4 SECTION
SCALE 3/4"=1'-0"

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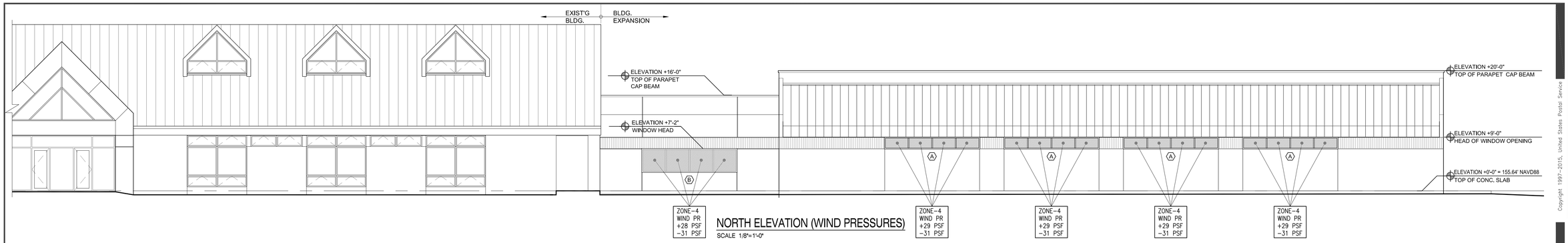
S-4.4

Scale: NOTED
Project: 21-23
USPS File Number: E54635

Date: 06/16/2022

Revisions:

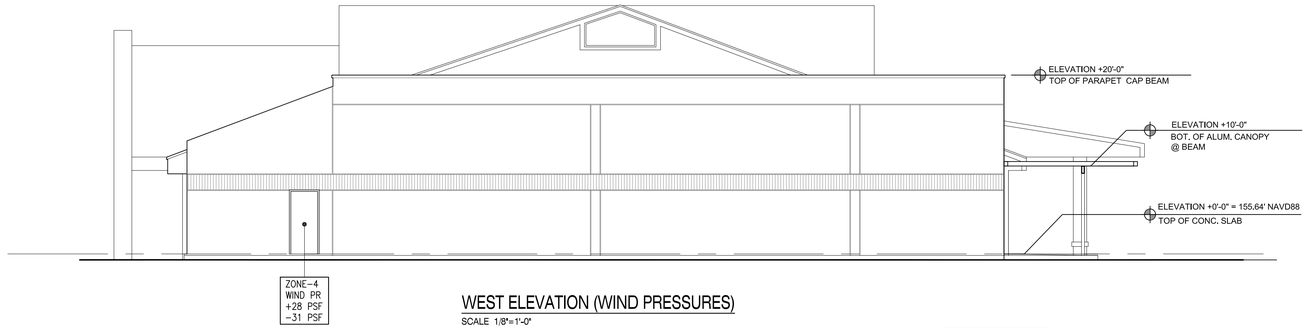


NORTH ELEVATION (WIND PRESSURES)

SCALE 1/8"=1'-0"

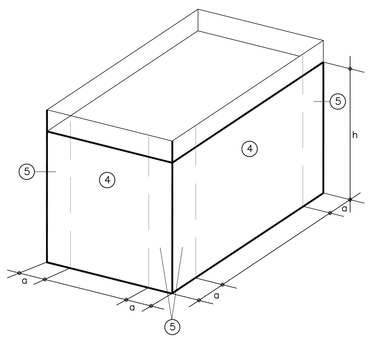
SOUTH ELEVATION (WIND PRESSURES)

SCALE 1/8"=1'-0"



WEST ELEVATION (WIND PRESSURES)

SCALE 1/8"=1'-0"



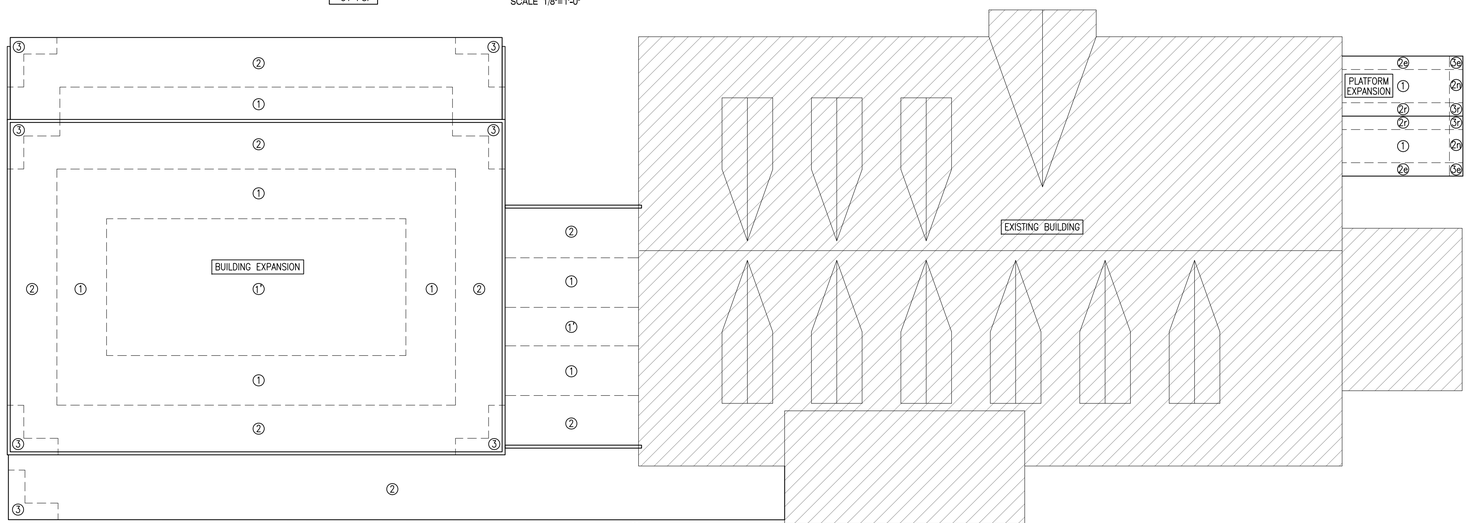
WIND PRESSURE DIAGRAMS WALLS

N.T.S.

WIND LOAD PRESSURE FOR ROOFING DESIGN (BUILDING EXPANSION)		
ROOF ZONE	PRESSURE (PSF) (NET UPLIFT)	a
1	45 PSF	7'-6"
1'	24 PSF	
2	60 PSF	
3	84 PSF	

WIND LOADS SHOWN IN THESE DRAWINGS ARE ALLOWABLE VALUES. NO FURTHER REDUCTION IS ALLOWED.

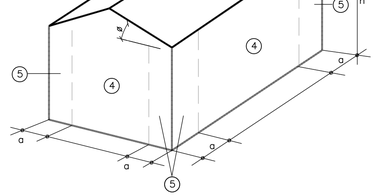
WINDOWS AND DOORS SHALL HAVE A VALID N.O.A. FROM MIAMI-DADE COUNTY OR FLORIDA APPROVAL.



N

WIND PRESSURE ROOF PLAN

SCALE 1/16"=1'-0"



WIND PRESSURE DIAGRAMS WALLS

N.T.S.

WIND LOAD PRESSURE FOR ROOFING DESIGN (PLATFORM EXPANSION)		
ROOF ZONE	PRESSURE (PSF) (NET UPLIFT)	a
1	56 PSF	3'-0"
2e	56 PSF	
2n	84 PSF	
2r	84 PSF	
3e	84 PSF	
3r	101 PSF	

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S-5.1

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