

A. GENERAL NOTES

1. ALL STRUCTURAL WORK SHALL CONFORM TO THE PROJECT SPECIFICATIONS, DRAWINGS, AND THE 2021 IBC W/ NH STATE BUILDING CODE AMENDMENTS
2. CONTRACTOR SHALL COORDINATE STRUCTURAL WORK WITH RELATED TRADES AND WITH OTHER DESIGN DISCIPLINE REQUIREMENTS PRIOR TO MAKING SUBMITTALS. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO PERFORMING WORK.
3. REFER TO OTHER DESIGN DISCIPLINE DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION REQUIRED FOR THE SUBMITTALS AND INSTALLATION OF STRUCTURES, INCLUDING BUT NOT LIMITED TO DIMENSIONS, ELEVATIONS, SLOPES, LOCATIONS OF OTHER SYSTEMS AND EQUIPMENT, OPENINGS, WALLS, STAIRS, FINISHES, COATINGS, AND OTHER NON-STRUCTURAL ITEMS. NOTES PROVIDED ON THE DRAWINGS ARE INTENDED FOR USE IN CONJUNCTION WITH PROJECT SPECIFICATIONS
4. DETAILS LABELED AS TYPICAL DETAILS ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH TYPICAL DETAILS SHALL APPLY WHETHER OR NOT THEY ARE DEMARKED AT EACH LOCATION IN THE DRAWINGS, FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS OF A SIMILAR NATURE. VERIFY APPLICABILITY BY SUBMITTALS.
5. CONTRACTOR IS RESPONSIBLE FOR COORDINATION DETAILS AND ACCURACY OF THE WORK; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS, FOR SELECTING FABRICATION PROCESSES, FOR TECHNIQUES OF ASSEMBLY IN ACCORDANCE WITH GENERAL CONDITIONS AND DIVISION 1 SPECIFICATION REQUIREMENTS; AND FOR PERFORMING ALL WORK IN A SAFE AND SECURE MANNER IN ACCORDANCE WITH GOVERNING JOB SAFETY STANDARDS.
6. CONTRACTOR SHALL VERIFY ALL CONDITIONS AT THE SITE, INCLUDING LOCATIONS OF ALL EXISTING STRUCTURES AND EXISTING UTILITIES ABOVE AND BELOW GROUND (AS ANY INFORMATION SHOWN IS APPROXIMATE AND NOT NECESSARILY COMPLETE). CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO PERFORMING WORK.
7. LOADS APPLIED DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS NOTED ON THE DRAWINGS OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTIONS AS DETERMINED BY THE CONTRACTOR. THE STRUCTURAL ELEMENTS OF THE PROJECT AS SHOWN IN THE CONSTRUCTION DOCUMENTS HAVE BEEN DESIGNED FOR THE SPECIFIED VERTICAL AND LATERAL FORCES ACTING ON THE COMPLETED BUILDING. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN AND PROVIDE ALL REQUIRED SHORING AND BRACING NEEDED DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF THE PARTIALLY-COMPLETED STRUCTURE AND FOR CONSTRUCTION LOADINGS THAT EXCEED THE SPECIFIED DESIGN LOADS
8. SHORING, BRACING, PROTECTING, AND MAINTAINING THE INTEGRITY OF ANY EXISTING, ADJACENT, AND/OR ONGOING PARTIALLY COMPLETED STRUCTURES IS THE RESPONSIBILITY OF THE CONTRACTOR.

B. EXISTING BUILDING NOTES

1. DIMENSIONS, ELEVATIONS, MEMBER SIZES, AND DETAILS OF EXISTING STRUCTURE SHOWN IN THE STRUCTURAL DRAWINGS HAVE BEEN EXTRACTED FROM RECORD DRAWINGS AND/OR LIMITED FIELD MEASUREMENTS. AS SUCH THEY ARE NOT TO BE CONSIDERED SUITABLY ACCURATE FOR ANY CONSTRUCTION WORK SHOWN, INCLUDING FABRICATIONS, SUBMITTALS, ETC. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS OF THE EXISTING CONSTRUCTION, INCLUDING PLUMBNESS OR FLATNESS OF WALLS, FLOORS, ETC., AT THE JOB SITE PRIOR TO SUBMITTAL, FABRICATION OR CONSTRUCTION WORK. ANY DEVIATIONS FOUND IN THE FIELD FROM WHAT IS SHOWN ON THE DRAWINGS SHALL BE REPORTED TO THE ARCHITECT PRIOR TO FABRICATION OR CONSTRUCTION.
2. TEMPORARY SHORING AND BRACING OF FLOORS, WALLS, AND OTHER STRUCTURAL ELEMENTS OF THE EXISTING BUILDINGS REQUIRED TO ACHIEVE THE INSTALLATION OF NEW AND/OR THE REMOVAL OF EXISTING STRUCTURAL ELEMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL, AT THEIR DISCRETION AND WHERE SPECIFIED, EMPLOY ENGINEERING SERVICES FOR DESIGN OF TEMPORARY BRACING, SHORING AND PROTECTION. EXISTING BUILDING MOVEMENTS SHALL BE LIMITED TO PREVENT DISTRESS FROM OCCURRING.
3. REPORT EXISTING CONDITIONS UNCOVERED, REVEALED, FOUND OR DEVELOPED DURING CONSTRUCTION INDICATIVE OF STRUCTURAL INTEGRITY LOSS OR DETERIORATION, UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.
4. DO NOT CUT, DRILL OR ALTER ANY EXISTING STRUCTURAL ELEMENTS UNLESS SHOWN OR NOTED ON THE STRUCTURAL DRAWINGS WITHOUT NOTIFY THE ARCHITECT FOR REVIEW, INCLUDING TEMPORARY MEASURES OR FOR THE INSTALLATION OF OTHER DESIGN DISCIPLINE WORK.
5. MONITORING OF CONSTRUCTION WORK SHALL INCLUDE, BUT IS NOT LIMITED TO FIRE WATCH DURING AND AT LEAST 24 HOURS AFTER ALL STEEL WELDING OR DRILLING, WOOD DRILLING, AND HEAT TRANSFERRING CONSTRUCTION MEASURES. DO NOT ALLOW HEAT OR ENERGY FROM EQUIPMENT TO DAMAGE OR OTHERWISE ALTER EXISTING STRUCTURAL ELEMENTS TO REMAIN.
6. FOR EXISTING STEEL ELEMENTS, DO NOT ALLOW THE THROUGH THICKNESS TEMPERATURE OF THE STEEL TO EXCEED 300° FAHRENHEIT DURING WELDING PROCESSES UNLESS SPECIFICALLY NOTED OTHERWISE. USE ACTIVE, OBSERVABLE SURFACE MONITORING METHODS.
7. PRE-BID SITE VISIT/DOCUMENTATION: EXISTING STRUCTURAL CONDITIONS AND BUILDING FEATURES MAY NOT BE COMPLETELY OR ACCURATELY SHOWN ON THESE DRAWINGS OR DOCUMENTS OR THOSE OF OTHER CONSULTANTS. THEREFORE, PRIOR TO SUBMITTING A BID FOR THE PROJECT, THE CONTRACTOR SHALL REVIEW ALL AVAILABLE EXISTING DRAWINGS AND CONDUCT A PRE-BID SITE VISIT TO VIEW THE EXISTING STRUCTURAL CONDITIONS AND BUILDING FEATURES.

C. TEMPORARY SHORING

1. THE CONTRACTOR SHALL PROVIDE ALL THE NECESSARY MEASURES AND PRECAUTIONS TO PREVENT DAMAGE AND SETTLEMENT OF EXISTING OR NEW CONSTRUCTION.
2. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER DESIGN, ADEQUATE INSTALLATION AND MAINTENANCE OF ANY AND ALL SHEETING, SHORING AND UNDERPINNINGS AGAINST EXISTING STRUCTURES AS REQUIRED, SO THAT THEY ARE NOT ENDANGERED BY THIS CONSTRUCTION.
4. TEMPORARY MEMBERS AND CONNECTIONS SHALL NOT BE REMOVED UNTIL PERMANENT MEMBERS ARE IN PLACE AND FINAL CONNECTIONS ARE MADE.
5. NEITHER THE ENGINEER NOR HIS/HER CONSULTANTS SHALL BE RESPONSIBLE FOR OR HAVE CONTROL OR CHARGE OF CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS/PROGRAMS IN CONNECTION WITH THIS PROJECT.
6. NEITHER THE ENGINEER NOR HIS/HER CONSULTANTS SHALL BE RESPONSIBLE FOR, OR HAVE CONTROL OVER, THE ACTS OF OMISSIONS BY THE CONTRACTOR, SUBCONTRACTORS, ANY OF THEIR AGENTS, EMPLOYEES, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THE ENTITIES OR INDIVIDUALS MENTIONED HEREIN ABOVE, TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
7. THE ATTACHED PLANS AND DETAILS ARE A GRAPHICAL REPRESENTATION OF THE EXISTING CONDITIONS AND MAY NOT REFLECT ACTUAL FIELD CONDITIONS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS OF THE EXISTING BUILDING PRIOR TO CONSTRUCTION.
8. CONTRACTOR SHALL SUBMIT A CONSTRUCTION PHASING PLAN. THE CONSTRUCTION PHASING PLAN SHALL INCLUDE AS A MINIMUM: A PLAN IN EITHER GRAPHICAL OR NARRATIVE FORMAT DESCRIBING THE SEQUENCING OF THE BRACING, SHORING, DEMO, AND NEW CONSTRUCTION; BRACING AND SHORING DRAWINGS AS REQUIRED; BRACING AND SHORING
9. CONTRACTOR SHALL PROVIDE ALL SHORING AND BRACING DURING REMOVAL AND ERECTION OF NEW FOUNDATION WALLS.

D. FOUNDATION RELATED EARTHWORK

1. THE CONTRACTOR SHALL CARRY THE FOUNDATIONS TO THE ELEVATIONS INDICATED TO SUPPORT AN ALLOWABLE SOIL BEARING PRESSURE OF 3,000 PSF. THE CONTRACTOR SHALL ENGAGE A GEOTECHNICAL ENGINEER/TESTING AGENCY AS REQUIRED TO CONFIRM THESE ASSUMPTIONS PRIOR TO CONSTRUCTION.
2. THE GEOTECHNICAL REPORTING CONTAINS SPECIFIC REQUIREMENTS PERTAINING TO GRUBBING, SITE, SUBFLOOR AND BEARING SURFACE PREPARATION AND PROTECTION; STRUCTURAL FILL AND COMPACTION REQUIREMENTS; GROUND WATER MANAGEMENT; ETC. THAT ARE NOT NECESSARILY SHOWN BY THE DRAWINGS AND SPECIFICATIONS. ALSO, IBC CHAPTER 18 "SOILS AND FOUNDATION" REQUIREMENTS APPLY, UNLESS SPECIFICALLY NOTED OTHERWISE BY THE GEOTECHNICAL REPORTING, DRAWINGS OR SPECIFICATIONS. REPORT CONFLICTS BETWEEN THE REPORTING AND THE DRAWINGS AND SPECIFICATIONS TO THE ARCHITECT PRIOR TO COMMENCING ANY AFFECTED WORK.
3. A LICENSED GEOTECHNICAL ENGINEER SHALL INSPECT AND REPORT ON ALL NATIVE SUB-GRADES FOR SLABS-ON-GRADE AND FOUNDATION PREPARED SOIL SURFACES PRIOR TO THE PLACEMENT OF ANY BACKFILL, FILL, AND FOUNDATION STRUCTURAL ELEMENTS. FOUNDATIONS AND FOOTING SHALL BEAR ON COMPETENT NATIVE SOILS OR COMPACTED STRUCTURAL FILLS IN ACCORDANCE WITH THE GEOTECHNICAL REPORTING.
4. FOOTINGS, PILE CAPS, AND SLABS CAST DIRECTLY AGAINST THE EARTH SHALL BE SIDE-FORMED AS REQUIRED TO KEEP EARTH OUT OF THE CONCRETE. COMPACT DISTURBED LOAD BEARING SOIL IN DIRECT CONTACT WITH FOUNDATIONS TO ORIGINAL BEARING CAPACITY. AS WET WEATHER OR GROUND CONDITIONS WARRANT, PLACE A MINIMUM OF 6 INCHES OF CRUSHED STONE OR 12 INCHES OF SAND-GRAVEL WRAPPED IN GEOTEXTILE FABRIC FOR SUBGRADE PROTECTION BENEATH FOUNDATIONS. DO NOT ALLOW FOR STANDING WATER ON EARTH. IF OVER-EXCAVATION OCCURS, REPLACE MATERIAL WITH BACKFILL MEASURES SPECIFIED FOR USE UNDER FOUNDATIONS, AFTER ACCEPTANCE BY GEOTECHNICAL ENGINEER.
5. UNLESS NOTED OTHERWISE, PLACE AND COMPACT BACKFILL IN EQUAL CONTINUOUS LAYERS NOT EXCEEDING A MAXIMUM OF 8" OF COMPACTED DEPTH FOR HAND-HELD COMPACTION EQUIPMENT AND A MAXIMUM OF 12" INCHES COMPACTED DEPTH FOR VIBRATORY ROLLERS. MAINTAIN OPTIMUM MOISTURE CONTENT OF BACKFILL MATERIALS TO ATTAIN COMPACTION DENSITY.
6. AT EARTH RETAINING AND FOUNDATION WALLS, BACKFILL LIFTS TO NOT EXCEED 12 INCH DIFFERENCE IN ELEVATION UNTIL FINAL ELEVATION ARE REACHED ON BOTH SIDES OF THE WALL. AT BASEMENT WALLS, DO NOT BACKFILL UNTIL GROUND FLOOR AND CONNECTED ELEVATED FRAMED LEVELS SLABS HAVE BEEN COMPLETED AND THE CONCRETE AT WALLS AND FLOORS HAS ACHIEVED FULL DESIGN STRENGTH.
7. THE CONSTRUCTION CONSIDERATIONS IN THE GEOTECHNICAL REPORTING AND PROJECT SPECIFICATIONS SHALL APPLY TO THIS PROJECT, INCLUDING BUT NOT LIMITED TO PROOFROLLING SUBGRADES AT THE EXCAVATION AND/OR BEARING ELEVATIONS; REMOVING AND REPLACING LOOSE OR SOFT POCKETS, FILL SLOPE CONSTRUCTIONS, ETC.
8. BACKFILL REQUIREMENTS:
- A. FILL WITHIN BUILDING ENVELOPE AND EXTENDING OUTWARD AT 1:1 SLOPE TO ACCEPTABLE NATIVE SOIL CONDITIONS.
MATERIAL: "SAND-GRAVEL"; "GRANULAR"; "CRUSHED STONE" WITH GEOTEXTILE WRAP (SEE SECTIONS)
COMPACTION: 95% MODIFIED PROCTOR
- B. BACKFILL DIRECTLY BELOW INTERIOR SLABS-ON-GRADE ASSEMBLIES (12 INCHES UNLESS NOTED OTHERWISE); "CRUSHED STONE" WITHOUT GEOTEXTILE
COMPACTION: 95% MODIFIED PROCTOR
- C. BACKFILL BELOW PAVEMENT, WALKS, ENTRY SLABS IN VICINITY OF BUILDING:
MATERIAL: "SAND-GRAVEL" "GRANULAR" "CRUSHED STONE" (SEE SECTIONS, LAND ARCH AND CIVIL)
COMPACTION: 95% MODIFIED PROCTOR
- D. BACKFILL BEHIND RETAINING WALLS AND BASEMENT WALLS, OUTSIDE BUILDING ENVELOPE AND UNDER PAVEMENT, WALKS, ENTRY SLABS:
MATERIAL: "GRANULAR BACKFILL"
COMPACTION: 95% MODIFIED PROCTOR
- E. BACKFILL ALONG EXTERIOR OF BUILDING AGAINST WALLS AND NOT UNDER PAVEMENT, WALKS, ENTRY SLABS:
MATERIAL: "SUITABLE NATIVE SOIL" COVERED BY 2 FEET DEEP BY 4 FEET WIDTH OF "LESS PERMEABLE FILL
COMPACTION: 92% MODIFIED PROCTOR
9. BACKFILL MATERIALS: RECYCLED CONCRETE AGGREGATE TO BE USED IN WHOLE OR BLENDED WITH OTHER AGGREGATES TO ACHIEVE GRADATIONS BELOW, ONSITE MATERIALS MEETING THE FOLLOWING CLASSIFICATIONS MAY BE USED:

A. "SAND-GRAVEL":	SIEVE DESIGNATION	% BY WEIGHT PASSING SIEVES
	4 INCH	100
	1/2 INCH	50-85
	No. 4	45-75
	No. 100	10-35
B. "GRANULAR":	SIEVE DESIGNATION	% BY WEIGHT PASSING SIEVES
	3 INCH	100
	No. 4	45-70
	No. 40	0-12
	No. 200	0-6
C. "CRUSHED STONE" WITH GEOTEXTILE FABRIC:	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10
D. "SUITABLE NATIVE SOIL": ON SITE SAND OR GRAVEL REASONABLY FREE OF LOAM, SILT, CLAY, OR ORGANIC MATTER.	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10
E. "LESS PERMEABLE FILL" GLACIAL TILL (SEE GEOTECHNICAL REPORT)	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10
F. "RECYCLED CONCRETE AGGREGATE" STOCKPILED ON SITE FROM DECONSTRUCTION PROJECT. SUBMIT GRADATIONS PRIOR TO ANY REQUIRED BLENDING; AS WELL AS FOR BLENDED AGGREGATES. MUST BE USED AS IS OR INTEGRAL WITH "SAND-GRAVEL"; "GRANULAR"; "CRUSHED STONE" FILLS OR BACKFILLS ABOVE.	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10
10. GEOTEXTILE FABRIC: NON-WOVEN WITH 12 LAPPED SEAMS SEE GEOTECHNICAL REPORTING FOR USE AND MEETING:	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10
11. INSULATION AT EXTERIOR SLABS AND WALKS (NOT PAVEMENTS) EXTRUDED POLYSTYRENE, STRENGTH OF 40 PSI (UNO) AND RATED FOR UNDERSLAB/UNDERGROUND USE. STAGGER AND DO NOT TAPE BOARD JOINTS.	SIEVE DESIGNATION	% WEIGHT BY PASSING SIEVES
	1 INCH	100
	3/4 INCH	90-100
	3/8 INCH	0-55
	No. 4	0-10

E. CAST-IN-PLACE CONCRETE

1. CODES AND STANDARDS: COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF:
- A. ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
- B. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE"
- C. ACI 304 "GUIDE FOR MIXING, TRANSPORTING AND PLACING CONCRETE"
- D. ACI 305 "HOT WEATHER CONCRETING"
- E. ACI 306 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING"
- F. ACI 308 "STANDARD PRACTICE FOR CURING CONCRETE"
- G. ACI 302 "GUIDE TO CONCRETE FLOOR AND SLAB CONSTRUCTION"
2. CONCRETE TESTING: THE CONTRACTOR SHALL PREPARE A SET OF 4 CYLINDERS/TEST SET TO BE TESTED AT AN INDEPENDENT LABORATORY. THE CYLINDERS SHALL BE TAKEN FROM ONE CONCRETE TRUCK AND LABELED WITH DATE, TRUCK NUMBER, AND LOCATION OF CONCRETE PLACEMENT. EACH SAMPLE SHALL ALSO BE TESTED FOR SLUMP, AIR CONTENT, AND TEMPERATURE. THE CYLINDERS SHALL BE TESTED AS FOLLOWS: 1 AT 7 DAYS; 2 AT 28 DAYS; AND A THIRD HELD FOR A 56 DAY BREAK IF REQUIRED. TEST CYLINDERS SHALL BE TAKEN AT LEAST ONCE PER PLACEMENT OR AT THE FOLLOWING INCREMENTS:
- A. WALLS AND FOOTINGS: 50 CUBIC YARDS
- B. ISOLATED FOOTINGS: 25 CUBIC YARDS
- C. SLABS: 50 CUBIC YARDS
- FIELD TESTING SHALL BE PERFORMED BY A GRADE I ACI (MINIMUM) FIELD TESTING TECHNICIAN.
3. SUBMIT MIX DESIGN AND EITHER TRIAL MIX DESIGNS OR HISTORIC FIELD DATA FOR APPROVAL IN ACCORDANCE WITH ACI 318, CHAPTER 5, INCLUDE TECHNICAL DATA SHEETS, GRADATIONS, AND MATERIAL VERIFICATIONS ON ALL COMPONENTS. SUBMIT MIX DESIGNS, PRIOR TO PLACEMENT OF CONCRETE, TRANSIT MIX SHALL CONFORM TO ASTM C94.
4. COMPRESSIVE MIXTURES AS DELINEATED IN CONCRETE MIX TABLE; SEE NOTES BELOW FOR ADDITIONAL INFORMATION.
- A. SLUMP: 3"-5" BEFORE ADDITION OF WATER REDUCER, 6"-8" AFTER ADDITION OF WATER REDUCER
- B. ALL CONCRETE NORMALWEIGHT, UNLESS NOTED OTHERWISE.
5. MAXIMUM AGGREGATE SIZE IN ACCORDANCE WITH ACI 301; CLEARLY NOTE LOCATION WHERE AGGREGATES GREATER THAT 3/4" MAXIMUM SIZE ARE PROPOSED FOR USE.
6. NO CHLORIDE OR OTHER UNAUTHORIZED ADMIXTURES SHALL BE USED. MAINTAIN MAXIMUM WATER SOLUBLE CHLORIDE ION (CL-) IN CONCRETE, BY WEIGHT OF CEMENT AT LESS THAN 1.00 FOR NON-EXPOSED CONCRETES AND 0.30 FOR EXTERIOR EXPOSED CONCRETES
7. WHEN AMBIENT TEMPERATURE IS BELOW 40° FAHRENHEIT OR MORE THAN 90° FAHRENHEIT PLACE AND PROTECT CONCRETE IN ACCORDANCE WITH ACI STANDARDS LISTED ABOVE.
8. CONCRETE PLACEMENT MAY REQUIRE ADJUSTMENT OF REINFORCEMENT, EMBEDDED ITEMS OR ANCHOR BOLTS. REVIEW DRAWINGS IDENTIFY THESE LOCATIONS TO ARCHITECT PRIOR TO SUBMITTALS. PROVIDE ADDITIONAL SUPERVISION AT ALL STEEL TO CONCRETE CONNECTION LOCATIONS AND MODIFY PLACEMENT MEASURES TO ACCOUNT FOR CONGESTIONS.
9. COMPLY WITH ACI CODES AND PLACE CONCRETE IN A CONTINUOUS OPERATION WITHIN PLANNED JOINTS OR SECTIONS. DO NOT PERMIT COLD JOINTS TO OCCUR.
10. CURING: COVER OR WET CURE ALL ELEMENTS. BEGIN INITIAL CURING AS SOON AS FREE WATER HAS DISAPPEARED FROM EXPOSED SURFACES. WHERE POSSIBLE, KEEP CONTINUOUSLY WET FOR 72 HOURS. CONTINUE CURING BY USE OF MOISTURE RETAINING COVER. USE OF MEMBRANE-FORMING CURING COMPOUNDS IS PROHIBITED.
11. SURFACE FINISHES:
- A. GENERAL: COMPLY WITH ACI 302-1R RECOMMENDATIONS FOR SCREEDING, RESTRAIGHTENING, AND FINISHING OPERATIONS FOR CONCRETE SURFACES. DO NOT WET CONCRETE SURFACES
12. PROVIDE CONTROL AND CONSTRUCTION JOINTS BY DETAIL AND SPECIFICATION REQUIREMENTS. SHOW LOCATION ON REINFORCING SUBMITTAL FOR COORDINATION WITH FLOORING, EQUIPMENT AND OTHER CONTRACTOR REQUIREMENTS.
- A. SLABS SAW-CUT CONTROL JOINTS AS SOON AS CONCRETE HAS HARDENED ENOUGH TO WALK ON SURFACE WITHOUT DAMAGING CONCRETE AND NO MORE THAN 4 HOURS AFTER FINAL TROWEL. JOINT SPACING SHALL, UNLESS NOTED OTHERWISE, NOT EXCEED 36 TIMES THE SLAB THICKNESS OR 18 FEET
- B. WALLS CONTROL JOINTS: NOT EXCEEDING 20 FEET AND AT EACH INTEGRAL PILASTER; CONSTRUCTION JOINTS AT 80 FEET OF MAXIMUM SPACING.

F. CONCRETE REINFORCEMENT

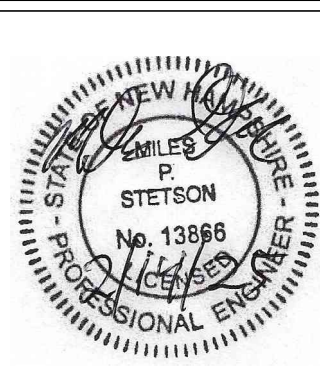
1. SHOP DRAWINGS SHALL BE PROVIDED PRIOR TO START OF CONCRETE PLACING AND BE IN ACCORDANCE WITH:
- A. ACI 301
- B. ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT"
- C. ACI SP-66 "ACI DETAILING MANUAL"
- D. CRSI MSP "MANUAL OF STANDARD PRACTICE"
- SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. SHOW ALL SLABS IN PLAN AND ALL WALLS IN ELEVATION WITH OPENINGS SHOWN BASED ON MEP COORDINATION SUBMITTALS AND ARCHITECTURAL REQUIREMENTS. SUBMIT PROPOSED CONTROL AND CONSTRUCTION JOINTS FOR REVIEW ON REINFORCING SUBMITTALS
2. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60, STEEL BARS PER ASTM A305, UNLESS NOTED OTHERWISE.
3. PROVIDE EPOXY-COATED BARS WHERE SHOWN MEETING ASTM A775 AND USING EPOXY COATED SUPPORTS, COATED WIRE, AND EPOXY COATING FOR REPAIR OF SURFACE PRIOR TO POURING.
4. WHERE SPECIFICALLY SHOWN ON THE DRAWINGS, WELD REINFORCING BARS IN ACCORDANCE WITH AWS D1.4 "PRE-QUALIFIED JOINT, ELECTRODE 900 LOW HYDROGEN" AND PROCESS REQUIREMENTS INCLUDING COORDINATED WITH MILL CERTIFIED CARBON EQUIVALENT. ALTERNATIVELY, ASTM A706, GRADE 60 MAY BE SUBSTITUTED, INDICATE MATERIAL AND WELDING REQUIREMENTS ON SUBMITTAL. DO NOT WELD AT LOCATIONS NOT DETAILED, UNLESS SUBMITTED AND REVIEWED BY ARCHITECT.
5. FIELD BENDING OR REINFORCEMENT SHALL CONFORM TO ACI 301, INCLUDING PRE-HEAT REQUIREMENTS.
6. WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185 WITH A MINIMUM ULTIMATE TENSILE STRENGTH OF 70,000 PSI. LAP ONE CROSS WIRE SPACING PLUS 2". SUPPORT MESH ON CHAIRS PER CRSI WITH #4 AT 4'-0"oc, EACH WAY.
7. PROVIDE MINIMUM CONCRETE COVER TO REINFORCEMENT AS FOLLOWS, UNLESS OTHERWISE NOTED:
- A. BOTTOM OF FOOTINGS, GRADE BEAMS, AND SLABS-ON-GRADE: 3"
- B. SIDES OF FOOTINGS AND GRADE BEAMS: 2"
- C. FOUNDATION WALLS, FROST WALLS, RETAINING WALLS, PIT WALLS: 2"
- D. EXTERIOR WALLS (EXPOSED TO WEATHER): 2"
- E. FACES OF WALLS OTHER THAN THOSE NOTED ABOVE: 3/4"
- F. FOUNDATION PIERS: 2" TO TIES
- G. ALL FACES OF BEAMS AND COLUMNS: 1-1/2" TO TIES
- H. TOP AND BOTTOM OF ELEVATED SLABS: 3/4"
- I. TOPPING SLAB: 3/4"
- J. SLAB-ON-DECK: 3/4" FROM DECK, 3/4" FROM TOP SURFACE
8. ALL LAPS SHALL BE FULL TENSION LAPS (CLASS B SPLICE) UNLESS SPECIFICALLY NOTED OTHERWISE. DOWELS SHALL MATCH SIZE AND SPACING OF MAIN REINFORCEMENT, UNLESS OTHERWISE NOTED.
9. HEADED STUD ANCHORS, DEFORMED BAR ANCHORS (DBA'S), AND OTHER EMBEDDED ITEMS AS SPECIFIED FOR STRUCTURAL STEEL. ALL WELDS FOR STUDS AND DBA'S SHALL BE AUTOMATICALLY WELDED WITH MANUFACTURER'S EQUIPMENT AND RECOMMENDATIONS FOR FLUX FILLED HEADS.
10. CHAIRS AND SPACERS SHALL BE PLACED TO ADEQUATELY SUPPORT REINFORCING DURING PLACEMENT. FOREIGN MATERIALS SUCH AS WOOD, CLAY BRICK OR OTHER UNSUITABLE SUPPORTS SHALL NOT BE USED TO SUPPORT REINFORCING. SET WIRE TIES SO ENDS ARE DIRECTED INTO CONCRETE WHERE CONCRETE WILL BE EXPOSED. DO NOT USE CONCRETE SUPPORTS OR PUDDLING FOR SLABS UNLESS SUBMITTED AND ACCEPTABLY REVIEWED.

G. CONCRETE FORMWORK

1. CONCRETE FORMS SHALL BE CLEAN AND FREE FROM DEBRIS. IF FORMS ARE COATED WITH A VEGETABLE BASED (SOY) RELEASE AGENT, WHICH SHALL NOT STAIN CONCRETE OR ABSORB MOISTURE OR IMPAIR NATURAL BONDING OF CONCRETE.
2. COORDINATE WITH REINFORCING SUBMITTAL FOR OPENING AND ADDITIONAL REQUIREMENTS. SUBMIT, BEFORE FRAMING OPENINGS IN STRUCTURAL ELEMENTS WHICH ARE NOT INDICATED ON DRAWINGS.
3. PROVIDE BRACING TO ENSURE STABILITY OF FORMWORK, FOR PLACEMENT OPERATIONS. DO NOT REMOVE FORMS OR BRACING UNTIL CONCRETE HAS GAINED SUFFICIENT STRENGTH TO CARRY ITS OWN WEIGHT AND IMPOSED LOADS.
4. ALL WALL SIDES AND SLAB EDGES EXPOSED TO VIEW TO HAVE CLASS A -- CLASS OF SURFACE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

H. POST-INSTALLED ANCHORS INTO CONCRETE AND MASONRY

1. WHERE A MANUFACTURER'S ANCHORS IS SPECIFICALLY CALLED OUT ON THE DRAWINGS, IT SHALL BE CONSIDERED THE DESIGN BASIS FOR THE REQUIRED ANCHOR. ALTERNATES MEETING OR EXCEEDING ANCHOR SYSTEM DEMANDS, INCLUDING, BUT NOT LIMITED TO CAPACITY LOADING, EDGE DISTANCE, SUBSTRATE THICKNESS FOR CONNECTION ELEMENTS AND BASE MATERIAL SHALL BE SUBMITTED FOR PROPOSED USE PENDING ACCEPTABLE REVIEW. SUBMIT ICC-ES CODE REPORTS.
2. ADHESIVE ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL BE:
- A. FOR CONCRETE AND CONCRETE MASONRY: HILTI HIT-HY-200
- B. FOR EXISTING BRICK MASONRY: HILTI HIT-HY 270
- INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS. USE 3/4 INCH DIAMETER AT MINIMUM EMBEDMENT UNLESS OTHERWISE INDICATED BY DETAIL. SEE NOTE 1.
3. EXPANSION ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL BE:
- A. FOR CONCRETE: HILTI KWIK BOLT T2
- B. FOR MASONRY: HILTI KWIK BOLT 3.
- INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS. USE 3/4 INCH DIAMETER AT MINIMUM EMBEDMENT UNLESS OTHERWISE INDICATED BY DETAIL. SEE NOTE 1.
4. SCREW TYPE ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL, FOR CONCRETE AND MASONRY: SIMPSON TITEN-HD INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS. USE 3/4 INCH DIAMETER AT MINIMUM EMBEDMENT UNLESS OTHERWISE INDICATED BY DETAIL. SEE NOTE 1.



Chkd.									
Rev.	No.								

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Client:

BARRETT ARCHITECTURE
281 THE BEND RD.
GREENSBORO BEND, VT 05842

Sheet Title:

STRUCTURAL NOTES

Project Title:

**LITTLETON COMMUNITY
CENTER ANNEX RENOVATIONS**

120 MAIN ST. LITTLETON, NH 03561

Designed By:	MS
Checked By:	MS
Drawn By:	MS
Scale:	AS NOTED
Date:	2/14/24

S0.1

EV PROJECT #24169

1. UNLESS OTHERWISE SPECIFIED, EACH PIECE OF LUMBER SHALL BEAR THE GRADE MARK, STAMP, OR OTHER IDENTIFYING MARKS INDICATING GRADES OF MATERIAL, AND RULES OR STANDARDS UNDER WHICH PRODUCED. SUCH IDENTIFYING MARKS ON A MATERIAL SHALL BE IN ACCORDANCE WITH THE RULE OR STANDARD UNDER WHICH MATERIAL IS PRODUCED, INCLUDING REQUIREMENTS FOR QUALIFICATIONS AND AUTHORITY OF THE INSPECTION ORGANIZATION, USAGE OF AUTHORIZED IDENTIFICATION AND INFORMATION INCLUDED IN THE IDENTIFICATION. THE INSPECTION AGENCY FOR LUMBER SHALL BE APPROVED BY THE BOARD OF REVIEW, AMERICAN LUMBER STANDARDS COMMITTEE, TO GRADE SPECIES USED.
2. PROTECT LUMBER AND OTHER PRODUCTS FROM DAMPNESS BOTH DURING AND AFTER DELIVERY AT THE SITE. PILE PLYWOOD AND LUMBER IN STACKS IN SUCH A MANNER AS TO PROVIDE ADEQUATE AIR CIRCULATION AND TO PREVENT WARPING. LOCATE STACKS IN WELL DRAINED AREAS, SUPPORTED AT LEAST SIX INCHES ABOVE GRADE AND COVER WITH WELL VENTILATED SHEDS HAVING A FIRMLY CONSTRUCTED OVERHANGING ROOF AS WELL AS SUFFICIENT END WALL TO PROTECT LUMBER FROM DRIVING RAIN.
3. STORE SEASONED MATERIALS IN DRY PORTIONS OF BUILDING.
4. PROTECT SHEET MATERIALS FROM CORNERS BREAKING AND DAMAGING SURFACES WHILE UNLOADING.
5. NOMINAL SIZES ARE INDICATED EXCEPT AS SHOWN BY DETAIL DIMENSIONS. PROVIDE ACTUAL SIZES AS REQUIRED BY PRODUCT STANDARD 20, DEPARTMENT OF COMMERCE.
6. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19%.
7. LIGHT GAGE METAL CONNECTIONS SHALL BE SIMPSON, SUBMIT MANUFACTURERS SPECIFICATION SHEETS.

1. 2x6 AND 2x4 BEARING WALLS, INTERIOR AND EXTERIOR LOCATIONS: SPRUCE-PINE-FIR No. 1/ No. 2 AS GRADED BY NLGA
2. STRUCTURAL ROOF AND FLOOR FRAMING: SPRUCE-PINE-FIR No. 1/ No. 2 AS GRADED BY NLGA
3. PRESERVATIVE PRESSURE TREATED (P.P.T. or P.T.) LUMBER: SOUTHERN PINE No. 2, AS GRADED BY SPIB
4. LAMINATED VENEER LUMBER (LVL):
 - A. PROVIDE LVL HEADERS AND BEAMS AS INDICATED.
 - B. LVL FRAMING SHALL BE LAMINATED DOUGLAS FIR OR SOUTHERN PINE (GP LAM BY GEORGIA PACIFIC OR MICROLAM BY TRUS-JOIST OR EQUAL) MEETING THE FOLLOWING MINIMUM ALLOWABLE STRESS CRITERIA:
FB (BENDING STRESS) = 2600 PSI
FV (HORIZ. SHEAR STRESS) = 285 PSI
E (MODULUS OF ELASTICITY) = 1,900,000 PSI
FC (COMPRESSIONS PERPENDICULAR TO GRAIN) = 750 PSI
5. LAMINATED STRAND LUMBER (LSL):
 - A. PROVIDE LSL HEADERS AND BEAMS AS INDICATED.
 - B. LSL FRAMING (TIMBERSTRAND BY TRUS-JOIST OR EQUAL) TO MEET THE FOLLOWING MINIMUM ALLOWABLE STRESS CRITERIA:
FB (BENDING STRESS) = 2600 PSI
FV (HORIZ. SHEAR STRESS) = 400 PSI
E (MODULUS OF ELASTICITY) = 1,700,000 PSI
FC (COMPRESSION PERPENDICULAR TO GRAIN) = 880 PSI
6. GLULAM BEAMS AND COLUMNS
 - A. PROVIDE GLULAM BEAMS AND COLUMNS AS INDICATED.
 - B. GLULAM BEAMS AND COLUMNS SHALL BE PRODUCED BY NORDIC ENGINEERED WOOD, OR EQUIVALENT MEETING THE FOLLOWING MINIMUM ALLOWABLE STRESS CRITERIA:
FB (BENDING STRESS) = 2400 PSI
FV (HORIZ. SHEAR STRESS) = 300 PSI
E (MODULUS OF ELASTICITY) = 1,900,000 PSI
FC (COMPRESSION PERPENDICULAR TO GRAIN) = 600 PSI
APPEARANCE GRADE - ARCHITECTURAL
STRESS GRADE - 24F-ENDPNG
7. MISCELLANEOUS LUMBER: PROVIDE WOOD FOR SUPPORT OR ATTACHMENT OF THE WORK INCLUDING NON-BEARING PARTITION WALL STAYS, BUSHES, NAILERS, BLOCKING, FURRING, GROUNDS, STRIPPING AND SIMILAR MEMBERS. PROVIDE LUMBER OF SIZES AND SHAPES INDICATED. GRADE: SPRUCE-PINE-FIR STUD GRADE AS GRADED BY NLGA.

1. ROOF SHEATHING: 5/8" ADVANTECH ROOF SHEATHING, STRUCTURAL 1 PANEL GRADE, MANUFACTURED BY HUBER ENGINEERED WOODS WITH A PS-2 SPAN RATING OF 40/20 AS DENOTED BY CODE EVALUATION REPORT ESR 1785.
2. FLOOR SHEATHING: 23/32" ADVANTECH SUBFLOOR SHEATHING, STRUCTURAL 1 PANEL GRADE, MANUFACTURED BY HUBER ENGINEERED WOODS WITH A PS-2 SPAN RATING OF 24-INCHES AS DENOTED BY CODE EVALUATION REPORT ESR 1785.
3. FASTENERS AND ANCHORS: FURNISH ITEMS OF ROUGH HARDWARE, METAL CONNECTORS, BOLTS, ETC., REQUIRED TO COMPLETE THE WORK. BOLTS, NUTS AND WASHERS SHALL BE HOT DIPPED ELECTRO GALVANIZED STEEL.
4. SILL GASKET ON TOP OF FOUNDATION WALL: 1/4" THICK, PLATE WIDTH WIDE, CLOSED CELL POLYETHYLENE URETHANE FOAM FROM FOUNDATION ROLLS.
5. SUBFLOOR GLUE: AIA AFG-01, WATERPROOF OF WATER SOLVENT BASE, AIR CURC TYPE, CARTRIDGE DISPENSED.
6. BUILDING PAPER: NO. 15 ASPHALT FELT, PLAIN UNTREATED CELLULOSE BUILDING PAPER.
7. WOOD PRESERVATIVE (PRESSURE TREATMENT): AWP-A TREATMENT ACQ USING WATER BORNE PRESERVATIVE WITH 0.40 PERCENT RETANAGE.
8. SET STRUCTURAL MEMBERS LEVEL AND PLUMB, IN CORRECT POSITION.
9. MAKE PROVISIONS FOR ERECTION LOADS, AND FOR SUFFICIENT TEMPORARY BRACING TO MAINTAIN STRUCTURE SAFE, PLUMB, AND IN TRUE ALIGNMENT UNTIL COMPLETION OF ERECTION AND INSTALLATION OF PERMANENT BRACING.
10. PLACE HORIZONTAL MEMBERS, CROWN SIDE UP.
11. CONSTRUCT LOAD BEARING FRAMING FULL LENGTH WITHOUT SPLICES.
12. DOUBLE MEMBERS AT OPENINGS OVER 24 INCHES WIDE. SPACE SHORT STUDS OVER AND UNDER OPENING TO STUD SPACING.
13. CONSTRUCT DOUBLE JOIST HEADERS AT FLOORS AND CEILING OPENINGS AND UNDER WALL STUD PARTITIONS THAT ARE PARALLEL TO FLOOR JOISTS. FRAME RIGIDLY INTO JOISTS.
14. BRIDGE JOISTS FRAMING IN EXCESS OF 8 FEET SPAN AT MID-SPAN AND WHERE SHOWN ON DRAWINGS. FIT SOLID BLOCKING OR BRIDGING AT ENDS OF MEMBERS.
15. SECURE ROOF SHEATHING WITH LONGER EDGE PERPENDICULAR TO FRAMING MEMBERS AND WITH ENDS STAGGERED AND SHEET ENDS OVER BEARING.
16. USE SHEATHING CLIPS BETWEEN SHEETS BETWEEN ROOF FRAMING MEMBERS.
17. WHERE TONGUE AND GROOVE PLYWOOD IS USED, FULLY ENGAGE TONGUE AND GROOVE EDGES.
18. SECURE WALL SHEATHING WITH LONG DIMENSION PERPENDICULAR TO WALL STUDS, WITH ENDS OVER FIRM BEARING AND STAGGERED.
19. PLACE BUILDING PAPER HORIZONTALLY OVER WALL SHEATHING; WEATHER LAP EDGES AND ENDS.
20. SECURE SUB-FLOOR SHEATHING WITH LONGER EDGE PERPENDICULAR TO FLOOR FRAMING AND WITH END JOINTS STAGGERED AND SHEET ENDS OVER BEARING. ATTACH WITH SUB-FLOOR GLUE AND 8D NAILS AT 6" ON CENTER AT PERIMETER AND 12" ON CENTER ON INTERIOR OF PANEL.
21. TOLERANCES:
A. FRAMING MEMBERS: 1/4 INCH FROM TRUE POSITION, MAXIMUM.
B. SURFACE FLATNESS OF FLOOR: 1/4 INCH IN 10 FEET MAXIMUM, AND 1/2 INCH IN 30 FEET MAXIMUM.
22. ALL POSTS AND COLUMNS FROM HEADERS AND BEAMS SHALL BEAR CONTINUOUSLY TO CONCRETE FOUNDATIONS INCLUDING BLOCKING IN FLOOR AND ROOF SPACES. BLOCKING SHALL BE OF THE SIZE AND SHAPE TO CARRY THE REQUIRED LOADING.
23. ALL BOTTOM BEARING PLATES, FOR STUD WALLS OR BEAM BEARING, SHALL BE ANCHORED TO THE FOUNDATION WITH 1/2" DIAMETER ANCHOR BOLTS AT 4'-0" ON CENTER, UNLESS NOTED OTHERWISE.
24. ALL BEARING WALLS SHALL BE BLOCKED AT 4'-0" ON CENTER, VERTICALLY, UNLESS NOTED OTHERWISE.
25. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE PRESSURE TREATED, P.P.T.
26. ALL FASTENERS FOR PRESSURE TREATED WOOD TO BE G90 HOT-DIPPED GALVANIZED.
27. ALL HANGERS FOR PRESSURE TREATED WOOD TO BE G90 HOT-DIPPED GALVANIZED.
28. PROVIDE 1/4" NOMINAL GAP BETWEEN WOOD FRAMING AND HORIZONTAL FACES OF CONCRETE WALLS.

UNLESS OTHERWISE NOTED, STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

A. WIDE FLANGE SECTIONS: ASTM A572 GRADE 50 OR ASTM A992 (FY = 50 KSI)
ANGLES, CHANNELS, PLATE AND OTHER HOT ROLLED SHAPES: ASTM A36 (FY = 36 KSI)
C. TUBES: ASTM A503 GRADE C, RECTANGULAR; 50 KSI; ROUND: 46 KSI
D. PIPES: ASTM A53 GRADE B TYPE E OR S (FY = 35 KSI)
E. BASEPLATES, CONNECTION PLATES, STIFFENER PLATES: ASTM A572 GRADE 50 OR ASTM A992 (FY = 50 KSI)
F. THREADED RODS: ASTM A572 GRADE 50
G. STAINLESS STEEL (SUS) BARS & PLATES: ASTM A304, FY = 30 KSI
H. ANCHOR BOLTS: ASTM F1554 GRADE 55, UNLESS NOTED OTHERWISE, WITH SUPPLEMENTARY REQUIREMENT S1 FOR WELDABILITY.

2. BOLTS, NUTS AND WASHERS: ASTM A325 TYPE 1 BOLTS (3/4" MINIMUM DIAMETER), ASTM A563 D4 HEAVY HEX NUTS WITH ASTM A490 HARDENED WASHERS. PROVIDE BOLT ASSEMBLIES GALVANIZED TO ASTM A123 AT GALVANIZED STRUCTURAL MEMBERS. PROVIDE ASTM A490 BOLTS WHERE NOTED ON DRAWINGS OR WHERE NEEDED FOR SPECIFIED LOADS. DO NOT MIX BOLT SIZES BETWEEN A325 AND A490 BOLTS. HIGH STRENGTH LOAD INDICATOR BOLTS MAY BE USED AT THE CONTRACTOR'S OPTION.

3. DESIGN OF STEEL CONNECTIONS: CONTRACTOR IS RESPONSIBLE FOR DESIGN OF ALL STEEL CONNECTIONS OR PORTIONS OF CONNECTIONS NOT FULLY DETAILED IN THE CONTRACT DOCUMENTS, FOR THE SPECIFIED CONNECTION FORCES. SEE SPECIFICATIONS FOR DESIGN REQUIREMENTS. IN NO CASE SHALL LOADS BE LESS THAN 12 KIPS (WORKING).

4. SUBMITTALS FOR REVIEW

A. SHOP DRAWINGS: INDICATE PROFILES, SIZES, SPACING, AND LOCATIONS OF STRUCTURAL MEMBERS, DECKING, OPENINGS, ATTACHMENTS, AND FASTENERS. SHOW ALL CONNECTION DETAILS. PROVIDE DESIGN OF CONNECTIONS NOT DETAILED ON DRAWINGS. INDICATE WELDED CONNECTIONS WITH AWS S2.0 WELDING SYMBOLS. INDICATE NET WELD LENGTHS

B. ERM SHOP DRAWINGS: SHALL BE DATED AND IDENTIFIED WITH A UNIQUE DRAWING NUMBER AND REVISION NUMBER. RESUBMITTED SHOP DRAWINGS SHALL BE GIVEN A NEW REVISION NUMBER, AND ALL CHANGES/ADDITIONS/DELETIONS FROM THE PREVIOUS SUBMISSION SHALL BE CLEARLY IDENTIFIED.

C. ERECTION DRAWINGS SHALL INCLUDE DETAILS OF ALL FIELD WELDING AND ANY OTHER SPECIAL FIELD INSTRUCTIONS SEE SPECIFICATION SECTION 05120 AND NOTES BELOW FOR ADDITIONAL REQUIREMENTS

5. SUBMITTALS FOR INFORMATION

A. MANUFACTURER'S MILL CERTIFICATE: CERTIFY THAT PRODUCTS MEET OR EXCEED SPECIFIED REQUIREMENTS.

B. MILL TEST REPORTS: SUBMIT INDICATING STRUCTURAL STRENGTH, DESTRUCTIVE AND NON-DESTRUCTIVE TEST ANALYSIS

C. WELDERS CERTIFICATES: CERTIFY WELDERS EMPLOYED ON THE WORK, VERIFYING AWS QUALIFICATION WITHIN THE PREVIOUS 12 MONTHS.

6. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE "MANUAL OF STEEL CONSTRUCTION", 14TH EDITION, BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AND THE STRUCTURAL WELDING CODE (AWS D1.1) LATEST EDITION, BY THE AMERICAN WELDING SOCIETY.

7. STRUCTURAL STEEL CONNECTIONS SHALL BE AS FOLLOWS:

A. ALL CONNECTIONS UNLESS INDICATED OTHERWISE SHALL BE MADE WITH 3/4 INCH DIAMETER A325 BOLTS. AT MOMENT CONNECTIONS, THE BEAM SHALL BE DESIGNED AS TYPE "SC" OR CRITICAL CONNECTIONS SHALL BE USED, AND THE USE OF FULLY-TENSIONED SLP-CRITICAL BOLTS INSTALLED IN ACCORDANCE WITH THE AISI "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" IS REQUIRED AT THE FOLLOWING LOCATIONS:

- BRACING CONNECTIONS
- CONNECTIONS CARRYING DEFINED AXIAL LOADS
- COLUMN SPICES FOR BRACING MOMENTS
- FLANGE AND WELB BOLTS AT MOMENT CONNECTIONS AND MOMENT SPICES

ALL OTHER BOLTS MAY BE BEARING BOLTS (THREADS INCLUDED) TIGHTENED TO THE SNUG-TIGHT CONDITION, UNLESS NOTED OTHERWISE.

B. THE MINIMUM NUMBER OF BOLTS IN ANY CONNECTION SHALL BE TWO 3/4 INCH DIAMETER A325 BOLTS UNLESS INDICATED OTHERWISE.

C. IN CONNECTIONS OF BEAMS AND GIRDERS, THE MINIMUM NUMBER OF BOLTS SHALL BE REQUIRED TO DEVELOP THE BEAM BEARING "Y" NOTED ON THE CONTRACT DRAWINGS. THE PLAN NOTES SHALL ALSO BE ADHERED. IF THE BEAM SHEAR IS NOT NOTED, THE CONNECTIONS SHALL DEVELOP THE BEAM SHEAR (V=2W/3) WHERE W = THE TOTAL ALLOWABLE BEAM UNIFORM LOAD BASED ON SIMPLE SPAN MOMENTS AND BRACED COMPRESSION FLANGES. (SEE AISI MANUAL OF STEEL CONSTRUCTION, BEAMS) LOADS SHOWN ARE THE SERVICE (UNFACTORED) DESIGN FORCES INDICATED ON THE DRAWINGS. ANY 1/3 ALLOWABLE STRESS INCREASE MAY NOT BE TAKEN WITH THE SPECIFIED DESIGN FORCES.

D. IN CONNECTIONS OF BEAMS AND GIRDERS, SHALL DEVELOP THE BEAM SHEAR DESCRIBED ABOVE IN ADDITION TO AXIAL FORCES LISTED ON THE STRUCTURAL DRAWINGS. (COMPRESSION OR TENSION) DO NOT USE SINGLE PLATE CONNECTIONS FOR FRAMING WITH AXIAL LOADS.

E. ALL MOMENT CONNECTIONS SHALL DEVELOP THE FULL MOMENT CAPACITY OF THE BEAM OR GIRDER, UNLESS INDICATED OTHERWISE.

F. THE CONNECTION FOR DIAGONAL BRACING, VERTICAL AND HORIZONTAL, SHALL DEVELOP THE FORCE INDICATED ON THE DRAWINGS BUT NOT LESS THAN 50% OF THE AXIAL CAPACITY OF THE BRACE IN TENSION. DESIGN CALCULATIONS, SIGNED AND SEALED BY CONTRACTOR'S PROFESSIONAL ENGINEER SHALL BE SUBMITTED FOR ALL CONNECTIONS NOT FULLY DETAILED IN THE STRUCTURAL DRAWINGS.

G. CALCULATIONS FOR STANDARD, REPETITIVE BEAM CONNECTIONS AND AT SUBMITTED. THE FORM OF SUMMARIZED CAPACITY TABLES FOR EACH TYPE OF CONNECTION USED (DOUBLE ANGLE, SHEAR PLATE, ETC.). THE TABLES ARE TO BE ACCOMPANIED BY A DETAIL CLEARLY SHOWING THE CONNECTION GEOMETRY, INCLUDING THE NUMBER, SIZE, GRADE AND TYPE (BEARING OR SLP-CRITICAL) OF BOLTS; SIZE OF BOLT HOLES; SIZE, GRADE, AND GEOMETRY OF CONNECTION ANGLES OR PLATES; SIZE AND LENGTH OF WELDS; AND MINIMUM WELD STRENGTH FOR FULL CAPACITY. COMPUTER PRINTOUTS OF DETAILED CONNECTION CHECKS FOR EACH INDIVIDUAL PIECE (AS GENERATED BY SOME DETAILING SOFTWARE) ARE NOT ACCEPTABLE FOR THE STANDARD, REPETITIVE BEAM CONNECTIONS.

I. CALCULATIONS FOR BRACING AND OTHER NON-STANDARD CONNECTIONS SHALL INCLUDE A DETAIL SHOWING THE CONNECTION GEOMETRY AND DESIGN FORCES, AND FULL CALCULATIONS DEMONSTRATING THE ADEQUACY OF THE CONNECTION.

J. CONTRACTOR TO SUBMIT CONNECTION DETAILS AND CALCULATIONS IN ADVANCE OF PREPARING PIERCE DRAWINGS SO AS TO STREAMLINE THE SHOP DRAWING PREPARATION AND REVIEW PROCESS. SEE SPECIFICATION SECTION 05120 FOR ADDITIONAL REQUIREMENTS

K. DESIGN OF STEEL CONNECTIONS FOR SHEAR SHALL INCLUDE THE EFFECTS OF ECCENTRICITY.

L. CONTRACTOR SHALL PROVIDE STIFFENERS, WELD DOUBLER PLATES, AND OTHER REINFORCEMENT AS NECESSARY TO RESIST LOAD EFFECTS DUE TO THE SPECIFIED CONNECTION LOADS.

M. MOMENT CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL MOMENT CAPACITY OF THE MEMBER UNLESS NOTED OTHERWISE.

N. SINGLE-ANGLE CONNECTIONS MAY NOT BE USED FOR BEAM CONNECTIONS.

8. ALL STRUCTURAL SHOP AND FIELD WELDING SHALL BE MADE WITH ELECTRODES DESIGNED BY E70XX LOW HYDROGEN, IN ACCORDANCE WITH AWS D1.1, PERFORMED BY CERTIFIED WELDERS.

9. PROVIDE 3/8" WEB STIFFENERS IN BEAMS OVER OR UNDER ALL COLUMNS OR POSTS. TWO ON EACH SIDE IN LINE WITH COLUMN FLANGES.

10. THE MINIMUM THICKNESS OF GUSSET AND FIN PLATES SHALL BE 3/8".

11. PROVIDE 3/4" DIAMETER SOLD FLUXED SHEAR CONNECTIONS STUDS AUTOMATICALLY END WELDED THROUGH THE METAL DECK AS INDICATED AND IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. STUDS SHALL BE 9 INCHES LONG AFTER WELDING.

12. GROUT: NON-SHRINK TYPE, PRE-MIXED COMPOUND CONSISTING OF NON-METALLIC AGGREGATE CEMENT, WATER REDUCING AND PLASTICIZING ADDITIVES, CAPABLE OF DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI AT 28 DAYS AS MANUFACTURED BY FIVE STAR PRODUCTS, INC., FAIRFIELD, CT, OR APPROVED EQUIVALENT.

13. SHOP AND TOUCH-UP PRIMER (DESIGN BASIS): TNEPEC SERIES 88HS. COORDINATE WITH ARCHITECTURAL COATINGS AND FINISHES.

14. TOUCH-UP PRIMER FOR GALVANIZED SURFACES (DESIGN BASIS): TNEPEC SERIES 90G-K197. COORDINATE WITH ARCHITECTURAL COATINGS AND FINISHES.

15. FINISH

A. COORDINATE ALL COATINGS WITH DIVISION 9 SPECIFICATION REQUIREMENTS

B. WHERE INDICATED, STRUCTURAL STEEL MEMBERS ARE TO BE GALVANIZED IN ACCORDANCE WITH ASTM A123. PROVIDE MINIMUM 1.5 OZ/SQ FT GALVANIZED COATING. ALL MEMBERS EXPOSED TO THE EXTERIOR OR EXTENDING THROUGH AND BEYOND BUILDING ELEMENT SHALL BE GALVANIZED

16. ERECTION

A. ALLOW FOR ERECTION LOADS, AND FOR SUFFICIENT TEMPORARY BRACING TO MAINTAIN STRUCTURE SAFE, PLUMB, AND IN TRUE ALIGNMENT UNTIL COMPLETION OF ERECTION AND INSTALLATION OF PERMANENT BRACING.

B. FIELD WELD COMPONENTS INDICATED ON DRAWINGS AND SHOP DRAWINGS.

C. DO NOT FIELD CUT OR ALTER STRUCTURAL MEMBERS WITHOUT APPROVAL OF ARCHITECT/ENGINEER.

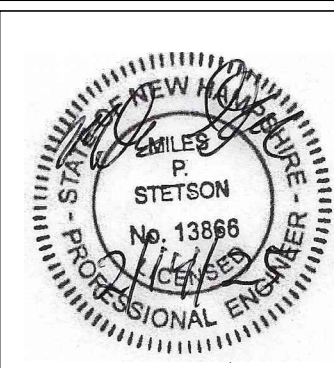
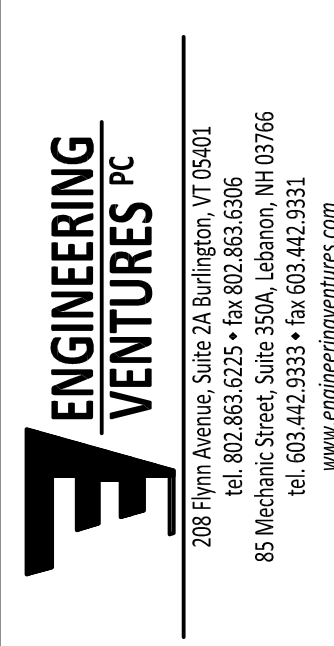
D. AFTER ERECTION, BRUISE WELDS, ABRASIONS, AND SURFACES NOT SHOP PRIMED, EXCEPT SURFACES TO BE IN CONTACT WITH CONCRETE.

E. GROUT UNDER BASE PLATES WITH PRE-MIXED NON-SHRINK GROUT WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 7,000 PSI

1.	Building Code:	The 2021 IBC / W / N State Building Code Amendments and Referenced Standards Including IBC and Applicable Provisions for a Building That is Listed on a National Historic Registry.
2.	Dead Loads:	
	a. Roof Dead Load:	15 psf
	b. Floor Dead Load:	15 psf
3.	Live Loads:	
	a. Roof Live Load:	Snow Load Governs
	b. Ground Floor Live Load (live load reduction not used):	100 psf
	c. Attic Floor Live Load:	30 psf*
4.	Roof Snow Load:	
	a. Ground Snow Load, P_g :	68 psf
	b. Flat Roof Snow Load, P_f :	57 psf
	c. Snow Exposure Factor, C_e :	1.0
	d. Snow Load Importance Factor, I_s :	1.0
	e. Thermal Factor, C_t :	1.2
5.	Wind Design Data:	
	a. Basic Wind Speed (3-second gust): V :	108 mph
	b. Wind Exposure:	C
	c. Internal Pressure Coefficients:	+/- 0.18
	d. Components and Cladding Wind Pressure:	per ASCE 7
6.	Earthquake Design Data:	Not Considered
7.	Allowable Soil Bearing Pressure:	2000 psf**
*	THE AUTHORITY HAVING JURISDICTION SHALL IMPLEMENT CONTROLS TO LIMIT THE OCCUPANCY OF THE ATTIC FLOOR TO NOT EXCEED 30 PSF.	
**	ASSUMED - TO BE VERIFIED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION	

AB	ANCHOR BOLT	MC	MOMENT CONNECTION
AFF	ABOVE FINISH FLOOR	N.S.	NEAR SIDE
AL	ALUMINUM	oc	ON CENTER
B.O.F.	BOTTOM OF FOOTING	P#	PIER DESIGNATION
DWG	DRAWING	PL	PLATE
E.F.	EACH FACE	SS	STAINLESS STEEL
ELEV.	ELEVATION	STD	STANDARD
EP	EMBED PLATE	T.O.C.	TOP OF CONCRETE
EQ	EQUAL	T.O.S.	TOP OF STEEL
E.S.	EACH SIDE	T.O.SHELF	TOP OF SHELF
E.W.	EACH WAY	T.O.W.	TOP OF WALL
EX.	EXISTING	TYP.	TYPICAL
#	FOOTING DESIGNATION	U.N.O.	UNLESS NOTED OTHERWISE
FND	FOUNDATION	V.I.F.	VERIFY IN FIELD
F.S.	FAR SIDE		
H.T.	HEAVY TIMBER		

NOTE: NOT ALL SYMBOLS AND NOTATIONS USED			
	NORTH ARROW		CONCRETE
	ELEVATION		GROUT or FINE CRUSHED GRAVEL
[XX' - XX']	TOP OF FOOTING ELEV.		GRATING
	SECTION NUMBER DRAWING WHERE SHOWN		LEDGE/ROCK
	DECK SPAN DIRECTION OR GRATING DIRECTION		3/4" CRUSHED STONE
	SLOPE DIRECTION, and MAGNITUDE		COMPACTED GRANULAR
	BEAM/COLUMN SPLICE		RIGID INSULATION
	ROOF PITCH		WOOD
	FOOTING STEP		UNDISTURBED SUBGRADE
	OPENING		CMU BLOCK
	MOMENT CONNECTION		BRICK
	GUARDRAIL/RAILING		
	BEAM PENETRATION		

[illegible]

Client: **BARRETT ARCHITECTURE**
281 THE BEND RD.
GREENSBORO BEND, VT 05842

Sheet Title: **STRUCTURAL NOTES**
BASIS OF DESIGN

Project Title: **LITTLETON COMMUNITY**
CENTER ANNEX RENOVATIONS

Designed By:	MS/IR
Checked By:	MS
Drawn By:	IR
Scale:	AS NOTED
Date:	2/14/24

S0.2

EV PROJECT #24169

1. SOILS AND FOUNDATIONS
2. CAST-IN-PLACE CONCRETE
3. STRUCTURAL STEEL
4. WOOD

STRUCTURAL TESTS AND INSPECTIONS ARE TO BE PERFORMED BY THE REGISTERED DESIGN PROFESSIONAL AND/OR A QUALIFIED PERSON, COMPETANT IN THE INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION. THE REGISTERED DESIGN PROFESSIONALS AND SUGGESTIONS FOR TESTING AGENCIES ARE LISTED BELOW:

INSPECTION AGENT	FIRM	ADDRESS
STRUCTURAL ENGINEER OF RECORD (EV)	ENGINEERING VENTURES, PC.	288 FLYNN AVE, SUITE 2A BURLINGTON, VT 05401
ARCHITECT OF RECORD (AOR)	BARRETT ARCHITECTURE	281 THE BEND RD GREENSBORO BEND, VT 05842
TESTING AGENCY (TA)	TBD	TBD
GEOTECHNICAL ENGINEER (GE)	TBD	TBD

ITEM	FREQUENCY	AGENT	REFERENCES
VERIFICATION AND INSPECTION (APPROVED SOILS REPORT AND CONSTRUCTION DOCUMENTS SHALL BE USED TO DETERMINE COMPLIANCE)			
VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	TA/GE	
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	TA	
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	FOR EACH SOURCE OF FILL MATERIAL	TA	SPECIFICATIONS
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	CONTINUOUS	TA	
PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	ONCE FOR EACH AREA OF PLACEMENT	TA	
COMPACTION TESTING	EVERY OTHER LIFT, EVERY 1000 SF	TA	ASTM D2922

ITEM	SCOPE	FREQUENCY	REFERENCES	AGENT
MIX DESIGN	REVIEW MIX DESIGN FOR CONFORMANCE WITH SPECIFICATIONS	PRIOR TO CONSTRUCTION	ACI 318 SPECIFICATIONS	EV
MATERIAL CERTIFICATION	REVIEW SUBMITTALS FOR CONFORMANCE WITH SPECIFICATIONS	PRIOR TO CONSTRUCTION	ACI 318 SPECIFICATIONS	EV
REINFORCING INSTALLATION	REVIEW REINFORCING FOR SIZE, QUANTITY, CONDITION, AND PLACEMENT	PERIODIC	ACI 318 SPECIFICATIONS	TA
CONCRETE PLACEMENT	OBSERVE CONCRETE PLACEMENT OPERATIONS. VERIFY CONFORMANCE WITH HOT OR COLD WEATHER REQUIREMENTS. PERFORM SLUMP, DENSITY, AND AIR CONTENT TESTS AT POINT OF DISCHARGE.	PERIODIC	ACI 318 SPECIFICATIONS	TA
CURING AND PROTECTION	OBSERVE PROCEDURES FOR CONFORMANCE WITH SPECIFICATIONS	PERIODIC	ACI 318 SPECIFICATIONS	TA
EVALUATION OF CONCRETE STRENGTH	TEST (TA) AND EVALUATE (EV) FOR CONFORMANCE WITH SPECIFICATIONS	CONTINUOUS	ACI 318 SPECIFICATIONS	TA EV

ITEM	SCOPE	FREQUENCY	REFERENCES	AGENT
FABRICATOR CERTIFICATION / QUALITY CONTROL PROCEDURES	FABRICATOR QUALITY CONTROL PROCEDURE SPECIFICATIONS AND DETAILED FABRICATION PROCEDURES	PRIOR TO CONSTRUCTION	IBC SECTION 1704.2.1	EV
MATERIAL VERIFICATION OF STRUCTURAL STEEL	IDENTIFICATION MARKINGS TO CONFORM TO AISC 360	PERIODIC	AISC 360, SECTION M5.5	TA
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS	IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	AISC 360, A3.3 AND APPLICABLE ASTM STANDARDS	TA
HIGH-STRENGTH BOLTING	DETERMINATION THAT THE REQUIREMENTS FOR BOLTS, NUTS, WASHERS AND PAINT, BOLTED PARTS AND INSTALLATION AND TIGHTENING STANDARDS ARE MET	PERIODIC	AISC 360, SECTION M2.5	TA
MATERIAL VERIFICATION OF WELD FILLER MATERIALS	IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	AISC 360, SECTION A3.5, AND APPLICABLE AWS A5 DOCUMENTS	TA
WELDING	WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATIONS FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1	PERIODIC/CONTINUOUS	AWS D1.1 AND D1.3	TA
STRUCTURAL FRAMING, DETAILS AND ASSEMBLIES	REVIEW SUBMITTALS AND SHOP DRAWINGS FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS	PRIOR TO CONSTRUCTION	AISC 360, SPECIFICATION	EV

ITEM	SCOPE	FREQUENCY	REFERENCES	AGENT
QUALITY CONTROL	FABRICATOR QUALITY CONTROL PROCEDURE SPECIFICATIONS	PRIOR TO CONSTRUCTION	IBC SECTION 1704.2	TA
SHEAR WALL NAILING	VERIFY MATERIALS AND NAILING PATTERN	PERIODIC	IBC SECTION 1705.12.2	TA

(TABLE 2304.9.1, IBC 2012)

CONNECTION		FASTENING a,m		LOCATION	
1. JOIST TO SILL OR GIRDER	(3) 8d COMMON (2-1/2" x 0.131") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			TOENAIL	
2. BRIDGING TO JOIST	(2) 8d COMMON (2-1/2" x 0.131") (2) 3" x 0.131" NAILS (2) 3" 14 GAGE STAPLES			TOENAIL EACH END	
3. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST	(2) 8d COMMON (2-1/2" x 0.131")			FACE NAIL	
4. WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST	(3) 8d COMMON (2-1/2" x 0.131")			FACE NAIL	
5. 2" SUBFLOOR TO JOIST OR GIRDER	(2) 16d COMMON (3-1/2" x 0.162")			BLIND AND FACE NAIL	
6. SOLE PLATE TO JOIST OR BLOCKING	16d (3-1/2" x 0.135") AT 16"oc 3" x 0.131" NAILS AT 8"oc 3" 14 GAGE STAPLES AT 12"oc			TYPICAL FACE NAIL	
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	(3) 16d (3-1/2" x 0.135") AT 16"oc (4) 3" x 0.131" NAILS AT 16"oc (4) 3" 14 GAGE STAPLES AT 16"oc			BRACE WALL PANELS	
7. TOP PLATE TO STUD	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			END NAIL	
8. STUD TO SOLE PLATE	(4) 8d COMMON (2-1/2" x 0.131") (4) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			TOENAIL	
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			END NAIL	
9. DOUBLE STUDS	16d (3-1/2" x 0.135") AT 24"oc 3" x 0.131" NAIL AT 8"oc 3" 14 GAGE STAPLE AT 8"oc			FACE NAIL	
10. DOUBLE TOP PLATES	16d (3-1/2" x 0.135") AT 16"oc 3" x 0.131" NAIL AT 12"oc 3" 14 GAGE STAPLE AT 12"oc			TYPICAL FACE NAIL	
DOUBLE TOP PLATES	(8) 16d COMMON (3-1/2" x 0.162") (12) 3" x 0.131" NAILS (12) 3" 14 GAGE STAPLES			LAP SPLICE	
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON (2-1/2" x 0.131") AT 24"oc (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			TOENAIL	
12. RIM JOIST TO TOP PLATE	8d (2-1/2" x 0.131") AT 6"oc 3" x 0.131" NAIL 6"oc 3" 14 GAGE STAPLE AT 6"oc			TOENAIL	
13. TOP PLATES, LAPS AND INTERSECTIONS	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			FACE NAIL	
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON (3-1/2" x 0.162")			16"oc ALONG EDGE	
15. CEILING JOISTS TO PLATE	(3) 8d COMMON (2-1/2" x 0.131") (5) 3" x 0.131" NAILS (5) 3" 14 GAGE STAPLES			TOENAIL	
16. CONTINUOUS HEADER TO STUD	(4) 8d COMMON (2-1/2" x 0.131")			TOENAIL	
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3) 16d COMMON (3-1/2" x 0.162") MINIMUM, TABLE 2308.10.4.1 (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES			FACE NAIL	
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3) 16d COMMON (3-1/2" x 0.162") MINIMUM, TABLE 2308.10.4.1 (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES			FACE NAIL	
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	(3) 8d COMMON (2-1/2" x 0.131") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			TOENAIL	
20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE	(2) 8d COMMON (2-1/2" x 0.131") (2) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES			FACE NAIL	
21. 1" x 8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2-1/2" x 0.131")			FACE NAIL	
22. WIDER THAN 1" x 8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2-1/2" x 0.131")			FACE NAIL	
23. BUILT-UP CORNER STUDS	16d COMMON (3-1/2" x 0.162") 3" x 0.131" NAILS 3" 14 GAGE STAPLES	24"oc 16"oc 16"oc			
24. BUILT-UP GIRDER AND BEAMS	20d COMMON (4" x 0.192") AT 32"oc 3" x 0.131" NAILS 3" 14 GAGE STAPLES	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES			
	(2) 20d COMMON (4" x 0.192") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL NAIL AT ENDS AND AT EACH SPLICE			
25. 2" PLANKS	16d COMMON (3-1/2" x 0.162")	AT EACH BEARING			
26. COLLAR TIE TO RAFTER	(3) 10d COMMON (3" x 0.148") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL			
27. JACK RAFTER TO HIP	(3) 10d COMMON (3" x 0.148") (4) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAILS			
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL			
28. ROOF RAFTER TO 2-bY RIDGE BEAM	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAILS			
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL			
29. JOSIT TO BAND JOIST	(3) 16d COMMON (3-1/2" x 0.162") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL			
30. LEDGER STRIP	(3) 16d COMMON (3-1/2" x 0.162") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL AT EACH JOIST			

FOR SIZ: 1" = 25.4 mm

a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.

b. NAILS SPACED AT 8" ON CENTER AT PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6" AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.

c. COMMON OR DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

d. COMMON (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

e. DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

f. CORROSION-RESISTANT SIDING (6d - 1-7/8" x 0.106"; 8d - 2-3/8" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 2-1/2" x 0.113").

g. FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS.

h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16-INCH-DIAMETER HEAD AND 1-1/2-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1-3/4-INCH LENGTH FOR 25/32-INCH SHEATHING.

i. CORROSION-RESISTANT STAPLES WITH NOMINAL 7/16-INCH CROWN OR 1-INCH CROWN AND 1-1/4-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1-1/2-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS NOTED OTHERWISE).

j. CASING (1-1/2" x 0.089") OR FINISH (1-1/2" x 0.072") NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.

k. PANEL SUPPORTS AT 24 INCHES, CASING OR FINISH NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.

l. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2-1/2" x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.

m. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.

n. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8" AT INTERMEDIATE SUPPORTS.

o. FASTENERS SPACED AT 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.

p. FASTENERS SPACED AT 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

TABLE 1. CONCRETE MINIMUM REQUIREMENTS					
CONCRETE USAGE	MIN. COMPRESSIVE STRENGTH (f'c)	CONCRETE TYPE	EXPOSURE CLASSES	MAX. W/C RATIO	MIN. AIR CONTENT (%)
COMBINED FOOTINGS, CONTINUOUS FOOTINGS, SPREAD FOOTINGS	3,000 psi AT 56 DAYS	NWC	C0, F0	N/A	5
FOUNDATION WALLS, COLUMNS AND PIERS	4,000 psi AT 28 DAYS	NWC	C1, F1	0.45	5
INTERIOR SLAB-ON-GRADE	4,000 psi AT 28 DAYS	NWC	C0, F0	0.50	5
EXTERIOR SLAB-ON-GRADE	5,000 psi AT 56 DAYS	NWC	C2, F2	0.40	5
NOTES: 1. ALL CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS F0, S0, P0 AND C0 ACCORDING TO ACI 318-08 UNLESS NOTED OTHERWISE ON DRAWINGS. 2. CONCRETE NOTED ABOVE OR ON PLAN AS EXPOSURE CLASS F1, F2, S1, S2, S3, P1, C1 OR C2 SHALL BE PROPORTIONED TO COMPLY WITH THE REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE CONTENT, ETC. 3. FOR SLAB, COORDINATE AND PROVIDE MIX DESIGNS MEETING MAXIMUM CEMENT CONTENT FOR AGGREGATE SIZE TO COMPLY WITH EXPOSURE CLASS. 4. WHERE INDICATED IN THE "ADDITIONAL REMARKS" ABOVE, CONCRETE SHALL BE PROPORTIONED FOR A MAXIMUM ALLOWABLE CHLORIDE CONTENT DETERMINED BY ASTM C157, USING AIR STORAGE. 5. WALLS AND PIERS THAT ARE LOCATED EXTERIOR TO THE BUILDING FOOTPRINT AND EXTEND ABOVE THE FROST LINE ARE EXPOSED TO FREEZING AND THAWING. FOR SUCH WALLS AND PIERS, 4,500 psi AT 28 DAYS, HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45 AND HAVE A MINIMUM 4.5% AIR ENTRAINMENT.					

BEARING WALL SCHEDULE	
PLAN MARK	DESCRIPTION
BW1	2x6 AT 16"oc
BW2	2x6 AT 12"oc
BW3	2x4 AT 12"oc

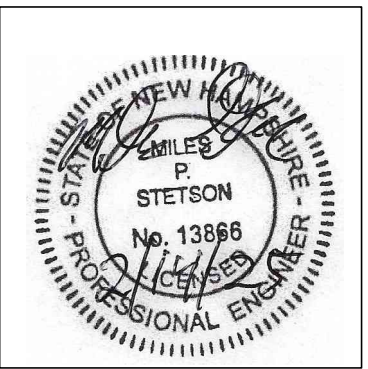
CONCRETE USAGE	MIN. COMPRESSIVE STRENGTH (F _c)	CONCRETE TYPE	EXPOSURE CLASSES	MAX. W/CM RATIO	PERMISSIBLE AIR CONTENT	REQUIRED CEMENT REPLACEMENT	MAX. AGGREGATE SIZE	ADDITIONAL REMARKS
COMBINED FOOTINGS, CONTINUOUS FOOTINGS, SPREAD FOOTINGS	3,000 psi AT 56 DAYS	NWC	C0, F0	N/A	N/A	0 - 70%	3/4"	
FOUNDATION WALLS, COLUMNS AND PIERS	4,000 psi AT 28 DAYS	NWC	C1, F1	0.45	4.5% ±1.5%	0 - 50%	3/4"	
INTERIOR SLAB-ON-GRADE	4,000 psi AT 28 DAYS	NWC	C0, F0	0.50	N/A	0 - 50%	3/4"	
EXTERIOR SLAB-ON-GRADE	5,000 psi AT 56 DAYS	NWC	C2, F2	0.40	5.5% ±1.5%	15 - 25%	3/4"	

NOTES:

- ALL CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS F0, S0, P0 AND C0 ACCORDING TO ACI 318-08 UNLESS NOTED OTHERWISE IN TABLE ABOVE, IN NOTES BELOW OR ELSEWHERE ON THE STRUCTURAL DRAWINGS.
- CONCRETE NOTED ABOVE OR ON PLAN AS EXPOSURE CLASS F1, F2, S1, S2, S3, P1, C1 OR C2 SHALL BE PROPORTIONED TO COMPLY WITH ACI 318-08 TABLES 4.3.1, 4.4.1 AND 4.4.2 IN ADDITION TO THE NOTATIONS IN THE REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES AND AIR CONTENT LIMITS, CHLORIDE LIMITS AND POZZOLAN LIMITS.
- FOR SLAB, COORDINATE AND PROVIDE MIX DESIGNS MEETING MAXIMUM CEMENT CONTENT FOR AGGREGATE SIZE TO COMPLY WITH TABLE 8.4.1B OF ACI 302-15.
- WHERE INDICATED IN THE "ADDITIONAL REMARKS" ABOVE, CONCRETE SHALL BE PROPORTIONED FOR A MAXIMUM ALLOWABLE UNIT SHRINKAGE OF 0.035% MEASURED 28 DAYS AFTER CURING IN LIME WAS AS DETERMINED BY ASTM C157, USING AIR STORAGE.
- WALLS AND PIERS THAT ARE LOCATED EXTERIOR TO THE BUILDING FOOTPRINT AND EXTEND ABOVE THE FROST LINE ARE EXPOSURE CLASS C1 AND F1 AND SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4,500 psi AT 28 DAYS, HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45 AND HAVE A MINIMUM 4.5% AIR ENTRAINMENT.

PLAN MARK	DESCRIPTION
BW1	2x6 AT 16"oc
BW2	2x6 AT 12"oc
BW3	2x4 AT 12"oc

1. STUDS SHALL ALIGN WITH FRAMING ABOVE AND BELOW
2. BEARING WALLS SHALL BE BLOCKED AT 4'-0" O.C. U.N.O
3. ALL BEARING WALLS TO BE TYPE BW1, U.N.O.
4. ALL BEARING WALLS TO HAVE (2) 2X TOP PLATE, U.N.O.
5. WOOD IN CONTACT W/CONCRETE TO BE P.T.

[illegible]

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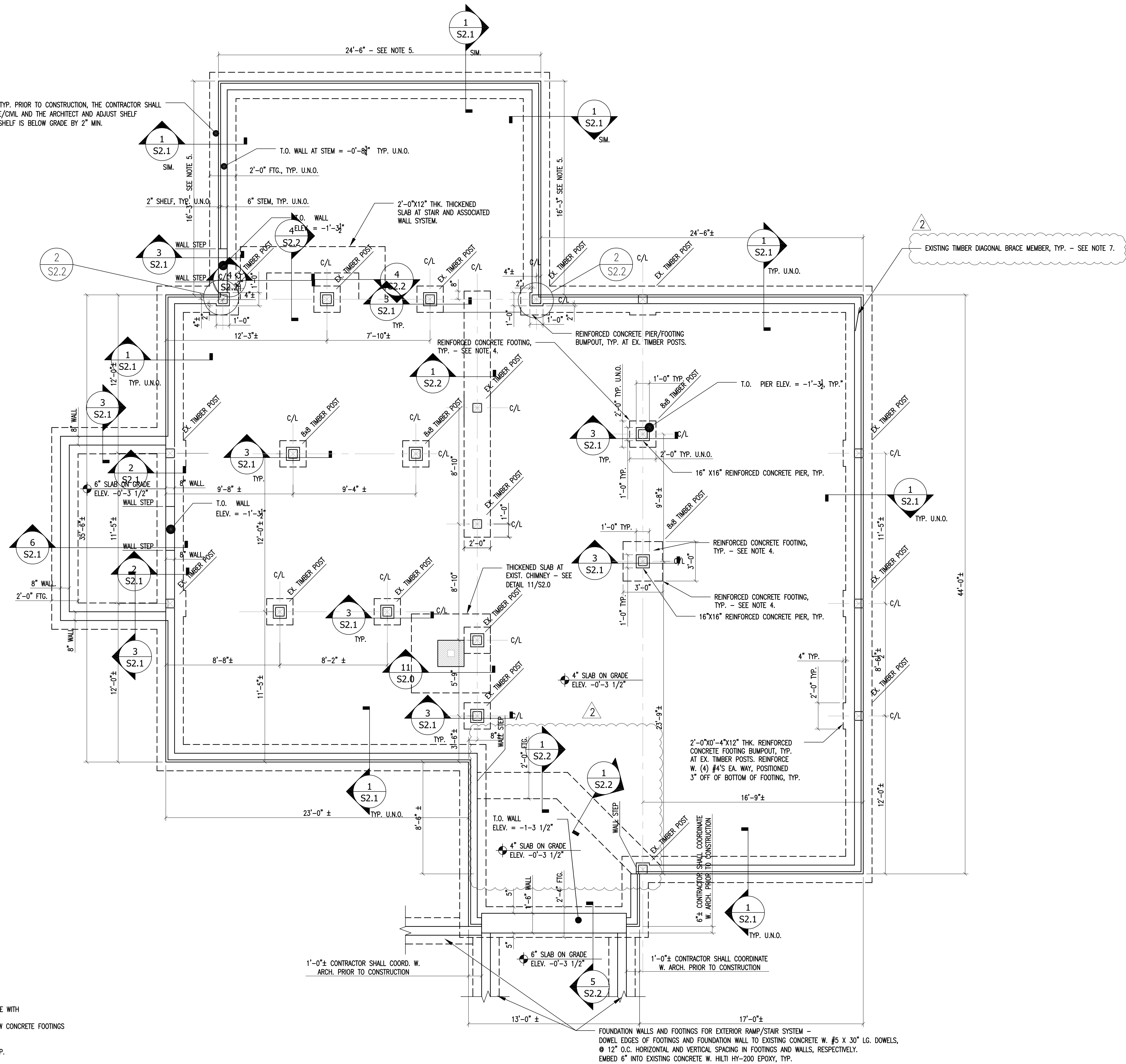
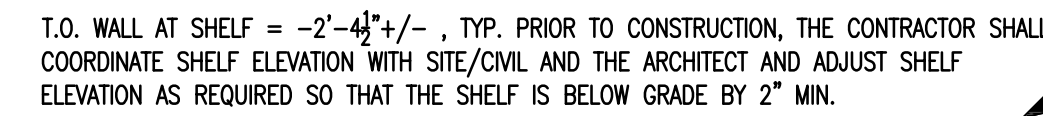
Client: **BARRETT ARCHITECTURE**
281 THE BEND RD.
GREENSBORO BEND, VT 05842

Sheet Title: STRUCTURAL NOTES SPECIAL INSPECTIONS
Project Title: LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS
120 MAIN ST. LITTLETON, NH 03561

Designed By:	MS/IP
Checked By:	MS
Drawn By:	IP
Scale:	AS NOTED
Date:	2/14/24

S0.3

EV PROJECT #24169



NOTES:

1. ALL DIMENSIONS AND ELEVATIONS ARE APPROXIMATE. THE CONTRACTOR SHALL VERIFY ELEVATIONS AND DIMENSIONS IN THE FIELD AND COORDINATE WITH THE ARCHITECTURAL DOCUMENTS PRIOR TO CONSTRUCTION.
2. REMOVE UNSUITABLE SOILS BENEATH NEW FOUNDATION AND SLAB ON GRADE AND BACKFILL EXISTING BASEMENT/CRAWL SPACE AND BENEATH NEW CONCRETE FOOTINGS WITH COMPACTED GRANULAR FILL. SEE SPECIFICATIONS AND SLAB ON GRADE DETAILS.
3. REMOVE EXISTING TIMBER FRAMING FOR EXISTING WALL FRAMES AS REQUIRED FOR PROPER FLUSH TO TOP OF FOUNDATION WALL.
4. REINFORCED CONCRETE FOOTINGS SHALL BE 12" THICK. REINFORCE 2"22' FOOTINGS W. (3) #4'S EA. WALL, 3" OFF OF BOTTOM OF FOOTING; TY. AND 3"x3' FOOTING W. (4) #4'S EA. WALL, 3" OFF OF BOTTOM OF FOOTING; TY.
5. DEMOLISH EXISTING WALL FRAMING AND REPLACE WITH 2"x4 STUDS @ 12" O.C.. PROVIDE FULL-DEPTH BLOCKING AT 4'-0" O.C. SPACING OVER STUD HEIGHT, TY.
6. ELEVATIONS ARE BASED ON FINISHED FLOOR ELEVATION OF 0'-0".
7. THE CONTRACTOR SHALL VERIFY QUANTITY AND LOCATIONS OF EXISTING TIMBER DIAGONAL BRACES AT BASES OF EXISTING TIMBER POSTS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL ATTACH BOTTOM ENDS OF EXISTING BRACES TO THE TOP OF THE NEW MUDDO SLIP PLATE WITH (2) 8" LG. TIMBERLOK SCREWS, TY. FOR ALL.

FOUNDATION PLAN

$$1/4^n = 1' - 0^n$$

	Rev.	No.	Description	Circlod
	1	7/28 /25	COORDINATED REVISIONS	
	2	8/28 /25	DESIGN UPDATES	

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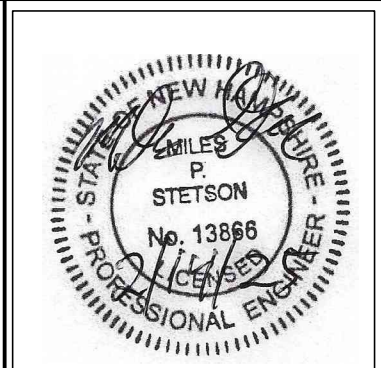
BARRETT ARCHITECTURE
281 THE BEND RD.
GREENSBORO BEND, VT 05842

FOUNDATION PLAN

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Scale:	AS NOTED
Date:	2/14/24

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EV PROJECT #24169



Rev. No.	Description	Chkd.
1	7/28/25 COORDINATED REVISIONS	
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Client:

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281 THE BEND RD. 05842
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Sheet Title:

PARTIAL ROOF FRAMING PLAN

Project Title:

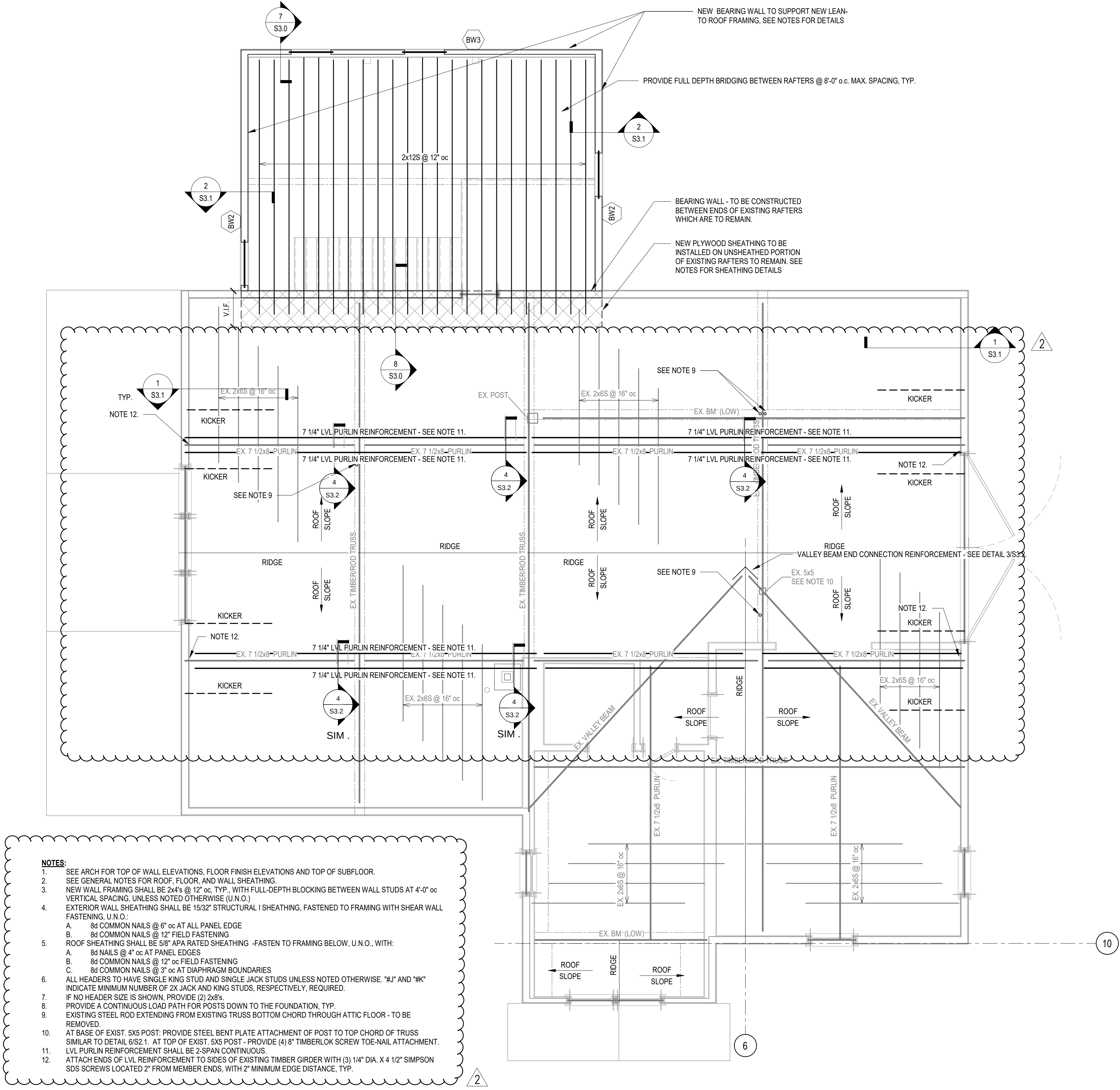
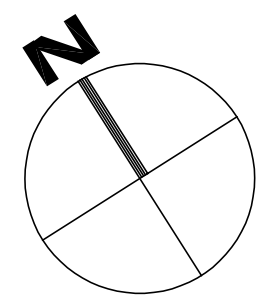
LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS

120 MAIN ST. LITTLETON, NH 03561

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Checked By:	MS
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Scale:	AS NOTED
Date:	2/14/24

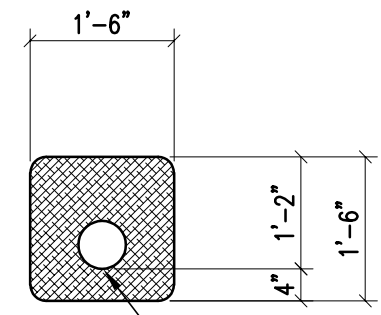
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EV PROJECT #24169



- NOTES:**
- SEE ARCH FOR TOP OF WALL ELEVATIONS, FLOOR FINISH ELEVATIONS AND TOP OF SUBFLOOR.
 - SEE GENERAL NOTES FOR ROOF, FLOOR, AND WALL SHEATHING.
 - NEW WALL FRAMING SHALL BE 2x4s @ 12" oc, TYP. WITH FULL-DEPTH BLOCKING BETWEEN WALL STUDS AT 4'-0" oc VERTICAL SPACING, UNLESS NOTED OTHERWISE (U.N.O.).
 - EXTERIOR WALL SHEATHING SHALL BE 15/32" STRUCTURAL I SHEATHING, FASTENED TO FRAMING WITH SHEAR WALL FASTENING, U.N.O.:
 - 8d COMMON NAILS @ 6" oc AT ALL PANEL EDGE
 - 8d COMMON NAILS @ 12" FIELD FASTENING
 - ROOF SHEATHING SHALL BE 5/8" APA RATED SHEATHING - FASTEN TO FRAMING BELOW, U.N.O., WITH:
 - 8d NAILS @ 4" oc AT PANEL EDGES
 - 8d COMMON NAILS @ 12" oc FIELD FASTENING
 - 8d COMMON NAILS @ 3" oc AT DIAPHRAGM BOUNDARIES
 - ALL HEADERS TO HAVE SINGLE KING STUD AND SINGLE JACK STUDS UNLESS NOTED OTHERWISE. "HJ" AND "HK" INDICATE MINIMUM NUMBER OF 2X JACK AND KING STUDS, RESPECTIVELY, REQUIRED.
 - IF NO HEADER SIZE IS SHOWN, PROVIDE (2) 2x8s.
 - PROVIDE A CONTINUOUS LOAD PATH FOR POSTS DOWN TO THE FOUNDATION, TYP.
 - EXISTING STEEL ROD EXTENDING FROM EXISTING TRUSS BOTTOM CHORD THROUGH ATTIC FLOOR - TO BE REMOVED.
 - AT BASE OF EXIST. 5X5 POST: PROVIDE STEEL BENT PLATE ATTACHMENT OF POST TO TOP CHORD OF TRUSS. SIMILAR TO DETAIL 6/S2.1. AT TOP OF EXIST. 5X5 POST - PROVIDE (4) 8" TIMBERLOK SCREW TOE-NAIL ATTACHMENT.
 - LVL PURLIN REINFORCEMENT SHALL BE 2-SPAN CONTINUOUS.
 - ATTACH ENDS OF LVL REINFORCEMENT TO SIDES OF EXISTING TIMBER GIRDER WITH (3) 1/4" DIA. X 4 1/2" SIMPSON SDS SCREWS LOCATED 2" FROM MEMBER ENDS, WITH 2" MINIMUM EDGE DISTANCE, TYP.

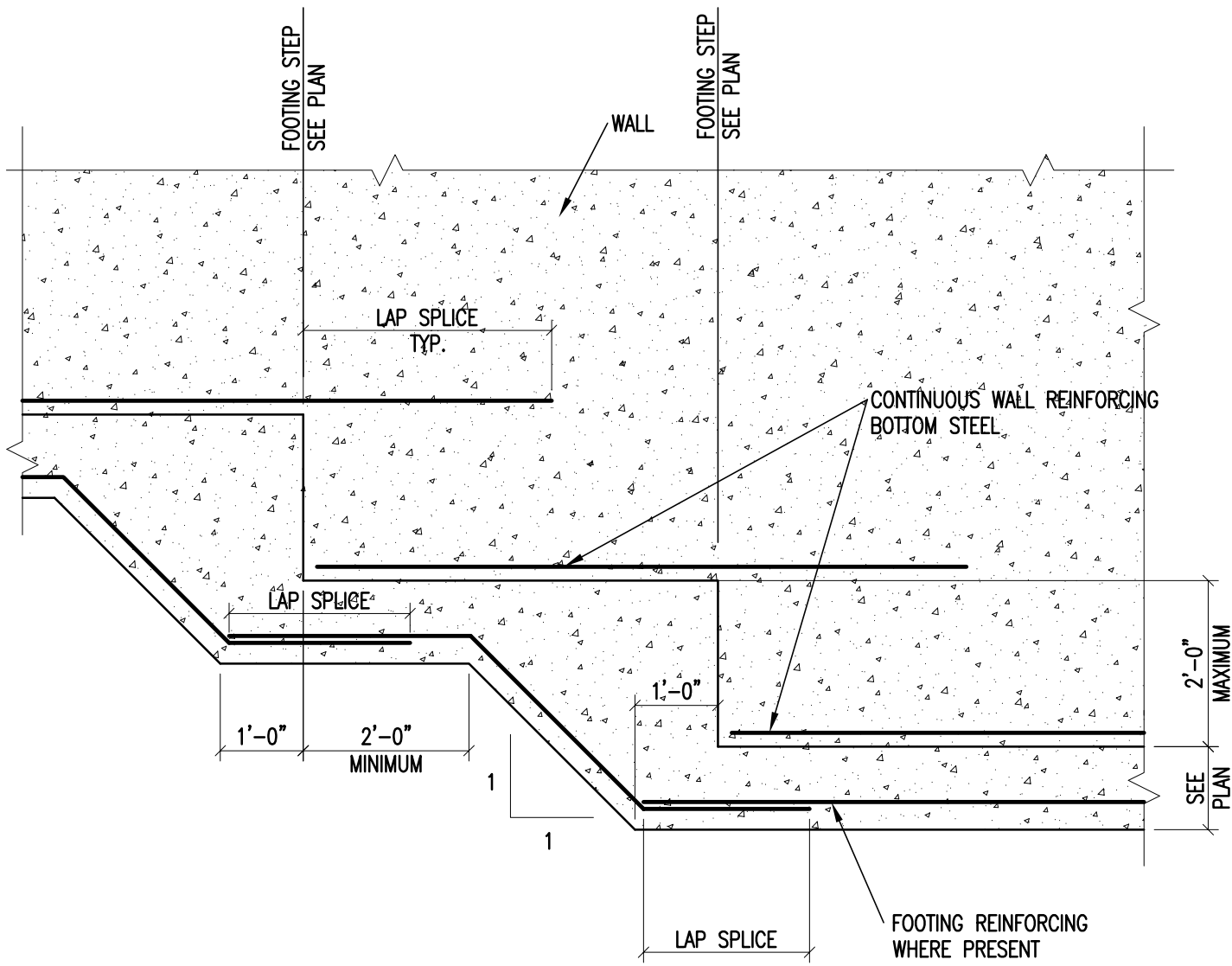
1 ROOF FRAMING PLAN
1/4" = 1'-0"



6" PERFORATED PVC DRAIN. PLACE IN 18" OF CRUSHED STONE. WRAP IN MIRAFLEX 140N FILTER FABRIC OR EQUIVALENT. REFER TO SITE ENGINEERING DRAWINGS FOR OUTLET LOCATION AND DETAILS. PROVIDE CLEAN OUTS AT ALL CORNERS AND 50' MAXIMUM SPACING. COORDINATE WITH SPECIFICATIONS. FOOTING DRAIN SHALL BE PITCHED TO DAYLIGHT OR CATCH BASIN - COORDINATE W. CIVIL.

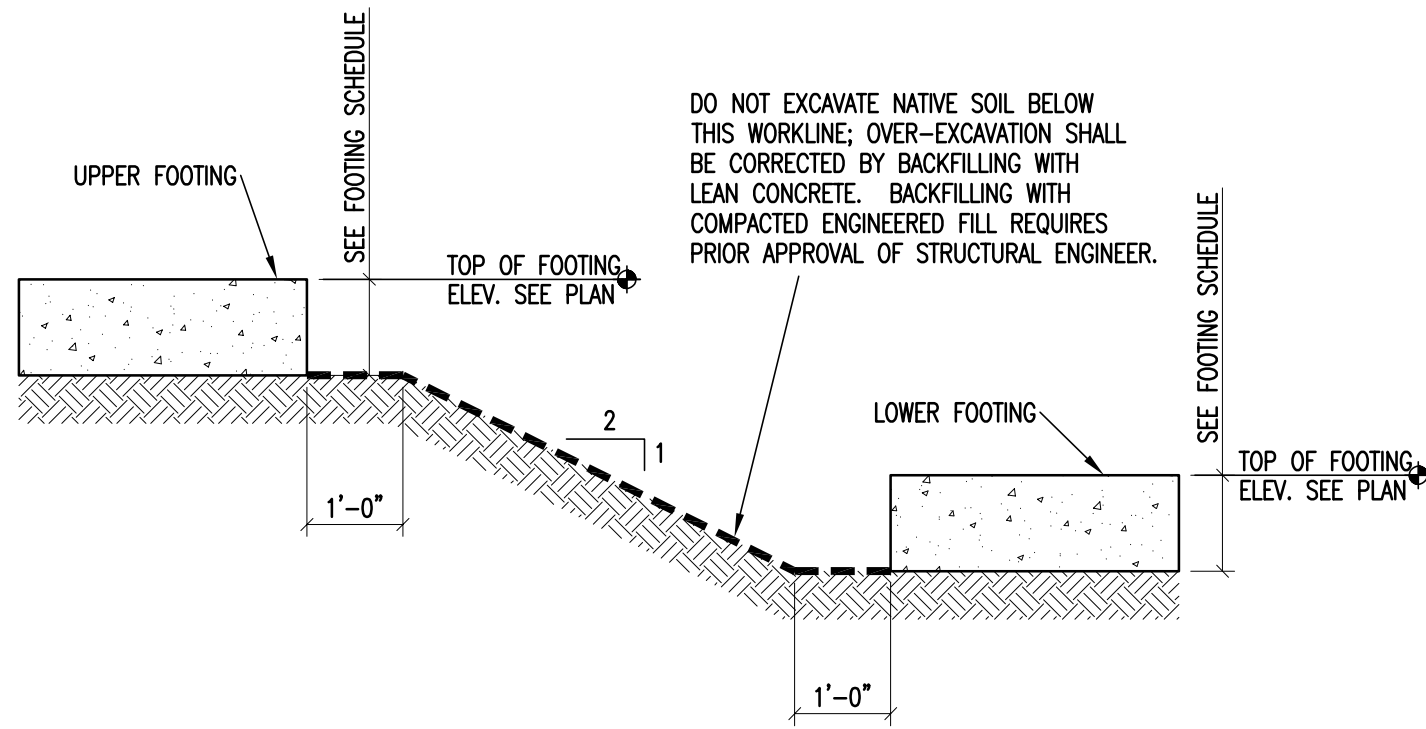
TYPICAL FOOTING DRAIN

1/2" = 1'-0"



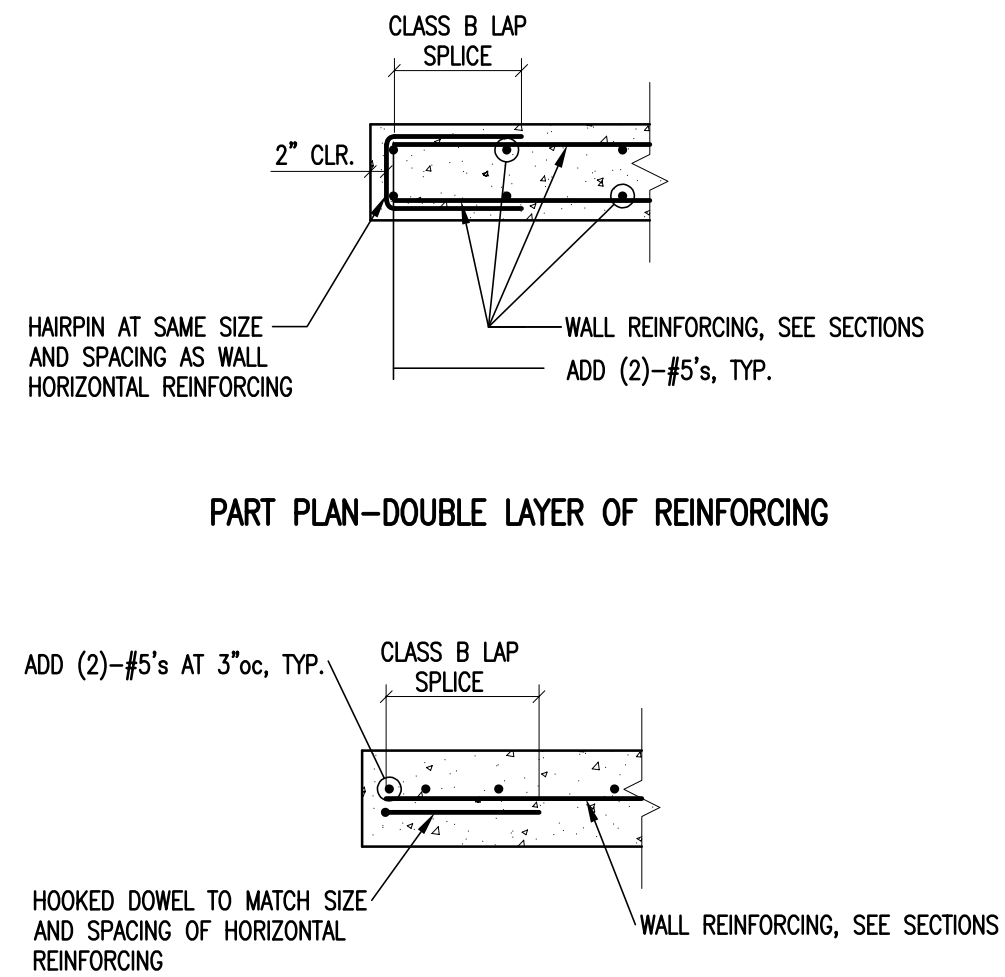
TYPICAL FOOTING STEP

1/2" = 1'-0"



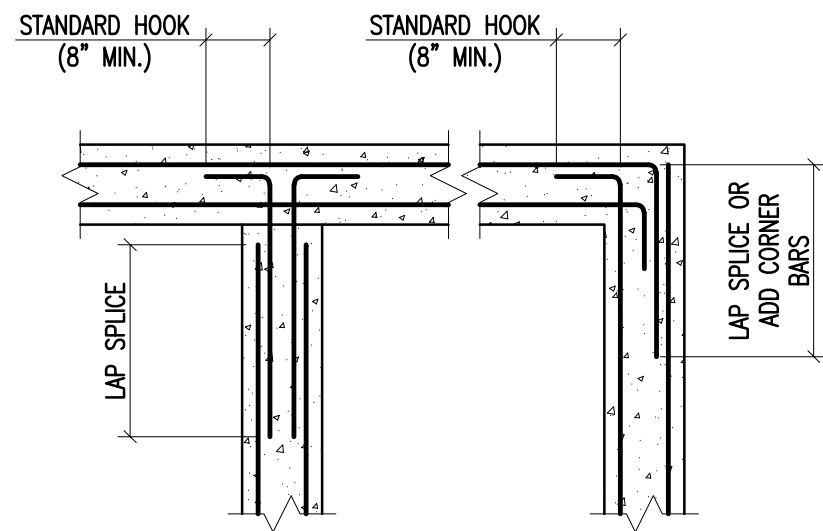
TYPICAL EXCAVATION AT ADJACENT FOOTING

1/2" = 1'-0"



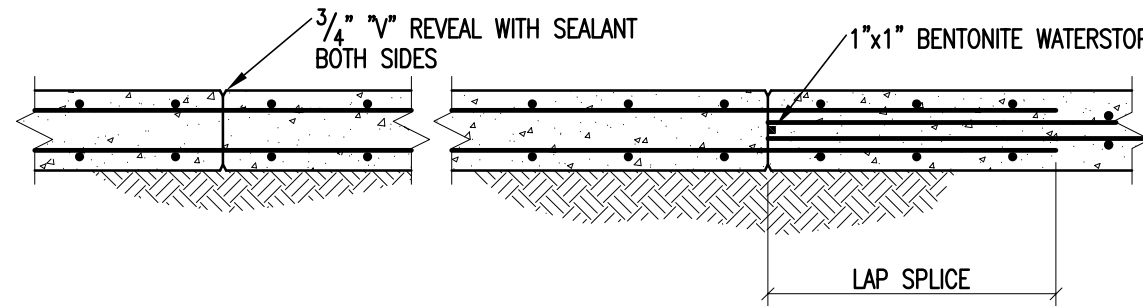
TYPICAL CONCRETE WALL END

1/2" = 1'-0"



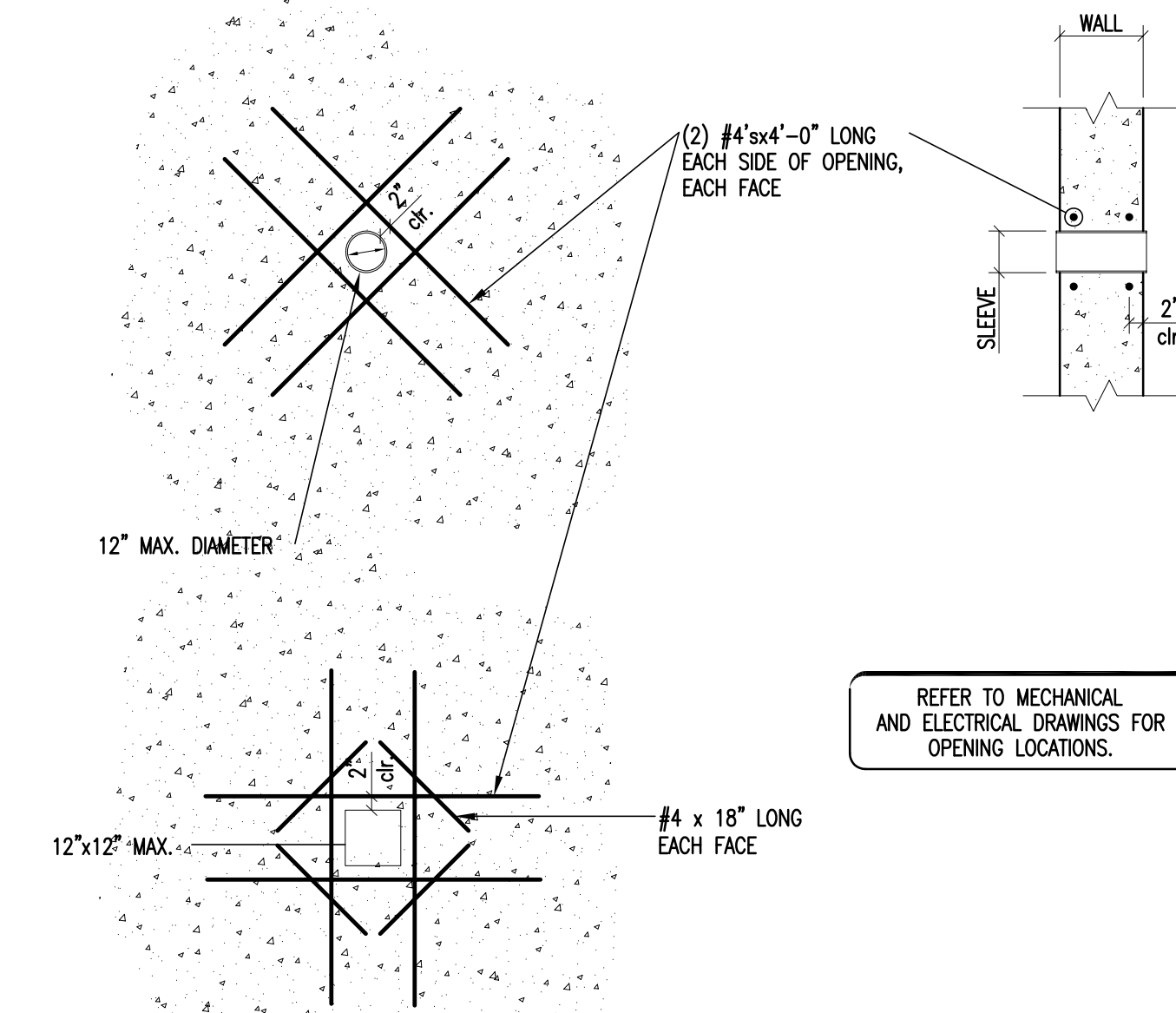
TYPICAL CONCRETE WALL CORNER REINFORCING

1/2" = 1'-0"



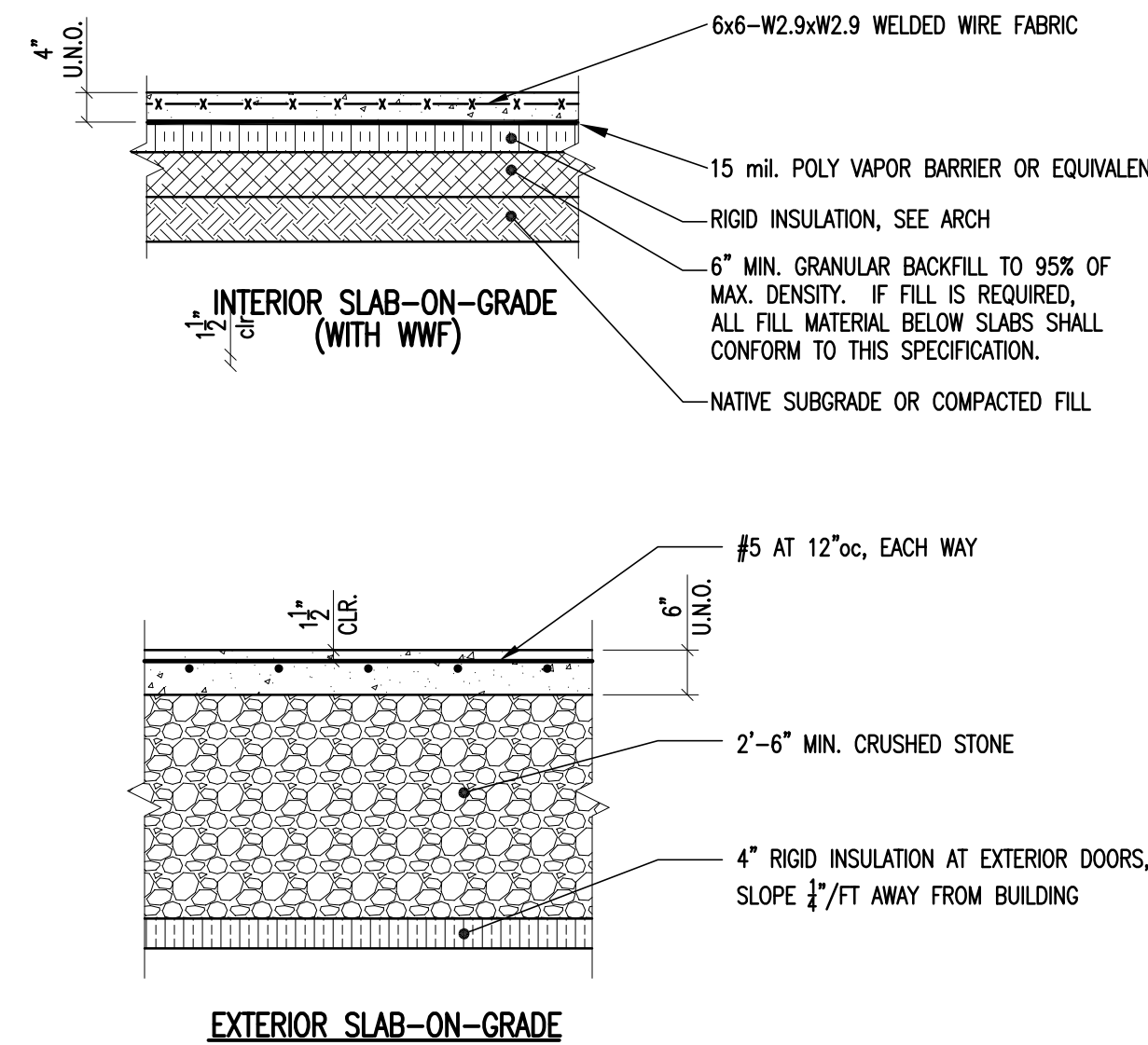
TYPICAL CONCRETE WALL JOINTS

1/2" = 1'-0"



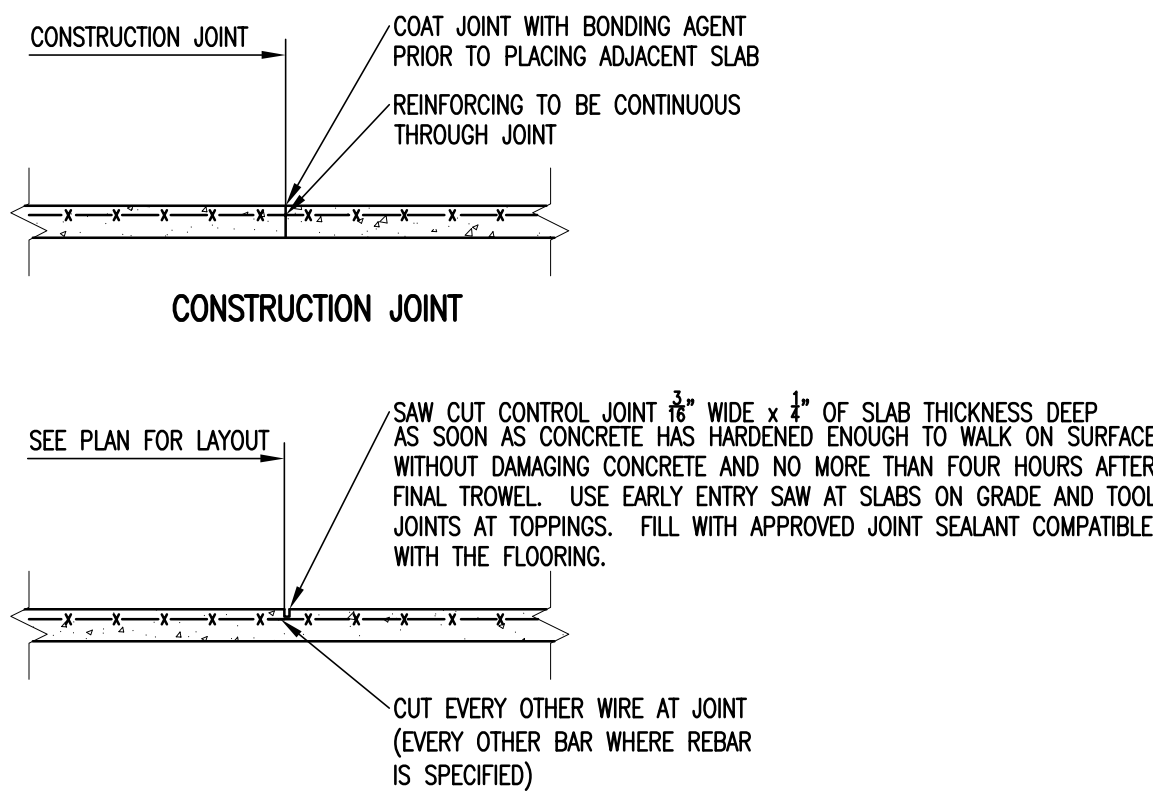
TYPICAL CONCRETE WALL OPENING (<12")

1/2" = 1'-0"



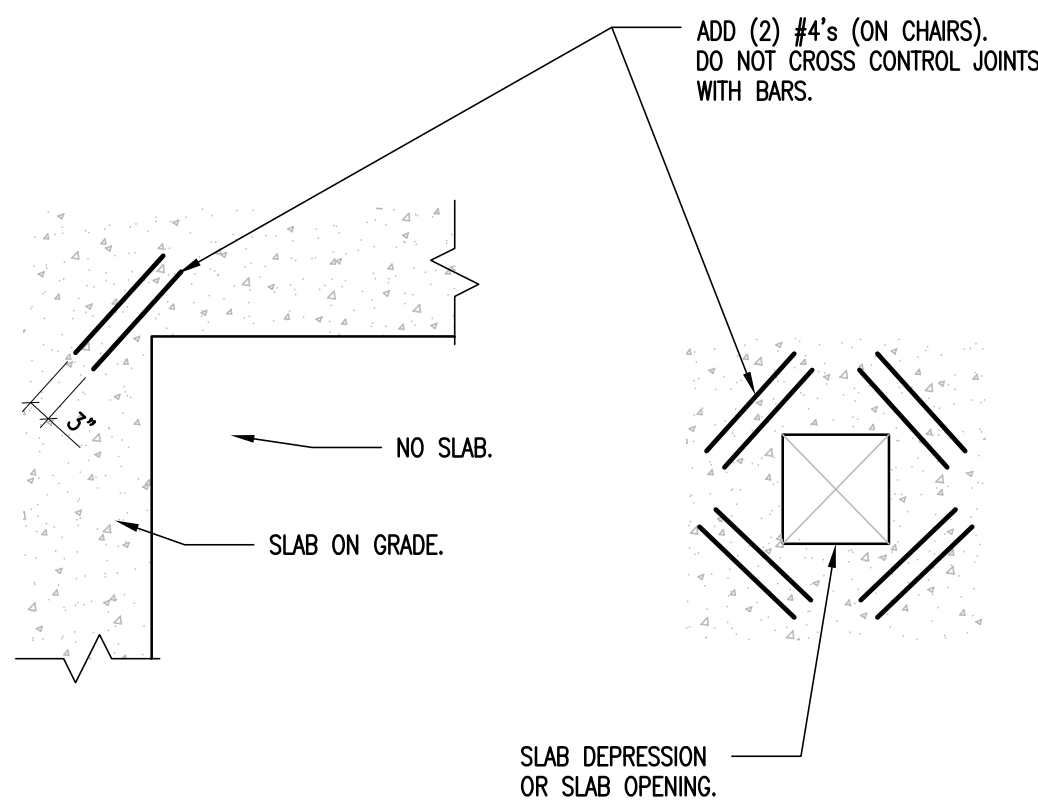
TYPICAL SLAB-ON-GRADE SECTIONS

1/2" = 1'-0"



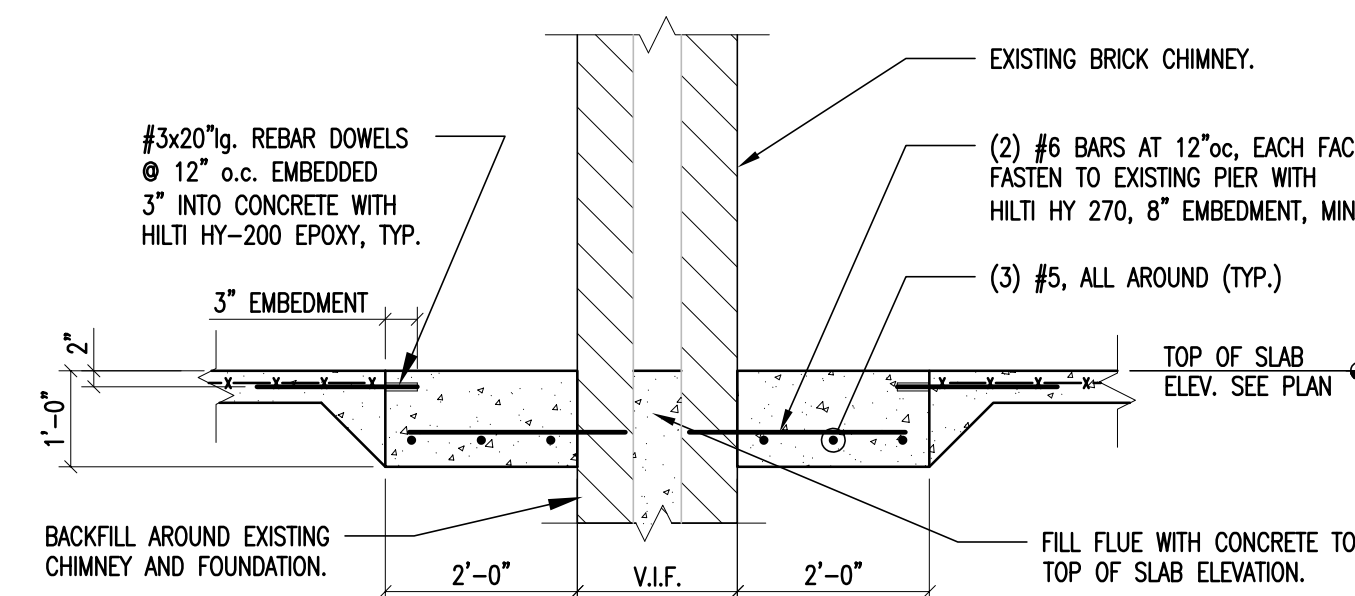
TYPICAL SLAB-ON-GRADE JOINTS

1/2" = 1'-0"



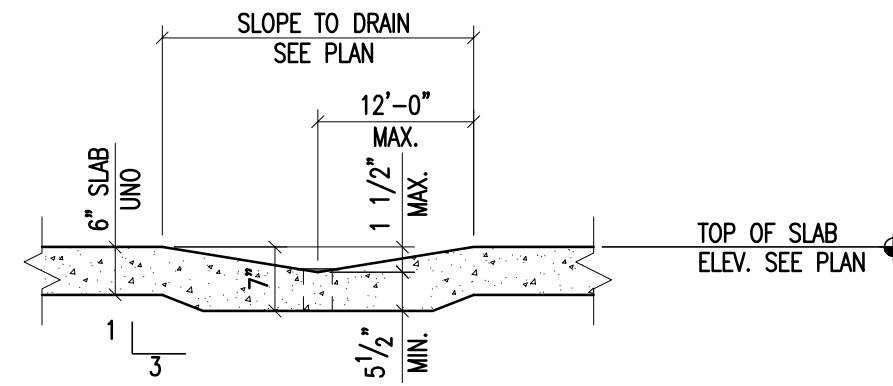
TYPICAL RE-ENTRANT CORNERS AT SLAB ON GRADE

1/2" = 1'-0"



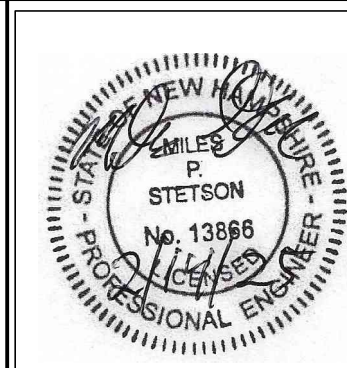
DETAIL - NEW FOOTING REINFORCING AT EX. CHIMNEY

1/2" = 1'-0"



TYPICAL SLOPE-TO-DRAIN AT SLAB-ON-GRADE

1/2" = 1'-0"



Rev. No.	Description	Chkd.
1	7/26/25 COORDINATED REVISIONS	

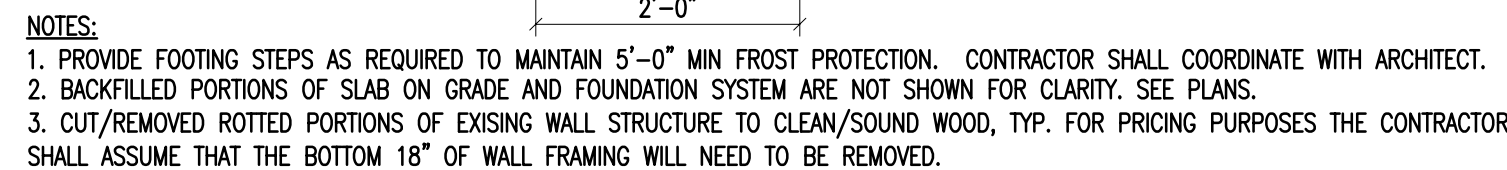
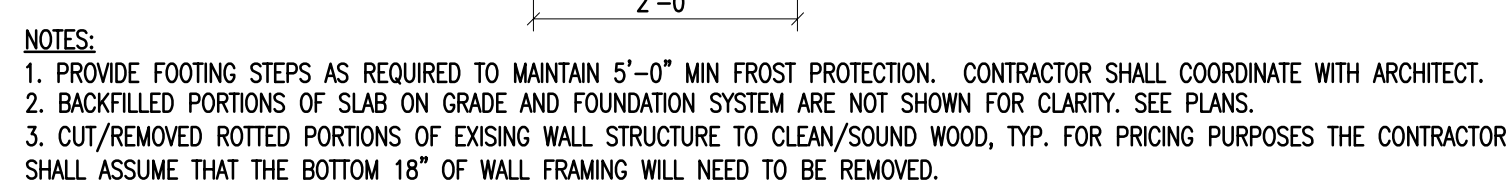
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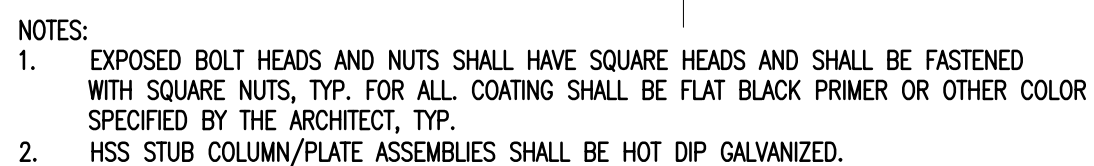
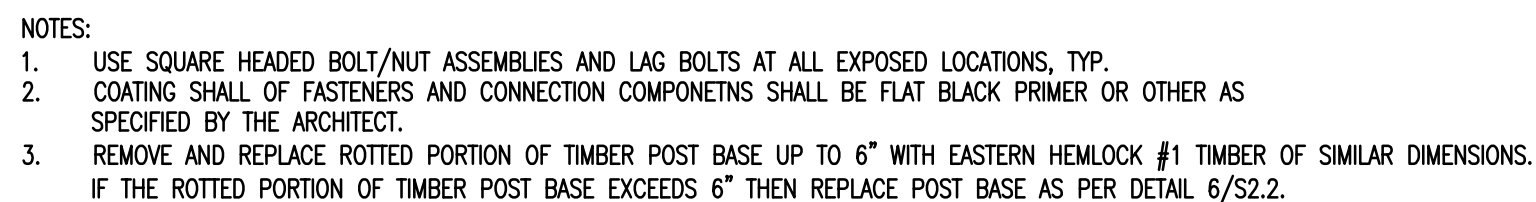
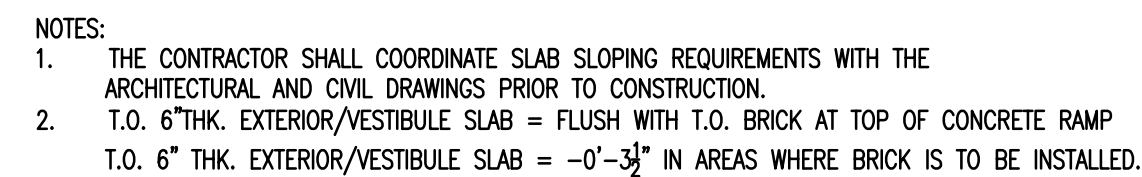
Client:
BARRETT ARCHITECTURE
281 THE BEND RD. 05842
GREENSBORO BEND, VT

Sheet Title:
FOUNDATION DETAILS
Project Title: **LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS**
120 MAIN ST. LITTLETON, NH 03561

Designed By: **MS**
Checked By: **MS**
Drawn By: **MS**
Scale: **AS NOTED**
Date: **2/14/24**

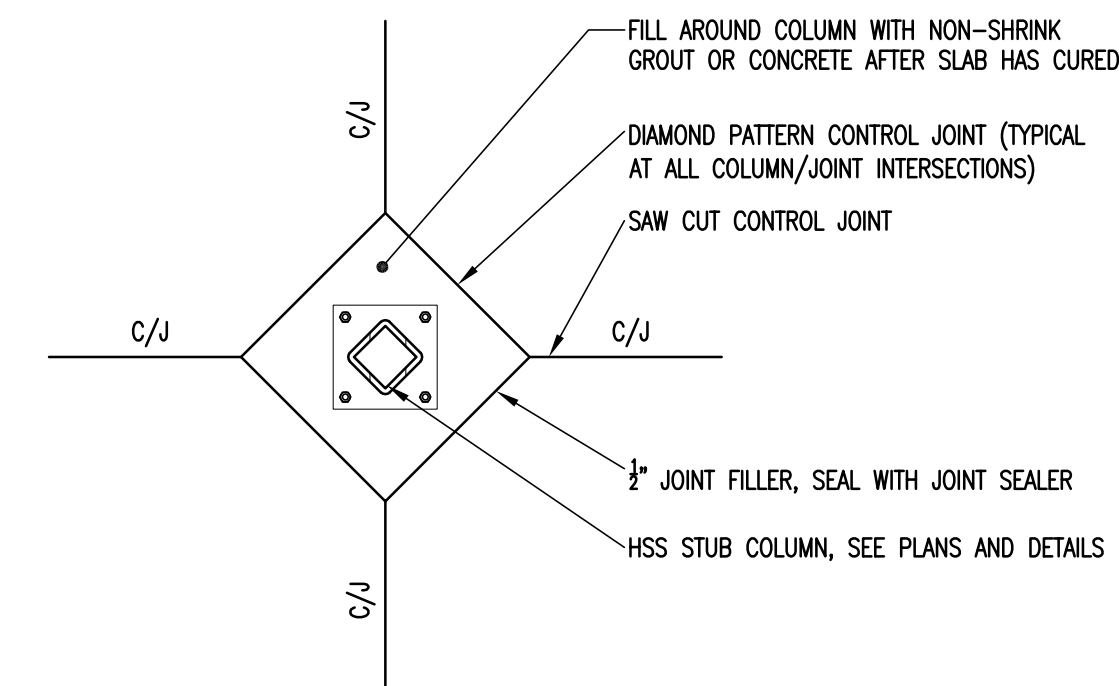
S2.0
EV PROJECT #24169


$$1/2^n = 1' - 0^n$$

$$1/2'' = 1'-0''$$

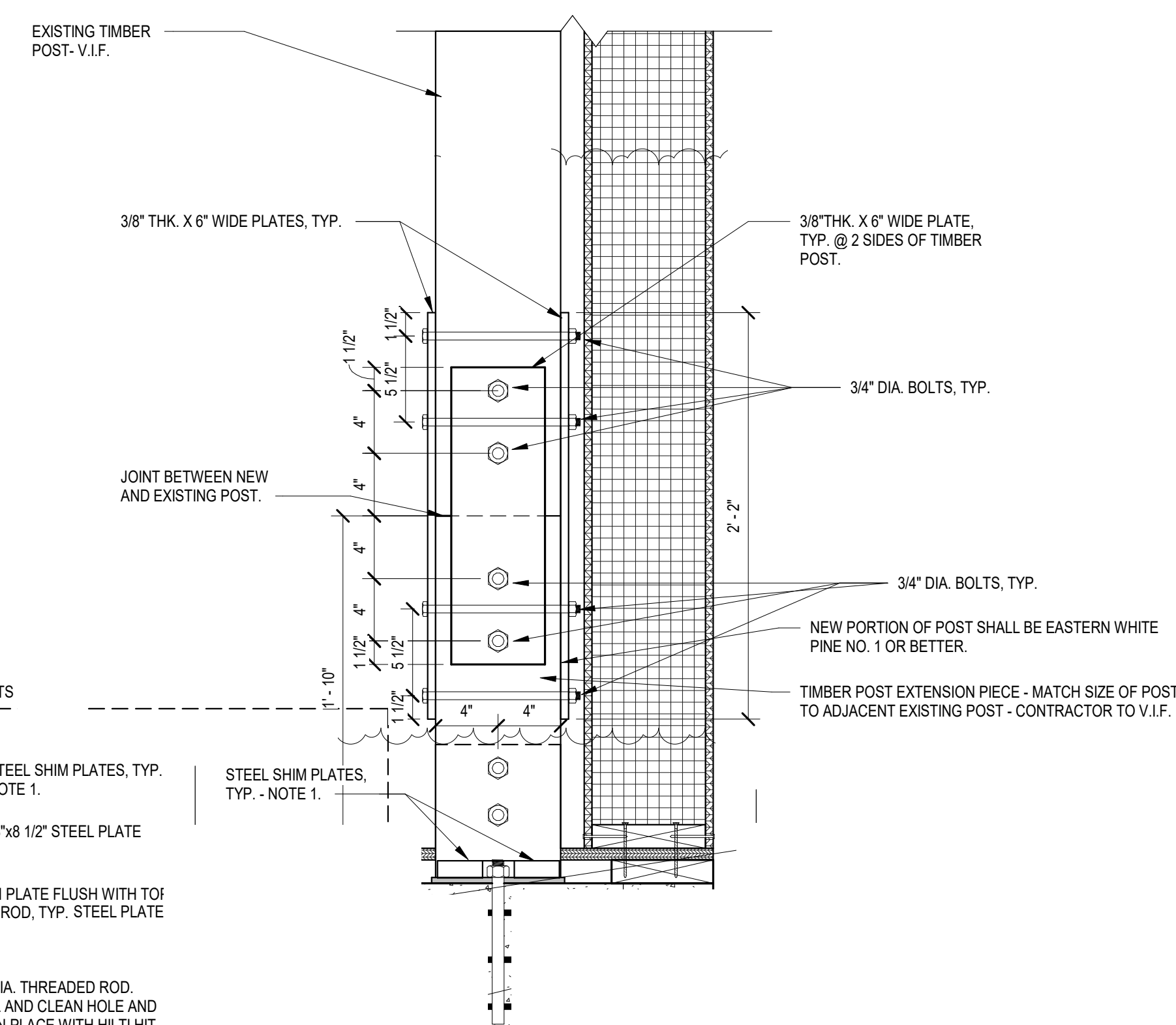
$$1/2^n = 1'-0^n$$

$$3/4'' = 1'-0''$$

$$\frac{3}{4}'' = 1'-0''$$

$$\frac{3}{4}'' = 1'-0''$$

S2.1

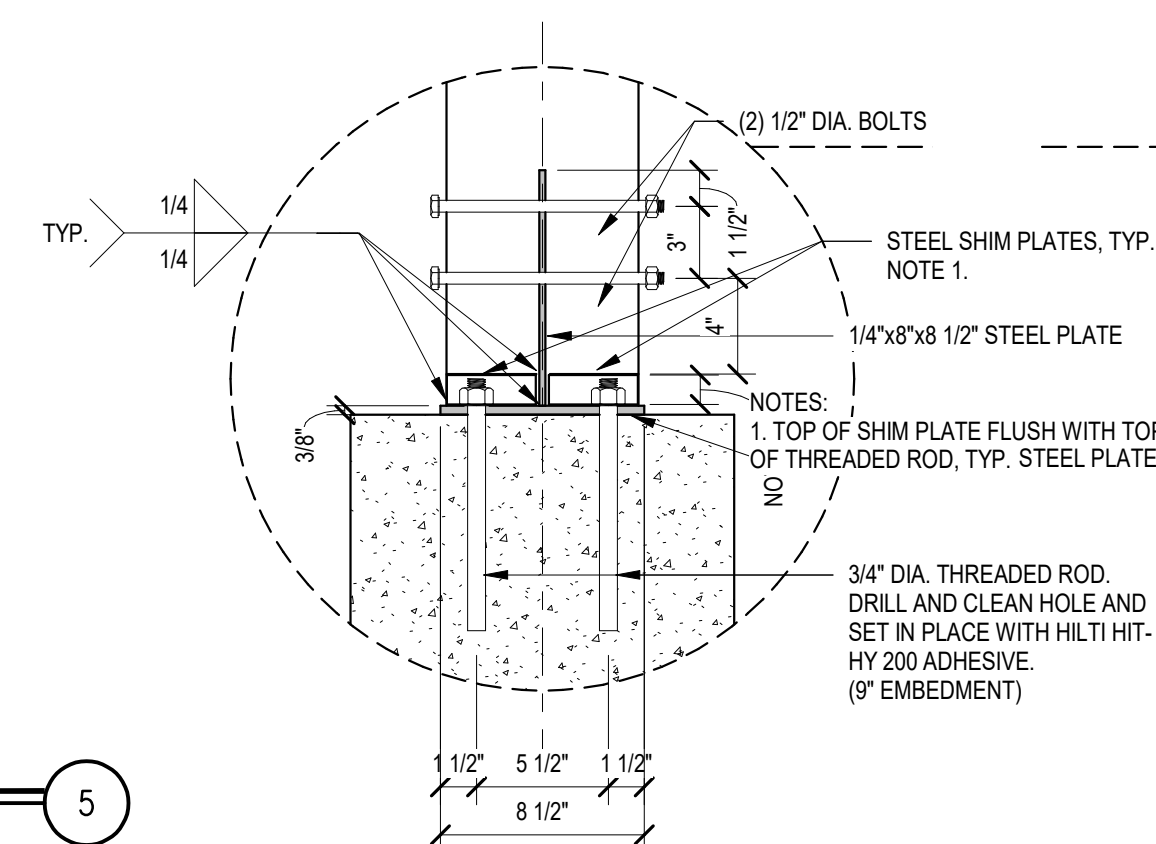
EV PROJECT #24169



TYPICAL COLUMN
ISOLATION JOINT AT SLAB-ON-GRADE



TYPICAL POST ROT REPAIR DETAIL - ROT
EXTENDING GREATER THAN 6" FROM POST BASE



STATE OF NEW HAMPSHIRE
P. STETSON
No. 13886
PROFESSIONAL ENGINEER

[illegible]

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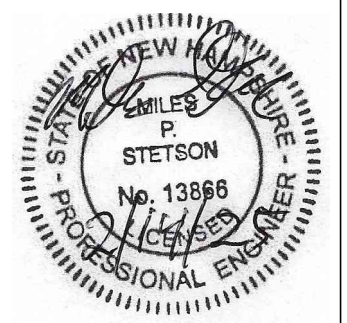
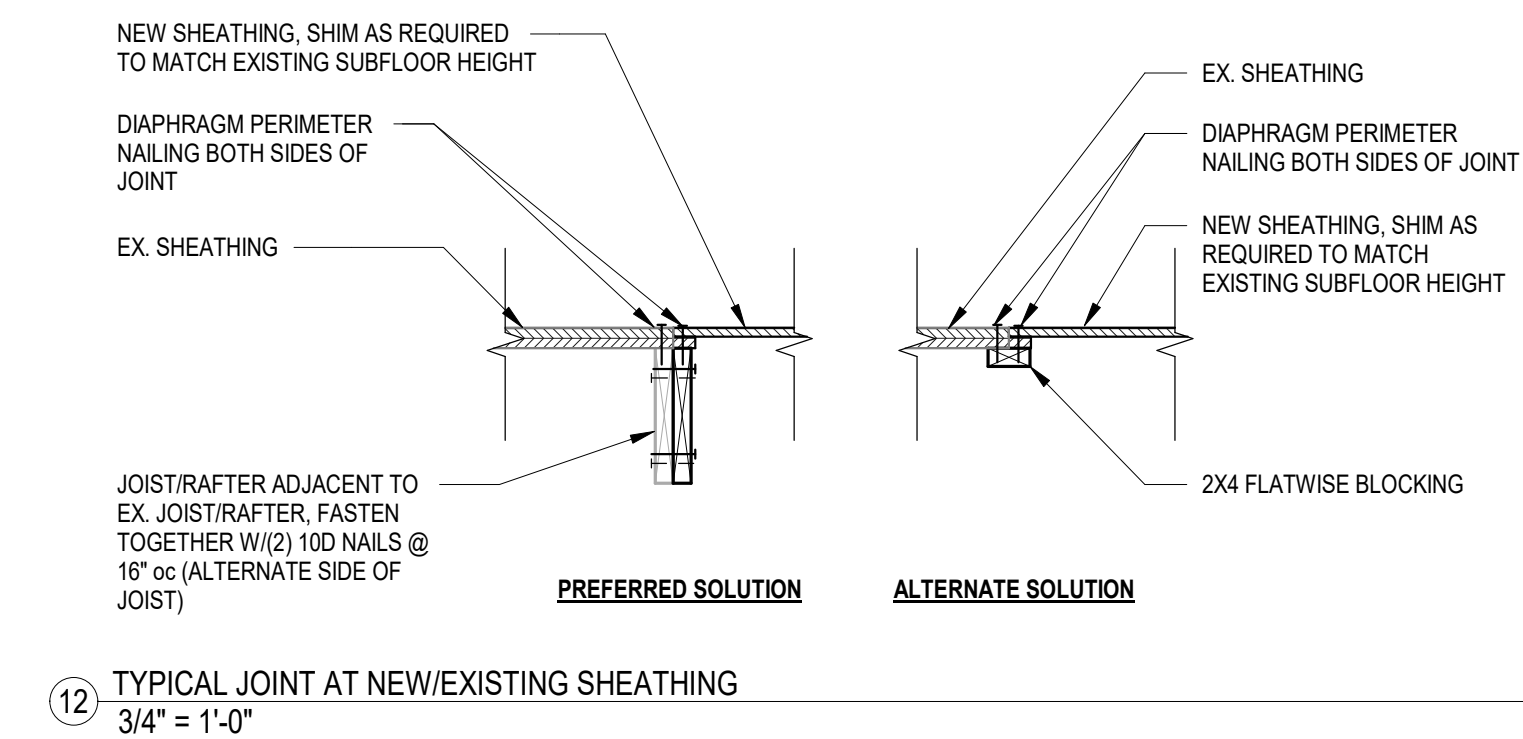
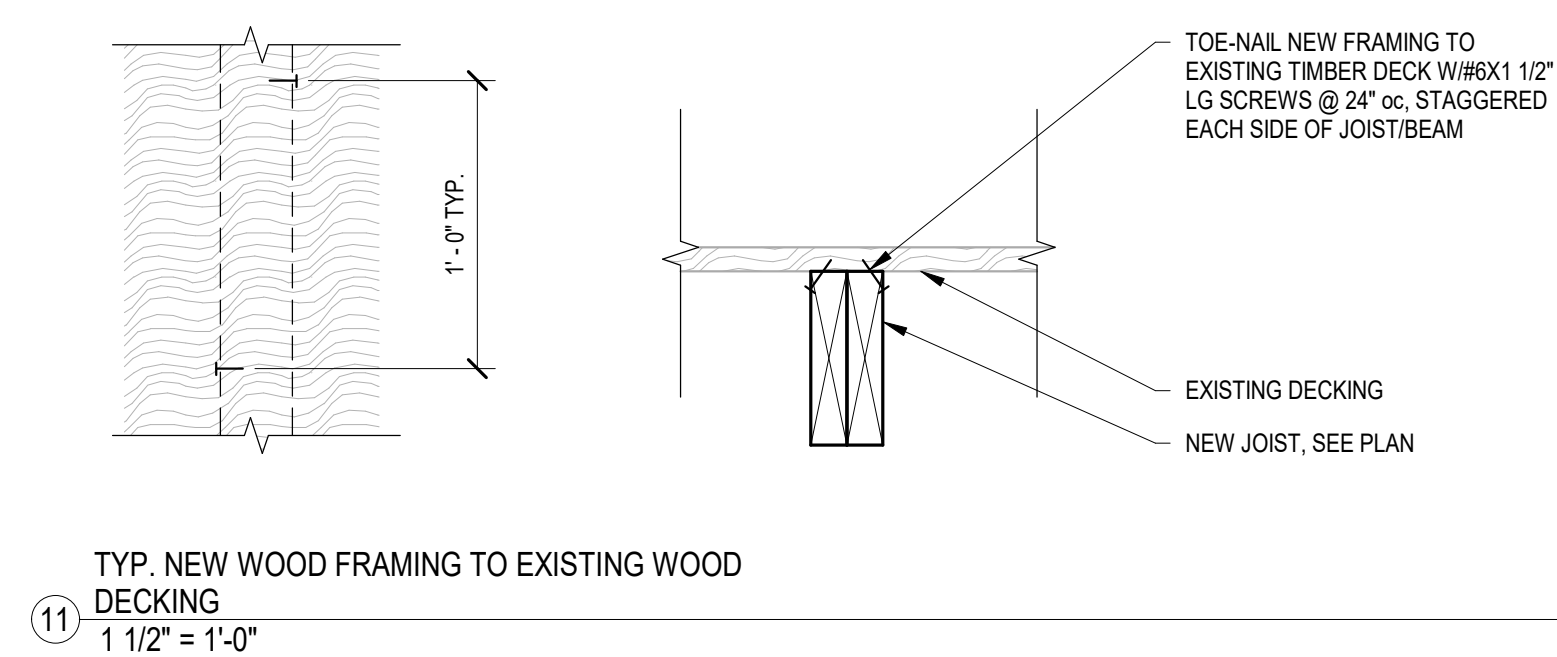
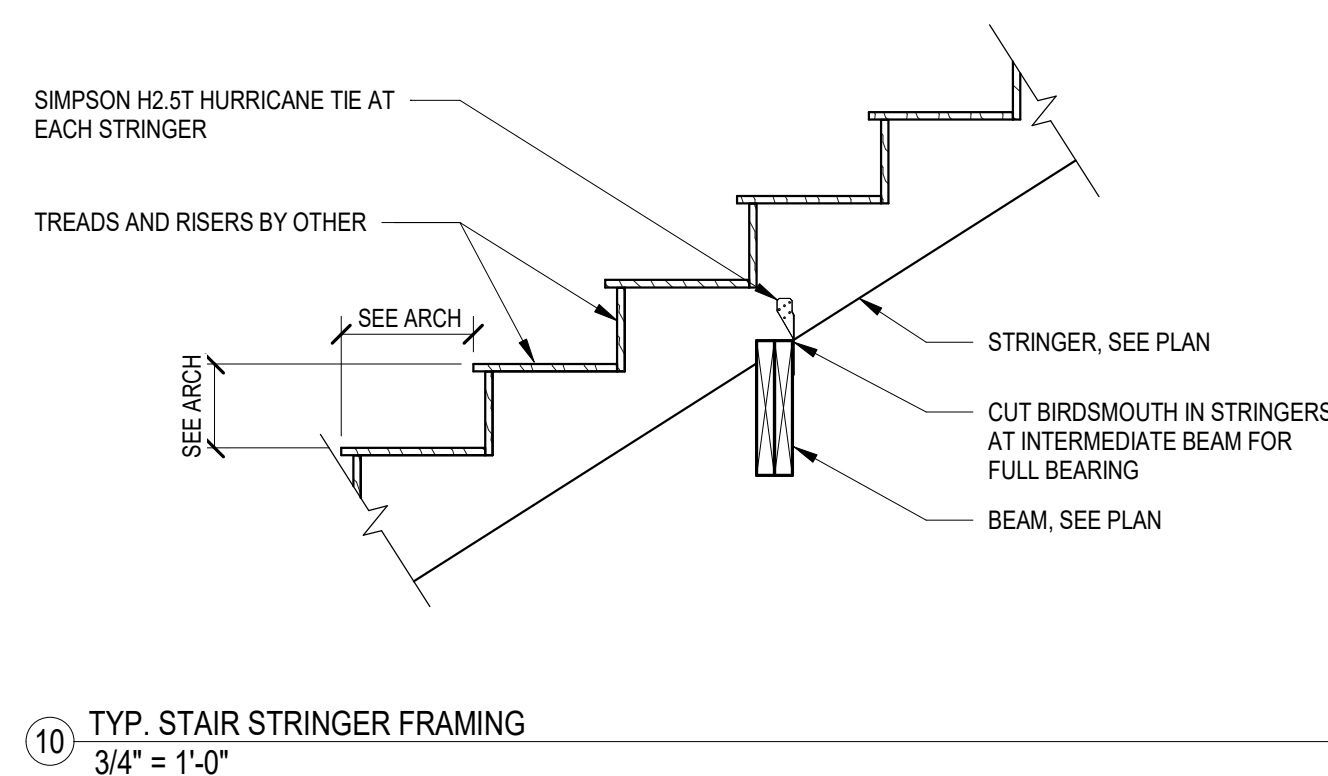
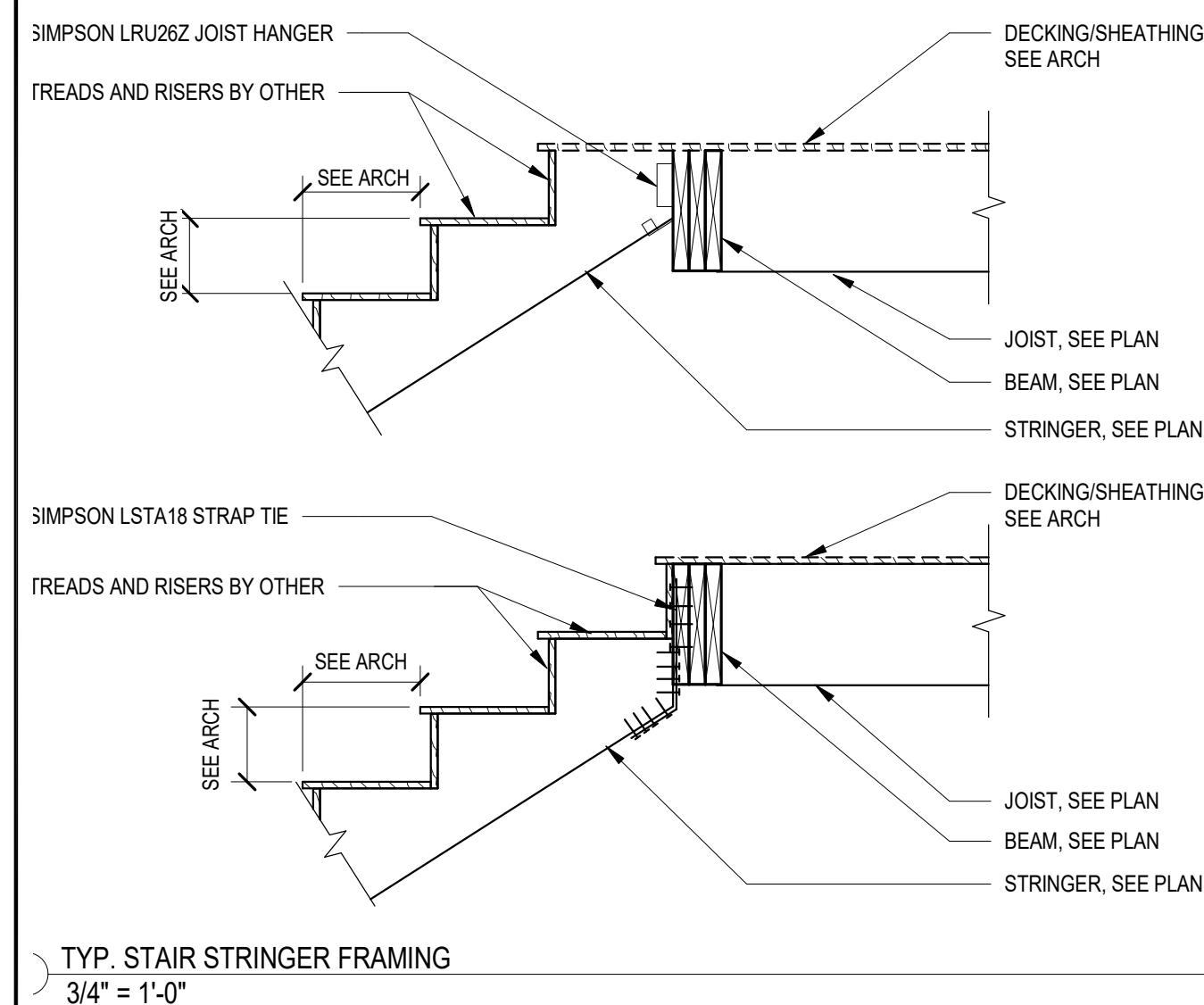
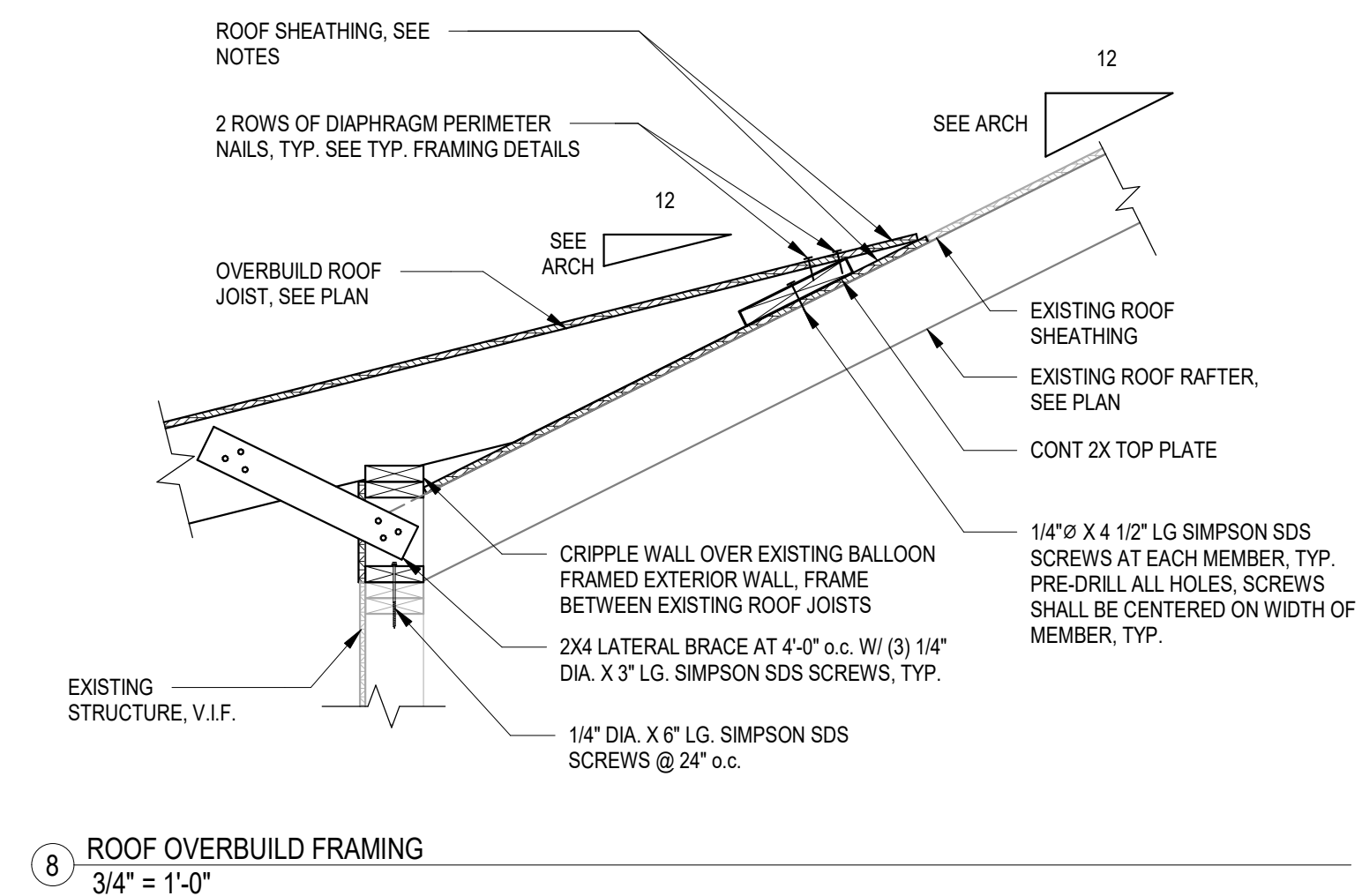
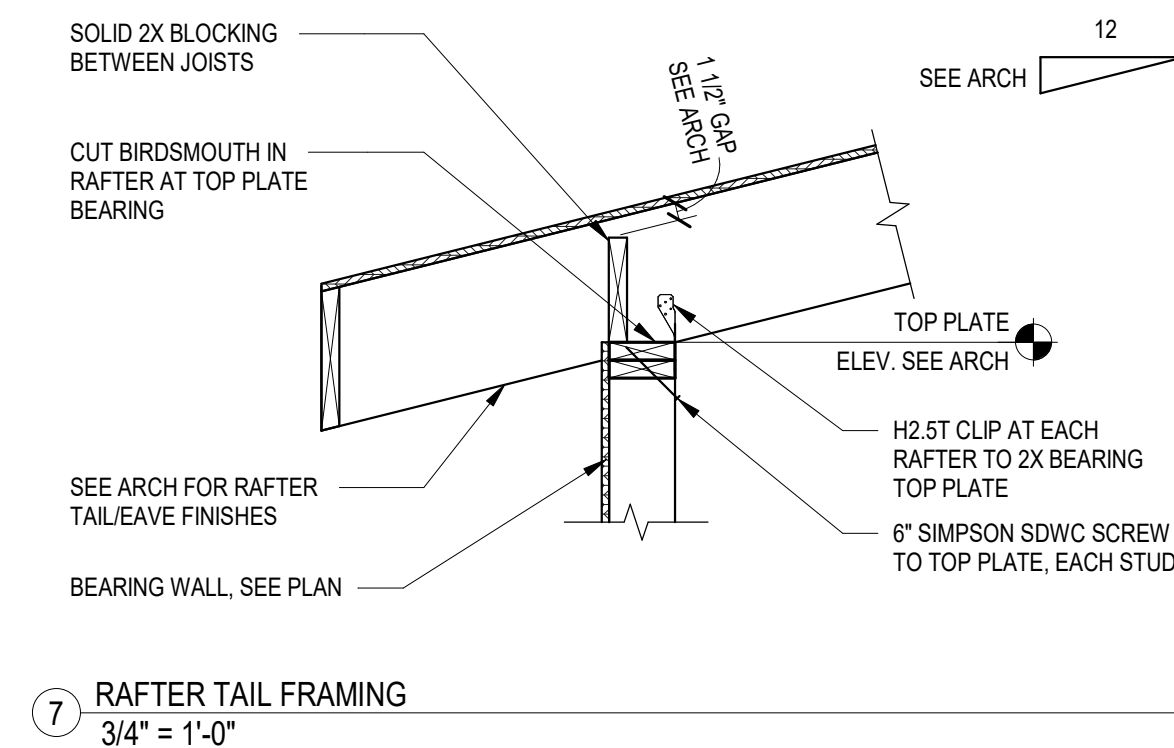
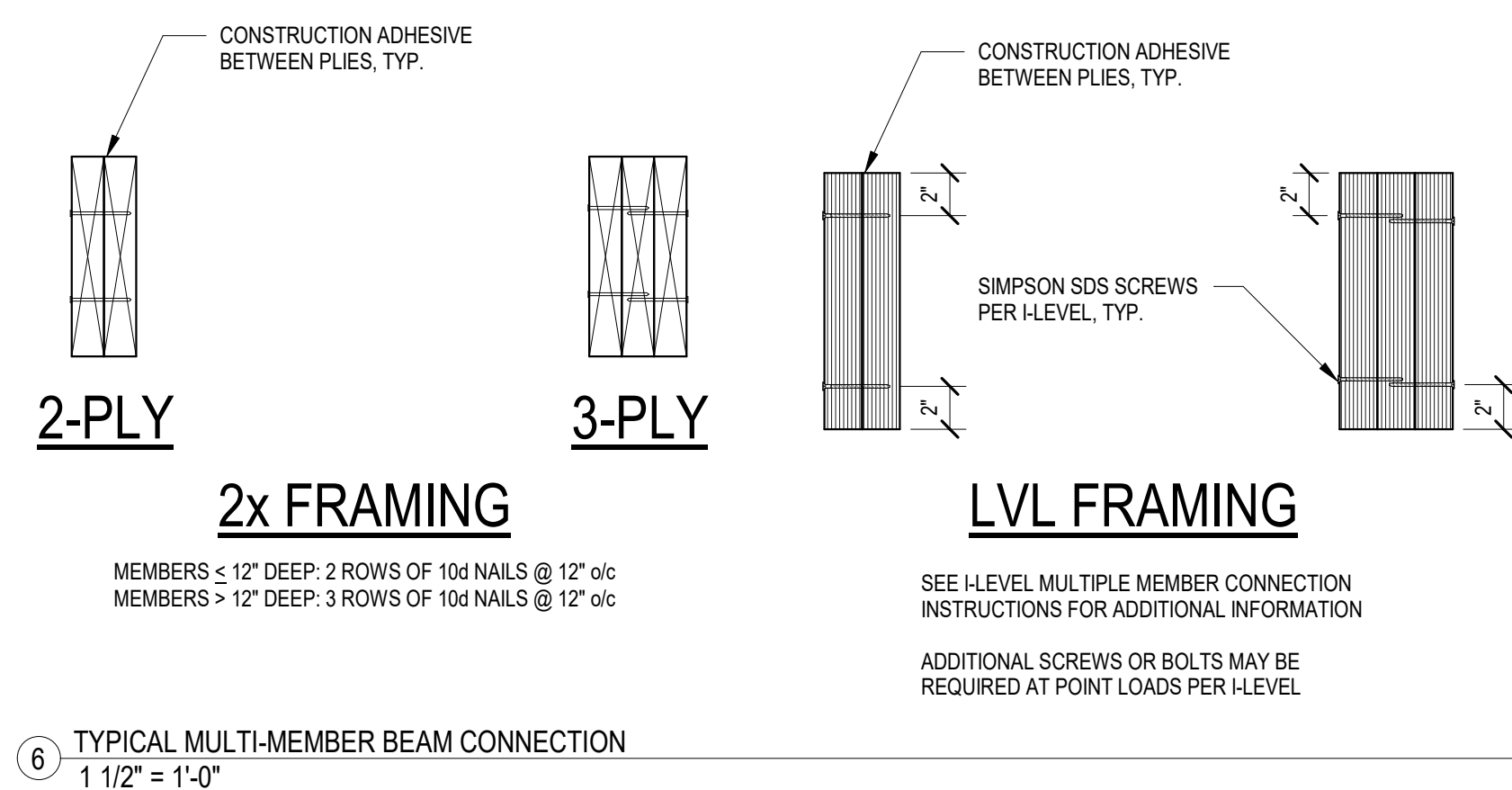
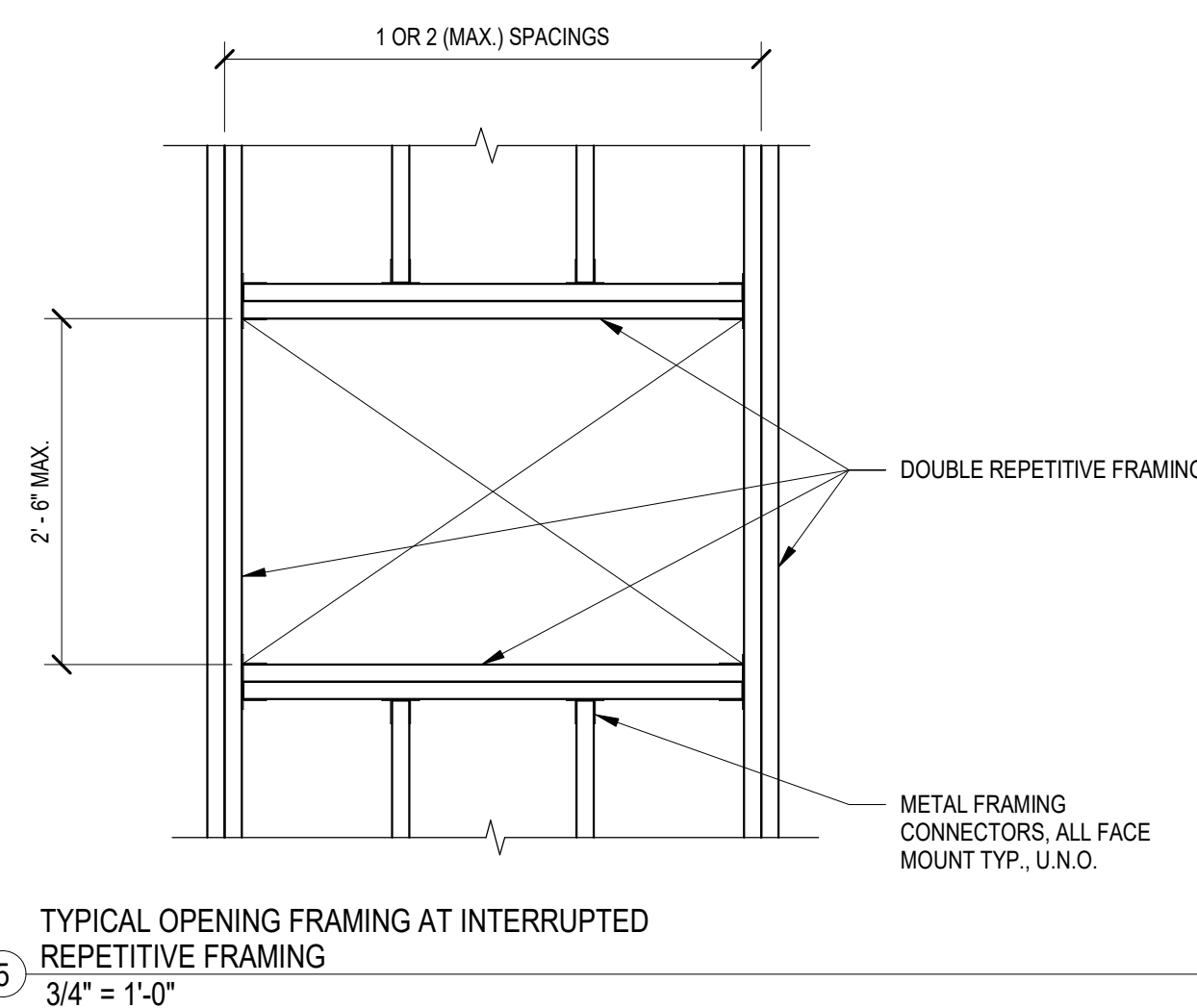
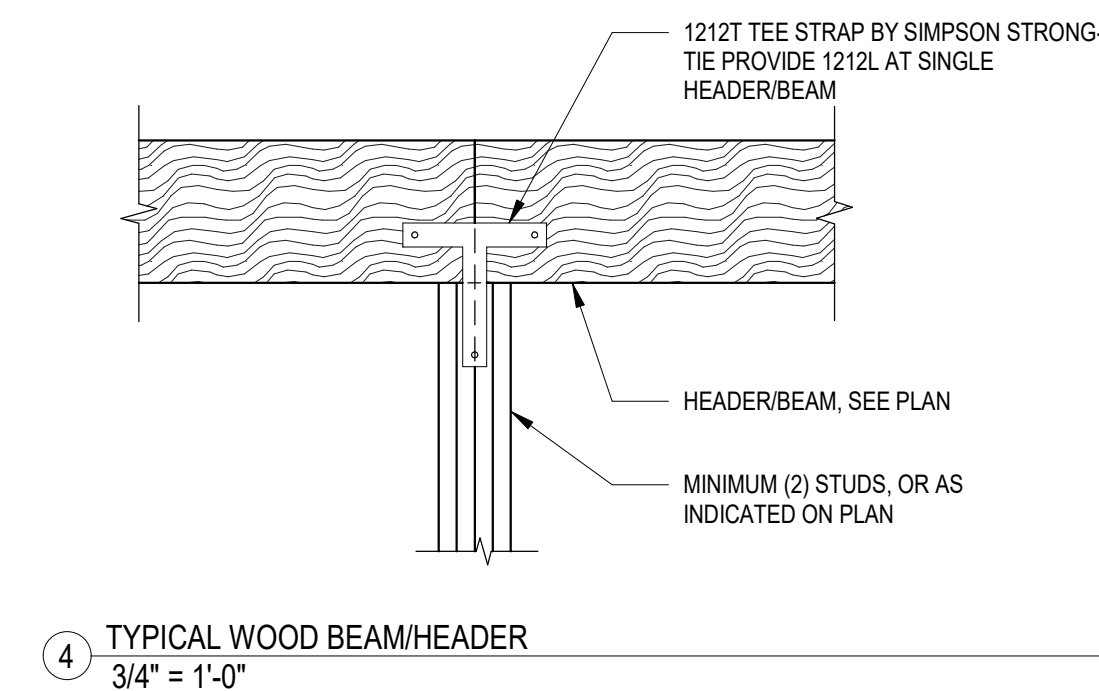
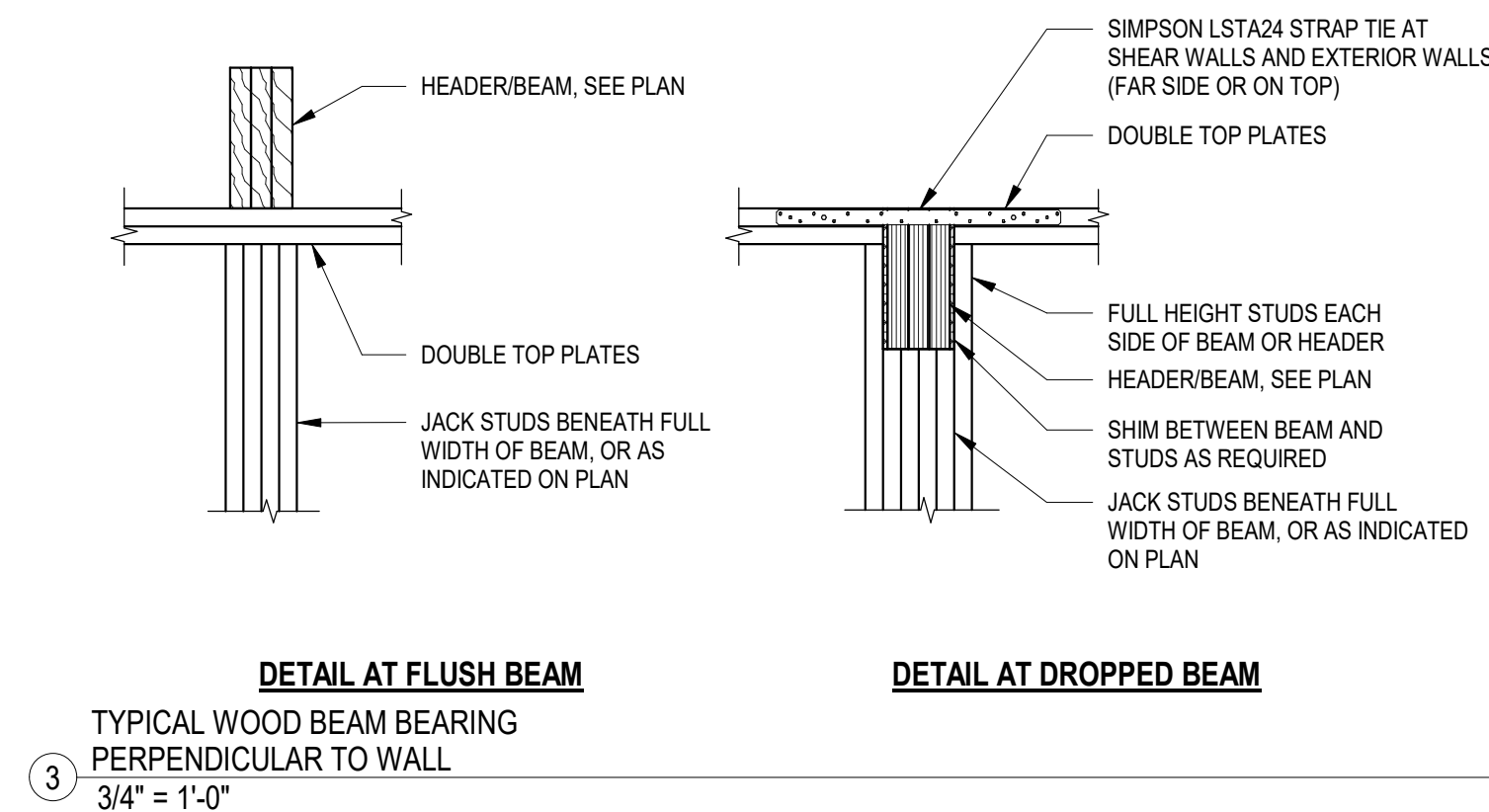
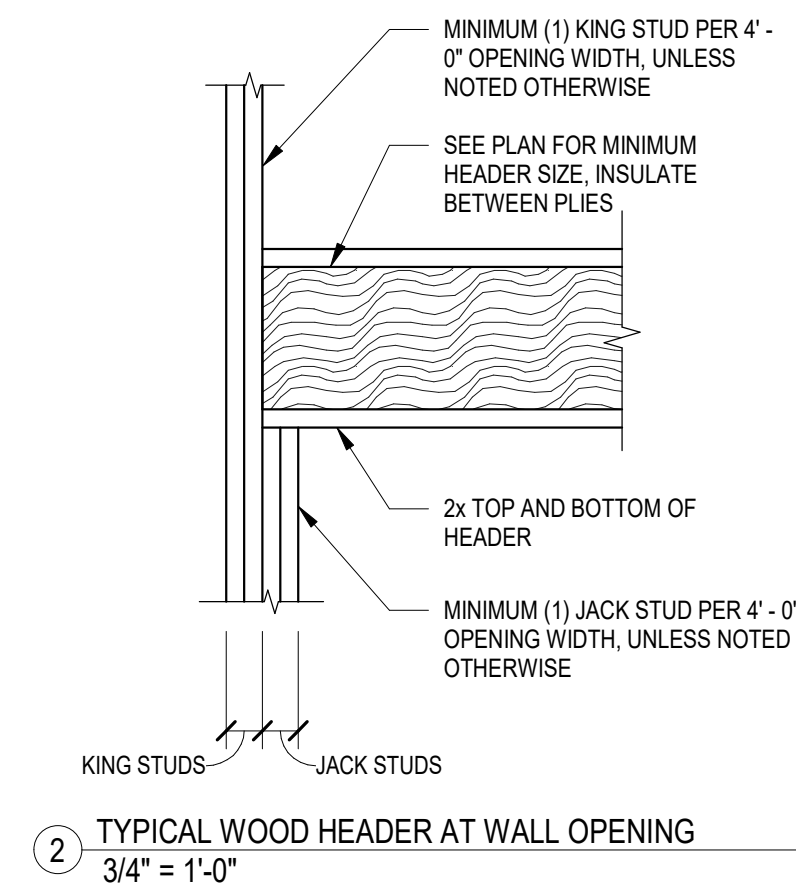
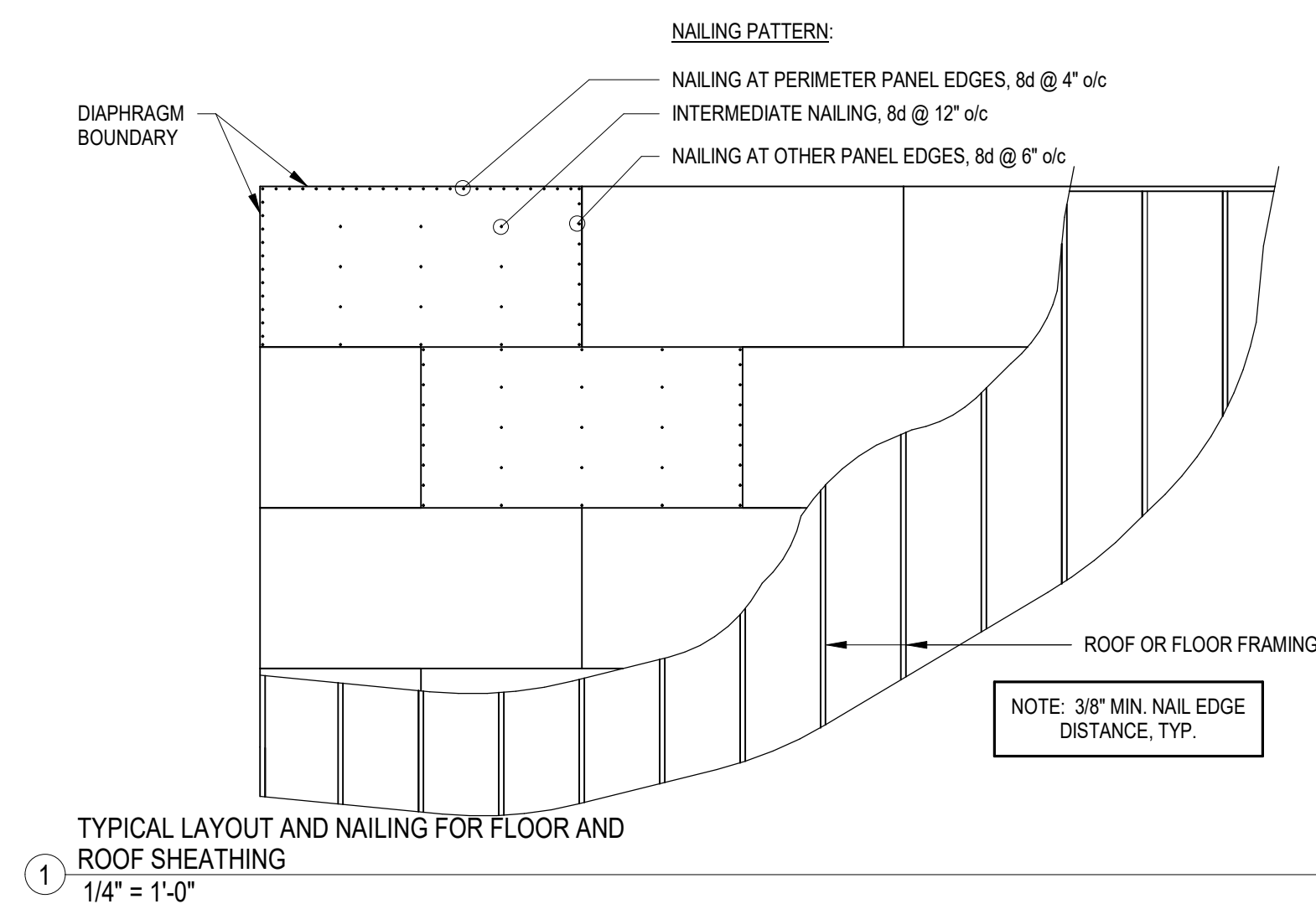
Client: **BARRETT ARCHITECTURE**
281 THE BEND RD.
GREENSBORO BEND, VT 05842

Sheet Title: FOUNDATION DETAILS
Project Title: LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS 120 MAIN ST. LITTLETON, NH 03561

Designed By:	MS
Checked By:	MS
Drawn By:	MS
Scale:	AS NOTED
Date:	2/14/24

S2.2

EV PROJECT #24169

[illegible]

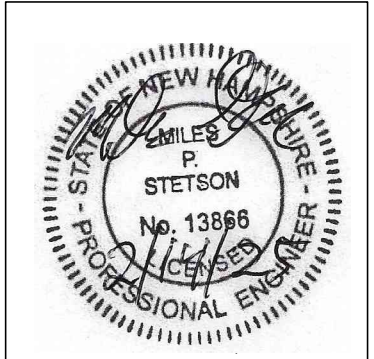
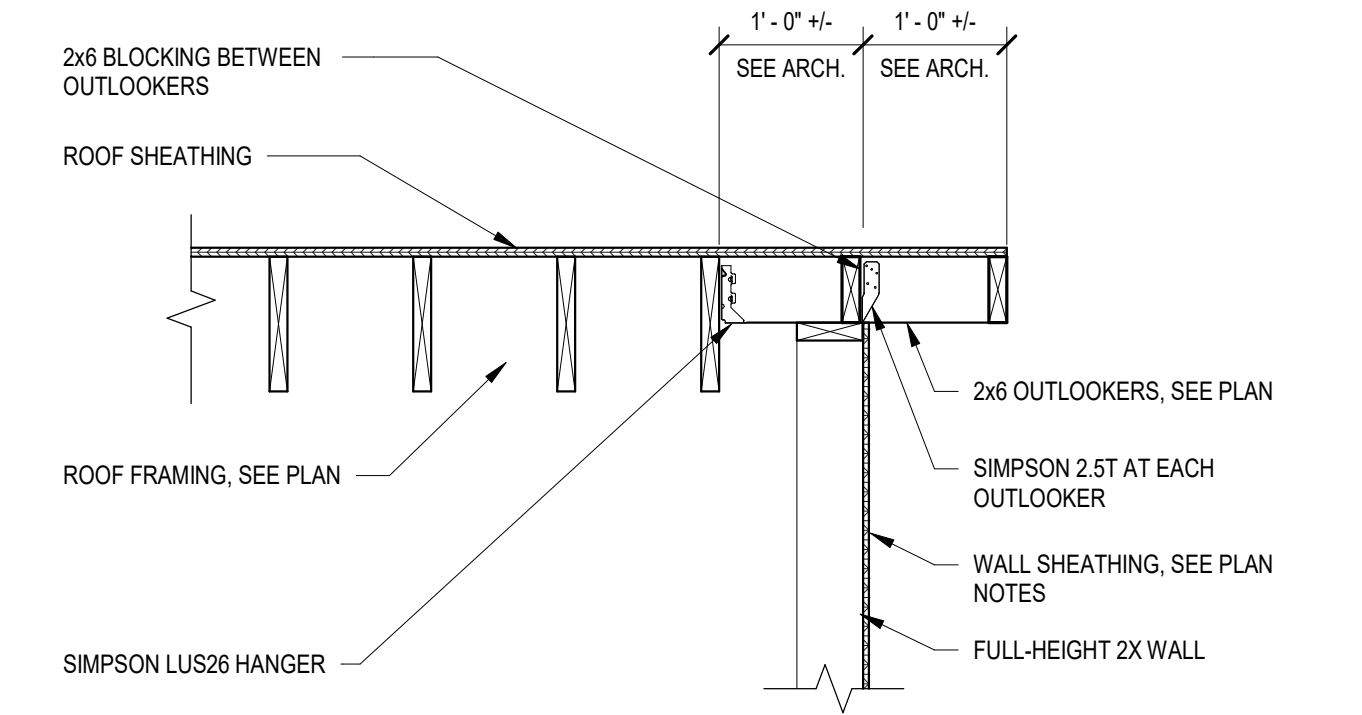
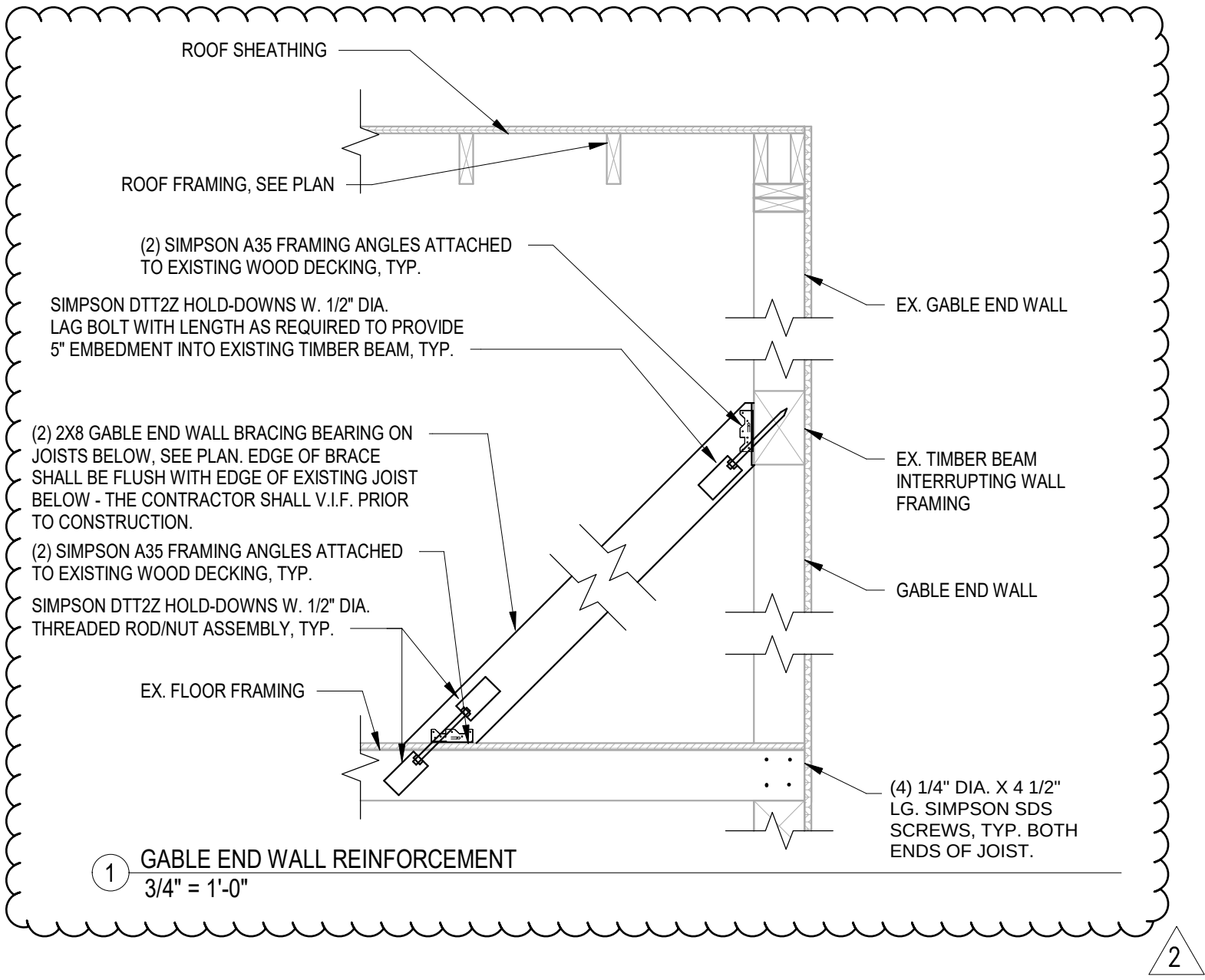
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Client: **BARRETT ARCHITECTURE**
281 THE BEND RD.
GREENSBORO BEND, VT 05842

Sheet Title:	TYPICAL FRAMING DETAILS
Project Title:	LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS
	120 MAIN ST. LITTLETON, NH 03561

Designed By:	MS
Checked By:	MS
Drawn By:	MS
Scale:	AS NOTED
Date:	2/14/24



Rev.	No.	Description	Chkd.

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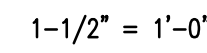
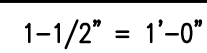
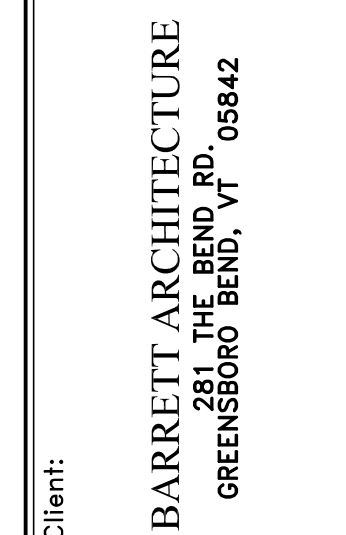
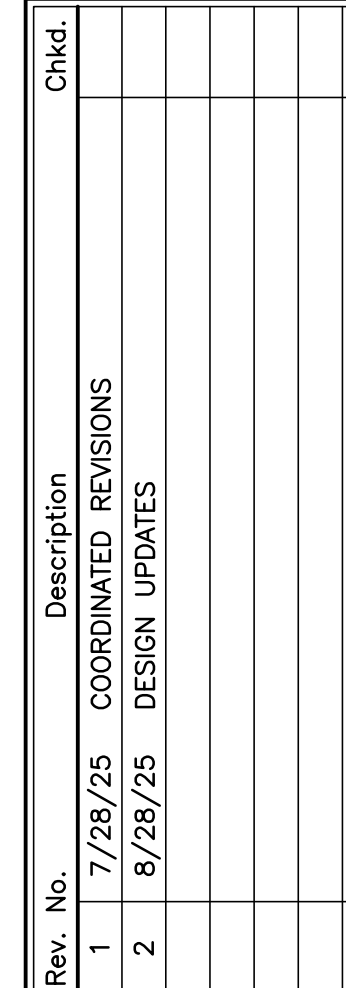
Sheet Title: **FRAMING DETAILS**

Project Title: **LITTLETON COMMUNITY CENTER ANNEX RENOVATIONS**
120 MAIN ST., LITTLETON, NH 03561

Designed By:	MS
Checked By:	MS
Drawn By:	MS
Scale:	AS NOTED
Date:	2/14/24

S3.1

EV PROJECT #24169


$$1-1/2'' = 1'-0''$$

$$1-1/2'' = 1'-0''$$


Project Title: **LITTLETON COMMUNITY
CENTER ANNEX RENOVATIONS**

S3.2

EV PROJECT #24169