

Village of Bayside Plan Commission Meeting October 17, 2019 Village Board Room, 5:45 pm

#### PLAN COMMISSION AGENDA

**PLEASE TAKE NOTICE** that a meeting of the Village of Bayside Plan Commission will be held at the Bayside Village Hall, 9075 North Regent Road, Bayside, Wisconsin at the above noted time and date, at which the following items of business will be discussed and possibly acted upon:

- I. CALL TO ORDER AND ROLL CALL
- II. APPROVAL OF MINUTES
  - A. Plan Commission meeting minutes, August 6, 2019.
- III. BUSINESS
  - **A.** Discussion/Recommendation on Amended Conditional Use Permit for 877 W Glencoe Place to replace and upgrade equipment on existing cell tower.
- IV. ANY OTHER BUSINESS AS MAY PROPERLY COME BEFORE THE COMMISSION
- V. ADJOURNMENT

Lynn Galyardt Administrative Services Director October 10, 2019

Upon reasonable notice, efforts will be made to accommodate the needs of disabled individuals through appropriate aids and services. Contact Village Hall at 414-206-3915. It is possible that members of and possibly a quorum of members of other Boards, Commissions, or Committees of the Village including in particular the Board of Trustees may be in attendance in the above-stated meeting to gather information; no action will be taken by any other Boards, Commissions, or Committees of the Village except by the Board, Commission, or Committee noticed above. Agendas and minutes are available on the Village website (www.bayside-wi.gov)



### Village of Bayside Plan Commission Meeting Minutes August 6, 2019

#### I. CALL TO ORDER

President Dickman called the meeting to order at 6:00pm

#### II. ROLL CALL

Chairman:

Sam Dickman

Jeff Jubelirer

Commissioners:

Edward Harris

Ari Friedman-excused

John Krampf

Marisa Roberts

Robb DeGraff

Also present:

Village Manager Andy Pederson

Administrative Services Director Lynn Galyardt Assistant Village Manager La'Neka Horton

Assistant to Administrative Service Director Richard Kerns

Village Attorney Chris Jaekels

There were 120 people in the audience

#### III. INFORMAL PUBLIC HEARING

A. Request for a Planned Unit Development District bounded by N. Port Washington Road, W. Brown Deer Road, W. White Oak Lane, and U.S. Highway Interstate 43 North.

Attorney Jaekels outlined the details of the process regarding the request for a Planned Unit Development District bounded by N. Port Washington Road, W. Brown Deer Road, W. White Oak Lane, and U.S. Highway Interstate 43 North.

Scott Yauck, President and Chief Executive Officer of Cobalt Partners provided an overview of the requested development parameters. Mr. Yauck stated the request is to create two zones for development generally bounded by N. Port Washington Road, W. Brown Deer Road, W. White Oak Lane and U.S. Highway Interstate 43 North.

The following people spoke at the meeting:
Robert F. Kohn, 8904 N Port Washington Rd
Barbara J. Becker, 9733 N Lake Dr
Ellen Daroga, 8950 N Bayside Dr
Ralph H. McClure, 8926 N King Dr
Kait Krueger, 664 W Aspenwood Ct
Kari Schmidt, 510 W Manor Cir
R. J. Siegel, 9260 N Pelham Pkwy
Mark Schrager, 9059 N Meadowlark Ln
Ellen La Fouge, 9154 N Fielding Rd
Sheila Schmitz-Lammers, 849 E Fairy Chasm Rd

provide a traffic study, review height of buildings, green space percentage, storm water discharge, and lighting spilling onto neighboring facilities within the next month.

## VI. ANY OTHER BUSINESS AS MAY PROPERLY COME BEFORE THE COMMISSION

There was none.

#### VII. ADJOURNMENT

Motion by Chairman Dickman, seconded by Commissioner Harris, to adjourn the meeting at 8:00pm. Motion carried unanimously.

Lynn Galyardt Administrative Services Director

Jennifer Nissen, 235 W Manor Cir Gerald Feldman, 133 E Glencoe Pl Gina Kilian, 1110 E Standish Pl F. Tessa Bartels, 208 E Ravine Baye Rd Hal Buell, 9481 N Sequoia Dr Mary K Mc Cann, 9490 N Sleepy Hollow Ln Pam Ringsred, 565 E Glencoe Pl David Frick, 9317 N Lake Dr Steve Schultz, 9359 N Lake Dr Elizabeth Levins, 825 E Donges Rd Linda Hauke, 9555 N Sequoia Dr Mark McCormick, 809 E Ellsworth Ln Herb Zien, 825 E Donges Rd Gail Becker, 9280 N Lake Dr Chris Marks, 306 W Ellsworth Ln Karen Siegel, 9260 N Pelham Pkwy Marcie Mangas Prill, 9109 N Bayside Dr

Chairman Dickman closed the public hearing at 7:31pm.

#### IV. APPROVAL OF MINUTES

A. Plan Commission meeting minutes, June 18, 2019.

Motion by commissioner Harris, seconded by Commissioner Jubelirer, to approve the June 18, 2019 Plan Commission Public Hearing and Meeting minutes. Motion carried unanimously.

#### V. BUSINESS

A. Discussion/Recommendation on Amended Conditional Use Permit for 8989 N Port Washington Road to replace and upgrade equipment on existing cell tower.

Manager Pederson stated the request of the amended Conditional Use Permit is for ATT to upgrade its equipment on the existing cell tower.

Motion by Commissioner DeGraff, seconded by Commissioner Krampf, to recommend approval to the Board of Trustees on the request for an amended Conditional Use Permit for 8989 N Port Washington Road to replace and upgrade equipment on existing cell tower. Motion carried unanimously.

B. Discussion on Proposed Planned Unit Development District Presentation by Cobalt Partners LLC and La Macchia Holdings LLC for proposed planned unit development generally bounded by N. Port Washington Road, W. Brown Deer Road, W. White Oak Lane, and U.S. Highway Interstate 43 North.

The committee discussed the Proposed Planned Unit Development presented by Cobalt Partners LLC and questioned Mr. Yauck on the height of the buildings, aesthetic appearance, impervious surface, municipal water source, and how the area would be luminated. Chairman Dickman requested that staff provide additional information to the committee on parking,



# CONDITIONAL USE PERMIT APPLICATION

PLEASE PRINT OR TYPE

Applicant Name(s) Sprint C/O NTP Wireless attn: Kyra Ambrose	
Name of business or developmentSprint	
Address of proposed business Cellular Antenna site	, Bayside, WI 53217
Applicant address125 S Clark St. Chicago, IL 60616  Applicant phone number(s)773-941-6973	
Property owner name Same as Applicant	, ,
	_ Phone number
Parcel number	

Conditional Use Permit Plan of Operation

Please Answer all questions and attach additional sheets as necessary. If you do not answer a question, provide a justification for why it does not apply to you.

New Condition	nal Use Permit □ Amended Conditional Use Permit 🗹
Address of Business:	877 W. Glencoe Place. Bayside, WI 53217
	c uses of entire property or lease space and summary of type of
ousiness planned:	Sprint is proposing to do an antenna & equipment upgrade.
A brief description of on	-site operations:
a brief description of on	5120 Spo. 40.0115.
Legal description of pro	perty:
Fay Key TD Number/Pai	cel Number:
_	
Lot size or lease space s	ize (in square feet): 12'x11' 132 sq ft.
Building dimensions and	number of floors:N/A
Total floor area (in squa	re feet): N/A
Number of sniπs and ma	aximum number of employees per shift: N/A
,	
Days and hours of opera	ition:N/A
Frequency of deliveries	to site and type(s) of vehicles that will deliver:
Technicians visit the si	e approximately once per month in a van-sized vehicle.
	tion: N/A
Projected traffic circula	
	size, location, existing or new etc.) *All signs must be approved
the ARC: No new s	gnage is proposed. All existing signs are in accordance with FCC re-

	nonitored. The site has
alarm system which is connected to a regional switch office.	
	•
Describe the noise, odors, glare, dust, potential fire hazards, or smoke	e resulting from the
Proposed use: None. Site has existing wireless telecommunications fa	cility.
Status of interior plans requiring State approval:N/A	
Status of State License(s) and/or Certificate(s) required for operation	ı:N/A
List the timetable for completion of all building construction or interio	or ·
	ipon approval and order
of equipment.	
·	
Anticipated maximum number of facility users and visitors at one tim	e (including special
events): N/A	
events): N/A	
events)	
· ·	
Total number of estimated parking spots needed for operation: Nor	
Total number of estimated parking spots needed for operation: Nor	ne ·
Total number of estimated parking spots needed for operation: Nor	ne ·
Total number of estimated parking spots needed for operation: Nor	ne ·
Total number of estimated parking spots needed for operation: Nor  Nor  Nor  Dumpster enclosure and trash removal: N/A	ne
Total number of estimated parking spots needed for operation: Nor  Nor  Nor  Nor  N/A  Does the applicant have the legal authority to act for and obligate the company or corp	ooration? Yes_XNo
Total number of estimated parking spots needed for operation: Nor  Nor  Nor  Nor  Nor  Nor  Nor  N/A  Does the applicant have the legal authority to act for and obligate the company or corp  Does the applicant have the legal authority to act for and obligate the property owner?	ooration? Yes_XNo
Total number of estimated parking spots needed for operation: Nor	ooration? Yes_X_No
Total number of estimated parking spots needed for operation: Nor Dumpster enclosure and trash removal: N/A  Does the applicant have the legal authority to act for and obligate the company or corp Does the applicant have the legal authority to act for and obligate the property owner? Is the property owner(s) knowledgeable of the request for a Conditional Use?	oration? Yes_X_NoNoYes_X_No

\*Attach a legal description of the property requested for a conditional use, a plat of survey of the property, and a drawing of any proposed development. OFFICE USE ONLY: pd 9/23/9 \$300.00 application fee: Application received by: \$100.00 occupancy permit fee: Public Hearing date: · Approved by Board of Trustees: Board of Trustees Meeting: NSFD Permit Issued?: Occupancy Permit Issued?:



1033 WATERVLIET SHAKER RD, ALBANY, NY 12205

# **Structural Analysis Report**

#### **December 4, 2018**

Site ID	ML03XC113
Infinigy Job Number	3086-В0002-В
Client	NTP Wireless
Proposed Carrier	Sprint
Site Location	877 W. Glencoe Place, Bayside, WI 53217 43° 10' 43.9" N NAD83 87° 55' 3.5" W NAD83
Structure Type	116' Monopole
Structural Usage Ratio	94.7%
Overall Result	Pass

Upon reviewing the results of this analysis, it is our opinion that the structure meets the specified TIA code requirements. The tower and foundations are therefore deemed adequate to support the existing and proposed loading as listed in this report.



Ian Geery Structural Engineer II

## Structural Analysis Report

## December 4, 2018

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Calculations	Appended

## **Introduction**

Infinigy Engineering has been requested to perform a structural analysis on the existing 116' Monopole. All supporting documents have been obtained from the client and are assumed to be accurate and applicable to this site. The tower was analyzed using tnxTower version 8.0.2.1 tower analysis software.

#### **Supporting Documentation**

<b>Construction Drawings</b>	Infinigy CDs, dated December 5, 2017
Previous Analysis	W-T Job #T140908, dated March 27, 2015
Previous Analysis	Infinigy SA, dated May 18, 2018
<b>Tower Drawings</b>	EEI Job # WI 1854, dated July 26, 1996
Tower Loading Form	Semaan Engineering solutions, Inc., dated October 9, 2018

#### П

### **Analysis Code Requirements**

Wind Speed	90 mph (3-Second Gust, V <sub>ASD</sub> ) / 115 mph (3-Second Gust, V <sub>ULT</sub> )
Wind Speed w/ ice	30 mph (3-Second Gust, V <sub>ASD</sub> ) w/ 3/4" ice
TIA Revision	ANSI/TIA-222-G
Adopted IBC	2012 IBC
Structure Class	II
Exposure Category	C
Topographic Category	1
Calculated Crest Height	0 ft.

### **Conclusion**

Upon reviewing the results of this analysis, it is our opinion that the structure meets the specified TIA code requirements. The tower and foundations are therefore deemed adequate to support the existing and proposed loading as listed in this report.

If you have any questions, require additional information, or actual conditions differ from those as detailed in this report please contact me via the information below:

Ian Geery
Structural Engineer II | **INFINIGY**4636 E University Dr, Phoenix, AZ 85034
(M) (623) 670-2138
igeery@infinigy.com | www.infinigy.com

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## **Existing and Reserved Loading**

Mount Height (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
	12	Amphenol BXA-70080 / 8' Panels		(12) 1 5/8"	
	12	RRUS A2 Modules			
	6	RRH 3JR52709AA 2X60			Verizon
115.0	3	RRH 4X30-4T4R-B13	Platform w/	(3) Hybrid	
113.0	3	RRH 4X30-4T4R-BAND 25 (PCS	Handrails	Cables	
	3	60W RRH)		1.56"	
	3	OVP Junction Boxes			
	12	10"x7"x2" TMAs			
	1	KMW ET-X-TS-70-15-62-18-iR-RD			
	2	KMW ET-X-WM-18-65-8P	DB   Platform w/ (3) 1		Sprint
	2	Commscope SBCHH-1D90B			
102.0	0 1	Alpha AW3266		(3) 1 ¼" Hybrid	
103.0	3	Samsung RRH-C2A			
		Samsung RRH-P4			
		Samsung RRH-RRU 2.5LTEV3 10	] -		-
		KM			
90.0	9	Celwave APL199014-42T2	Platform ·	(9) 1 5/8"	TeleCorp

## To be Removed Loading

Mount Height (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
	1	KMW ET-X-TS-70-15-62-18-iR-RD			
	2	KMW ET-X-WM-18-65-8P	·		
103.0	2	Commscope SBCHH-1D90B			Sprint
	3	Samsung RRH-RRU 2.5LTEV3 10 KM			,
	1	Alpha AW3266			

# **Proposed Loading**

Mount Height (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
102.0	3	KMW ETCR-654L12H6			Sprint
103.0	3	Samsung RRH-B8		644 and	Spriit

### **Final Configuration**

Mount Height (ft)	Qty.	Appurtenance	Mount Type	Coax & Lines	Carrier
	12	Amphenol BXA-70080 / 8' Panels		(12) 1 5/8"	
	12	RRUS A2 Modules			
	6	RRH 3JR52709AA 2X60			
115.0	3	RRH 4X30-4T4R-B13	Platform w/	(3) Hybrid	Verizon
3		RRH 4X30-4T4R-BAND 25 (PCS	Handrails	Cables	VCHZOH
	3	60W RRH)		1.56"	
	3	OVP Junction Boxes			
	12	10"x7"x2" TMAs			
	3	KMW ETCR-654L12H6			
103.0	3	Samsung RRH-C2A	Platform w/	(3) 1 1/4"	Sprint
103.0	3	Samsung RRH-P4	Handrails	Hybrid	Spriit
	3	Samsung RRH-B8			
90.0	9	Celwave APL199014-42T2	Platform	(9) 1 5/8"	TeleCorp

## **Structure Usages**

	Summary	
Pole (L1)	84.2	Pass
Base Plate	86.1	Pass
RATING =	86.1	Pass

## **Foundation Reactions**

Reaction Data	Design Reactions x 1.35	Analysis Reactions	Result
Moment (kip-ft)	2104.5	1992.9	94.7%
Shear (kip)	24.5	22.7	92.7%

<sup>\*</sup> Design reactions are multiplied by 1.35 per ANSI/TIA-222-G 15.5.1

Tower base reactions are acceptable when compared to the original design reactions.

## **Deflection, Twist, and Sway**

Antenna Elevation (ft)	Deflection (in)	Twist (°)	Sway (°)
103.0	18.151	0.014	1.757

<sup>\*</sup>Per ANSI/TIA-222-G Section 2.8.2 maximum serviceability structural deflection limit is 3% of structure height.

<sup>\*</sup>Per ANSI/TIA-222-G Section 2.8.2 maximum serviceability structural twist and sway limit is 4 degrees.

<sup>\*</sup>Per ANSI/TIA-222-G Section 2.8.3 deflection, Twist, and sway values were calculated using a basic 3-second gust wind speed of 60 mph.

<sup>\*</sup>It is the responsibility of the client to ensure their proposed and/or existing equipment will meet ANSI/TIA-222-G Annex D or other appropriate microwave signal degradation limits based on the provided values above.

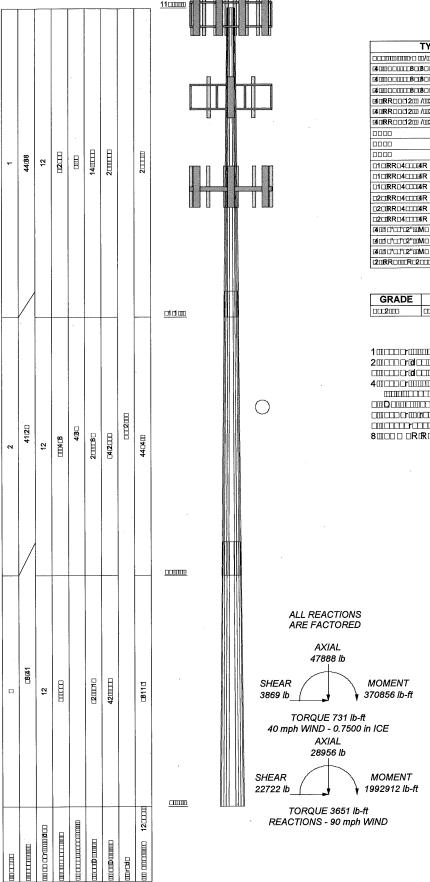
### **Assumptions and Limitations**

Our structural calculations are completed assuming all information provided to Infinigy Engineering is accurate and applicable to this site. For the purposes of calculations, we assume an overall structure condition of "like new" and all members and connections to be free of corrosion and/or structural defects. The structure owner and/or contractor shall verify the structure's condition prior to installation of any proposed equipment. If actual conditions differ from those described in this report Infinigy Engineering should be notified immediately to complete a revised evaluation.

Our evaluation is completed using standard TIA, AISC, ACI, and ASCE methods and procedures. Our structural results are proprietary and should not be used by others as their own. Infinigy Engineering is not responsible for decisions made by others that are or are not based on our supplied assumptions and conclusions.

This report is an evaluation of the tower structure only and does not reflect adequacy of any existing antenna mounts, mount connections, or cable mounting attachments. These elements are assumed to be adequate for the purposes of this analysis and are assumed to have been installed per their manufacturer requirements.

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#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
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01@RR04@04R	tri 🗆	BROID4	tico
0101RR040004R	dri 🗆	BROD8	tico
020fR040004R	dof 🗆	BR□⊞8	tico
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#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
mm <b>o</b> non	CTTTTTT	BUTTO			

#### **TOWER DESIGN NOTES**

Infinigy Engineering	<sup>™</sup> ML03XC113	
a 0000 00raww0000riRd0		
mmoonmad22000		
mc18am1	<b>™™™™™222</b> ™□ <b>™™</b> 12/□4/18 <b>™</b> □□□□□□	
		][]

Infinigy Engineering 1033 Watervliet Shaker Rd. Albany, NY 12205 Phone: (518) 690-0790

FÁX:

Job□		Page □
,	M 000001100	1 00000
Project		Date□
	□□8 <b>□</b> 4□1□	- C8[2:0008[12/04/18]
Client□		Designed by □

	Tower Input Data □□
The tower is	a monopole.
	s designed using the TIA-222-G standard.
	ng design criteria apply:
	Tower is located in Milwaukee County, Wisconsin.
	Basic wind speed of 90 mph.
	Structure Class II.
	Exposure Category C.
	Topographic Category 1.
	Crest Height 0.00 ft.
	Nominal ice thickness of 0.7500 in.
	Ice thickness is considered to increase with height.
	Ice density of 56 pcf.
	A wind speed of 40 mph is used in combination with ice.
	Temperature drop of 50 °F.
	Deflections calculated using a wind speed of 60 mph.
	A non-linear (P-delta) analysis was used.
	Pressures are calculated at each section.
	Stress ratio used in pole design is 1.
	Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- Use Code Stress Ratios
- Use Code Safety Factors Guys Escalate Ice Always Use Max Kz Use Special Wind Profile
- Include Bolts In Member Capacity
- Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided)
  - SR Members Have Cut Ends SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- Assume Rigid Index Plate
- Use Clear Spans For Wind Area
- Use Clear Spans For KL/r
- Retension Guys To Initial Tension Bypass Mast Stability Checks
- Use Azimuth Dish Coefficients
- Project Wind Area of Appurt.
- Autocalc Torque Arm Areas Add IBC .6D+W Combination
- Sort Capacity Reports By Component
- Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder

- Use ASCE 10 X-Brace Ly Rules
- Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable
- Offset Girt At Foundation Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are

## **Tapered Pole Section Geometry** □□

Known

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	
. 7 1	116.00-71.12	44.88	3.75	10	14.0000	25.3900	0.2500	1.0000	A 572-65

#### Page 🗆 Job□ tnxTower 2 ...... M 0000001100 Date□ Project Infinigy Engineering 1033 Watervliet Shaker Rd. □8[2□[[[8][12/□4/18[ Albany, NY 12205 Phone: (518) 690-0790 FAX: Client□ Designed by □

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L2	71.12-33.58	41.29	4.83	12	23.9383	34.2900	0.3438	1.3750	(65 ksi) A572-65 (65 ksi)
L3	33.58-0.00	38.41		12	32.3916	42.0000	0.3750	1.5000	A572-65 (65 ksi)

				Ta	pered P	ole Pr	opertie	<b>S</b> 🗆 🗆			
Section	Tip Dia.	Area in²	I in <sup>4</sup>	r in	C in	I/C in³	J in <sup>4</sup>	It/Q in²	w in	w/t	
L1	in 14.4057	11.0688	267.109		7.2520	36.8326	541,2370	5,4477	3.0820	12.328	
LI	26.1975	20.2377	1632.59		13.1520	124.1324	3308.0739	9.9604	6.1345		
L2	25.6348	26.1162	1855.749	95 8.4468	12.4000	149.6568	3760.2516	12.8536	5.4942	15.983	
	35.3784	37.5743	5526.624	43 12.1528	17.7622	311.1449	11198.4392	18.4929	8.2685	24.054	
L3	34.6528	38.6600	5058.224	41 11.4619	16.7788	301.4645	10249.3334	19.0273	7.6759		
	43.3493	50.2622	11115.67	47 14.9018	21.7560	510.9246	22523.3706	24.7375	10.2510	27.336	
										'	
Tower	Gus	set	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mi				Double Angle
Elevatio	n Are	ea Ti	hickness		$A_f$	Factor		Stitch		Stitch Bolt	Stitch Bolt
	(per f	Gace)				$A_r$		Spac	_	Spacing	Spacing
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	Ji		ITL		1	1	1			111	177

	sussei Area	Gusset Thickness	Gussei Graae	Aajusi, Facior $A_{\ell}$	Aajusi. Factor	weigni muii.	Stitch Bolt	Stitch Bolt	Stitch Bolt
	er face)	11110141035		12)	$A_r$		Spacing	Spacing	Spacing
u	,						Diagonals	Horizontals	Redundants
ft	ft²	in					in	in	in
L1				1	1	1			
116.00-71.12									
L2 71.12-33.58				1	1	1			
L3 33.58-0.00				1	1	1			

# Feed Line/Linear Appurtenances - Entered As Area □

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
•	Leg		JF	ft			ft²/ft	plf
1 5/8" Coax	C	No	Inside Pole	115.00 - 0.00	12	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
1.56" Hybrid	C	No	Inside Pole	115.00 - 0.00	3	No Ice	0.00	1.66
•						1/2" Ice	0.00	1.66
						1" Ice	0.00	1.66
***							*	
1 1/4" Hybriflex Cable	C	No	Inside Pole	103.00 - 0.00	3	No Ice	0.00	1.00
						1/2" Ice	0.00	1.00
						1" Ice	0.00	1.00
***								
1 5/8" Coax	C	No	Inside Pole	90.00 - 0.00	9	No Ice	0.00	0.82
	_					1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82

			-			
Feed Line/L	inear Ap	purtenances	Section	Areas		

#### Page 🗆 Job□ tnxTower M000001100 Date□ Project Infinigy Engineering 1033 Watervliet Shaker Rd. **□□**8 **□**4 **□**1 **□** \_8<u>\_2</u>\_\_\_8<u>\_1</u>2/<u>\_4/18</u>\_ Albany, NY 12205 Phone: (518) 690-0790 FAX: Client□ Designed by □

Tower Section	Tower Elevation	Face	$A_R$	$A_F$	$C_A A_A$ In Face	$C_A A_A$ Out Face	Weight
	ft		ft²	$ft^2$	ft²	ft <sup>2</sup>	lb
L1	116.00-71.12	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	885.28
L2	71.12-33.58	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	946.01
L3	33.58-0.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	0.000	846.22

Feed Line/Linear Appurtenances S	Section Areas - With Ice□
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Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_AA_A$	$C_AA_A$	Weight
Section	Elevation	or	Thickness	-2	-2	In Face	Out Face	**
	ft	Leg	in	ft²	ft²	ft²	Jt²	<u>lb</u>
L1	116.00-71.12	Α	1.661	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	885.28
L2	71.12-33.58	Α	1.569	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	946.01
L3	33.58-0.00	Α	1.400	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	846.22

# Shielding Factor Ka□

_						
	Tower	Feed Line	Description	Feed Line	$K_{a}$	$K_a$
	Section	Record No.	_	Segment Elev.	No Ice	Ice

## Discrete Tower Loads □

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	۰	ft		ft²	ft²	lb
Angle Platform w/ Handrails	С	From Face	0.00 0.00 0.00	. 0.0000	115.00	No Ice 1/2" Ice 1" Ice	42.40 48.40 54.40	42.40 48.40 54.40	2000.00 2450.00 2900.00
(4) BXA-70080-8CF	A	From Face	3.00 0.00 0.00	0.0000	115.00	No Ice 1/2" Ice 1" Ice	8.32 8.92 9.52	8.35 9.75 11.01	52.20 120.70 198.50
(4) BXA-70080-8CF	В	From Face	3.00 0.00 0.00	0.0000	115.00	No Ice 1/2" Ice 1" Ice	8.32 8.92 9.52	8.35 9.75 11.01	52.20 120.70 198.50
(4) BXA-70080-8CF	C	From Face	3.00 0.00 0.00	0.0000	115.00	No Ice 1/2" Ice 1" Ice	8.32 8.92 9.52	8.35 9.75 11.01	52.20 120.70 198.50

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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weight
	Leg	->F -	Lateral	,5					
			Vert	0			£2	ω2	lb
			ft ft	v	ft		$ft^2$	$ft^2$	10
			ft						
(4) RRUS-12 W/ A2	Α	From Face	3.00	0.0000	115.00	No Ice	2.49	1.33	79.10
			0.00			1/2" Ice	2.68	1.48	101.12
(A) DD110 10 111/ 10	ъ	в в	0.00	0.0000	115.00	1" Ice No Ice	2.89 2.49	1.64 1.33	126.21 79.10
(4) RRUS-12 W/ A2	В	From Face	3.00 0.00	0.0000	115.00	1/2" Ice	2.49	1.33	101.12
			0.00			1" Ice	2.89	1.64	126.21
(4) RRUS-12 W/ A2	С	From Face	3.00	0.0000	115.00	No Ice	2.49	1.33	79.10
(4) 14(05 12 11/112	Ü	110111111100	0.00	0,000		1/2" Ice	2.68	1.48	101.12
			0.00			1" Ice	2.89	1.64	126.21
COVP	Α	From Face	3.00	0.0000	115.00	No Ice	3.20	1.01	19.00
			0.00			1/2" Ice	3.42	1.15	40.19
			0.00			1" Ice	3.65	1.30	64.49
COVP	В	From Face	3.00	0.0000	115.00	No Ice	3.20	1.01	19.00
			0.00			1/2" Ice	3.42	1.15	40.19 64.49
COMP	0	E E	0.00 3.00	0.0000	115.00	1" Ice No Ice	3.65 3.20	1.30 1.01	19.00
COVP	С	From Face	0.00	0.0000	113.00	1/2" Ice	3.42	1.01	40.19
			0.00			1" Ice	3.65	1.30	64.49
B13 RRH4x30-4R	Α	From Face	3.00	0.0000	115.00	No Ice	2.16	1.62	51.00
<i>D13 1441 1130 12</i>	••		0.00			1/2" Ice	2.35	1.79	70.61
			0.00			1" Ice	2.55	1.97	93.18
B13 RRH4x30-4R	В	From Face	3.00	0.0000	115.00	No Ice	2.16	1.62	51.00
			0.00			1/2" Ice	2.35	1.79	70.61
			0.00			1" Ice	2.55	1.97	93.18
B13 RRH4x30-4R	C	From Face	3.00	0.0000	115.00	No Ice	2.16	1.62	51.00
			0.00		,	1/2" Ice 1" Ice	2.35 2.55	1.79 1.97	70.61 93.18
D25 DD11420 4D	٨	From Face	0.00 3.00	0.0000	115.00	No Ice	2.33	1.31	51.00
B25 RRH4x30-4R	A	From Face	0.00	0.0000	113.00	1/2" Ice	2.14	1.46	68.46
			0.00			1" Ice	2.53	1.63	88.75
B25 RRH4x30-4R	В	From Face	3.00	0.0000	115.00	No Ice	2.14	1.31	51.00
DED IGGI MED 1.10	_		0.00			1/2" Ice	2.33	1.46	68.46
			0.00			1" Ice	2.53	1.63	88.75
B25 RRH4x30-4R	C	From Face	3.00	0.0000	115.00	No Ice	2.14	1.31	51.00
			0.00			1/2" Ice	2.33	1.46	68.46
			0.00			1" Ice	2.53	1.63	88.75
(4) 10"x7"x2" TMA	Α	From Face	3.00	0.0000	115.00	No Ice	0.58	0.18	15.00
			0.00			1/2" Ice 1" Ice	0.68 0.79	0.25 0.33	19.02 24.46
(A) 1005-705-20 TMA	В	From Face	0.00 3.00	0.0000	115.00	No Ice	0.79	0.33	15.00
(4) 10"x7"x2" TMA	ь	rioni race	0.00	0.0000	115.00	1/2" Ice	0.68	0.16	19.02
			0.00			1" Ice	0.79	0.33	24.46
(4) 10"x7"x2" TMA	C	From Face	3.00	0.0000	115.00	No Ice	0.58	0.18	15.00
(1) 10 11/112 11/111	-		0.00			1/2" Ice	0.68	0.25	19.02
			0.00			1" Ice	0.79	0.33	24.46
(2) RRH 3JR52709AA 2X60	Α	From Face	3.00	0.0000	115.00	No Ice	3.50	2.10	60.00
			0.00			1/2" Ice	3.76	2.34	84.31
	_		0.00	0.0000	1100	1" Ice	4.03	2.58	112.31
(2) RRH 3JR52709AA 2X60	В	From Face	3.00	0.0000	115.00	No Ice 1/2" Ice	3.50	2.10	60.00 84.31
			0.00			1/2" Ice 1" Ice	3.76 4.03	2.34 2.58	84.31 112.31
(2) DDH 21D52700 A A 2V60	C	From Face	3.00	0.0000	115.00	No Ice	3.50	2.38	60.00
(2) RRH 3JR52709AA 2X60	С	110m Face	0.00	0.0000	113.00	1/2" Ice	3.76	2.34	84.31
			0.00			1" Ice	4.03	2.58	112.31
***			2.00						
Angle Platform w/ Handrails	C	From Face	0.00	0.0000	103.00	No Ice	42.40	42.40	2000.00
-			0.00			1/2" Ice	48.40	48.40	2450.00

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Description `	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft	0	ft		ft²	ft²	lb
,			ft 0.00	~		1" Ice	54.40	54.40	2900.00
ETCR-654L12H6	Α	From Face	3.00	0.0000	103.00	No Ice	15.71	6.00	84.90
			0.00			1/2" Ice	16.28	6.52	167.97
			0.00			1" Ice	16.86	7.05	258.50
ETCR-654L12H6	В	From Face	3.00	0.0000	103.00	No Ice	15.71	6.00	84.90
•			0.00			1/2" Ice	16.28	6.52	167.97
			0.00			1" Ice	16.86	7.05	258.50
ETCR-654L12H6	С	From Face	3.00	0.0000	103.00	No Ice	15.71	6.00	84.90
			0.00			1/2" Ice	16.28	6.52	167.97
			0.00			1" Ice	16.86	7.05	258.50
RRH-C2A (28.8")	Α	From Face	3.00	0.0000	103.00	No Ice	5.47	2.61	55.00
			0.00			1/2" Ice	5.76	2.82	96.39
			0.00			1" Ice	6.06	3.05	142.03
RRH-C2A (28.8")	В	From Face	3.00	0.0000	103.00	No Ice	5.47	2.61	55.00
			0.00			1/2" Ice	5.76	2.82	96.39
	~		0.00	0.0000	100.00	1" Ice	6.06	3.05	142.03
RRH-C2A (28.8")	C	From Face	3.00	0.0000	103.00	No Ice	5.47	2.61	55.00
			0.00			1/2" Ice	5.76	2.82	96.39
DDIT D4		. P	0.00	0.0000	102.00	1" Ice	6.06	3.05	142.03 60.00
RRH-P4	Α	From Face	3.00	0.0000	103.00	No Ice	2.25 2.44	1.05	
			0.00			1/2" Ice 1" Ice	2.63	1.19 1.34	77.56 97.92
DDII D4	В	Enous Eooo	0.00 3.00	0.0000	103.00	No Ice	2.03	1.05	60.00
RRH-P4	В	From Face	0.00	0.0000	103.00	1/2" Ice	2.44	1.19	77.56
			0.00			1" Ice	2.63	1.19	97.92
RRH-P4	С	From Face	3.00	0.0000	103.00	No Ice	2.25	1.05	60.00
KKII-F4	C	Prom Pace	0.00	0.0000	105.00	1/2" Ice	2.44	1.19	77.56
			0.00			1" Ice	2.63	1.34	97.92
RRH-B8	Α	From Face	3.00	0.0000	103.00	No Ice	2.64	1.66	60.00
RRT-B0	21	1 tom 1 dec	0.00	0.0000	105.00	1/2" Ice	2.85	1.84	82.85
			0.00			1" Ice	3.06	2.02	108.85
RRH-B8	В	From Face	3.00	0.0000	103.00	No Ice	2.64	1.66	60.00
iddi bo		11011111100	0.00	0,000	100.00	1/2" Ice	2.85	1.84	82.85
			0.00			1" Ice	3.06	2.02	108.85
RRH-B8	C	From Face	3.00	0.0000	103.00	No Ice	2.64	1.66	60.00
			0.00			1/2" Ice	2.85	1.84	82.85
			0.00			1" Ice	3.06	2.02	108.85
***					·				
Angle Low Profile Platform	C	From Face	0.00	0.0000	90.00	No Ice	26.10	26.10	1500.00
Ç			0.00			1/2" Ice	31.60	31.60	1700.00
			0.00			1" Ice	37.10	37.10	1900.00
APL199014-42T2	Α	From Face	3.00	0.0000	90.00	No Ice	2.09	2.09	6.00
			0.00			1/2" Ice	2.39	2.39	20.81
			0.00			1" Ice	2.70	2.70	39.44
APL199014-42T2	В	From Face	3.00	0.0000	90.00	No Ice	2.09	2.09	6.00
			0.00			1/2" Ice	2.39	2.39	20.81
			0.00			1" Ice	2.70	2.70	39.44
APL199014-42T2	C	From Face	3.00	0.0000	90.00	No Ice	2.09	2.09	6.00
			0.00			1/2" Ice	2.39	2.39	20.81
			0.00			1" Ice	2.70	2.70	39.44

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		Load Combinations
Comb.		Description
<i>No.</i>	Dood Only	
1	Dead Only	
2	1.2 Dead+1.6 Wind 0 deg - No Ice	
3	0.9 Dead+1.6 Wind 0 deg - No Ice	
4	1.2 Dead+1.6 Wind 30 deg - No Ice	
5	0.9 Dead+1.6 Wind 30 deg - No Ice	
6	1.2 Dead+1.6 Wind 60 deg - No Ice 0.9 Dead+1.6 Wind 60 deg - No Ice	
7		
8 9	1.2 Dead+1.6 Wind 90 deg - No Ice 0.9 Dead+1.6 Wind 90 deg - No Ice	
	1.2 Dead+1.6 Wind 120 deg - No Ice	
10		•
11 12	0.9 Dead+1.6 Wind 120 deg - No Ice 1.2 Dead+1.6 Wind 150 deg - No Ice	
13	0.9 Dead+1.6 Wind 150 deg - No Ice 1.2 Dead+1.6 Wind 180 deg - No Ice	•
14		
15	0.9 Dead+1.6 Wind 180 deg - No Ice	
16	1.2 Dead+1.6 Wind 210 deg - No Ice	
. 17	0.9 Dead+1.6 Wind 210 deg - No Ice	
18	1.2 Dead+1.6 Wind 240 deg - No Ice	
19	0.9 Dead+1.6 Wind 240 deg - No Ice 1.2 Dead+1.6 Wind 270 deg - No Ice	•
20	•	
21	0.9 Dead+1.6 Wind 270 deg - No Ice	
22	1.2 Dead+1.6 Wind 300 deg - No Ice 0.9 Dead+1.6 Wind 300 deg - No Ice	
23	1,2 Dead+1.6 Wind 330 deg - No Ice	
24 25		
25 26	0.9 Dead+1.6 Wind 330 deg - No Ice 1.2 Dead+1.0 Ice+1.0 Temp	
26 27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	1
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	
39	Dead+Wind 0 deg - Service	
40	Dead+Wind 30 deg - Service	
41	Dead+Wind 60 deg - Service	
42	Dead+Wind 90 deg - Service	
43	Dead+Wind 120 deg - Service	
44	Dead+Wind 150 deg - Service	
45	Dead+Wind 180 deg - Service	
46	Dead+Wind 210 deg - Service	
47	Dead+Wind 240 deg - Service	
48	Dead+Wind 270 deg - Service	
49	Dead+Wind 300 deg - Service	
50	Dood+Wind 220 dag Sarvice	

	Maximum Tower Deflections - Service Wind □											
Section	Elevation	Horz.	Gov.	Tilt	Twist	•						
No.	ft	Deflection in	Load Comb.	, o	٥							

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Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	
L1	116 - 71.12	22.634	45	1.9251	0.0167
L2	74.87 - 33.58	8.590	45	1.1678	0.0052
L3	38.41 - 0	2.121	45	0.5177	0.0016
22	30.11	2		5.51,7	0.0020

## Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	. •	ft
115.00	Angle Platform w/ Handrails	45	22.255	1.9068	0.0163	17949
103.00	Angle Platform w/ Handrails	45	17.753	1.6873	0.0125	6903
90.00	Angle Low Profile Platform	45	13.167	1.4484	0.0087	3451

## Maximum Tower Deflections - Design Wind □

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	116 - 71.12	90.378	14	7.6375	0.0663
L2	74.87 - 33.58	34.499	. 14	4.6835	0.0210
L3	38.41 - 0	8.534	14	2.0826	0.0062

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	vation Appurtenance		Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in .	٥	0	ft
115.00	Angle Platform w/ Handrails	14 ·	88.871	7.5667	0.0650	4674
103.00	Angle Platform w/ Handrails	14	70.982	6.7159	0.0497	1795
90.00	Angle Low Profile Platform	14	52.742	5.7863	0.0347	894

# Compression Checks

## Pole Design Data

Section	Elevation	Size	L	$L_{u}$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.									$P_{u}$
	ft		ft	ft		in <sup>2</sup>	lb	lb	$\overline{\phi P_n}$
L1	116 - 71.12 (1)	TP25.39x14x0.25	44.88	116.00	160.8	19.4716	-12735.90	170231.00	0.075
L2	71.12 - 33.58 (2)	TP34.29x23.9383x0.3438	41.29	116.00	118.8	36.2339	-19374.40	580198.00	0.033
L3	33.58 - 0 (3)	TP42x32.3916x0.375	38.41	116.00	93.4	50.2622	-28937.50	1301300.00	0.022

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Section	Elevation	Size	L	$L_{\mu}$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.	ft		ft	ft		in²	· <i>lb</i>	lb	$\frac{P_u}{\Phi P_u}$
0	y .								Ψ1 η

Pole Bending De	sign Data
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Section No.	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio M <sub>ux</sub>	Muy	$\phi M_{ny}$	Ratio M <sub>uy</sub>
	ft		lb-ft	lb-ft	$\phi M_{nx}$	lb-ft	lb-ft	$\phi M_{ny}$
L1	116 - 71.12 (1)	TP25.39x14x0.25	522473.33	681150.00	0.767	0.00	681150.00	0.000
L2	71.12 - 33.58	TP34.29x23.9383x0.3438	1178916.67	1724791.67	0.684	0.00	1724791.67	0.000
L3	33.58 - 0 (3)	TP42x32,3916x0,375	1992908.33	2870150.00	0.694	0.00	2870150.00	0.000

Pole Shear Design Data	
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Section No.	Elevation	Size	Actual V <sub>u</sub>	$\phi V_n$	Ratio $V_u$	Actual T <sub>u</sub>	$\phi T_n$	Ratio T <sub>u</sub>
	ft		lb	lb	φ <i>V</i> ,,	lb-ft	lb-ft	$\phi T_n$
L1	116 - 71.12 (1)	TP25.39x14x0.25	16490.80	692784.00	0.024	0.00	1385991.67	0.000
L2	71.12 - 33.58 (2)	TP34.29x23.9383x0.3438	19564.00	1296440.00	0.015	0.00	3509758.33	0.000
L3	33.58 - 0 (3)	TP42x32.3916x0.375	22743.40	1694100.00	0.013	0.00	5837516.67	0.000

# Pole Interaction Design Data

Section No.	Elevation ft	Ratio P <sub>u</sub> $\phi P_n$	$Ratio$ $M_{ux}$ $\phi M_{nx}$	$Ratio$ $M_{uy}$ $\phi M_{ny}$	$Ratio$ $V_u$ $\phi V_n$	Ratio $T_u$ $\phi T_n$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	116 - 71.12 (1)	0.075	0.767	0.000	0.024	0.000	0.842	1.000	4.8.2
L2 .	71.12 - 33.58	0.033	0.684	0.000	0.015	0.000	0.717	1.000	4.8.2
L3	33.58 - 0 (3)	0.022	0.694	0.000	0.013	0.000	0.717	1.000	4.8.2

# Section Capacity Table □

Section	Elevation	Component	Size	Critical	P	øP <sub>allow</sub>	%	Pass Fail
No.	Jī	Туре		Element	10	ID.	Capacity	ran
L1	116 - 71.12	Pole	TP25.39x14x0.25	1	-12735.90	170231.00	84.2	Pass

Infinigy Engineering 1033 Watervliet Shaker Rd. Albany, NY 12205 Phone: (518) 690-0790 FAX:

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Client□		Designed by □
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Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	øP <sub>allow</sub> lb	% Capacity	Pass Fail
L2	71.12 - 33.58	Pole	TP34.29x23.9383x0.3438	2	-19374.40	580198.00	71.7	Pass
L3	33.58 - 0	Pole	TP42x32.3916x0.375	3	-28937.50	1301300.00	71.7	Pass
							Summary	
						Pole (L1)	84.2	Pass
			*			RATING =	84.2	Pass

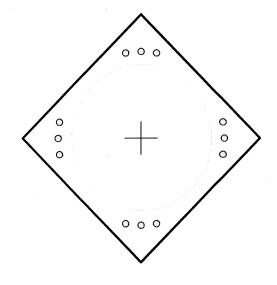
Program Version 8.0.2.1 - 5/2/2018 File:C:/Users/BDavenport/Desktop/ML03XC113.eri

Customer:
Engineer:
Job #:
Baseplate/Flange:
Plate Shape:

Date:

12/4/2018 NTP Wireless BD 568-401 Base Plate Square

Lo	ading Data							
TIA Code Revision:	Rev-G							
Axial:	29	kips						
Moment:	1992.9	k-ft						
Shear:	22.7	kips						
Plate Data								
Pole Base Diameter:	42	in						
Pole Base Shape:	12 Sided							
Pole thickness:	0.375	in						
Base Weld Size:	0.25	in						
Plate Width	50.5	in						
Plate Thickness:	2.75	in						
Plate Steel Grade:	A572 Gr. 60	ksi						
Bolt Data								
Bolt Hole Diameter:	2.375	in						
Bolt Diameter	2.25	in						
Bolt Quantity:	12							
Bolt Grade:	A615 Gr. 75	psi						
Bolt Circle:	50	in						
Bolt Spacing:	6	in						
Sti	ffener Data							
Stiffener Quantity:								
Stiffener Height:		in						
Stiffener Width:		in						
Stiffener Thickness:		in						
Stifferner Steel Grade:								
Vertical Weld Size:		in						
Horizontal Weld Size:		in						
Stiffener Notch width:		lin						



		_
Plate Ratio:	86.14	%
Bolt Ratio:	55.29	%
Bolt Shear Ratio:	63.57	%
Vertical Weld Ratio:	-	%
Horizontal Weld Ratio:	-	%
Stiffener Ratio:	-	%

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