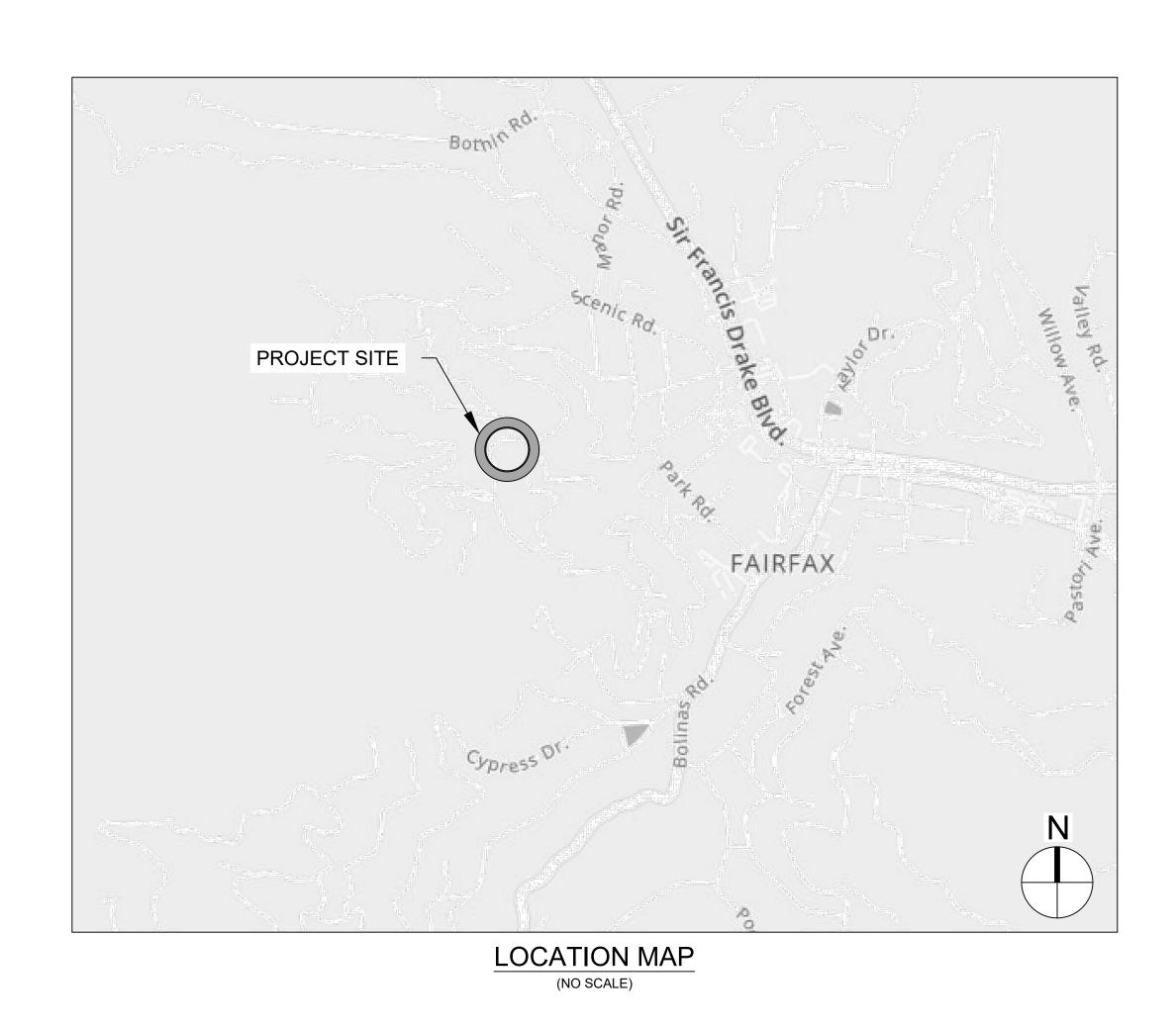
TOWN OF FAIRFAX DEPARTMENT OF PUBLIC WORKS

142 BOLINAS ROAD FAIRFAX, CALIFORNIA 94930 PHONE: (415) 453-0291

RETAINING WALL REPLACEMENT BERRY TRAIL AT TAMALPAIS DRIVE





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GENERAL

- ALL CONDITIONS AND DIMENSIONS SHOWN ON THE PLANS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DISCREPANCIES THAT REQUIRE CLARIFICATION OR REVISIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE STARTING WORK.
- 2. THE CONTRACTOR SHALL POSSES A CLASS "A" LICENSE.
- 3. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SAFETY, AND SEQUENCE.
- 4. CONTRACTOR SHALL NOTIFY UNDERGROUND SERVICE ALERT PRIOR TO START OF ANY CONSTRUCTION. CONTRACTOR SHALL NOTIFY ALL PUBLIC OR PRIVATE UTILITY COMPANIES IN ACCORDANCE WITH ALL APPLICABLE LAWS PRIOR TO COMMENCEMENT OF WORK NEAR EXISTING UTILITY LINES.
- 5. CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION OF ALL EXISTING UTILITIES IN THE FIELD. ANY UTILITIES DAMAGED BY THE CONTRACTOR SHALL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- 6. TOWN OF FAIRFAX ENCROACHMENT PERMIT IS REQUIRED FOR ALL WORK, INCLUDING STAGING OF MATERIALS AND EQUIPMENT IN THE TOWN'S RIGHT-OF-WAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING AN ENCROACHMENT PERMIT IN ACCORDANCE WITH THE PERMIT REQUIREMENTS.
- 7. THE CONTRACTOR SHALL COORDINATE WITH ENGINEER TO ESTABLISH THE RETAINING WALL, SOLDIER PILE AND TIEBACK LAYOUTS PRIOR TO DRILLING AND SOLDIER PILE AND TIEBACK INSTALLATION.
- 8. THE CONTRACTOR SHALL HAUL AWAY ALL UNUSED/EXCESS EXCAVATED MATERIAL OFF SITE FOR LEGAL DISPOSAL.
- 9. NO CONSTRUCTION MATERIALS, EQUIPMENT, DEBRIS OR WASTE SHALL BE PLACED OR STORED WHERE IT MAY BE SUBJECT TO WIND OR RAIN EROSION AND DISPERSION.
- 10. WORKMANSHIP TO BE PERFORMED IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS ALONG WITH 2023 CALTRANS STANDARD SPECIFICATIONS, TOWN OF FAIRFAX, MARIN WATER AND MARIN COUNTY STANDARDS AND GENERALLY ACCEPTED CONSTRUCTION PRACTICES.

SURVEY NOTES

- TOPOGRAPHIC INFORMATION IS BASED UPON A FIELD SURVEY PERFORMED BY CAPSTONE LAND SURVEYING, LLP ON DECEMBER 8, 2023.
- 2. VERTICAL DATUM: NORTH AMERICAN VERTICAL DATUM OF 1988.
- 3. MAPPING OF BOUNDARY/PROPERTY LINES WAS NOT INCLUDED AS PART OF THE SURVEY.

SPECIAL INSPECTIONS

- SPECIAL INSPACTION SHALL BE PERFORMED BY MILLER PACIFIC AND/OR A QUALIFIED TESTING AND INSPECTION AGENCY DURING CONSTRUCTION, INCLUDING THE FOLLOWING:
- 1.1 SOLDIER PILES: INTERMITTENT OBSERVATION OF DRILLING. FINISHED SOLDIER PILE EXCAVATIONS SHALL BE OBSERVED PRIOR TO INSTALLING STEEL BEAM. STEEL BEAM SHALL BE OBSERVED PRIOR TO PLACEMENT IN DRILLED HOLE.
- 1.2 CONCRETE: INTERMITTENT OBSERVATION DURING PLACEMENT. IF REQUESTED BY THE ENGINEER, CONCRETE SHALL BE SAMPLED AND CYLINDERS SHALL BE CAST FOR STRENGTH TESTING IN CONFORMANCE WITH ASTM C39. A MINIMUM OF 1 CYLINDER SHALL BE TESTED AT THREE 7 DAYS AND A MINIMUM OF 3 CYLINDERS SHALL BE TESTED AT 28 DAYS.
- 1.3 GROUT: INTERMITTENT OBSERVATION DURING PLACEMENT. IF REQUESTED BY THE ENGINEER, GROUT SHALL BE SAMPLED AND CYLINDERS SHALL BE CAST FOR STRENGTH TESTING IN CONFORMANCE WITH ASTM C39. A MINIMUM OF 1 CYLINDER SHALL BE TESTED AT 3 DAYS AND A MINIMUM OF 3 CYLINDERS SHALL BE TESTED AT 28 DAYS
- 1.4 TIEBACKS: INTERMITTENT OBSERVATION OF DRILLING. FINISHED EXCAVATIONS SHALL BE OBSERVED PRIOR TO INSTALLING THREADBAR. THREAD BAR SHALL BE OBSERVED PRIOR TO PLACEMENT IN DRILLED HOLE. LOAD TESTING SHALL BE PERFORMED AS SPECIFIED HEREIN AND OBSERVED BY THE ENGINEER.
- 1.5 DRAINAGE: INTERMITTENT OBSERVATION OF DRAINAGE PANELS, PERMEABLE MATERIAL AND RELATED COMPONENTS INSTALLED FOR WALL DRAINAGE.
- 1.6 BACKFILL: INTERMITTENT OBSERVATION AND FIELD DENSITY TESTING OF COMPACTED BACKFILL.

SOLDIER PILES

- REFER TO TECHNICAL SPECIFICATION SECTION 2296 FOR ADDITIONAL REQUIREMENTS FOR SOLDIER PILE CONSTRUCTION.
- 2. STEEL BEAMS FOR SOLDIER PILES SHALL BE ASTM A572, GRADE 50 OR APPROVED
- 3. STEEL BEAMS SHALL BE PAINTED WITH TWO COATS OF BLACK COAL TAR EPOXY, CARBOLINE BITUMASTIC NO. 300-M OR APPROVED EQUAL. COAL TAR EPOXY COATING SHALL EXTEND 12 IN BELOW THE TOP OF CONCRETE ENCASEMENT.

CONCRETE

- 1. ALL CONCRETE SHALL BE NORMAL WEIGHT AND READY-MIXED WITH A MAXIMUM WATER TO CEMENT RATIO OF 0.45.
- 2. ALL AGGREGATE USED IN CONCRETE PRODUCTION SHALL CONFORM TO ASTM C33.
- 3. ALL WATER USED IN CONCRETE PRODUCTION SHALL BE CLEAN POTABLE AND NOT DETRIMENTAL TO THE CONCRETE.
- 4. READY-MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH
- 5. CEMENT SHALL CONFORM TO ASTM C 150, TYPE II.
- 6. CONCRETE FOR SOLDIER PILES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS.
- 7. CONCRETE FOR LAGGING SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS.
- 8. REBAR SHALL CONFORM TO ASTM A615, GRADE 60.

TIEBACKS

- 1. REFER TO TECHNICAL SPECIFICATION 2286 FOR TIEBACK REQUIREMENTS.
- 2. TIEBACKS SHALL BE DYWIDAG STEEL THREAD BARS (OR APPROVED EQUIVALENT) AS DESIGNATED ON THE PLANS AND MANUFACTURED EXPRESSLY FOR USE AS TIEBACKS.
- 3. GRADE 75 THREADBAR SHALL CONFORM TO ASTM A615 GRADE 150 THREADBAR SHALL CONFORM TO ASTM 722
- BEARING PLATES AND HARDWARE SHALL CONFORM TO THE MANUFACTURER'S RECOMMENDATIONS AND APPLICABLE ASTM STANDARDS.
- 5. CORROSION PROTECTION SHALL BE DOUBLE CORROSION PROTECTION AS RECOMMENDED BY THE MANUFACTURER AND AS SHOWN ON THE PLANS.
- 6. CEMENT GROUT SHALL BE MADE OF PORTLAND CEMENT CONFORMING TO ASTM C 150 WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS AND
- 7. LOAD TESTING SCHEDULE:
- (DL = DESIGN LOAD, CTL = CREEP TEST LOAD)

WATER CEMENT RATIO BETWEEN 0.4 AND 0.5.

- 7.1 PROOF TESTING SHALL BE PERFORMED ON ALL TIEBACKS.
- 7.2 PROOF LOAD TEST: AL (0.05 DL), 0.25 DL, 0.50 DL, 0.75 DL, 1.00 DL, 1.33 DL (CTL)

HOLD CTL FOR 10 MINUTES WITH DISPLACEMENT MEASUREMENTS AT 1, 2, 3, 4, 5, 6, AND 10 MINUTES. IF THE TOTAL MOVEMENT BETWEEN 1 AND 10 MINUTES EXCEEDS 0.04 IN, THE TEST LOAD SHALL BE HELD FOR AN ADDITIONAL 50 MINUTES WITH FURTHER DISPLACEMENT READINGS MADE AT 15, 20, 25, 30, 45, AND 60 MINUTES. THE TOTAL MOVEMENT WITHIN THE PERIOD OF 6 TO 60 MINUTES SHALL NOT EXCEED 0.08 IN.

7.3 THE TIEBACK DISPLACEMENT SHALL BE MEASURED WITH A DIAL GAUGE CAPABLE OF ACCURATELY MEASURING DISPLACEMENT TO THE NEAREST 0.001 IN.

WOOD

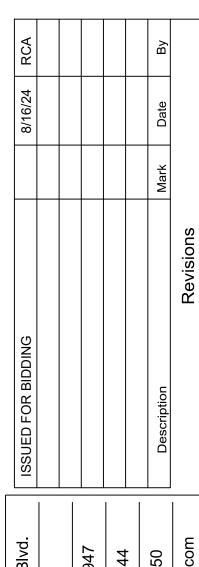
- 1. TIMBER LAGGING AND OTHER WOOD PRODUCTS SHALL BE DOUGLAS FIR NO. 1 OR BETTER. PRESERVATIVES AND TREATMENT SHALL COMPLY WITH AWPA STANDARD U1 FOR USE CATEGORY 4B.
- 2. WHERE CUTTING OF LAGGING IS REQUIRED, PAINT CUT ENDS WITH COMPLIANT PRESERVATIVE.

WALL DRAINAGE

- 1. PERMEABLE MATERIAL SHALL CONSIST OF CALTRANS CLASS 1 PERMEABLE MATERIAL. ENCASED IN FILTER FABRIC OR CLASS 2 PERMEABLE MATERIAL.
- 2. FILTER FABRIC SHALL CONSIST OF MIRAFI 140N OR APPROVED EQUAL.
- 3. DRAINAGE PIPE SHALL CONFORM TO ASTM D3034, SDR 35.

EROSION & SEDIMENT CONTROL

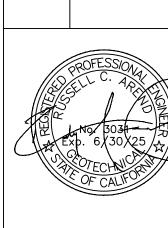
- EROSION AND SEDIMENT CONTROL MEASURES SHALL COMPLY WITH ALL REQUIREMENTS OUTLINED IN THE MARIN COUNTY STORMWATER POLLUTION PREVENTION PROGRAM (MCSTOPPP) MINIMUM CONTROL MEASURES FOR SMALL CONSTRUCTION PROJECTS AS OUTLINED IN THE MCSTOPPP CONSTRUCTION EROSION AND SEDIMENT CONTROL PLAN APPLICANT PACKAGE.
- ANY AREAS IN WHICH GROUND SURFACE AND VEGETATIVE COVER HAS BEEN DISTURBED BY CONSTRUCTION ACTIVITIES SHALL BE COVERED WITH A PRE-APPROVED SEED MIX AND BIODEGRADABLE EROSION CONTROL MATS UPON COMPLETION OF CONSTRUCTION.
- 3. EROSION CONTROL MATS SHALL CONSIST OF TYPE SC150 BY NORTH AMERICAN GREEN (OR APPROVED EQUAL).



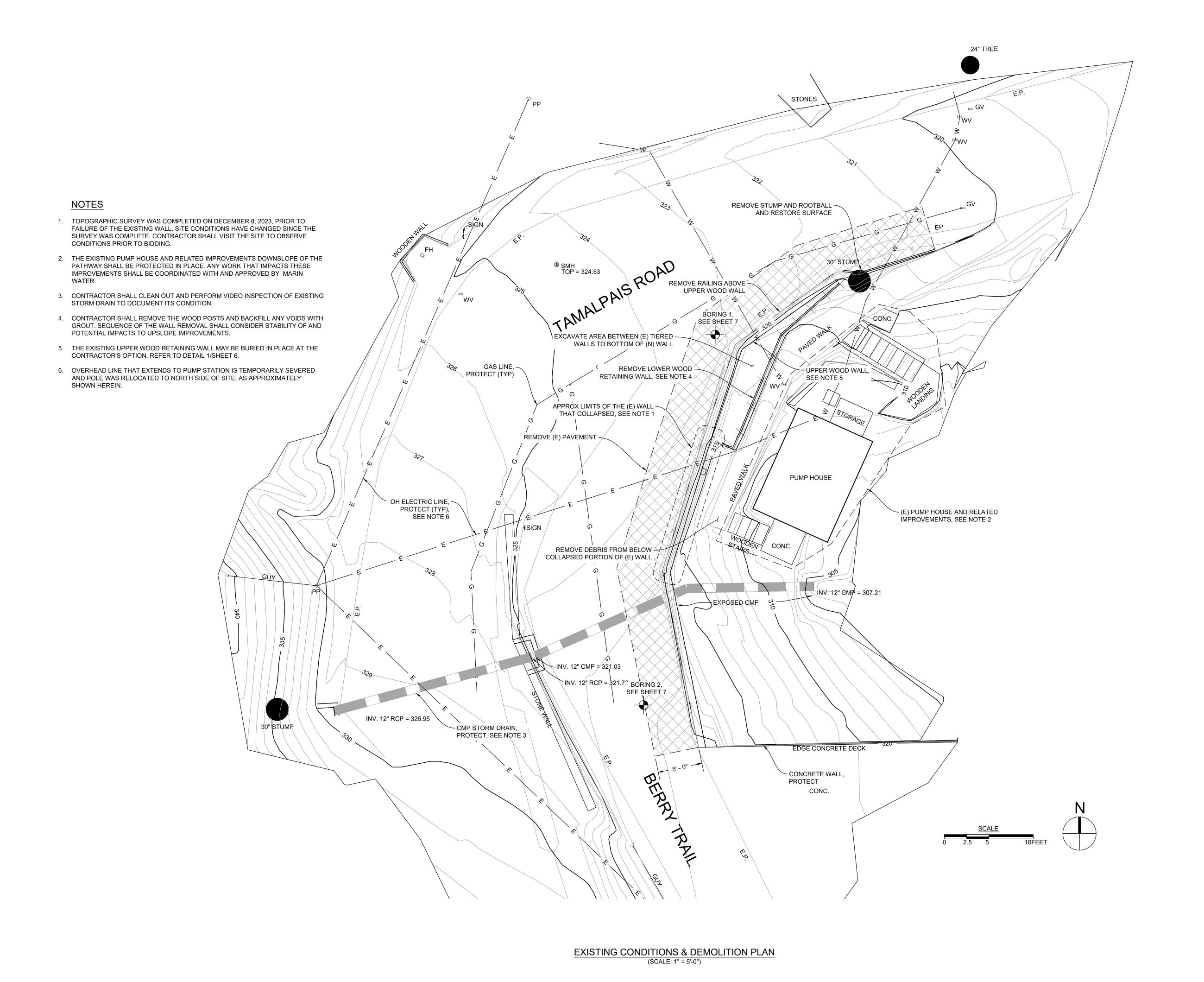
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all Replacement Tamalpais Drive California NOTE Retaining Berry Trail



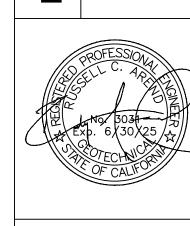
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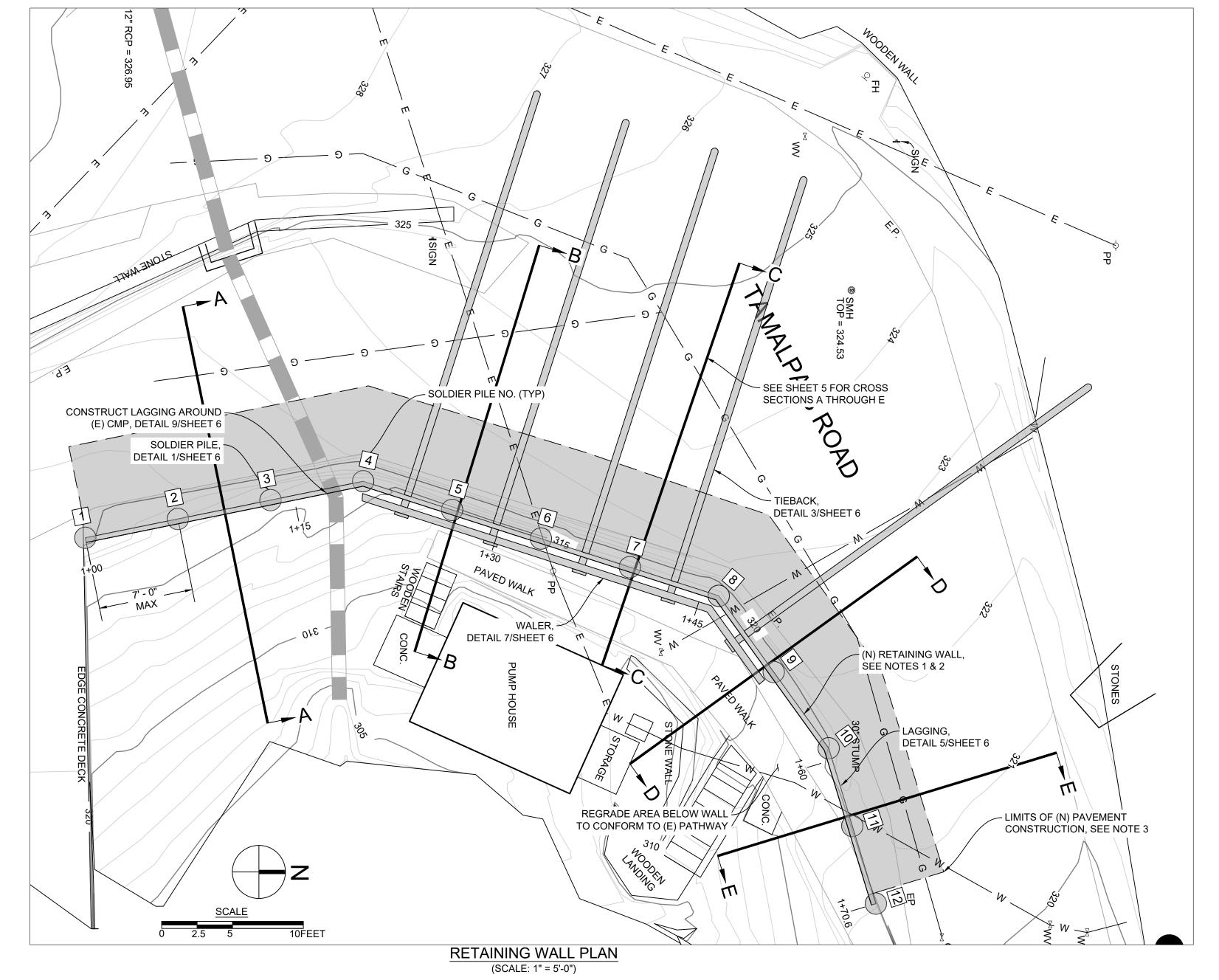
EXISTING CONDITIONS & DEMOLITION PLAN Retaining Wall Replacement Berry Trail at Tamalpais Drive Fairfax, California

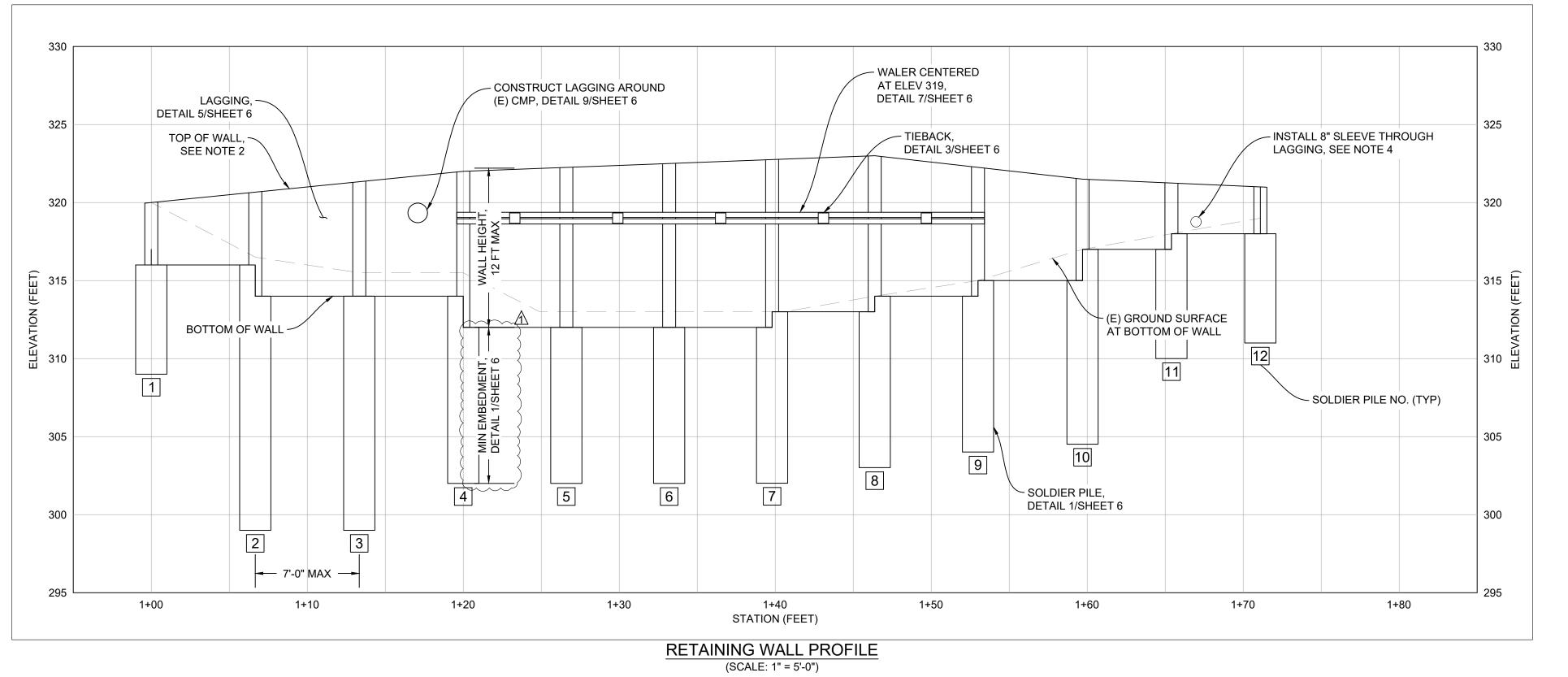


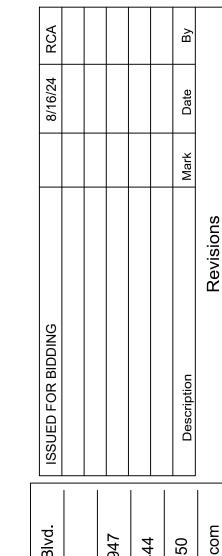
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NOTES

- 1. THE EXISTING UPPER WOOD RETAINING WALL MAY BE BURIED IN PLACE AT THE CONTRACTOR'S OPTION. REFER TO DETAIL 1/SHEET 6.
- 2. GUARDRAIL AND WALL RAILING ARE NOT SHOWN ON PLAN AND PROFILE FOR CLARITY. THESE WILL BE LOCATED ABOVE THE WALL FOR THE ENTIRE ALIGNMENT. REFER TO DETAIL 4/SHEET 6 FOR GUARDRAIL AND WALL RAILING INFORMATION.
- 3. NEW PAVEMENT SECTION TO CONSIST OF 3" OF ASPHALT OVER 6" OF AGGREGATE BASE COMPACTED TO 95% RC. SUBGRADE SHALL BE SCARIFIED, MOISTURE CONDITIONED AND COMPACTED TO 95% RC.
- 4. INSTALL SLEEVE THROUGH LAGGING FOR FUTURE UTILITY INSTALLATIONS. LOCATION SHOWN IS APPROXIMATE. FINAL LOCATION SHALL BE REVIEWED AND APPROVED BY MARIN WATER DURING CONSTRUCTION.





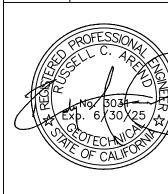




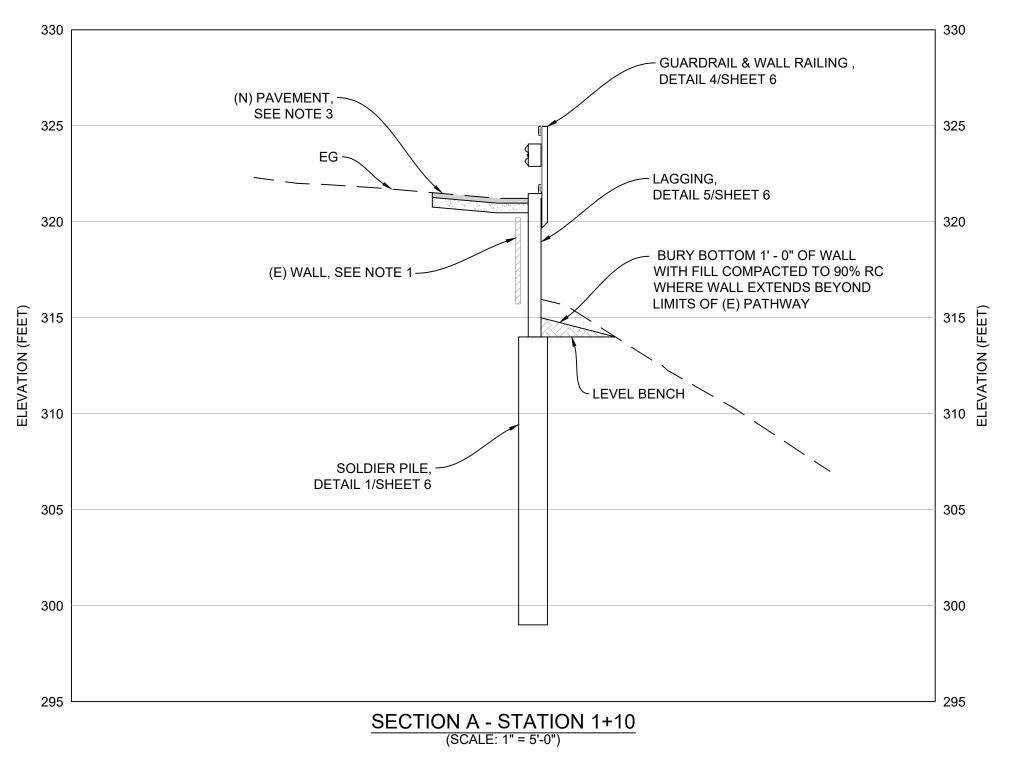
PROFILE

Retaining Wall Replacement Berry Trail at Tamalpais Drive Fairfax, California

RETAINING WALL PLAN &



SHEET



(N) PAVEMENT,

SEE NOTE 3

(E) UPPER WALL, -SEE NOTE 1

DETAIL 3/SHEET 6

TIEBACK, -

LAGGING, -DETAIL 5/SHEET 6

SOLDIER PILE, -

SECTION C - STATION 1+40 (SCALE: 1" = 5'-0")

DETAIL 1/SHEET 6

330

325

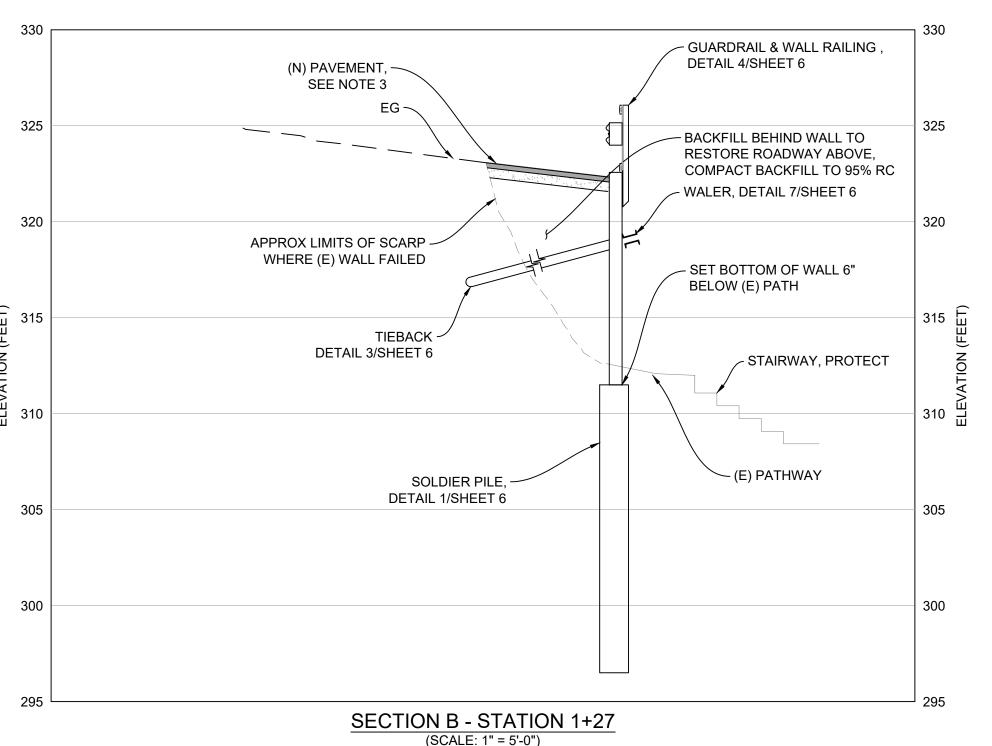
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(N) PAVEMENT, -SEE NOTE 3

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TIEBACK,

DETAIL 3/SHEET 6

(E) UPPER WALL, SEE NOTE 1

SET BOTTOM OF WALL -

6" BELOW (E) PATH

DETAIL 1/SHEET 6

SOLDIER PILE, -

<u>SECTION D - STATION 1+52</u> (SCALE: 1" = 5'-0")

325

320

出 310

305

300

GUARDRAIL & WALL RAILING ,
DETAIL 4/SHEET 6

- WALER,

(E) PATHWAY

DETAIL 7/SHEET 6

– (E) LOWER WALL, SEE NOTE 2

- (E) STONE WALL, PROTECT

-(E) STORAGE,

PROTECT

325

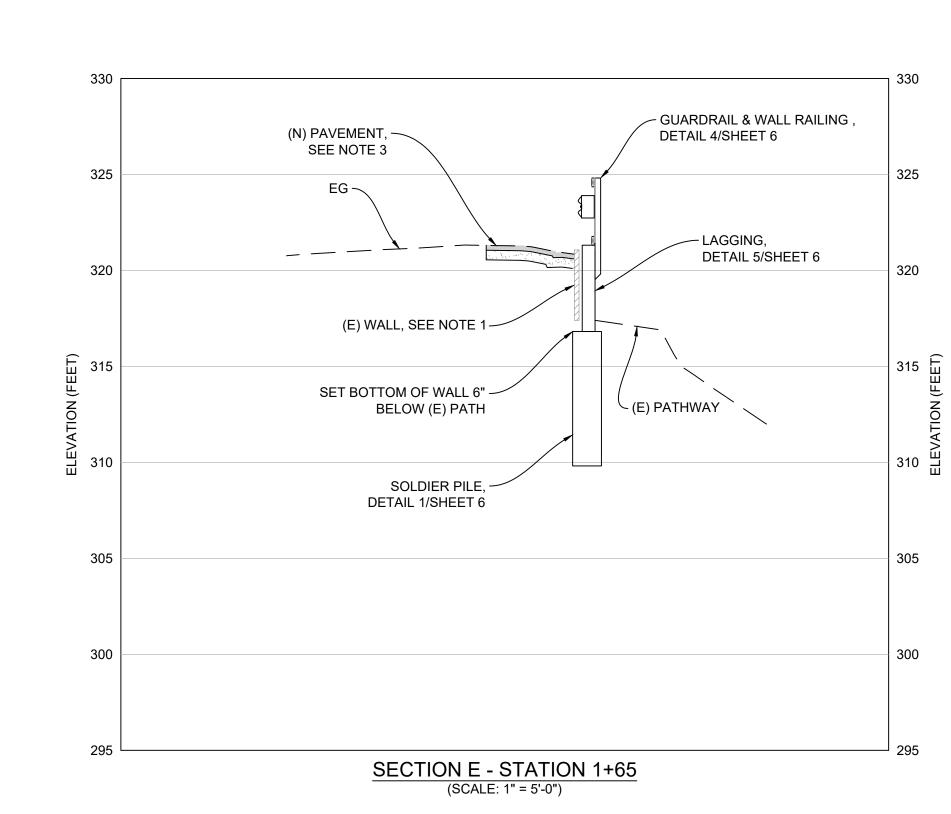
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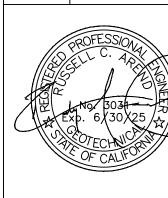
NOTES

- 1. THE EXISTING UPPER WOOD RETAINING WALL MAY BE BURIED IN PLACE AT THE CONTRACTOR'S OPTION. REFER TO DETAIL 1/SHEET 6.
- 2. WHERE TIERED WALLS EXIST REMOVE LOWER WOOD WALL AND REGRADE AREA IN FRONT OF THE NEW WALL AS SHOWN ON THE RETAINING WALL PLAN ON SHEET 3.
- 3. NEW PAVEMENT SECTION TO CONSIST OF 3" OF ASPHALT OVER 6" OF AGGREGATE BASE COMPACTED TO 95% RC. SUBGRADE SHALL BE SCARIFIED, MOISTURE CONDITIONED AND COMPACTED TO 95% RC

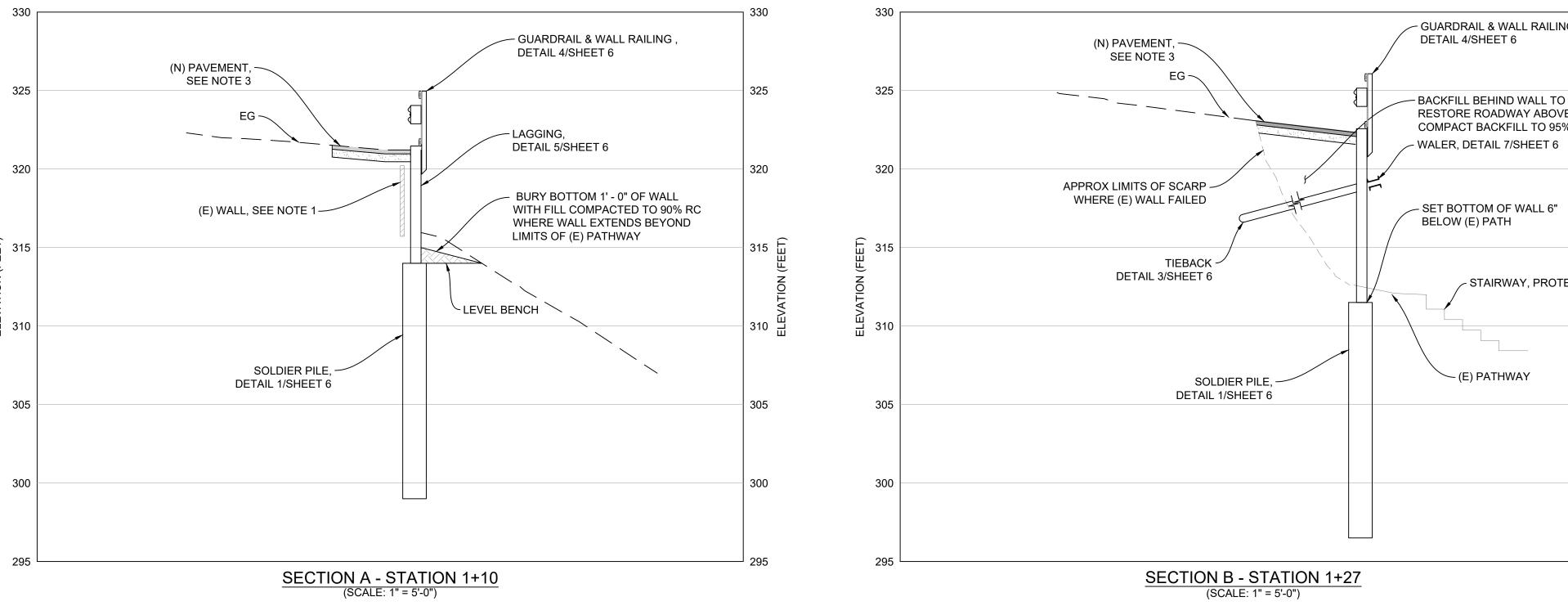


Retaining Wall Replacement Berry Trail at Tamalpais Drive Fairfax, California SECTIONS

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SHEET



- GUARDRAIL & WALL RAILING,

DETAIL 4/SHEET 6

~ WALER, DETAIL 7/SHEET 6

SEE NOTE 2

(E) PATHWAY

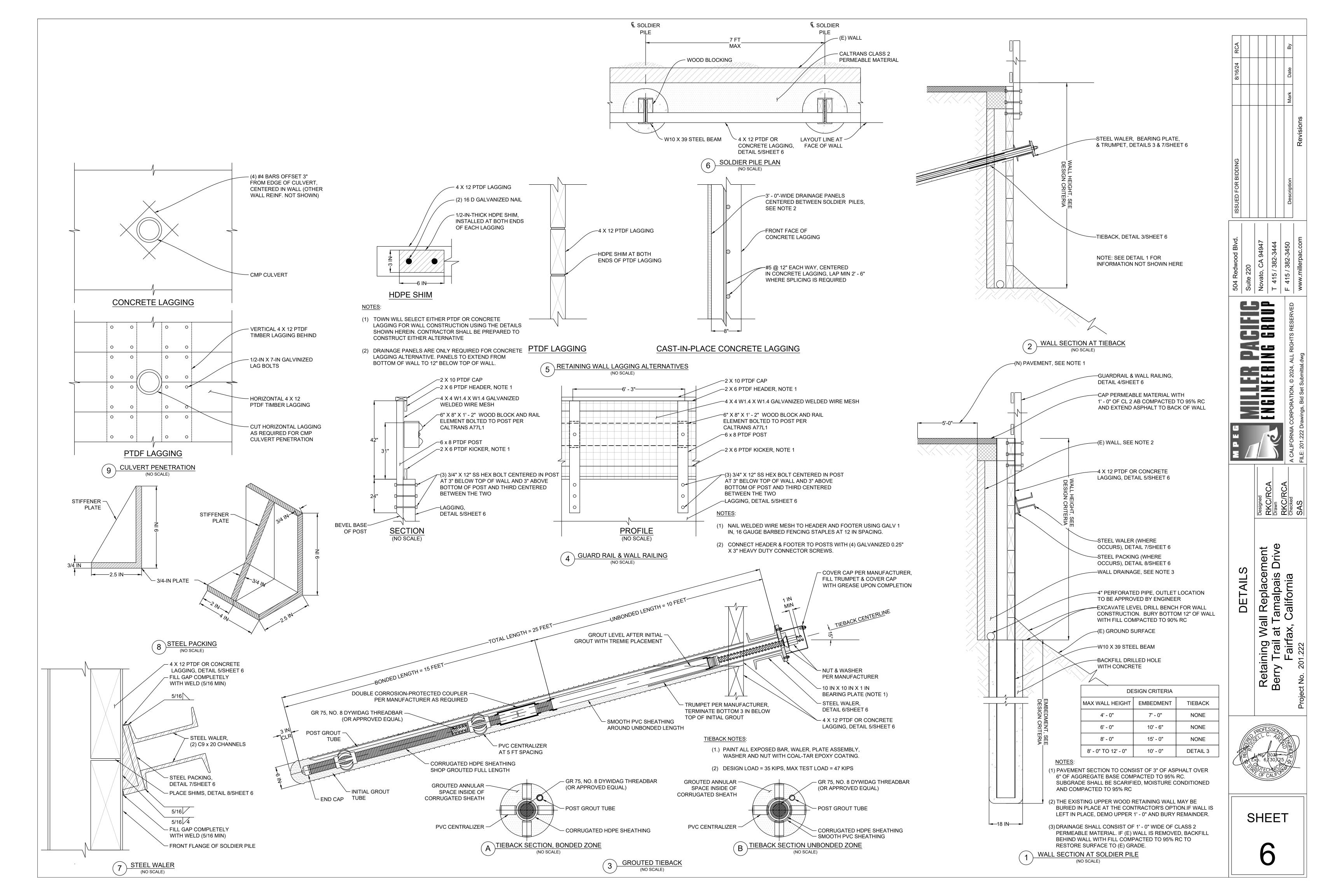
(E) LOWER WALL,

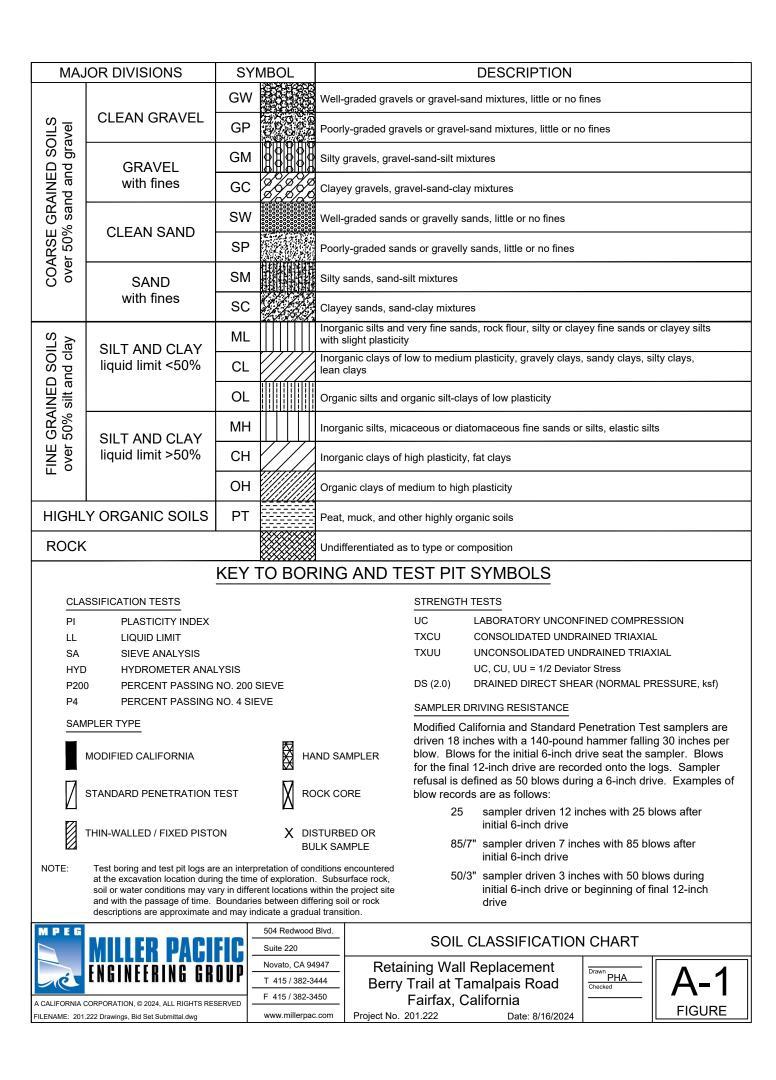
(E) PUMP STATION,

300

PROTECT

– SET BOTTOM OF WALL 6" BELOW (E) PATHWAY





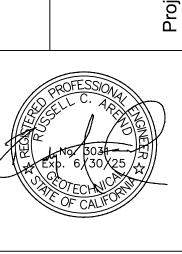
Fracture Classification	Spacing	Bedding Classification
Crushed Intensely fractured Closely fractured Moderately fractured Widely fractured Very widely fractured	less than 3/4 inch 3/4 to 2-1/2 inches 2-1/2 to 8 inches 8 to 24 inches 2 to 6 feet greater than 6 feet	Laminated Very thinly bedded Thinly bedded Medium bedded Thickly bedded Very thickly bedded
	HARDNESS	
Low Moderate Hard Very hard	Carved or gouged Easily scratched w Difficult to scratch, Rock scratches me	ith a knife, friable knife scratch leaves dust trace
	STRENGTH	
Friable Weak Moderate Strong Very strong	Crumbles by rubbing with fingers Crumbles under light hammer blows Indentations <1/8 inch with moderate blows, y Withstands few heavy hammer blows, y Withstands many heavy hammer blows,	ields large fragments
very suong		
very suong	WEATHERING	
Complete Minerals dec High Rock decom coated w Moderate Fracture sur Slight A few staine no affect	composed to soil, but fabric and structure position, thorough discoloration, all fracturith clay, oxides or carbonates faces coated with weathering minerals, multiple of the fractures, slight discoloration, no mineral on cementation	res are extensively oderate or localized discoloration al decomposition,
Complete Minerals dec High Rock decom coated w Moderate Fracture sur Slight A few staine no affect	composed to soil, but fabric and structure position, thorough discoloration, all fracturith clay, oxides or carbonates faces coated with weathering minerals, modifications, slight discoloration, no minerals	res are extensively oderate or localized discoloration al decomposition,
Complete Minerals dec High Rock decom coated v Moderate Fracture sur Slight A few staine no affect Fresh Rock unaffec	composed to soil, but fabric and structure position, thorough discoloration, all fracturith clay, oxides or carbonates faces coated with weathering minerals, multiple of the fractures, slight discoloration, no mineral on cementation	res are extensively oderate or localized discoloration al decomposition, rings under hammer impact at the location and time of exploration.
Complete Minerals dec High Rock decom coated v Moderate Fracture sur Slight A few staine no affect Fresh Rock unaffec	composed to soil, but fabric and structure position, thorough discoloration, all fracturith clay, oxides or carbonates faces coated with weathering minerals, mid fractures, slight discoloration, no mineral on cementation cted by weathering, no change with depth of the discoloration of conditions encountered and water conditions may differ in other locations and	res are extensively oderate or localized discoloration al decomposition, rings under hammer impact at the location and time of exploration.

o meters DEPTH	SAMPLE SYMBOL (4)		BLOWS / FOOT (1)	DRY UNIT WEIGHT pcf (2)	MOISTURE CONTENT (%)	SHEAR STRENGTH psf (3)	OTHER TEST DATA	OTHER TEST DATA
- - -1 - 5-		4.5" Asphalt Concrete over 6" Aggregate Base Sandy GRAVEL (GM) light gray, dry to slightly moist, loose Sandy SILT (ML) brown, slightly moist, soft to medium stiff, love medium plasticity	9 5 v to	91 110	4.3 16.3	500 UC		
-2 - -2 - - - -3 10-		SANDSTONE tan, low hardness, weak, highly to completel weathered SHALE gray and brown, low hardness, friable to weathered		100	7.1			
- -4 - -		weathered 47 119 13.7 grades hard, dark gray						
15- -5 -			91	132	7.2			
Water level encountered during drilling Water level measured after drilling Water level measured after drilling Value Va								HT (pcf)
Suite 220 Suite 220 Novato, CA 94947 T 415 / 382-3444 F 415 / 382-3450 Filename: 201.222 Drawings, Bid Set Submittal.dwg Suite 220 Suite 220 Novato, CA 94947 T 415 / 382-3444 F 415 / 382-3450 Www.millerpac.com Project No. 201.222 Date: 8/16/2024 FIGURE Drawings, Bid Set Submittal.dwg Figure					-3 JRE			

meters DEPTH S feet	SAMPLE	SYMBOL (4)	BORI (CONTI		BLOWS / FOOT (1)	DRY UNIT WEIGHT pcf (2)	MOISTURE CONTENT (%)	SHEAR STRENGTH psf (3)	OTHER TEST DATA	OTHER TEST DATA
			SHALE dark gray, hard, friable to	weak, highly weathered	64	127	12.1	2300 UC		
_		×××2	Boring terminated at 21.5 ft b Ground water measured at 2							
-7 -			drilling	no napon completion of						
_										
25-										
-8 - -										
_										
_										
9 30-										
-										
- -10 -										
_										
35-										
- -11 -										
_										
_										
- 12 40-										
	Ļ									
Water level encountered during drilling NOTES: (1) UNCORRECTED FIELD BLOW COUNTS (2) METRIC EQUIVALENT DRY UNIT WEIGHT kN/m³ = 0.1571 x DRY UNIT WEIGHT (pcf) (3) METRIC EQUIVALENT STRENGTH (kPa) = 0.0479 x STRENGTH (psf) (4) GRAPHIC SYMBOLS ARE ILLUSTRATIVE ONLY										
MPEG MILLER PACIFIC 504 Redwood Blvd. Suite 220 November 60 04047										
ENGINEERING GROUP A CALIFORNIA CORPORATION, © 2024, ALL RIGHTS RESERVED A CALIFORNIA CORPORATION, © 2024, ALL RIGHTS RESERVED STUDENIA CORPORATION, © 2024, ALL RIGHTS RESERVED WWW. millernac.com WWW. millernac.com WWW. millernac.com WWW. millernac.com Project No. 201 222 P					Tamalpais	s Road	P		A-	4

o meters DEPTH of feet SAMPLE SYMBOL (4)		BLOWS / FOOT (1)	DRY UNIT WEIGHT pcf (2)	MOISTURE CONTENT (%)	SHEAR STRENGTH psf (3)	OTHER TEST DATA	OTHER TEST DATA
- 1 - I	6" Asphalt Concrete over 6" Aggregate Base Sandy SILT (ML) dark gray-brown, slighty mottled red-orange, moist, stiff, low to medium plasticity	13	105	18.8	1225 UC		
5-	CLAY with Sand (CL) dark brown and gray mottled dark red-brown, moist, medium stiff, low plasticity	11	125	11.6	1025 UC	LL 31 PL 23 Pl 8	
	SHALE dark gray, crushed, low hardness, friable to weak, completely weathered [Bedrock]	28	108	19.9			
-3 10- - 4 - 		34	120	13.5			
-5 -	Boring terminated at 18.0 ft below ground surface	88/11"	132	9.9			
=	No groundwater encountered countered during drilling countered during drilling countered after drilling NOTES: (1) UNCORRECTED FIELD (2) METRIC EQUIVALENT (3) METRIC EQUIVALENT (3) METRIC EQUIVALENT	DRY UNIT V	VEIGHT kN	J/m ³ = 0.15 0479 x STF	71 x DRY L RENGTH (p	JNIT WEIGH	HT (pcf)
M P E G	LER PACIFIC NFFRING GROUP T. 445 (200 Auto) Retaining Wall I	BOR	ING LC	ILY			





Retaining Wall Replacement Berry Trail at Tamalpais Drive Fairfax, California

BORING LOGS

SHEET

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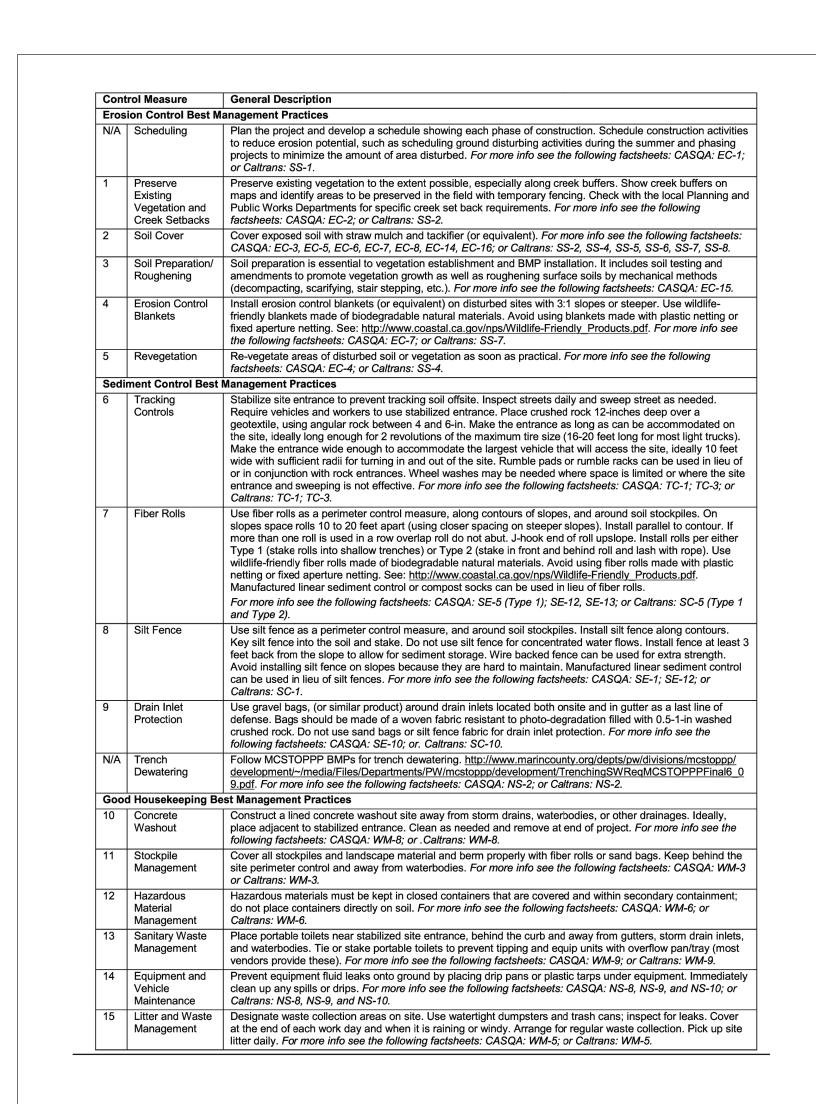


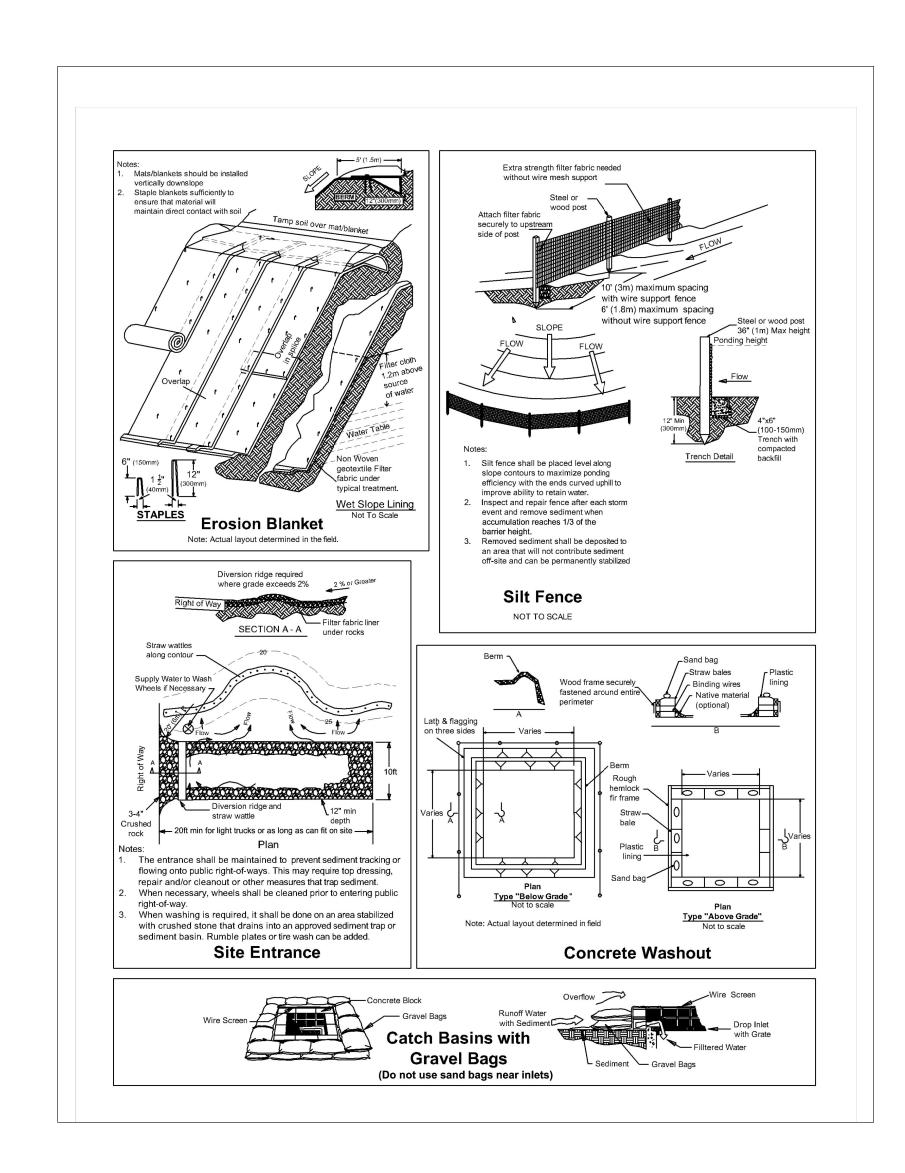
	Sediment Controls	Good Housekeeping			
6.	Tracking Controls	10. Concrete Washout			
3 7.	Fiber Rolls	11. Stockpile Management			
8.	Silt Fence	12. Hazardous Material Management			
9.	Drain Inlet Protection	13. Sanitary Waste Management			
NS	Trench Dewatering	14. Equipment and Vehicle Maintenance			
		15. Litter and Waste Management			
	8. 9.	6. Tracking Controls 5. 7. Fiber Rolls 6. Silt Fence 7. Fiber Rolls 7. Fiber Rolls 8. Silt Fence 9. Drain Inlet Protection			

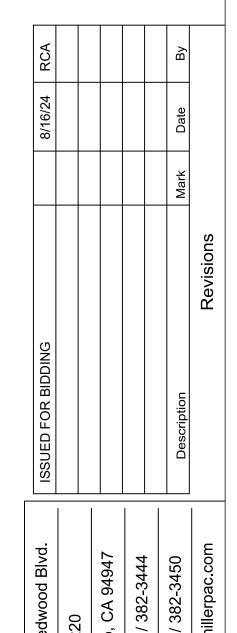
Note: Select an effective combination of control measures from each category, Erosion Control, Sediment Control, and Good Housekeeping. Control measures shall be continually implemented and maintained throughout the project until activities are complete, disturbed areas are stabilized with permanent erosion controls, and the local agency has signed off on permits that may have been required for the project. Inspect and maintain the control measures before and after rain events, and as required by the local agency or state permit.

More detailed information on the BMPs can be found in the related California Stormwater Quality Association (CASQA) and California Department of Transportation (Caltrans) BMP Factsheets. CASQA factsheets are available by subscription in the California Best Management Practices Handbook Portal: Construction at http://www.casqa.org. Caltrans factsheets are available in the Construction Site BMP Manual March 2003 at http://www.dot.ca.gov/hq/construc/stormwater/manuals.htm. Visit www.mcstoppp.org for more information on construction site management and Erosion and Sediment Control Plans.

If you require materials in alternative formats, please contact: 415-473-4381 voice/TTY or disabilityaccess@co.marin.ca.us







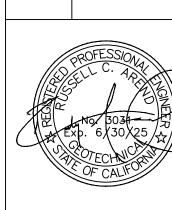
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SOL	Designed RKC/RCA Drawn RKC/RCA Checked
INT CONTROL	ement Drive

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EROSION & SEDIMENT Contract Retaining Wall Replacement

Retaining Wall Rep Berry Trail at Tamal



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