TOWN OF LOOMIS, CA

PRELIMINARY HYDROLOGIC AND HYDRAULIC STUDY

VERTICAL DATUM: NGVD29



Original Submittal: September 27, 2024 Revised Submittal: July 16, 2025

Prepared For:

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TABLE OF CONTENTS

1

1-2

2-6

6-7

7-8

8

9

Ι.	INTRODUCTION
II.	PROJECT LOCATION AND DESCRIPTION
III.	HYDROLOGY
IV.	HYDRAULICS
٧.	STORMWATER QAULITY
VI.	CONCLUSIONS
VII.	REFERENCES
APPEI	NDIX A — Vicinity Map
APPEI	NDIX B — FEMA Map
APPEI	NDIX C — Tentative Subdivision Map & Web Soil Survey
APPEI	NDIX D - Shed Maps
APPEI	NDIX E — Hydrology
APPEI	NDIX F — Hydraulics
APPEI	NDIX G - Storm Water Quality

I. INTRODUCTION

This report presents hydrologic and hydraulic analyses for The Reserve Subdivision (Project) by Premier 40, LLC, located in the Town of Loomis, Placer County, California (See Appendix A – Vicinity Map). This report estimates storm flow hydrology and hydraulics as outlined in the *Town of Loomis Land Development Manual* (Reference 1, hereinafter referred to as the *Manual*) and the *Placer County Stormwater Management Manual* (Reference 2, hereinafter referred to as the *SWMM*). Additionally, this report presents Low Impact Development (LID) Storm Water Quality calculations as outlined in the *West Placer – Storm Water Quality Design Manual* (Reference 3, hereinafter referred to as the *LID Manual*).

II. PROJECT LOCATION AND DESCRIPTION

The Project is proposed as a residential subdivision development located on 26.29± acres of currently undeveloped land in the Town of Loomis, Placer County. The site is located in the southwest corner of Rocklin Road and Barton Road (5780 Rocklin Road). Existing roadway improvements (Rocklin Road) border the Project to the north. Existing roadway improvements (Barton Road) border the Project east. Existing developed single-family large-lot parcels border the Project to the south and west. A large parcel (Parcel 2 – APN: 045-161-034) is situated between the Project site and the actual southwest corner of Rocklin and Barton Roads. This parcel (13.78± acres) is mostly undeveloped, with exception to agricultural fields and a small fruit stand structure.

See Vicinity Map – Appendix A

The development is proposed as a 20-lot residential subdivision and public right-of-way dedications along Rocklin and Barton Roads. The Project will construct roadway improvements, underground utilities and buildable areas on the proposed residential lots (see Tentative Subdivision Map – Appendix C). The Project's proposed improvements generally consist of the following:

- 20 Single-Family Residential Lots (Average Lot Size 54,628 SF);
- 1.21± acres of Public Right-of-Way;
- A paved single cul-de-sac to serve the subdivision (Private)
 - o Concrete curb, gutter & sidewalk (one side of road only)
- Offsite (Frontage Widening) Improvements along Rocklin Road
 - o Pavement widening
 - Concrete curb, gutter & sidewalk
 - Frontage Soundwall (or Fence)
- Offsite (Frontage Widening) Improvements along Barton Road
 - Pavement Widening + Entrance Tapers
 - o 3-FT Bike Lane
 - o Roadway Shoulder Grading & AC Curb
- Onsite & Offsite underground utilities;
- Water Quality (Vegetated) Swales, Preservation of Existing Tree Canopy and Preservation of Naturally Vegetated Areas (Buffers)

III. HYDROLOGY

METHODS

The methodology contained in this preliminary study is in compliance with the procedures presented in the *Manual and SWMM*. A hydrologic analysis was completed to estimate storm runoff from the proposed project. The drainage sheds and flow patterns for the undeveloped and developed conditions were determined from existing topography and the proposed preliminary grading plan. Both the Existing Shed Maps and Preliminary Developed Shed Maps can be seen in Appendix – D.

The SWMM's – Peak Flows from Small Watersheds was used to determine both the 10-year storm (for analysis of the local onsite storm drain system) and 100-year storm (for analysis of the overland flow routes). The associated detailed analysis, calculations, and estimates can be seen in Appendices – E & F.

EXISTING SITE CONDITIONS AND HYDROLOGY

Previously stated, the Project site encompasses 26.29± acres of undeveloped land and adjacent existing roadway improvements on Rocklin Road and Barton Road (See Appendix D – Existing Shed Maps). The property is located within the Town of Loomis and the County of Placer. Existing Conditions FEMA designation and Soil Group Classification are identified, as follows:

- FEMA National Flood Hazard Layer FIRMette Map, included in Appendix B, shows *The Project* is located within Non-Shaded Flood Zone "X" (Areas of Minimal Flood Hazard). There are no proposed building sites within a FEMA-designated Flood Zone or Special Flood Hazard Area (see Appendix B FEMA Map).
- Onsite soils belong to the Hydrologic Soil Group "B" (Reference 4, Soil Survey of Placer County, California – Western Part). An infiltration rate of 0.18 inches per hour for Natural Covers – Woodland, Grass – Fair Quality (per Table 5-3 of the SWMM) has been assumed for the site under existing conditions and underlying soils.

Native grasses, and heavily wooded rolling terrain comprise the majority of the site's ground cover. The existing drainage patterns are shown on the Existing Shed Maps (Appendix – D). The majority of the existing Project site, including adjacent Parcel 2, drain from the northeast to the southwest corner (denoted as Sheds X1-X11). The southeast corner of the Project site (Shed X13) drains from northwest to southeast and discharges offsite to the south along Barton Road. Additionally, (6) six offsite drainage sheds (denoted as Sheds O1-O2, O4-O7) contribute runoff to and through the Project site to the southwest corner. The southwest corner of the site (shown as Shed X1) is a large pond, containing water year round, receiving runoff from the Project site and adjacent properties to the south and west. Sheds X1-X13 and O1-O7 are more specifically described as follows:

SHED X1	The southwest corner of the Project site at 4.42± acres. Shed X1 is most defined as a year round pond, receiving runoff from the site and adjacent properties to the south and west. Stormwater collected in Shed X1 eventually discharges westerly (offsite) to natural conveyances of the Dry Creek watershed.
SHEDS X2 –X9 & X11	The majority of the Project site and adjacent Parcel 2 (APN: 045-161-034) totaling 33.87± acres. Generated runoff flows from the northeast to the southwest via natural drainage swales, entering the existing pond (Shed X1) in several locations.
SHEDS X8 & X10	Drainage sheds within the limits of the Project boundary and Parcel 2; however, located on the north side of Rocklin Road totaling 1.10± acres. Sheds X8 & X10 are characterized mostly by existing roadway runoff. Generated runoff is conveyed by existing roadside ditches to (2) two separate cross-culverts on Rocklin Road. Stormwater is then conveyed across Rocklin Road to the Project site and Parcel 2, combining with Sheds X2 –X9 & X11.
SHED X12	A small portion of Rocklin Road (north side) at 0.06± acres (2,500± SF) draining to an existing roadside ditch. Although technically within the Project's boundary, runoff from this small roadway shed, drain westerly down Rocklin Road to existing offsite drainage conveyances.
SHED X13	The southeast corner of the Project site at 0.99± acres. Shed X13 sheet flows east towards Barton Road and combines with offsite runoff from Shed O3. Runoff is conveyed south via existing AC curb along the west side of the road and discharges south along Barton Road.
SHEDS O1 & O2	Existing offsite drainage sheds contributing runoff to the Project site at 3.51± acres (See Offsite Existing Shed Map – EXH 2 – Appendix D). Existing Sheds O1 & O2 are comprised of large-lot single family properties. Runoff from these sheds combines with Onsite Sheds X2 & X3, respectively, prior to discharging the existing pond.
SHEDS 03 – 05	Existing offsite drainage sheds along Barton Road totaling 0.28± acres. Sheds O3 – O5 are the remainder area between the Project's boundary and roadway crown along Barton Road. Generated runoff from Sheds O3 – O5 sheet flows from the roadway crown to the west where it either continues to sheet flow to the Project site and Parcel 2 or is conveyed by AC curb along the west side of Barton Road. Runoff eventually combines with existing onsite Sheds.
SHEDS O6 & O7	Existing offsite drainage sheds on the north side of Rocklin Road totaling 11.60± acres. Sheds O6 & O7 generate runoff to the existing roadside ditches along the north side of Rocklin Road.

Runoff estimates for the Project's On-Site existing conditions and Offsite run-on were determined utilizing the *SWMM – Peak Flows from Small Watersheds*. Response times and associated runoff for the 10-year and 100-year event peak flows and supporting calculations are included in Appendix E - Table E1.

Table 1 below summarizes estimates for specific individual & cumulative shed storm runoff under existing conditions. Please refer to the detailed analysis performed in Appendix – E for supporting calculations.

	TABLE 1 EXISTING PEAK FLOW CHARACTERISTICS (10 & 100-YEAR)								
Location	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)							
Design Point #1 [SHED X1]	10.07	20.90							
Design Point #2 [SHEDS X2 + O1]	6.01	10.98							
Design Point #3 [SHED X13 + O3]	2.70	5.39							
Design Point #5 [SHEDS X3-X4, O2]	15.18	27.93							
Design Point #7 [SHEDS X5 + O4]	14.44	26.47							
Design Point #8 [SHED X6]	4.23	8.21							
Design Point #15 [SHEDS X7-X11, O5-O7]	53.72	96.80							
Design Point #16 [SHED X12]	0.15	0.30							

With respect to the existing peak flow runoff rates, shown in Table 1, above, please note the following:

- Attenuation of existing runoff from Offsite Sheds O6 & O7 and Onsite Sheds X8 & X10, due
 to the existing culverts in Rocklin Road is not accounted for at this time. Hydraulically,
 roadway cross-culverts will develop a certain amount of headwater on their upstream end
 to convey flow. This developed headwater condition will attenuate peak flows from the
 contributing sheds.
- 2. Attenuation of existing runoff from Onsite Sheds X2 X9 & X11, due to localized ponding is not accounted for at this time. Further evaluation of existing topography to determine localized low-points and/or depressions (if any) will be addressed at final design.

DEVELOPED SITE CONDITIONS & HYDROLOGY

The $SWMM-Peak\ Flows\ from\ Small\ Watersheds$ was used to determine the 10-year developed storm flows for the analysis of proposed storm drains and drainage swales. The same methodology was also utilized for the 100-year storm and used to evaluate overland release patterns. The proposed single-family subdivision portion of the Project site has been divided into several drainage sub-sheds shown on the Preliminary Developed Shed Maps (See Appendix – D) as Shed's A1 – A6, Shed B1 – B3 and Sheds C1 – C3, & C6. Existing Onsite Sheds (X1 – X12) were either replaced, modified or unchanged under proposed Developed Conditions.

The proposed sub-sheds, together, encompass the same total area of existing drainage Sheds X1 – X13 shown on the Existing Shed Maps (See Appendix – D). The smaller sub-sheds, under developed conditions, are used to determine 10-year peak flows rates to specific points of concentration (i.e. – drainage swales, vegetated swales, cross culverts, etc.). Sub-shed flows will be used in the hydraulic analysis of the proposed storm drain system under final design.

Similar to existing conditions, the proposed drainage patterns will drain from the northeast to southwest to Shed X1 (Existing Pond). However, in the developed condition the majority of the site that is composed of impervious surfaces will be conveyed to proposed roadside water quality swales (See Section V & Appendix - G). Additionally, developed runoff from impervious surfaces will be disconnected and flow across naturally and/or proposed landscape areas, prior to discharging to the Existing Pond. Runoff will be conveyed via paved surfaces, drainage swales, vegetated (water quality) swales and underground storm drain to the existing pond within the southwest corner of the Project site. The developed sub-sheds of Sheds A1 - A6, B1 - B3 and Sheds C1 - C3 & C6 are more specifically described, as follows:

SHEDS A1 – A6	Onsite developed Sheds totaling 9.27 \pm acres located on the southwest side of the proposed cul-de-sac. These Sheds are comprised primarily of residential homes construction and areas intentionally left undisturbed. Shed A1 $-$ A6 drain directly to the existing pond (Shed X1). Prior to discharging to the pond, developed runoff will drain over/through natural vegetation providing water quality treatment.
SHEDS B1 – B3	Onsite Developed Sheds south of Parcel 2, totaling 5.08± acres. These sheds are comprised five residential lots, roadway and vegetated roadside swales on the south side of the road. Shed B3 drains to a proposed swale between two residential lots. Sheds B1 – B2 drain to the proposed vegetated swales along the south side of the road. The proposed vegetated swale provides water quality treatment. From the swales, stormwater is conveyed to a downstream drainage swale via proposed storm drain, beyond which it ultimately discharges to the existing pond (Shed X1).
SHEDS C1 – C3, C6	Onsite Developed Sheds adjacent to and uphill of the proposed cul-de-sac, totaling 5.88± acres. These Sheds are comprised primarily of residential home construction and areas intentionally left undisturbed. Sheds C2 – C3 also include portions of the proposed cul-de-sac and vegetated roadside swales.

Shed C6 is comprised primarily of the required roadway widening improvements (i.e. – AC pavement, Concrete Curb, Gutter & Sidewalk, etc.) on Rocklin Road. Sheds C2 – C3 drain to the proposed water quality (vegetated) swales along the south side of the cul-de-sac. Sheds C1 drains to the drainage swale running down the middle of the shed. The proposed vegetated swales provide water quality and convey stormwater to downstream drainage swales, eventually discharging to the existing pond (Shed X1).

It should be noted that specific shed descriptions for Sheds X1, X5, X7 - X10, X11-A, X11-B, X12 and Sheds O1 - O7 have not been provided above, for the proposed Developed Condition. These sheds are similar in nature (if not exactly the same) as Existing Conditions. Sheds X2 - X4 and X13 are replaced by the proposed developed Sheds. Shed X5 is reduced under developed conditions. Sheds X11-A & X11-B are a result of the proposed development. These existing sheds drain through the proposed residential subdivision and discharge to the existing pond, similar to existing conditions.

Response times and associated runoff for the 10-year and 100-year event peak flow and supporting calculations are included in Appendix E - Tables E2 - E5. Table 2 below summarizes estimates for specific individual & cumulative shed storm runoff under developed conditions.

	TABLE 2 PROPOSED PEAK FLOW CHARACTERISTICS (10 & 100-YEAR)									
Location	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)								
Design Point #1 [SHED X1]	10.07	20.90								
Design Point #2 [SHED A1-A2, O1-O2]	15.69	28.89								
Design Point #3 [SHED O3 + O3-A]	1.57	3.26								
Design Point #7 [SHEDS A3, B1-B5, C1-C3, X5]	14.36	25.95								
Design Point #8 [SHED A4]	3.20	6.30								
Design Point #15 [SHEDS A5, C4-C6, X7-X11, O5-O7]	50.88	92.54								
Design Point #15A [SHED A6]	2.64	4.73								
Design Point #16 [SHED X12]	0.15	0.30								

IV. HYDRAULICS

STORM DRAIN SYSTEM

The proposed site layout and grading were used in determining the drainage swale locations, cross-culvert size, location, slopes, and inverts in accordance with the *Manual*. Stormwater flows from the sub-shed areas described above will be evaluated using Bentley StormCAD software, under final project design and analyzed for swale & culvert capacity and hydraulic grade line (10-Year Storm Event).

Based on the estimated Design Runoff (10-yr) and the associated hydraulic characteristics within the proposed drainage system, the analysis will verify that the swales and cross-culverts have adequate capacity to convey Design Runoff (10-yr). During final design the StormCAD Hydraulic Reports (Appendix – F) will show that HGL's within the onsite drainage system will maintain freeboard requirements of *Manual* – Section 11.

DETENTION SYSTEM

Under preliminary design, no detention or retention of storm water flows is proposed for this site. This report did however analyze the pre- and post-developed condition volumetric runoff to confirm the existing pond can adequately accommodate the developed stormwater. The Pond Volume Calculations are provided in Appendix E. The volumetric runoff is summarized in Table 3 below:

TAE	BLE 3													
VOLUMETRIC RUNOFF SUMMARY														
Existing Condition, CF	Developed Condition, CF													
205,927	243,252													

Based on the analysis, there is an increase of 37,325 CF of runoff volume in the developed condition. This increase in volume equates to an increase of 1.57 inches in depth when spread over the surface area of the existing pond. As the increase in pond depth is minimal, it is determined that the basin can accommodate the developed condition runoff.

V. STORM WATER QUALITY

The Project is required to implement Low Impact Development (LID) standards designed to reduce runoff, treat stormwater, and provide baseline hydromodification management (if required) per the *Manual* and *LID Manual*. "LID is a design strategy where storm water runoff is treated as a valuable resource that can recharge groundwater supplies, protect and enhance natural habitat and biodiversity, and add value to new development or redevelopment projects" *LID Manual*.

The Project design includes the use of four (4) Water Quality Swales (Swales) as Site Design Measures to treat and effectively reduce runoff from the impervious surfaces within DMA's 1 & 4 - 6 (See Preliminary Developed DMA SWQ Map - Appendix G). Naturally and/or Developed vegetated areas (Rooftop and Impervious Area Disconnection) and Existing Tree Canopy Preservation are proposed to treat and effectively reduce runoff from the impervious surfaces of DMA's 2 - 3, & 8 - 13. The LID Manual's template for Post-Construction Storm Water Quality Plans was utilized to determine the effectiveness for water quality treatment and runoff reduction of the proposed Swales, Rooftop & Impervious Area Disconnection and Existing Tree Canopy Preservation. (See Appendix G – LID Storm Water Quality). The Project's proposed drainage sheds were analyzed through the template for their impervious and pervious compositions (Each DMA's Surface Data is also provided on Preliminary Developed DMA SWQ Map - Appendix G). Within the template, drainage sheds are identified as Drainage Management Areas (DMA's) and are designated accordingly in the Preliminary Developed DMA SWQ Map (See Appendix - G). LID measures as outlined in the LID Manual were applied to the proposed drainage sheds or DMA's where appropriate. The Project utilizes Site Design Measures as Existing Tree Canopy Preservation, Rooftop and Impervious Area Disconnection and Vegetated Swales within individual drainage sheds (DMA's) prior to reaching the existing pond in the southwest corner of the Project site.

Runoff is treated/reduced from DMA's 1 & 4 - 6 via Existing Tree Canopy Preservation and Vegetated Swales. Per template Forms 3-4 & 3-5 each DMA's impervious surfaces are treated by Site Design Measures. Vegetated swale calculations are provided in Appendix G verifying minimum retention time (10 minutes) and maximum velocity (1 ft/s) requirements are met. Stormwater flow rates above the 2-year, 24-hour storm event are to be conveyed through the *Swales* and storm drain to the existing pond.

Runoff from rooftops and impervious surfaces within DMA's 2 - 3 & 8 - 13 is proposed to be disconnected. To achieve this, naturally and/or proposed vegetated areas will be utilized to capture the runoff volume from the 85th percentile, 24-hour storm event. Per the template's Form 3-4 the required runoff volume to be captured (after taking into consideration Existing Tree Canopy Preservation) is:

DMA 2 = 495-cubic feet DMA 3 = 0-cubic feet DMA 8 = 650-cubic feet DMA 9 = 0-cubic feet **DMA 10** = 400-cubic feet **DMA 11** = 722-cubic feet **DMA 12** = 578-cubic feet = 387-cubic feet **DMA 13**

Per the *LID Manual*, disconnected rooftops and impervious surfaces should be directed to established vegetated areas via splash pads, bubble-up emitters and/or sheet flow. Landscaped and/or naturally vegetated areas receiving runoff from rooftops and imperious surfaces should be

sized on an impervious:pervious ratio of 2:1. Under final design, this area ratio will be verified for DMA's 2-3 & 8-13 to effectively treat the runoff volumes listed above.

Results of the *LID Manual's* template are provided in Appendix G.

VI. CONCLUSIONS

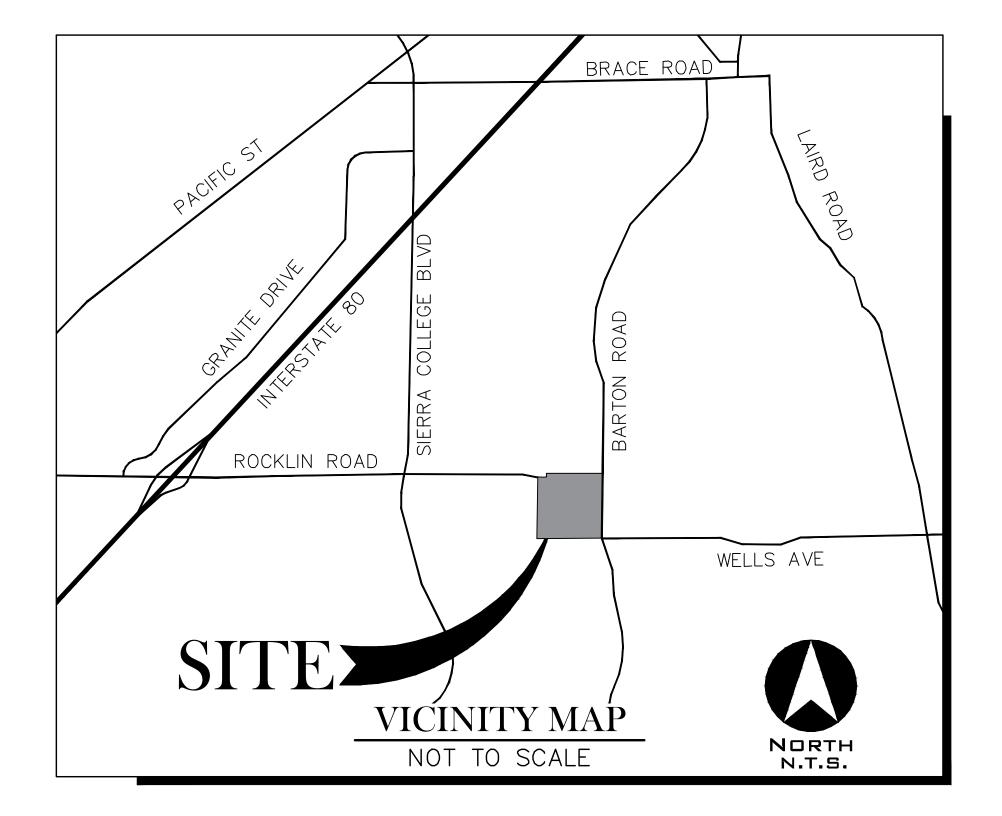
- 1. The methods used to calculate storm runoff for this project are in compliance with the *Manual and SWMM*.
- 2. The proposed Project lies outside of a FEMA-designated Flood Zone or Special Flood Hazard Area.
- 3. 10-Year storm flows generated from the site are conveyed through onsite swales, vegetated swales and underground storm drain.
- 4. Onsite storm runoff from impervious surfaces is reduced through proposed Site Design Measures consisting of Existing Tree Canopy Preservation, Disconnected Rooftops and impervious surfaces, and Vegetated Swales, providing permanent BMP facilities and LID measures.
- 5. The proposed Site Design Measures will provide the required water quality treatment and runoff reduction needed for the proposed Reserve Residential subdivision.

VII. REFERENCES

- 1. Town of Loomis. Town of Loomis Land Development Manual. March, 2004
- 2. Placer County Flood Control and Water Conservation District. *Stormwater Management Manual*. September 1, 1990.
- 3. West Placer. Storm Water Quality Design Manual. April 2016.
- 4. Natural Resources Conservation Service. *Soil Survey of Placer County, California Western Part*. Survey Area Data: Version 15, August 31, 2023.

APPENDIX A - VICINITY MAP

Vicinity Map



APPENDIX B - FEMA MAP

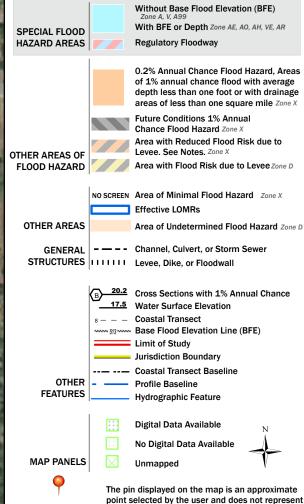
FEMA Map

National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 7/10/2024 at 3:30 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

an authoritative property location.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



APPENDIX C - TSM & WEB SOIL SURVEY

Tentative Subdivision Map
Web Soil Survey Data

TENTATIVE SUBDIVISION MAP

OWNER/APPLICANT

8483 DOUGLAS PLAZA DR, SUITE 110 GRANITE BAY, CA 95746 ATTN: STEFAN HORSTSCHRAER EMAIL: stefan@premierhomesca.com

785 ORCHARD DRIVE, SUITE 110

SOUTH PLACER MUNICIPAL UTILITY DISTRICT (SPMUD)

PLACER COUNTY WATER AGENCY (PCWA)

ELEM. SCHOOL DISTRICT

ZONE X (AREAS DETERMINED TO BE OUTSIDE

THE 0.2% ANNUAL CHANCE FLOODPLAIN) PER THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP, MAP NO. 06061C0962H, DATED NOVEMBER 2, 2018.

THE BASIS OF BEARINGS FOR THIS MAP IS THE NORTHERLY LINE OF LOTS 43, 44, & 45, AS SAID LOTS ARE SHOWN ON THE AMENDED PLAT OF "ST. FRANCIS WOODS", FILED ON AUGUST 14, 1991, IN BOOK R OF MAPS, AT

ROCKLIN ROAD WELLS AVE VICINITY MAI

INDEX OF DRAWINGS PAGE TITLE C-1 TENTATIVE SUBDIVISION MAP

C-8 PRELIMINARY SECTIONS

C-9 PRELIMINARY STRIPING PLAN

PROJECT SUMMARY

EXISTING ZONING RR - RURAL RESIDENTIAL PROPOSED ZONING 5780 ROCKLIN RD RR - RURAL RESIDENTIAL RESIDENTIAL LOTS - 20 LOTS SINGLE FAMILY, AGRICULTURAL MINIMUM: 40.000 SF MAXIMUM: 136,612 SF SINGLE FAMILY RESIDENTIAL, 20 LOTS AVERAGE: 54,628 SF RR - RURAL RESIDENTIAL GROSS: 26.29± AC (1,145,269 SF) R/W: $1.21 \pm AC (52,702 \text{ SF})$

NET: 25.08± AC (1,092,567 SF)

DEVELOPMENT	STANDARDS (RR)
CRITERIA	STANDARD
NET LOT AREA (SF)	40,000
MIN. LOT DEPTH (FT)	100
MIN. LOT WIDTH (FT)	135
MAX LOT COVERAGE (%)	20
FRONT SETBACK (FT)	75 AVG. (50 MIN.)
SIDE SETBACK (FT)	20
REAR SETBACK (FT)	20
MAX HFIGHT (FT)	35

FRONT SETBACK VARIANCE

THE COMBINED FRONT SETBACK FOR LOTS WITHIN THE COMMUNITY SHALL BE AN AVERAGE OF 75' OR MORE FROM THE CENTERLINE OF THE ROADWAY. IN NO EVENT SHALL THE FRONT SETBACK OF A LOT BE LESS THAN 50' FROM THE CENTERLINE OF THE ROADWAY. FRONT SETBACKS OF LESS THAN 75' SHALL BE GRANTED WHERE BENEFICIAL FOR THE PROTECTION OF NATURAL RESOURCES LIKE WETLANDS, TREES, ROCK OUTCROPPINGS AND OTHER DESIRABLE NATURAL FEATURES CURRENTLY EXISTING ON

TOWN OF LOOMIS

BENCHMARK - L36

341.89 FEET BEING THE NORTHWEST BOLT ON A TRAFFIC SIGNAL LIGHT STAND FOR THE SIGNAL LIGHT ARM OVER SIERRA COLLEGE BLVD., AT THE NORTHEAST CORNER OF THE INTERSECTION OF SIERRA

COLLEGE BLVD. AND ROCKLIN RD. (NGVD29 DATUM)

SW Corner of Rocklin Road and Barton Road Loomis, California

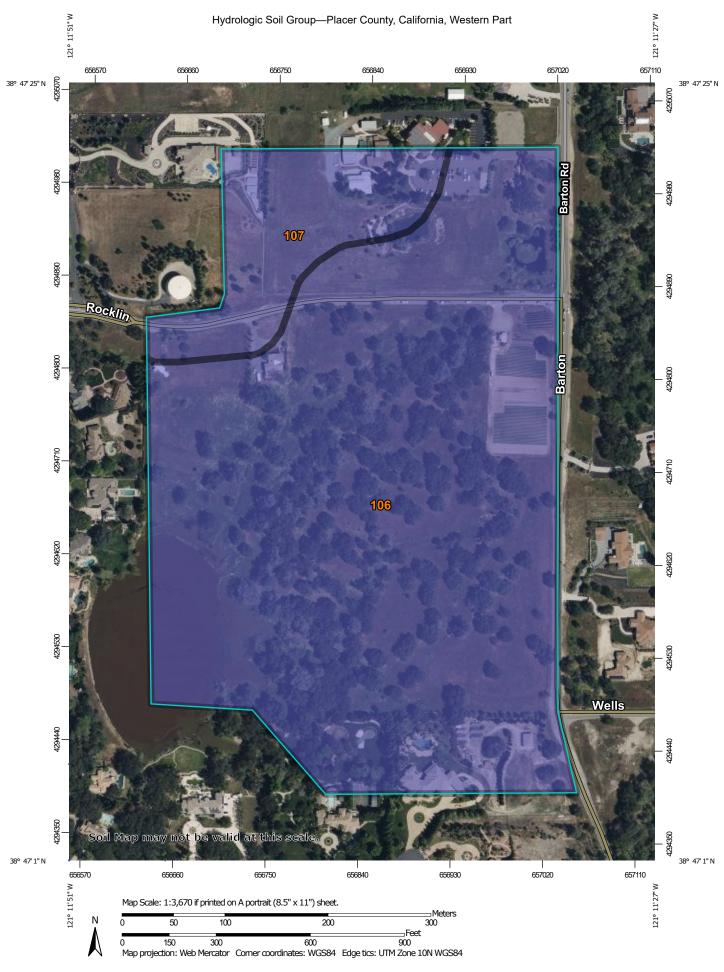
Proposed

PREMIER HOMES 8483 Douglas Plaza Dr Granite Bay, CA 95746

JULY, 2025 **3RD SUBMITTAL**



785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:24.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: Placer County, California, Western Part Survey Area Data: Version 15, Aug 31, 2023 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Apr 23, 2022—Apr 24. 2022 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

		,		
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
106	Andregg coarse sandy loam, 2 to 9 percent slopes	В	48.9	86.9%
107	Andregg coarse sandy loam, 9 to 15 percent slopes	В	7.4	13.1%
Totals for Area of Inter-	est		56.3	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

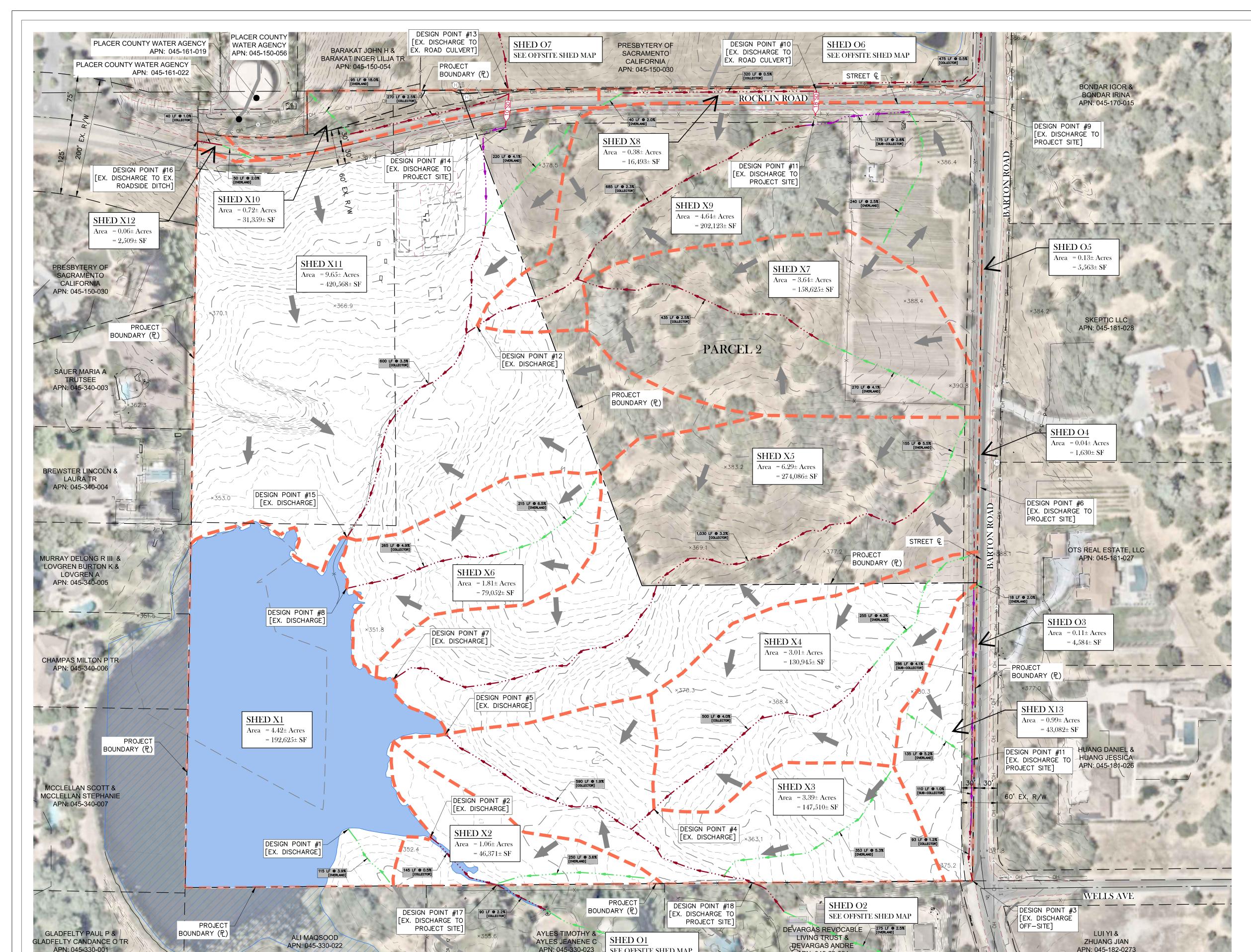
APPENDIX D - SHED MAPS

EXH 1 – Existing Shed Map (Onsite)

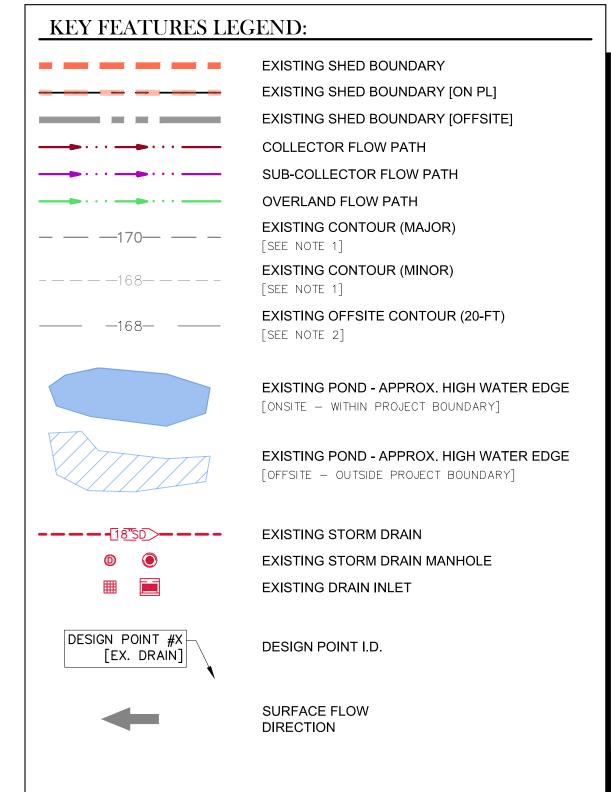
EXH 2 – Existing Shed Map (Offsite)

EXH 3 – Developed Shed Map (Onsite)

EXH 4 – Developed Shed Map (Offsite)

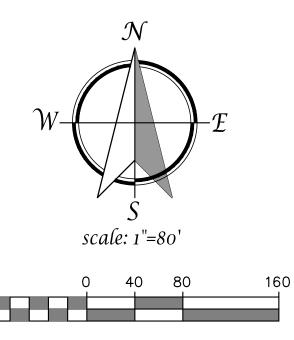


Existing Shed Map (Onsite)



GENERAL NOTES:

- EXISTING GROUND CONTOURS (5-FT MAJOR & 1-FT MINOR) BASED AERIAL TOPOGRAPHIC SURVEY PREPARED BY TSD ENGINEERING, INC. - DATE OF SURVEY: APRIL 2024.
- 2. EXISTING GROUND CONTOURS (20—FT) BASED ON PLACER COUNTY OPEN DATA (GIS CONTOUR DATA) - UPDATED APRIL 18, 2023.





PREMIER HOMES 8483 Douglas Plaza Dr Granite Bay, CA 95746

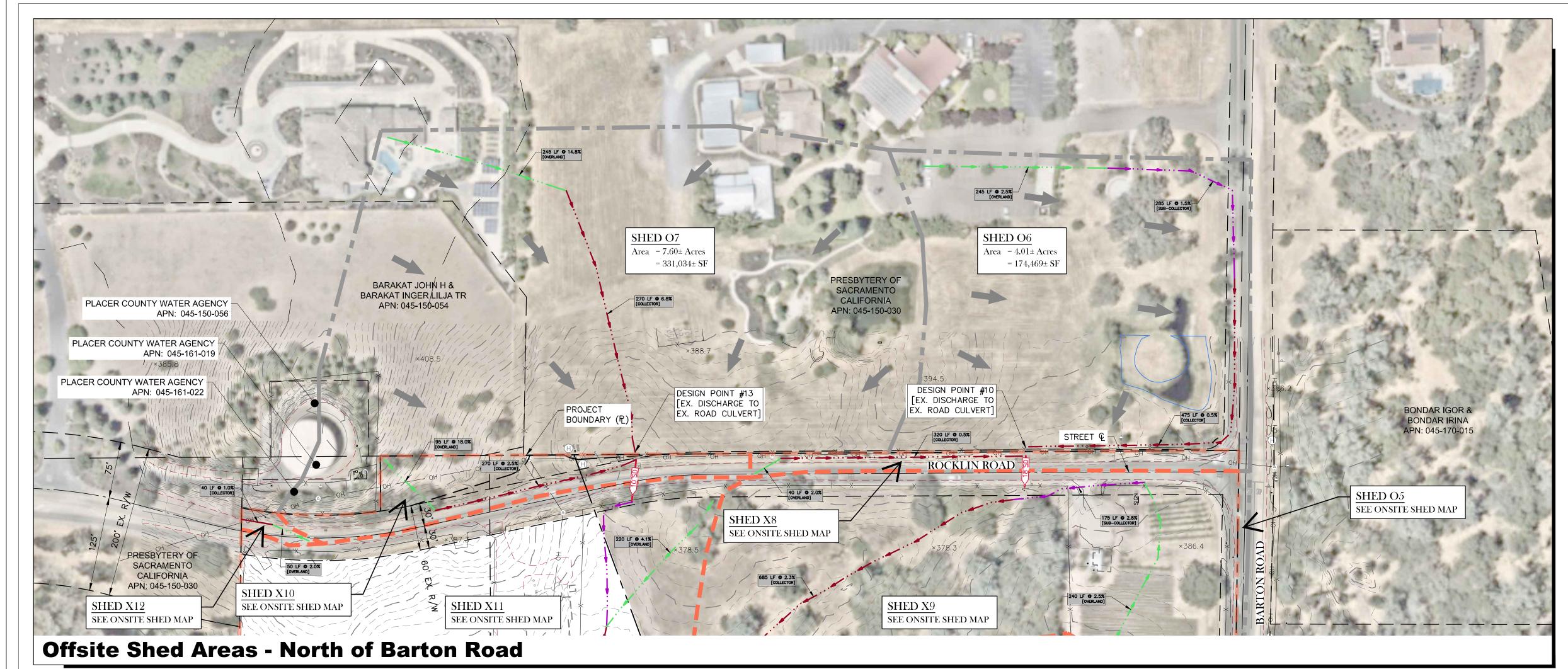
APN: 045-182-0273

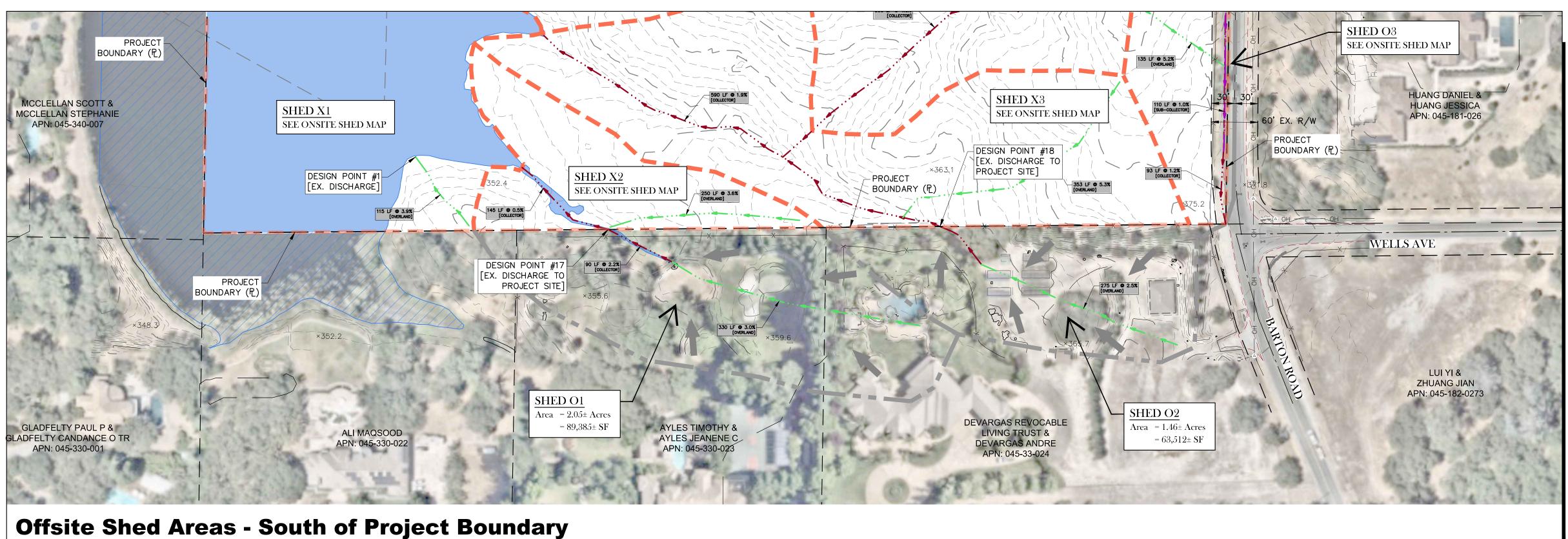
APRIL 1, 2025



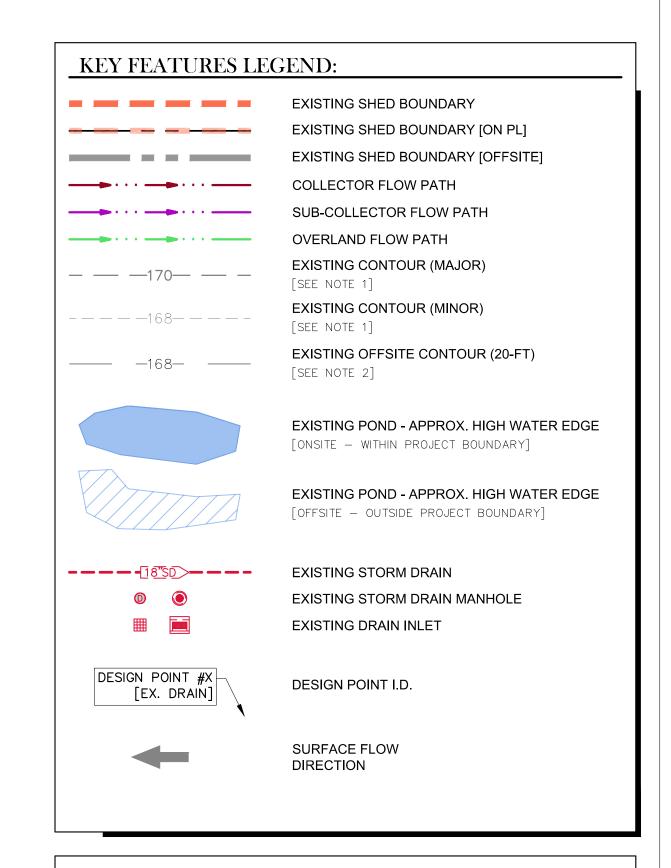
785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701

EXH-I



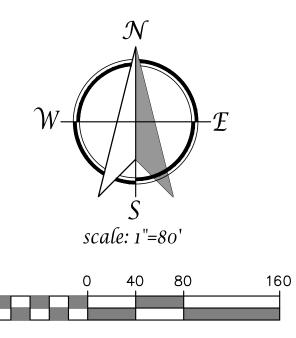


Existing Shed Map (Offsite)



GENERAL NOTES:

- EXISTING GROUND CONTOURS (5-FT MAJOR & 1-FT MINOR) BASED AERIAL TOPOGRAPHIC SURVEY PREPARED BY TSD ENGINEERING, INC. - DATE OF SURVEY: APRIL 2024.
- EXISTING GROUND CONTOURS (20-FT) BASED ON PLACER COUNTY OPEN DATA (GIS CONTOUR DATA) - UPDATED APRIL 18, 2023.





EXH-2

SW Corner of Rocklin Road and Barton Road Loomis, California

Proposed

PREMIER HOMES 8483 Douglas Plaza Dr Granite Bay, CA 95746

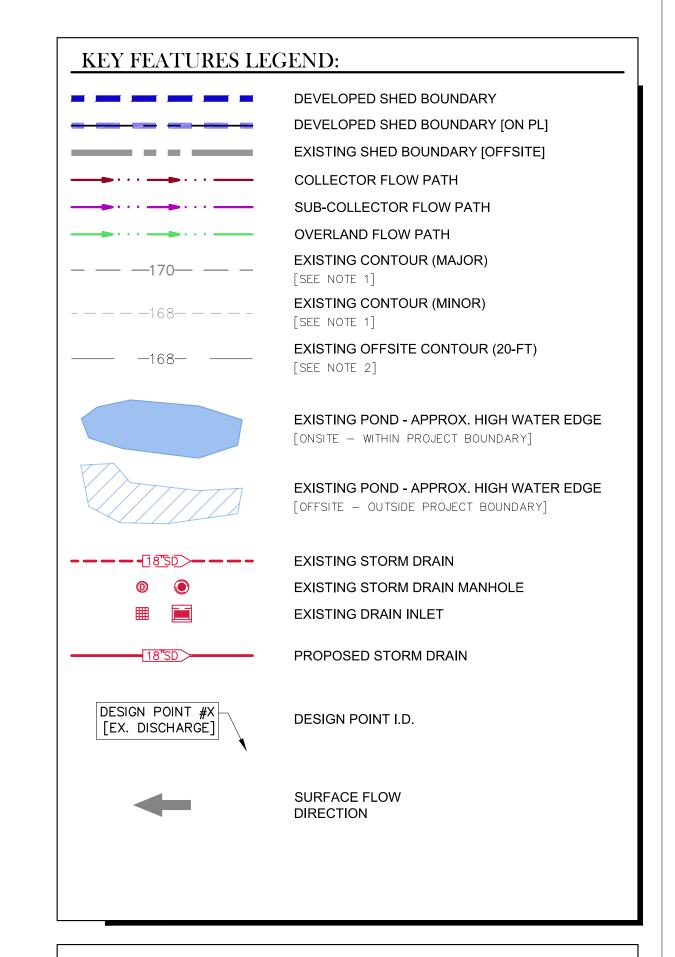
APRIL 1, 2025 2ND SUBMITTAL

785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701

AYLES JEANENE C

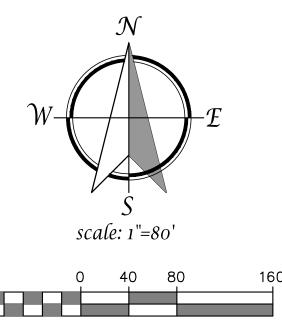
APN: 045-330-023

Developed Shed Map (Offsite)



GENERAL NOTES:

- . EXISTING GROUND CONTOURS (5-FT MAJOR & 1-FT MINOR) BASED - DATE OF SURVEY: APRIL 2024.
- 2. EXISTING GROUND CONTOURS (20-FT) BASED ON PLACER COUNTY OPEN DATA (GIS CONTOUR DATA) - UPDATED APRIL 18, 2023.





785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701

SW Corner of Rocklin Road and Barton Road Loomis, California

Offsite Shed Areas - South of Project Boundary

APN: 045-330-022

GLADFELTY CANDANCE O TR

APN: 045-330-001

Proposed

 $=63,512\pm$ SF

DEVARGAS ANDRE

APN: 045-33-024

PREMIER HOMES 8483 Douglas Plaza Dr Granite Bay, CA 95746

JULY 16, 2025 **3RD SUBMITTAL**

EXH-4

APPENDIX E - HYDROLOGY

Table E1 – Existing Conditions – Shed Data

Table E2 - Proposed Conditions - Shed "A" Data

Table E3 - Proposed Conditions - Shed "B" Data

Table E4 - Proposed Conditions - Shed "C" Data

Table E5 - Proposed Conditions - Shed "X" + Cumulative Shed Data

Pond Volume Calculations

THE RESERVE Date: 27-Sep-24 Revised: 2-Apr-25 TABLE E1 - EXISTING CONDITIONS - SHED DATA By: TNJ By: JM

		SHED CHARACTERISTICS											RESPONSE TIME					DISCHARGE RATE							
Drainage Shed I.D.	Shed Area	51. 5	6.117		Cover	Constant Loss Rate	Impervious	Slope	Travel Length	Manning's		Overland Flow Response Time	Collector Response Time	Collector Tributary Area	Total Response Time ¹	51 ti (6)		Unit Peak Discharge q100	Infiltration Factor Fi	Peak Flow					
[Design Point #]	(acres)	Flow Type	Soil Type	Ground Cover	Condition	(in/hr)	%	(Ft/Ft)	(ft)	n	(ft/ft)	(minutes)	(minutes)	(acres)	(minutes)	Elevation (ft)	(cfs/ac)	(cfs/ac)	(cfs/ac)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)				
SHED X1	4.42	Overland	В	Woodland,Grass Graded Swale	Fair	0.18	90	0.039 0.000	115 0	0.150 0.000	0.00	5.19	0.00	0.00	5	370	2.55	5.00	0.30	10.07	20.90				
[EXISTING POND]				Graded Swale				0.000		0.000	0.00	5.19	0.00	0.00											
																		, ,		1					
SHED X2	1.06	Overland	В	Woodland, Grass	Fair	0.18	5	0.036 0.005	250 145	0.150 0.040	5.00	8.47	1.03	1.06	10	370	2.10	3.85	0.30	2.21	4.07				
[DESIGN POINT #2]		Collector		Natural Swale				0.005	143	0.040	5.00	8.47	1.03	1.00											
[525.5.1.1.5.1.1.5.2]						l						<u> </u>													
SHED O1	2.05	Overland	В	Woodland, Grass	Fair	0.18	6	0.030	330	0.150		10.56			11	370	2.00	3.80	0.30	4.06	7.75				
		Collector		Natural Swale				0.022	90	0.035	5.00		0.28	2.05											
[DESIGN POINT #17]												10.56	0.28												
	3.11	Overland	В	Woodland,Grass	Fair	0.18	6	0.030	330	0.150		10.56			12	370	1.95	3.55	0.30	6.01	10.98				
SHEDS X2 + O1	0,11	Collector		Natural Swale	1 0.11	0.10	, i	0.022	90	0.035	5.00	20.00	0.28	2.05		3,0	1.33	3.33	0.30	0.02	10.50				
		Collector		Natural Swale				0.005	145	0.040	5.00		0.79	3.11											
[DESIGN POINT #2]												10.56	1.07												
	2.20	0 1 1	Т Б		F	1 040		0.052	252	0.150		0.27		T		270	2.20	1 420 [
SHED X3	3.39	Overland Collector	В	Woodland,Grass Natural Swale	Fair	0.18	5	0.053 0.019	353 590	0.150 0.035	5.00	9.27	1.73	3.39	11	370	2.30	4.20	0.30	7.75	14.19				
[DESIGN POINT #5]		Concetor		Natural Swale				0.013	330	0.000	3.00	9.27	1.73	0.00			<u> </u>								
	Leases			•		•								•	•	1									
SHED X4	3.01	Overland	В	Woodland, Grass	Fair	0.18	5	0.043	255	0.150		8.12			9	370	2.20	3.90	0.30	6.58	11.69				
		Collector		Natural Swale				0.040	500	0.035	5.00	242	1.14	3.01											
[DESIGN POINT #4]												8.12	1.14												
	1.46	Overland	В	Woodland,Grass	Fair	0.18	10	0.025	275	0.150		10.00		Τ	10	370	2.10	3.85	0.30	3.02	5.58				
SHED O2		Collector	_	Natural Swale		0.20		0.019	90	0.035	10.00		0.38	1.46											
[DESIGN POINT #18]												10.00	0.38												
			T -	I	T	1			075	0.450		10.00		T	1	T		1 T							
SHEDS	7.97	Overland Collector	В	Woodland, Grass	Fair	0.18	15	0.025 0.019	275 90	0.150 0.035	10.00	10.00	0.38	1.46	12	370	1.95	3.55	0.30	15.18	27.93				
X3-X4 + O2		Collector		Natural Swale Natural Swale				0.019	590	0.035	5.00		1.39	7.97											
[DESIGN POINT #5]												10.00	1.78												
SHED O3	0.11	Overland	В	AC Pavement	Fair	0.06	98	0.020	18	0.110	50.00	1.73			3	370	2.55	5.00	0.10	0.27	0.54				
[DESIGN POINT #11]		Sub-Collector		AC Curb				0.041	286	0.015	50.00	1.73	1.38 1.38	0.11							+				
[DESIGN FOINT #11]												1.75	1.36												
CUED V42	0.99	Overland	В	Woodland, Grass	Fair	0.18	5	0.052	135	0.150		5.24			5	370	2.55	5.00	0.30	2.51	4.94				
SHED X13				AC Curb				0.000	0	0.000	0.00		0.00	0.00											
[DESIGN POINT #11]												5.24	0.00												
	1.10	Overland	В	AC Pavement	Fair	0.06	98	0.020	18	0.110		1.73		T	2	370	2.55	5.00	0.10	2.70	5.39				
SHED X13 + O3	1.10	Sub-Collector		AC Pavement AC Curb	Fail	0.00	30	0.020	286	0.110	50.00	1.75	1.38	0.11	L	3/0	2.33	3.00	0.10	2.70	3.33				
		Sub-Collector		AC Curb				0.010	110	0.015	50.00		0.51	1.10											
		Collector		Natural Swale				0.012	93	0.035	5.00			1.10											
[DESIGN POINT #3]												1.73	0.51												
	1			T 11		1				0.450					T -	T				T					
SHED X5	6.29	Overland	В	Woodland, Grass	Fair	0.18	5	0.055	155 1030	0.150 0.035	5.00	5.60	2.12	6.29	8	370	2.30	4.20	0.30	14.37	26.32				
[DESIGN POINT #7]		Collector	+	Natural Swale		 		0.032	1030	0.033	5.00	5.60	2.12 2.12	0.29											
[520.0.110.111.117]				1		1																			
SHED O4	0.04	Overland	В	AC Pavement	Fair	0.06	98	0.020	18	0.110		1.73			3	370	2.55	5.00	0.10	0.10	0.20				
		Collector		AC Curb				0.030	120	0.015	50.00		0.84	0.04											
[DESIGN POINT #6]												1.73	0.84												
SHEDS	6.33	Overland	В	Woodland, Grass	Fair	0.18	6	0.055	155	0.150		5.60			8	370	2.30	4.20	0.30	14.44	26.47				
X5 + O4	0.33	Collector		Natural Swale	1 011	0.10	 	0.033	1030	0.035	5.00		2.12	6.33	•	370	2.30	7.20	0.50	17.77	20.77				
						1	l					5.60	2.12					1							

THE RESERVE
TABLE E1 - EXISTING CONDITIONS - SHED DATA

Date: 27-Sep-24

Revised: 2-Apr-25

By: TNJ

By: JM

					SHED	CHARACTERISTICS	5						RESPON	SE TIME				DISCHARG	E RATE		
Drainage Shed I.D. [Design Point #]	Shed Area (acres)	Flow Type	Soil Type	Ground Cover	Cover Condition	Constant Loss Rate (in/hr)	Impervious %	Slope (Ft/Ft)	Travel Length (ft)	Manning's	Side Slope (Z) (ft/ft)	Overland Flow Response Time (minutes)	Collector Response Time (minutes)	Collector Tributary Area (acres)	Total Response Time ¹ (minutes)	Elevation (ft)	Unit Peak Discharge q10 (cfs/ac)	Unit Peak Discharge q100 (cfs/ac)	Infiltration Factor Fi (cfs/ac)	Peak Flow Q ₁₀ (cfs)	Peak Flo
SUED VS	1.81	Overland	В	Woodland,Grass	Fair	0.18	5	0.065	215	0.150		6.48			7	370	2.35	4.55	0.30	4.23	8.21
SHED X6		Collector		Natural Swale				0.049	265	0.035	5.00		0.64	1.81							
[DESIGN POINT #8]												6.48	0.64								
011ED 1/E	3.64	Overland	В	Woodland,Grass	Fair	0.18	5	0.041	270	0.150		8.53			10	370	2.10	3.85	0.30	7.59	13.96
SHED X7		Collector		Natural Swale			-	0.025	435	0.035	5.00		1.13	3.64	-		_				
[DESIGN POINT #]												8.53	1.13								
	0.38	Overland	l B	AC Pavement	Fair	0.06	50	0.020	40	0.110		2.79	Π	I	5	370	2.55	5.00	0.10	0.95	1.88
SHED X8	0.38	Collector		Graded Swale	I dii	0.00	30	0.020	320	0.030	3.00	2.75	2.13	0.38		370	2.33	3.00	0.10	0.53	1.00
[DESIGN POINT #10]		Concetor		Graded Sware				0.003	020	0.000	0.00	2.79	2.13	0.50							
	Suntananananananan																				
CUED VO	4.64	Overland	В	Woodland,Grass	Fair	0.18	5	0.025	240	0.150	2.00	9.22	0.42	4.50	11	370	2.00	3.80	0.30	9.21	17.56
SHED X9		Sub-Collector		Graded Swale				0.028	175	0.030	3.00		0.43	1.50							
[DESIGN POINT #12]		Collector	+	Natural Swale				0.023	685	0.035	5.00	9.22	1.72 2.16	4.64							
[DESIGN FORNT #12]												3.22	2.10								
CUED V10	0.72	Overland	В	Woodland, Grass	Fair	0.18	45	0.180	95	0.150		2.92			4	370	2.55	5.00	0.30	1.74	3.50
SHED X10		Collector		Graded Swale				0.025	270	0.030	3.00		0.84	0.72							
[DESIGN POINT #13]												2.92	0.84								
	0.05	0 1 1			T 5.1.	0.40	_	0.044	220	1 0.150		7.54		1		270	2.20	2.00	0.20	24.00	27.40
SHED X11	9.65	Overland Collector	В	Woodland, Grass	Fair	0.18	5	0.041	220 600	0.150 0.035	5.00	7.54	1.10	9.65	9	370	2.20	3.90	0.30	21.08	37.49
[DESIGN POINT #15]		Collector		Natural Swale				0.033	000	0.055	5.00	7.54	1.10	9.03							
[82816111 61111 1125]												7.5									
SHED X12	0.06	Overland	В	AC Pavement	Fair	0.06	60	0.020	50	0.110		3.19			4	370	2.55	5.00	0.10	0.15	0.30
SUED VIS		Collector		Graded Swale				0.010	40	0.030	3.00		0.33	0.06							
[DESIGN POINT #16]												3.19	0.33								
	0.13	Overland	В	AC Davament	Fair	0.06	98	0.020	18	0.110		1.73	Τ	ı	5	370	2.55	5.00	0.10	0.32	0.64
SHED O5	0.13	Overland Collector	В	AC Pavement AC Curb	FdII	0.06	98	0.020	490	0.110	50.00	1.73	3.59	0.13	3	370	2.55	5.00	0.10	0.32	0.64
[DESIGN POINT #9]		Concetor		AC CUID				0.012	130	0.013	30.00	1.73	3.59	0.13							
	4.01	Overland	В	Woodland, Grass	Fair	0.18	15	0.025	245	0.150		9.33			12	370	1.95	3.55	0.30	7.64	14.05
SHED O6		Sub-Collector		Graded Swale				0.015	285	0.030	10.00		1.30	1.00							
[DECICAL DOINT #40]		Collector		Graded Swale				0.005	475	0.030	3.00	9.33	1.75 3.05	4.01							
[DESIGN POINT #10]												9.33	3.05								
	7.60	Overland	В	Woodland, Grass	Fair	0.18	15	0.148	245	0.150		5.47		I	6	370	2.40	4.80	0.30	17.90	36.14
SHED O7		Collector		Natural Swale				0.068	270	0.030	10.00		0.42	7.60							
[DESIGN POINT #13]												5.47	0.42								
	40.00			L 144 . 11 . 1 . 2	T			0.00-	245	0.450		0.33				1 275		1 222	2.25	1	
SHEDS	12.80	Overland	В	Woodland, Grass	Fair	0.18	18	0.025	245 285	0.150 0.030	10.00	9.33	1.30	1.00	14	370	1.80	3.20	0.30	22.35	40.27
X7-X9 + O5-O6		Sub-Collector Collector	+	Graded Swale Graded Swale				0.015 0.005	475	0.030	3.00		1.30	4.01							
71-V2 ± C2-O0		Collector		Natural Swale				0.003	685	0.035	5.00		1.34	12.80							
[DESIGN POINT #12]												9.33	4.39								
	30.77	Overland	В	Woodland,Grass	Fair	0.18	18	0.025	245	0.150	40.00	9.33	4.22	4.00	14	370	1.80	3.20	0.30	53.72	96.80
SHEDS		Sub-Collector		Graded Swale	-			0.015	285 475	0.030	10.00		1.30 1.75	1.00							
X7-X11 + O5-O7		Collector Collector		Graded Swale Natural Swale				0.005 0.023	475 685	0.030 0.035	3.00 5.00		1.75	4.01 12.80							
		Collector	 	Natural Swale				0.023	415	0.035	5.00		0.57	30.77							
ľ													1							90 . 996.6666666666666666	

Notes:

^{1.} Minimum response time of 5 minutes is used for all response times calculated to be less than 5 minutes.

Date: 27-Sep-24

Revised: 16-Jul-25

	-		== "" -								Dute.				10 301 23							
TABLE E2 - PROP	USED CO	NDITIONS - S	SHED "A" [DATA							Ву:	TNJ			JM							
					SHED	CHARACTERISTICS	3						RESPON				DISCHARGE RATE					
	Shed					Constant Loss			Travel		Side Slope	Overland Flow	Collector	Collector Tributary	Total Response		Unit Peak	Unit Peak	Infiltration			
Drainage Shed I.D.	Area				Cover	Rate	Impervious	Slope	Length	Manning's	(Z)	Response Time	Response Time	Area	Time ¹			Discharge q100	Factor Fi	Peak Flow	Peak Flow	
[Design Point #]	(acres)	Flow Type	Soil Type	Ground Cover	Condition	(in/hr)	%	(Ft/Ft)	(ft)	n	(ft/ft)	(minutes)	(minutes)	(acres)	(minutes)	Elevation (ft)	(cfs/ac)	(cfs/ac)	(cfs/ac)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	
SHED A1	1.52	Overland	В	Residential	Fair	0.18	26	0.010	180	0.150		10.21			11	370	2.00	3.80	0.30	2.92	5.66	
		Collector		Drainage Swale				0.030	305	0.020	3.00	40.24	0.54	1.52								
[DESIGN POINT #18]												10.21	0.54									
	1.46	Overland	В	Woodland, Grass	Fair	0.18	10	0.025	275	0.150	•	10.00	1	l	10	370	2.10	3.85	0.30	3.02	5.58	
SHED O2	1.40	Collector	В	Natural Swale	Tan	0.18	10	0.023	90	0.035	10.00		0.38	1.46	10	370	2.10	3.83	0.30	3.02	3.36	
[DESIGN POINT #18]								0.020				10.00	0.38									
SHEDS A1 + O2	2.98	Overland	В	Residential	Fair	0.18	18	0.010	180	0.150		10.21		_	11	370	2.00	3.80	0.30	5.80	11.16	
		Collector		Drainage Swale				0.030	305	0.020	3.00	40.24	0.54	1.52								
[DESIGN POINT #18]												10.21	0.54									
	3.22	Overland	R	Residential	Fair	0.18	20	0.027	205	0.150		8.19		I	10	370	2.10	3.85	0.30	6.57	12.20	
SHED A2	J.EE	Sub-Collector	5	Drainage Swale	i dii	0.10	20	0.020	470	0.020	3.00		0.94	1.75	10	370	2.10	3.03	0.50	0.57	12.20	
•		Collector		Natural Swale				0.005	100	0.040	5.00		0.54	3.22								
[DESIGN POINT #2]												8.19	1.48									
	1 1							1				40.56		ī							T	
SHED 01	2.05	Overland	В	Woodland, Grass	Fair	0.18	6	0.030	330	0.150 0.035	5.00	10.56	0.28	2.05	11	370	2.00	3.80	0.30	4.06	7.75	
[DESIGN POINT #17]		Collector		Natural Swale	+			0.022	90	0.035	5.00	10.56	0.28	2.05							<u> </u>	
[DESIGN FORNT #17]												10.50	0.20									
	8.25	Overland	В	Residential	Fair	0.18	16	0.010	180	0.150		10.21			12	370	1.95	3.55	0.30	15.69	28.89	
SHEDS A1		Collector		Drainage Swale				0.030	305	0.020	3.00		0.54	1.52								
A2 + O1-O2		Collector		Drainage Swale				0.020	470	0.020	3.00		0.68	6.20								
		Collector		Natural Swale				0.005	100	0.040	5.00		0.43	8.25								
[DESIGN POINT #2]												10.21	1.65									
	0.88	Overland	B	Residential	Fair	0.18	22	0.500	20	0.150		0.84	1	l	6	370	2.40	4.80	0.30	2.05	4.17	
SHED A3	0.00	Overland	В	Residential	i dii	0.10	22	0.044	90	0.150		4.32			· ·	370	2.40	4.00	0.50	2.03	7.1	
01122710		Collector		Drainage Swale				0.015	175	0.020	3.00		0.46	0.88								
[DESIGN POINT #7]												5.16	0.46									
	1 1							1				5.05		ī								
SHED A4	1.41	Overland	В	Residential	Fair	0.18	27	0.010	95 210	0.150 0.020	3.00	6.96	0.26	1 41	7	370	2.35	4.55	0.30	3.20	6.30	
[DESIGN POINT #8]		Collector		Drainage Swale	-			0.078	210	0.020	3.00	6.96	0.26	1.41								
[א ואוטא אוטוניזען												0.30	0.20							L		
CUED AS	1.01	Overland	В	Residential	Fair	0.18	20	0.010	140	0.150		8.78	I		9	370	2.20	3.90	0.30	2.16	3.88	
SHED A5	-	Collector		Drainage Swale		_	-	0.027	90	0.020	3.00		0.18	1.01	-		-					
[DESIGN POINT #]												8.78	0.18									
									427	0.450		0.20		ı								
SHED A6	1.23	Overland	В	Residential	Fair	0.18	18	0.010	127 186	0.150 0.020	3.00	8.28	0.29	1.23	9	370	2.20	3.90	0.30	2.64	4.73	
[DESIGN POINT #15A]		Collector	_	Drainage Swale				0.047	180	0.020	3.00	8.28	0.29	1.23								
[DESIGN POINT #15A]						<u> </u>						0.20	0.23	L		1				l	l	

^{1.} Minimum response time of 5 minutes is used for all response times calculated to be less than 5 minutes.

THE RESERVE

TABLE E3 - PROPOSED CONDITIONS - SHED "B" DATA

Date: 27-Sep-24

Revised: 16-Jul-25

By: TNJ

By: TNJ

By: JM

					A																	
					SHED (CHARACTERISTICS	S			RESPONSE TIME						DISCHARGE RATE						
	Shed					Constant Loss			Travel		Side Slope	Overland Flow	Collector	Collector Tributary	Total Response		Unit Peak	Unit Peak	Infiltration			
Drainage Shed I.D.	Area				Cover	Rate	Impervious	Slope	Length	Manning's	(Z)	Response Time	Response Time	Area	Time ¹		Discharge q10	Discharge q100	Factor Fi	Peak Flow	Peak Flov	
[Design Point #]	(acres)	Flow Type	Soil Type	Ground Cover	Condition	(in/hr)	%	(Ft/Ft)	(ft)	n	(ft/ft)	(minutes)	(minutes)	(acres)	(minutes)	Elevation (ft)	(cfs/ac)	(cfs/ac)	(cfs/ac)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	
	3.01	Overland	В	Residential	Fair	0.18	32	0.080	57	0.150		2.74			15	370	1.70	3.10	0.30	4.83	9.04	
SHED B1		Overland		Residential				0.023	174	0.150		7.79										
JULD DI		Overland		AC Pavement				0.020	46	0.110		3.04										
		Collector		Vegetated Swale				0.022	397	0.045	3.00		1.22	3.01								
[DESIGN POINT #]												13.57	1.22									
	1		T	T			T	ı		T	ı	1 001	ı			T		ı		1		
SHED B2	0.45	Overland	В	AC Pavement	Fair	0.18	67	0.020	53	0.110		3.31			6	370	2.40	4.80	0.30	0.99	2.07	
		Collector		Vegetated Swale				0.005	352	0.045	3.00		3.04	0.45								
[DESIGN POINT #]												3.31	3.04								1	
	1			1	1 .		_	T		1	T	274	1	<u> </u>		1	1 -	T T		1 -		
	3.46	Overland	В	Residential	Fair	0.18	37	0.080	57	0.150		2.74			15	370	1.70	3.10	0.30	5.50	10.34	
SHEDS B1 + B2		Overland		Residential				0.023	174	0.150		7.79										
3.1.2.3.3.2. · D.2		Overland		AC Pavement				0.020	46	0.110		3.04										
		Collector		Vegetated Swale				0.022	397	0.045	3.00		1.22	3.01								
[DESIGN POINT #]												13.57	1.22									
SHED B3	1.62	Overland	В	Residential	Fair	0.18	25	0.058	163	0.150		5.68			6	370	2.40	4.80	0.30	3.77	7.65	
JIILD DJ		Collector		Drainage Swale				0.018	205	0.020	2.00		0.40	1.62								
[DESIGN POINT #]		·				·					-	5.68	0.40									

Notes:

^{1.} Minimum response time of 5 minutes is used for all response times calculated to be less than 5 minutes.

TABLE E4 - PROPOSED CONDITIONS - SHED "C" DATA

By: TNJ

By: JM

	SHED CHARACTERISTICS												RESPON	SE TIME				DISCHARGE	RATE		
Drainage Shed I.D. [Design Point #]	Shed Area (acres)	Flow Type	Soil Type	Ground Cover	Cover Condition	Constant Loss Rate (in/hr)	Impervious %	Slope (Ft/Ft)	Travel Length (ft)	Manning's	Side Slope (Z) (ft/ft)	Overland Flow Response Time (minutes)	Collector Response Time (minutes)	Collector Tributary Area (acres)	Total Response Time ¹ (minutes)	Elevation (ft)	Unit Peak Discharge q10 (cfs/ac)	Unit Peak Discharge q100 (cfs/ac)	Infiltration Factor Fi (cfs/ac)	Peak Flow Q ₁₀ (cfs)	Peak Flow Q ₁₀₀ (cfs)
SHED C1	1.70	Overland	В	Residential	Fair	0.18	25	0.038	250	0.150		8.33			8	370	2.30	4.20	0.30	3.78	7.01
JILD CI		Collector		Drainage Swale				0.045	111	0.020	2.00		0.15	1.70							
[DESIGN POINT #]												8.33	0.15								
	3.72	Overland	В	Residential	Fair	0.18	26	0.150	142	0.150		3.93			13	370	1.90	3.40	0.30	6.78	12.36
		Overland		Residential				0.027	183	0.150		7.65									
SHED C2		Sub-Collector		Drainage Swale	i i			0.042	120	0.020	2.00		0.14	3.72							
		Sub-Collector		PCC Curb & Gutter				0.014	44	0.015	50.00		0.13	3.72							
		Collector		Vegetated Swale				0.006	138	0.045	3.00		0.66	3.72		<u> </u>					
[DESIGN POINT #]												11.59	0.93			<u> </u>					+
			•		•		•			•											
	0.46	Overland	В	AC Pavement	Fair	0.18	70	0.020	53	0.110		3.31			5	370	2.55	5.00	0.30	1.08	2.20
SHED C3		Collector		Vegetated Swale				0.042	324	0.045	3.00		1.25	0.46							
[DESIGN POINT #]												3.31	1.25								
																4					
CUED OC	0.50	Overland	В	AC Pavement	Fair	0.18	50	0.020	35	0.110		2.58			4	370	2.55	5.00	0.30	1.20	2.42
SHED C6		Collector		PCC Curb & Gutter				0.012	320	0.015	50.00		1.67	0.50							
[DESIGN POINT #]												2.58	1.67								

Date: 27-Sep-24

Revised: 16-Jul-25

Motos.

^{1.} Minimum response time of 5 minutes is used for all response times calculated to be less than 5 minutes.

[DESIGN POINT #__]

Revised: 16-Jul-25

THE RESERVE											Date:	: 27-Sep-24		Revised	l: 16-Jul-25						
TABLE E5 - PROP	OSED CO	ONDITIONS - S	HED "X" -	+ CUMMULATI	VE SHED D	ATA					Ву	: TNJ		Ву	r: JM						
					SHED C	HARACTERISTIC	CS .						RESPON					DISCHARG	E RATE		
Drainage Shed I.D. [Design Point #]	Shed Area (acres)	Flow Type	Soil Type	Ground Cover	Cover Condition	Constant Loss Rate (in/hr)	Impervious %	Slope (Ft/Ft)	Travel Length (ft)	Manning's	Side Slope (Z) (ft/ft)	Overland Flow Response Time (minutes)		Collector Tributary Area (acres)	Total Response Time ¹ (minutes)	Elevation (ft)	Unit Peak Discharge q10 (cfs/ac)	Unit Peak Discharge q100 (cfs/ac)	Infiltration Factor Fi (cfs/ac)	Peak Flow Q ₁₀ (cfs)	Peak Flow Q ₁₀₀ (cfs)
SHED X5	3.70	Overland	В	Woodland,Grass	Fair	0.18	5	0.055	155	0.150	Г 00	5.60	1.10	2.70	7	370	2.35	4.55	0.30	8.64	16.78
[DESIGN POINT#]	-	Collector		Natural Swale				0.032	506	0.035	5.00	5.60	1.19 1.19	3.70		 		-			+
[525:6:1:0:11:1:1:																					
SHED X11-A	0.68	Overland	В	Woodland, Grass	Fair	0.18	5	0.042	120	0.150		5.20			6	370	2.40	4.80	0.30	1.62	3.25
		Collector		Drainage Swale				0.017	235	0.020	3.00	F 20	0.63	0.68							
[DESIGN POINT #]												5.20	0.63								
SHED X11-B	0.69	Overland	В	Woodland, Grass	Fair	0.18	5	0.057	210	0.150		6.64			7	370	2.35	4.55	0.30	1.61	3.13
SUED YTT-D		Collector		Drainage Swale				0.046	65	0.020	3.00		0.12	0.69							
[DESIGN POINT #]												6.64	0.12								1
SHED O3-A	0.58	Overland	В	Woodland, Grass	Fair	0.18	90	0.050 0.024	46 501	0.150 0.015	50.00	2.78	1.95	0.58	5	370	2.55	5.00	0.30	1.32	2.74
[DESIGN POINT #]		Collector		AC Curb				0.024	301	0.013	30.00	2.78	1.95	0.36		 		+			
SHED 03 + 03-A	0.69	Overland	В	Woodland, Grass	Fair	0.18	90	0.045	46	0.150		2.87			5	370	2.55	5.00	0.30	1.57	3.26
[DESIGN POINT #3]		Collector		AC Curb				0.024	501	0.015	50.00	2.87	1.87 1.87	0.69		-					+
	5.00		B			0.10		0.055	155	0.150		5.60			7	270	2.25	4.55	0.20	42.22	24.00
SHEDS X5 + B3	5.32	Overland Collector	В	Woodland, Grass Natural Swale	Fair	0.18	11	0.055 0.032	506	0.130	5.00	3.00	1.19	3.70	/	370	2.35	4.55	0.30	12.33	24.03
511255 NS : 55		Collector		Drainage Swale				0.013	182	0.020	3.00		0.32	5.32							
[DESIGN POINT #]												5.60	1.51								
	8.78	Overland	- R	Residential	Fair	0.18	21	0.080	57	0.150		2.74	T	T	17	370	1.55	2.75	0.30	13.05	23.59
	8.78	Overland		Residential	Tall	0.10	21	0.023	174	0.150		7.79			17	370	1.55	2.73	0.30	13.03	23.33
SHEDS X5 + B1-B3		Overland		AC Pavement				0.020	46	0.110		3.04									
		Collector		Vegetated Swale				0.022	397	0.045	3.00		1.22	3.01				_			
[22222122222222222222222222222222222222		Collector		Circular Pipe				0.005	336	0.015	100.00	42.57	1.79	3.46		<u> </u>					
[DESIGN POINT #]												13.57	3.01								
	9.66	Overland	В	Residential	Fair	0.18	21	0.080	57	0.150		2.74		T	17	370	1.55	2.75	0.30	14.36	25.95
		Overland		Residential				0.023	174	0.150		7.79									
SHEDS X5 + B1-B3		Overland		AC Pavement				0.020	46	0.110		3.04	1.00	2.24		<u> </u>					
+A3		Collector Collector		Vegetated Swale Circular Pipe				0.022	397	0.045	3.00		1.22	3.01		-					
		Collector		Circular Pipe Circular Pipe				0.005 0.005	336 56	0.015	100.00 100.00		1.79	3.46 8.78		 		+			+
		Collector		Drainage Swale				0.003	190	0.015 0.020	3.00		0.24 0.16	9.66		+		+	 		
[DESIGN POINT #7]		Collector		Diamage Swale				0.039	190	0.020	3.00	13.57	3.41	9.00		+					
SHEDS X10 + X11-A	9.50	Overland	В	Woodland,Grass	Fair	0.18	30	0.148	245	0.150	10.00	5.47	2.12	7.00	6	370	2.40	4.80	0.30	21.94	44.74
		Collector Collector		Natural Swale Circular Pipe				0.068 0.005	270 55	0.030 0.015	10.00		0.42 0.09	7.60 8.32		 					1
+ C6 + O7		Collector		Drainage Swale				0.003	235	0.020	3.00		0.33	9.50		+			-		
[DESIGN POINT #]		00000		2. amage aware				0.017				5.47	0.84	0.00		<u> </u>					
					T			T													
	22.30	Overland Sub Collector	В	Woodland, Grass	Fair	0.18	30	0.025	245	0.150 0.030	10.00	9.33	1.30	1.00	14	370	1.80	3.20	0.30	38.12	69.34
SHEDS X7-X10 +		Sub-Collector Collector		Graded Swale Graded Swale			1	0.015 0.005	285 475	0.030	10.00 3.00		1.30	4.01							
(11-A + C6 + O5-O7	,	Collector		Circular Pipe				0.005	40	0.015	1.00		0.08	4.39							
		Collector		Natural Swale				0.023	540	0.035	5.00		1.15	9.03							
		Collector		Drainage Swale				0.011	180	0.020	3.00		0.24	22.30							
[DESIGN POINT # 1				1	1	· · · · · · · · · · · · · · · · · · ·		1	1			9.33	4.52			1			1		4

9.33

4.52

Date: 27-Sep-24

Revised: 16-Jul-25

TABLE E5 - PROP	PPOSED CONDITIONS - SHED "X" + CUMMULATIVE SHED DATA												By: TNJ By: JM												
					SHED	CHARACTERISTICS	3			RESPONSE TIME							DISCHARGE RATE								
Drainage Shed I.D. [Design Point #]	Shed Area (acres)	Flow Type	Soil Type	Ground Cover	Cover Condition	Constant Loss Rate (in/hr)	Impervious %	Slope (Ft/Ft)	Travel Length (ft)	Manning's n	Side Slope (Z) (ft/ft)	Overland Flow Response Time (minutes)	Collector Response Time (minutes)	Collector Tributary Area (acres)	Total Response Time ¹ (minutes)	Elevation (ft)	Unit Peak Discharge q10 (cfs/ac)	Unit Peak Discharge q100 (cfs/ac)	Infiltration Factor Fi (cfs/ac)	Peak Flow Q ₁₀ (cfs)	Peak Flow Q ₁₀₀ (cfs)				
	22.99	Overland	В	Woodland, Grass	Fair	0.18	30	0.025	245	0.150		9.33			14	370	1.80	3.20	0.30	39.30	71.49				
SHEDS X7-X10 +		Sub-Collector		Graded Swale				0.015	285	0.030	10.00		1.30	1.00											
X11-A + X11-B + C6		Collector		Graded Swale				0.005	475	0.030	3.00		1.75	4.01											
		Collector		Circular Pipe				0.005	40	0.015	1.00		0.08	4.39											
+ 05-07		Collector		Natural Swale				0.023	540	0.035	5.00		1.15	9.03											
		Collector		Drainage Swale				0.011	180	0.020	3.00		0.24	22.99											
[DESIGN POINT #]												9.33	4.51								1				
	1 - 1		1 1			1 1		T	0.45	0.150	1	0.22	T	l .		1	T			1					
	24.57	Overland	В	Woodland,Grass	Fair	0.18	30	0.025	245	0.150	10.00	9.33	4.20	4.00	14	370	1.80	3.20	0.30	42.01	76.40				
SHEDS X7-X10 +		Sub-Collector		Graded Swale				0.015	285	0.030	10.00		1.30	1.00											
X11-A + X11-B + C6		Collector Collector	+	Graded Swale Circular Pipe				0.005 0.005	475 40	0.030 0.015	3.00 1.00		1.75 0.08	4.01 4.39							-				
		Collector	+	Natural Swale				0.003	540	0.015	5.00		1.15	9.03							-				
+ 05-07 + C1		Collector		Drainage Swale				0.023	180	0.033	3.00		0.24	22.99						+	+				
		Collector		Drainage Swale				0.061	215	0.020	3.00		0.15	24.57						+	+				
[DESIGN POINT #]		Conector		Drainage Swale				0.001	213	0.020	3.00	9.33	4.66	24.57											
[DESIGNATIONAL III]												3.55									1				
	28.75	Overland	В	Woodland, Grass	Fair	0.18	30	0.025	245	0.150	I	9.33			14	370	1.80	3.20	0.30	49.15	89.40				
		Sub-Collector		Graded Swale		5125		0.015	285	0.030	10.00		1.30	1.00	= -										
SHEDS X7-X10 +		Collector		Graded Swale				0.005	475	0.030	3.00		1.75	4.01											
		Collector		Circular Pipe				0.005	40	0.015	1.00		0.08	4.39											
X11-A + X11-B + C6		Collector		Natural Swale				0.023	540	0.035	5.00		1.15	9.03											
+ O5-O7 + C1-C3		Collector		Drainage Swale				0.011	180	0.020	3.00		0.24	22.99											
		Collector		Drainage Swale				0.061	215	0.020	3.00		0.15	24.57											
		Overland		Circular Pipe				0.005	62	0.015	1.00		0.08	24.57											
[DESIGN POINT #]												9.33	4.74												
	1 1							<u> </u>					T	T			Ī			T					
	29.76	Overland	В	Woodland, Grass	Fair	0.18	30	0.025	245	0.150	10.00	9.33	1.00	1.00	14	370	1.80	3.20	0.30	50.88	92.54				
SHEDS X7-X10 +		Sub-Collector	1	Graded Swale				0.015	285	0.030	10.00		1.30	1.00		1									
X11-A + X11-B + C6		Collector	+	Graded Swale				0.005	475	0.030	3.00		1.75	4.01											
		Collector	+	Circular Pipe				0.005	40	0.015	1.00		0.08	4.39											
+ 05-07 + C1-C3 +		Collector	+ -	Natural Swale				0.023	540	0.035	5.00		1.15 0.24	9.03 22.99											
A5		Collector	+ +	Drainage Swale				0.011	180	0.020	3.00 3.00		0.24	22.99		-									
		Collector	+ +	Drainage Swale				0.061	215 62	0.020	1.00		0.15	24.57		-				+					
		Overland	+ +	Circular Pipe	-			0.005		+															
[DECICAL DOLLIT (115]		Collector	+	Drainage Swale				0.027	170	0.020	3.00	0.22	0.15	28.75											
[DESIGN POINT #15]						l				_[1	9.33	4.74	1							1				

Notes:

^{1.} Minimum response time of 5 minutes is used for all response times calculated to be less than 5 minutes.

Date: 4/7/2025 Revised:

¹Equations:

Volume = (Q)(Watershed Area)

 $Q = [(P - 0.2S)^{2}]/(P + 0.8S)$

S = (1000 / CN) - 10

Q = Direct Runoff (excess rainfall) in inches where:

> S = Potential Abstraction P = Rainfall Depth, inches

CN= Curve Number

Existing Condition

CN = 61

^{1,3}Curve Number for Pasture or range, good condition & Hydrologic Soil Group B

CN = (61)(1.30 - ((1.30 - 1.21) / (70 - 60))(61 - 60)

¹AMC III Curve Number, interpolated for worst case scenario

= 78.8

S = (1000 / 78.8) - 10

= 2.70

Existing Condition, potential abstraction

P = 4.65 + [(4.84 - 4.65) / (400 - 300)](370 - 300)

²Rainfall Depth for 100-yr Storm Event, interpolated for Elevation of 370-ft

= 4.78

 $Q = [(4.78 - (0.2)(2.70))^{2}] / (4.78 + (0.8)(2.70))$

Existing Condition runoff, inches

= 2.59

Volume = $(2.59 \text{ in})(26.29 \text{ ac} - 4.42 \text{ ac})(43,560 \text{ ft}^2/\text{ ac})(1 \text{ ft}/12 \text{ in})$

Existing Condition volume (site area excluding pond area), ft³

= 205,927

Date: 4/7/2025 Revised:

Developed Condition

CN = 70 - ((70 - 66) / (2 - 0.5))(1 - 0.5) = 68.7	^{1,3} Curve Number for Residential 1-acre lot, good condition (interpolated) & Hydrologic Soil Group B
CN = (68.7)(1.30 - ((1.30 - 1.21) / (70 - 60))(68.7 - 60) = 83.9	¹ AMC III Curve Number, interpolated for worst case scenario
S = (1000 / 83.9) - 10 = 1.92	Developed Condition, potential abstraction
P = 4.65 + [(4.84 - 4.65) / (400 - 300)](370 - 300) = 4.78	² Rainfall Depth for 100-yr Storm Event, interpolated for Elevation of 370-ft
$Q = [(4.78 - (0.2)(1.92))^{2}] / (4.78 + (0.8)(1.92))$ = 3.06	Developed Condition runoff, inches
Volume = (3.06 in)(26.29 ac - 4.42ac)(43,560 ft ² / ac)(1 ft / 12 in) = 243,252	Developed Condition volume (site area excluding pond area), ft ³
Change in Volume = Dev. Condition Vol Ex. Condition Vol. = 243,252 - 205,927 = 37,325	Increase in Runoff Volume, ft ³
Change in Pond Depth = Change in Vol. / Pond Surface Area = (44,869 ft ³ / 284,460 ft ²)(12 in / ft)	Increase in Pond Depth, inches

References

= 1.57

- 1. State Water Resources Control Boad. The Clean Water Team Guidance Compendium for Watershed Monitoring and Assessment.
- 2. Placer County Flood Control and Water Conservation District. Stormwater Management Manual. September 1, 1990.
- 3. Natural Resources Conservation Service. Soil Survey of Placer County, California Western Part. Survey Area Data: Version 15, August 31, 2023.

APPENDIX F - HYDRAULICS

Reserved for Final Design

APPENDIX G - STORM WATER QUALITY

Post Construction Storm Water Quality Plan
Preliminary Developed DMA SWQ Map
Water Quality - Vegetated Swale Calculations
Flowmaster Worksheets - Vegetated Swale Calculations

Post-Construction Storm Water Quality Plan

For:

THE RESERVE TOWN OF LOOMIS, CALIFORNIA

APN: 045-161-033

Prepared for:

Stefan Horstschraer
Owner/Developer
Premier 40, LLC
8483 Douglas Plaza Dr, Suite 110
Granite Bay, CA 95746
(916) 773-0207

Prepared by:

TSD Engineering, Inc. 785 Orchard Dr., Suite 110 Folsom, CA 95630 (916) 607-0707

Preparation Date: 07/16/2025	
PLACER COUNTY Approval Date:	

Section 1 General Project Information

The undersigned owner of the subject property, is responsible for the implementation of the provisions of this Storm Water Quality Plan (SWQP), including ongoing operations and maintenance (O&M), consistent with the requirements of the West Placer Storm Water Quality Design Manual and the State of California Phase II Small MS4 General Permit (Order No: 2013-0001-DWQ). If the undersigned transfers its interest in the property, its successors-in-interest shall bear the aforementioned responsibility to implement the SWQP.

For all Regulated Projects (As identified in Form 1-2 below), the undersigned owner hereby grants access to all representatives of the Jurisdictional Agency for the sole purpose of performing O&M inspections of the installed treatment system(s) and hydromodification control(s) if any.

A copy of the final signed and fully approved SWQP shall be available on the subject site for the duration of construction and then stored with the project approval documentation and improvement plans in perpetuity.

Form	1-1 Project Identification and O	wner's Certification
Project Site Address:	Located in the intersection of Wells Ave and Barto	n Road
Owner Name:	Stefan Horstschraer	
Title	Owner/Devloper	
Company	Premier 40, LLC	
Address	8483 Douglas Plaza Dr, Suite 110	
City, State, Zip Code	Granite Bay, CA 95746	
Email	stefan@premierhomesca.com	
Telephone #	(916) 773-0207	
Signature	Date	
Engineer:*	Tom Jones	PE Stamp*
Title	Project Engineer	(Required for all Regulated Projects)
Company	TSD Engineering, Inc.	
Address	785 Orchard Dr., Suite 110	
City, State, Zip Code	Folsom, CA 95630	
Email	tjones@tsdeng.com	
Telephone #	(916) 607-0707	
Signature		
Brief Description of Project:	The Reserve subdivision proposes 20 residential lo	ts that surround a cul-de-sac accessed from the
(Attach additional sheets as necessary)		

^{*} Not required for Small Projects as determined in Form 1-2 below. Project owners are responsible for ensuring that all storm water facilities are designed by an appropriately licensed and qualified professional.

Form 1-2 Project Category	
Development Category (Select all that apply)	
¹ Small Project – All projects, except LUPs, that create and/or replace between	
2,500-5,000 ft ² of impervious surface or detached single family homes that	
create and/or replace 2,500 ft ² or more of impervious surface and are not part of a larger plan of development.	
² Enter total new and/or replaced impervious surface (ft ²)	
³ Regulated Project – All projects that create and/or replace 5,000 ft ² or more of impervious surface.	
⁴ Regulated Redevelopment Project with equal to, or greater than 50 percent increase in impervious area	
⁵ Regulated Redevelopment Project with less than 50 percent increase in impervious area	
⁶ Enter total pre-project impervious surface (ft ²)	
⁷ Enter total new and/or replaced impervious surface (ft ²)	
⁸ Regulated Road or linear underground/overhead project (LUP) creating 5,000 ft ² or more of newly constructed contiguous impervious surface.	
⁹ Enter total new and/or replaced impervious surface (ft ²)	
¹⁰ Regulated Hydromodification Management Project – Regulated projects that create and/or replace 1 acre or more of impervious surface. A project that does not increase impervious surface area over the pre-project condition is not a hydromodification management project.	Х
¹¹ Enter total new and/or replaced impervious surface (ft ²)	445,112

Section 3 **Regulated Projects** Section 3 forms are to be completed for all Regulated Projects. Form 3-1 Site Location and Hydrologic Features Site coordinates: Elevation 85th Percentile, 24 Hour Design Storm ¹ Latitude ² Longitude (ft. above sea level) Depth (in): Take GPS measurement at approximate center of site 38.78631 -121.19472 370 0.9 Receiving waters Name of stream, lake or other downstream waterbody to Dry Creek which the site runoff eventually drains 303(d) listed pollutants of concern Refer to State Water Resources Control Board website Sediment, oils, fuel, mercury and debris www.waterboards.ca.gov/water issues/programs/water qualit ⁷Is Project going to be phased? No If yes, ensure that the SWQP evaluates each phase with distinct DMAs, requiring LID BMPs to address runoff at

⁸Use this form to show a conceptual schematic depicting DMAs and conveyance features connecting DMAs to the site outlet(s). An example is provided below that can be modified for the proposed project or a drawing clearly showing DMAs and flow routing may be attached.

See Attached Stormwater Quality Schematic &

Developed DMA SWQ Map

time of completion

DMA 1 DMA 2 DMA 3 DMA 4 Bioretention 1 Outfall

Form 3-2 Site Assessment and Layout Do	ocume	entation
	Has f	this Item been considered in the Site Layout and depicted in the Site Plan?
	Yes	Not Applicable (Include brief explanation)
Define the development envelope and protected areas, identifying areas that are most suitable for development and areas to be landscaped, or left undisturbed, and used for infiltration.	х	
Concentrate development on portions of the site with less permeable soils and preserve areas that can promote infiltration.		N/A Infiltration Rate is the same (Soil Type B)
Limit overall impervious coverage of the site with paving and roofs.	х	
Set back development from creeks, wetlands, and riparian habitats.	х	
Preserve significant trees.	х	
Conform site layout along natural landforms.	х	
Avoid excessive grading and disturbance of vegetation and soils.	х	
Replicate the site's natural drainage patterns.	х	
Detain and retain runoff throughout the site.	х	

Attach a Site Plan that incorporates the applicable considerations above. Ensure that the following items are included in the Site Plan:

Site Boundary

Soil types and areal extents, test pit and infiltration test locations

Topographic data with 1 ft. contours

Existing natural hydrologic features (depressions, watercourses, wetlands, riparian corridors)

Environmentally sensitive areas and areas to be preserved.

Proposed locations and footprints of improvements creating new, or replaced, impervious surfaces

Potential pollutant sources and locations

Entire site divided into separate DMAs with unique identifiers

Existing and proposed site drainage network with flow directions and site run-on and discharge locations

Proposed design features and surface treatments used to minimize imperviousness and reduce runoff

Proposed locations and footprints of treatment and hydromodification management facilities

Design features for managing authorized non-stormwater discharges

Areas of soil and/or groundwater contamination

Existing utilities and easements

Maintenance areas

	Form 3	3-3 Source	Control Measures
Potential Pollutant Generating Activity or Source	Ch	eck One	Describe the source control measures to be implemented for each potential pollutant generating activity or source present on the project as listed in Appendix C and in the CASQA Fact Sheets. Include any special features, materials, or methods of construction that will
	Present	Not Applicable	be used.
Accidental spills or leaks	✓		Materials will be stored indoors aways from storm drains or sensitive areas. (CASQA SC-11)
Interior floor drains		√	
Parking/storage areas and maintenance	V		Parking/Driveway areas drain to vegetated swales.
Indoor and structural pest control	V		Fed., State and local laws and regulatons for the use, storage and disposal of pesticides shall be followed. (CASQA SC-41)
Pools, spas, ponds, decorative fountains, and other water features	V		Never discharge wash water or wastewater to the driveway, street, gutter, or near a storm drain (use local regulation for storm drain discharge)
Landscape/outdoor pesticide use	V		Fed., State, and local laws and regulations for the use, storage, and disposal of pesticides shall be followed. (CASQA SC-41)
Restaurants, grocery stores, and other food service operations		V	
Refuse areas	V		Utilize Good Housekeeping measures and prevent runon and runoff from refuse toter storage areas. (CASQA SC-34)
Industrial Processes		V	
Outdoor storage of equipment or materials	V		Limit exposure of rainfall whenever possible. (CASQA SC-30, SC-31, SC-32)
Vehicle and equipment cleaning	V		If you choose to wash your car at home, wash it on a lawn or other unpaved surface to reduce the amount of car wash water runoff.
Vehicle and equipment repair and maintenance		V	
Fuel dispensing areas		✓	
Loading docks		V	
Fire sprinkler test water		V	
Drain or wash water from boiler drain lines, condensate drain lines, rooftop equipment, drainage sumps, and other sources		V	
Unauthorized non-storm water discharges		V	
Building and grounds maintenance	7		Switch to non-toxic chemical, encourage proper lawn management and native vegetation, integrated pest management, proper yard trimming recycling, and recycle residual paint, solvents, lumber, and other materials.

The source control measures identified in this table shall be designed consistent with recommendations from the CASQA Stormwater BMP Handbook for New Development and Redevelopment¹, or from another equivalent manual.

^[1] California Stormwater BMP Handbook New Development and Redevelopment. California Stormwater Quality Association (CASQA). January 2003.

	Form 3-4	Runoff Reduction Calculator fo	r Site	Design Me	asures	on Regula	ited Pro	ojects		
		¹ DMA ID No.		1		2		3		4
Site Design Measure		Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft³)
-	A _{imp} (ft ²)	impervious drainage area								
² Adjacent/On-Site Stream Setbacks and Buffers	V ₈₅ (in)	runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0
		ponding area								
Soil Quality Improvement		ponding depth								
and Maintenance	A _{sa} (ft ²)	soil amendment area		0		0		0		0
	D _{sa} (ft)	depth of amended soil								
	n	porosity of amended soil								
	n _e	number of new evergreen trees								
	n _d	number of new deciduous trees								
⁴ Tree Planting and Preservation	A _{tc} (ft ²)	canopy area of existing trees to remain on the property	22510	1519	10495	708	20667	1395	10146	685
reservation	V ₈₅ (in)	runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8	
5	A _{imp} (ft ²)	impervious drainage area			7332					
S Rooftop and Impervious Area Disconnection	V ₈₅ (in)	runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	495	0.8	0	0.8	0
	A _{res} (ft ²)	area of gravel storage layer								
⁶ Porous Pavement	D _{res} (ft)	depth of gravel storage layer		0		0		0		0
	n _{agg}	porosity of aggregate								
	С	efficiency factor								
	A _{imp} (ft ²)	impervious drainage area	19729						32400	
Vegetated Swales	V ₈₅ (in)	runoff volume from 85th percentile, 24-hour storm	0.8	1332	0.8	0	0.8	0	0.8	2187
N number of rain barre		number of rain barrels and/or cisterns		0		0		0		0
⁸ Rain Barrels and Cisterns	V _a (ft ³)	volume of each rain barrel and/or cistern		U		U		U		U
9 Do all Site Desig	n Measures	meet the design requirements outlined in the Fa	ct Sheets	;?	Yes	Х	No			
		ne Reduction (ft ³)		2851		1203		1395		2872
¹¹ Effec	tive Treated	I Impervious Area (ft²)		42239		17827		20667		42546

		Form 3-4	l Runc	off Reduction	on Cal	culator for	Site D	esign Mea	ures o	n Regulate	d Pro	niects					
DMA ID No.		5		6	3.1. Ca.	7	Site B	8		9		10		11		12	
Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft³)		Runoff Reduction (ft³)	
impervious drainage area																	
runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	
ponding area ponding depth soil amendment area depth of amended soil porosity of amended soil		0		0		0		0		0		0		0		0	
number of new evergreen trees number of new deciduous trees canopy area of existing trees to remain on the property	0	0	0	0		0	7878	532	39849	2690	2463	166	5790	391	101	7	
runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8		0.8		0.8		0.8		0.8		
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	9637	650	0.8	0	5927 0.8	400	0.8	722	0.8	578	
area of gravel storage layer depth of gravel storage layer porosity of aggregate efficiency factor		0		0		0		0		0		0		0		0	
impervious drainage area runoff volume from 85th percentile, 24-hour storm	13915 0.8	939	0.8	894	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	
number of rain barrels and/or cisterns volume of each rain barrel and/or cistern		0		0		0		0		0		0		0		0	
Total Volume Reduction		939		894		0		1182		2690		566		1112		585	
Effective Treated Impervious Area		13915		13246	0		17515		.7515 39849		8390			16481	8663		

		Form 3-4	Runc	off Reduction	on Ca	lculator fo	r Site	Design Mea	asures	on Regula	ted Pr	ojects				
DMA ID No.		13		14		15		16		17		18		19		20
Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft³)		Runoff Reduction (ft³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)
impervious drainage area																
runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
ponding area ponding depth soil amendment area depth of amended soil porosity of amended soil		0		0		0		0		0		0		0		0
number of new evergreen trees number of new deciduous trees canopy area of existing trees to remain on the property	3861	261		0		0	0	0		0		0		0		0
runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8		0.8		0.8		0.8		0.8	
impervious drainage area runoff volume from 85th percentile, 24-hour storm	5732 0.8	387	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
area of gravel storage layer depth of gravel storage layer porosity of aggregate efficiency factor		0		0		0		0		0		0		0		0
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
number of rain barrels and/or cisterns volume of each rain barrel and/or cistern		0		0		0		0		0		0		0		0
Total Volume Reduction		648		0		0		0		0		0		0		0
Effective Treated Impervious Area		9593		0		0		0		0		0		0		0

			orm 3-4 Runoff Reduction Calculator for Site Design													
	F	orm 3-4 R	Runoff Reduction Calculator for Site I		Site [Design Mea	sure	s on Regula	ated	Projects						
DMA ID No.		21		22		23		24		25		26		27		28
Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)
impervious drainage area																
runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
ponding area ponding depth soil amendment area depth of amended soil porosity of amended soil		0		0		0		0		0		0		0		0
number of new evergreen trees number of new deciduous trees canopy area of existing trees to remain on the property		0		0		0		0		0		0		0		0
runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8		0.8		0.8		0.8		0.8	
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
area of gravel storage layer depth of gravel storage layer porosity of aggregate efficiency factor		0		0		0		0		0		0		0		0
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
number of rain barrels and/or cisterns volume of each rain barrel and/or cistern		0		0		0		0		0		0		0		0
Total Volume Reduction		0		0		0		0		0		0		0		0
Effective Treated Impervious Area		0		0	0			0		0		0		0		0

			4 Runoff Reduction Calcula													
		Form 3-4 F			culator for	Site	Design Me	asure	s on Regul	ated	Projects					
DMA ID No.		29		30		31		32		33		34		35		36
Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)
impervious drainage area																
runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
ponding area ponding depth soil amendment area depth of amended soil porosity of amended soil		0		0		0		0		0		0		0		0
number of new evergreen trees number of new deciduous trees canopy area of existing trees to remain on the property		0		0		0		0		0		0		0		0
runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8		0.8		0.8		0.8		0.8	
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
area of gravel storage layer depth of gravel storage layer porosity of aggregate efficiency factor		0		0		0		0		0		0		0		0
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
number of rain barrels and/or cisterns volume of each rain barrel and/or cistern		0		0		0		0		0		0		0		0
Total Volume Reduction		0		0		0		0		0		0		0		0
Effective Treated Impervious Area		0		0	0			0		0		0		0		0

	F	orm 3-4 R	unof	f Reduction	n Calo	culator for	Site I	Design Mea	sure	s on Regula	ated	Projects				
DMA ID No.		37		38		39		40		41		42		43		44
Runoff Reduction Parameters		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)		Runoff Reduction (ft ³)
impervious drainage area																
runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
ponding area ponding depth soil amendment area depth of amended soil porosity of amended soil		0		0		0		0		0		0		0		0
number of new evergreen trees number of new deciduous trees canopy area of existing trees to remain on the property		0		0		0		0		0		0		0		0
runoff volume from 85th percentile, 24-hour storm	0.8		0.8		0.8		0.8		0.8		0.8		0.8		0.8	
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
area of gravel storage layer depth of gravel storage layer porosity of aggregate efficiency factor		0		0		0		0		0		0		0		0
impervious drainage area runoff volume from 85th percentile, 24-hour storm	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0	0.8	0
number of rain barrels and/or cisterns volume of each rain barrel and/or cistern		0		0		0		0		0		0		0		0
Total Volume Reduction		0		0		0		0		0		0		0		0
Effective Treated Impervious Area		0		0	0		0		0		0		0			0

	Form	1 3-5 Coi	nputatio	on of Wa	ter Qual	ity Desig	gn Crite	ria for Si	tormwat	er Treat	ment an	d Baseli	ne Hydi	romodif	ication	Measu	res					
DMA ID No.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22
¹ Total impervious area requiring treatment	42,239	17,827	18,393	42,546	13,915	13,246		17,515	27,853	8,390	16,481	8,663	9,593									
² Impervious area untreated by Site Design Measures (ft ²) Item 1 – Form 3-4 Item 11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
³ Additional pervious area draining to BMP (ft ²)	88,879	52,918	55,772	119,629	6,049	6,325		48,479	112,196	29,835	44,918	35,129	44,101									
⁴ Composite DMA Runoff Coefficient (Rc) Enter area weighted composite runoff coefficient representing entire DMA	0.44	0.39	0.39	0.40	0.72	0.71		0.40	0.35	0.36	0.40	0.35	0.33									
⁵ Water Quality Volume (WQV) (ft ³) WQV = 1/12 * [Item 2 + Item 3) *Item 4] * Unit WQV	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
⁶ Water Quality Flow (WQF) (cfs) WQF = 1/43,200 * [0.2* (Item 2 + Item 3) * Item4]	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

^{5,6} Values will equal zero if all impervious area has been treated by Site Design Measures.

DMA ID No.	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44
¹ Total impervious area requiring treatment																						
² Impervious area untreated by Site Design Measures (ft²) Item 1 – Form 3-4 Item 11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
³ Additional pervious area draining to BMP (ft ²)																						
⁴ Composite DMA Runoff Coefficient (Rc) Enter area weighted composite runoff coefficient representing entire DMA																						
⁵ Water Quality Volume (WQV) (ft ³) WQV = 1/12 * [Item 2 + Item 3) *Item 4] * Unit WQV	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
⁶ Water Quality Flow (WQF) (cfs) WQF = 1/43,200 * [0.2* (Item 2 + Item 3) * Item4]	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

 $^{^{5,\,6}}$ Values will equal zero if all impervious area has been treated by Site Design Measures.

	Form 5-1 BMP Inspection a	nd Maintenance
ВМР	Inspection Point and Frequency	Maintenance Activity Required
SWQ SWALES	Inlet/Outlet Structures (Bi-annually)	Ensure stable conveyance into facility,
		remove debris, repair any erosion
	Vegetation (on-going)	Prune and weed to keep water quality
		swales neat and orderly
	Irrigation (on-going)	Irrigate as required to establish and/or
		maintain vegetation
	Trash (Monthy, or as needed after storm	Remove obstructions, debris and trash
	events)	from swales and dispose of properly.
EXISITNG TREE	Irrigation (on-going)	Irrigate as required to establish and/or
CANOPY (SITE)		maintain trees
	Pruning (on-going)	Prune dead vegetation from trees on a
		regular basis
	Planting Areas (annually)	Remove fallen leaves and debris that
		could enter stormwater runoff
	Treating (on-going)	Minimize the use of chemical fertilizers
		& pesticides
NATURAL	Trash & Sediment (on-going)	Remove accumulated sediment, trash
AND/OR		debris, leaves and grass clippings.
DEVELOPED		Remove tree seedlings.
VEGETATED		
AREAS		
(Individual		
Lots)		

Form 6-1 Post-Construction Stormwater BMPs

Following is a summary of all BMPs included in the Project design. This checklist must be included on the cover sheet of the Improvement Plans for all Regulated Projects.

	ВМР	Plan Sheet Number(s)
Structural Source Controls (list BMPs)		
	Stream Setbacks and Buffers	C-7.0
	Soil Quality Improvement and Maintenance	N/A
Site Design Measures	Tree Planting and Preservation	C3.0 & C5.0
Site Design Measures	Rooftop and Impervious Area Disconnection	C7.0
	Porous Pavement	N/A
	Vegetated Swales	SWQP EXHIBIT
	Rain Barrels and Cisterns	N/A
	Bioretention with Infiltration	N/A
and Baseline Hydromodification Measures	Flow-Through Planters, Tree Box Filters and Media Filters	N/A
Hydromodification Management Measures	Supplemental Detention	N/A

Preliminary Developed DMA SWQ Map

	ARE	Α	
SURFACE DESCRIPTION	SF	AC	
TOTAL DMA #1 AREA =	131,118	3.01	
PERVIOUS			
LANDSCAPE AREAS ¹	88,879	2.04	
PERVIOUS AREA TOTAL ² =	88,879	2.04	
IMPERVIOUS			
BUILDING AREAS + DRIVEWAY ³	23,191	0.53	
ASSUMED FUTURE CONCRETE FLATWORK ⁴	4,500	0.10	
AC & PCC PAVEMENT AREAS (ROADWAY) ⁵	14,548	0.33	
IMPERVIOUS AREA TOTAL ⁶ =	42,239	0.97	
TOTAL AREA (CHECK) =	131,118	3.01	
Notes: 1. Total = DMA #1 Area Less Bldg + Driveway + Patios 8	& Walkways		

4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards.

IMPERVIOUS AREA TOTAL⁶ = 17,827

4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards.

TOTAL AREA (CHECK) = 70,745

2. Total pervious area for DMA #1 used in SWQP - Form 3-5, Item 3. 3. Based on Lot Fit Analysis and assumed driveway configurations.

5. AC & PCC pavement within Reserve Court for DMA #1.

ASSUMED FUTURE CONCRETE FLATWORK⁴

1. Total = DMA #2 Area Less Bldg + Driveway + Patios & Walkways.

2. Total pervious area for DMA #2 used in SWQP - Form 3-5, Item 3.

6. Total impervious area for DMA #2 used in SWQP - Form 3-5, Item 1

3. Based on Lot Fit Analysis and assumed driveway configurations.

5. AC & PCC pavement within Reserve Court for DMA #2.

AC & PCC PAVEMENT AREAS (ROADWAY)

LANDSCA

IMPERVIO

TOTAL DMA #4 AREA = 162,175 LANDSCAPE AREAS **BUILDING AREAS + DRIVEWAY** ASSUMED FUTURE CONCRETE FLATWORK4

TOTAL AREA (CHECK) = 162,175 1. Total = DMA #3 Area Less Bldg + Driveway + Patios & Walkways. 2. Total pervious area for DMA #3 used in SWQP - Form 3-5, Item 3. 3. Based on Lot Fit Analysis and assumed driveway configurations. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 5. AC & PCC pavement within Reserve Court for DMA #3.

6. Total impervious area for DMA #3 used in SWQP - Form 3-5, Item

IMPERVIOUS AREA TOTAL⁶ = 42,546

AC & PCC PAVEMENT AREAS (ROADWAY)

TOTAL DMA #5 AREA =	19,964	0.46
ERVIOUS		
ANDSCAPE AREAS ¹	6,049	0.14
PERVIOUS AREA TOTAL ² =	6,049	0.14
MPERVIOUS		
UILDING AREAS + DRIVEWAY ³	618	0.01
SSUMED FUTURE CONCRETE FLATWORK ⁴	0	0.00
C & PCC PAVEMENT AREAS (ROADWAY) ⁵	13,297	0.31
IMPERVIOUS AREA TOTAL ⁶ =	13,915	0.32
TOTAL AREA (CHECK) =	19,964	0.46
otes:		
. Total = DMA #3 Area Less Bldg + Driveway + Patios	& Walkways.	
. Total pervious area for DMA #3 used in SWQP - For	m 3-5, Item 3.	
. Based on Lot Fit Analysis and assumed driveway co	nfigurations.	
. Based on 1,500 SF of future flatwork per lot for pat	ios & walkways in ba	ack & side yards.
. AC & PCC pavement within Reserve Court for DMA	#3.	
. Total impervious area for DMA #3 used in SWQP - F	orm 3-5, Item 1.	

MA SURFACE DATA:	

	AF	REA		AREA		
URFACE DESCRIPTION	SF	AC	SURFACE DESCRIPTION	SF	AC	
TOTAL DMA #1 AREA =	131,118	3.01	TOTAL DMA #6 AREA =	19,571	0.45	
			PERVIOUS			
AREAS ¹	88,879	2.04	LANDSCAPE AREAS ¹	6,325	0.15	
PERVIOUS AREA TOTAL ² =	88,879	2.04	PERVIOUS AREA TOTAL ² =	6,325	0.15	
S			IMPERVIOUS			
REAS + DRIVEWAY ³	23,191	0.53	BUILDING AREAS + DRIVEWAY ³	856	0.02	
JTURE CONCRETE FLATWORK⁴	4,500	0.10	ASSUMED FUTURE CONCRETE FLATWORK ⁴	0	0.00	
AVEMENT AREAS (ROADWAY)5	14,548	0.33	AC & PCC PAVEMENT AREAS (ROADWAY) ⁵	12,390	0.28	
IMPERVIOUS AREA TOTAL ⁶ =	42,239	0.97	IMPERVIOUS AREA TOTAL ⁶ =	13,246	0.30	
TOTAL AREA (CHECK) =	131,118	3.01	TOTAL AREA (CHECK) =	19,571	0.45	
			Notes:			
#1 Area Less Bldg + Driveway + Patios &	Walkways.		1. Total = DMA #3 Area Less Bldg + Driveway + Patios &	Walkways.		

pervious area for DMA #1 used in SWQP - F	orm 3-5, Item 1.	6. Total impervious area for DMA #3 used in SWQP - Fo	rm 3-5, Item 1.	
	AR	REA		Α
SURFACE DESCRIPTION	SF	AC	SURFACE DESCRIPTION	SF
TOTAL DMA #2 AREA =	70,745	1.62	TOTAL DMA #8 AREA =	65,994
IS			PERVIOUS	
APE AREAS ¹	52,918	1.21	LANDSCAPE AREAS ¹	48,479
PERVIOUS AREA TOTAL ² =	52,918	1.21	PERVIOUS AREA TOTAL ² =	48,479
OUS			IMPERVIOUS	
G AREAS + DRIVEWAY ³	14,827	0.34	BUILDING AREAS + DRIVEWAY ³	14,515

TOTAL AREA (CHECK) = 65,994 1. Total = DMA #8 Area Less Bldg + Driveway + Patios & Walkways. . Total pervious area for DMA #8 used in SWQP - Form 3-5, Item 3. 3. Based on Lot Fit Analysis and assumed driveway configurations. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 5. AC pavement within Reserve Court for DMA #8.

6. Total impervious area for DMA #8 used in SWQP - Form 3-5, Item 1

IMPERVIOUS AREA TOTAL⁶ = 17,515

TOTAL DMA #9 AREA = 140,049

PERVIOUS AREA TOTAL² = 112,196

IMPERVIOUS AREA TOTAL⁶ = 27,853

TOTAL AREA (CHECK) = 140,049

2. Total pervious area for DMA #3 used in SWQP - Form 3-5, Item 3.

4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards.

1.11

0.07

2.58

0.54

0.10

3. Based on Lot Fit Analysis and assumed driveway configurations.

5. AC & PCC pavement within Reserve Court for DMA #3.

ASSUMED FUTURE CONCRETE FLATWORK⁴

	AN	EA	
SURFACE DESCRIPTION	SF	AC	SURFACE DESCRIPTION
TOTAL DMA #3 AREA =	74,165	1.70	TOTAL DMA #9 AREA =
rious			PERVIOUS
SCAPE AREAS ¹	55,772	1.28	LANDSCAPE AREAS ¹
PERVIOUS AREA TOTAL ² =	55,772	1.28	PERVIOUS AREA TOTAL ² =
RVIOUS			IMPERVIOUS
DING AREAS + DRIVEWAY ³	15,393	0.35	BUILDING AREAS + DRIVEWAY ³
MED FUTURE CONCRETE FLATWORK⁴	3,000	0.07	ASSUMED FUTURE CONCRETE FLATWORK ⁴
PCC PAVEMENT AREAS (ROADWAY) ⁵	0	0.00	AC PAVEMENT AREAS (ROADWAY) ⁵
IMPERVIOUS AREA TOTAL ⁶ =	18,393	0.42	IMPERVIOUS AREA TOTAL ⁶ =
TOTAL AREA (CHECK) =	74,165	1.70	TOTAL AREA (CHECK) =
1			Notes:

2. Total pervious area for DMA #3 used in SWQP - Form 3-5, Item 3. 2. Total pervious area for DMA #9 used in SWQP - Form 3-5, Item 3. 3. Based on Lot Fit Analysis and assumed driveway configurations. 3. Based on Lot Fit Analysis and assumed driveway configurations. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 5. AC & PCC pavement within Reserve Court for DMA #3. 6. Total impervious area for DMA #3 used in SWQP - Form 3-5, Iten 5. AC pavement within Reserve Court for DMA #9. 6. Total impervious area for DMA #9 used in SWQP - Form 3-5, Item 1

		Ai	L
	SURFACE DESCRIPTION	SF	AC
	TOTAL DMA #10 AREA =	38,225	0.88
٦	PERVIOUS		
	LANDSCAPE AREAS ¹	29,835	0.68
	PERVIOUS AREA TOTAL ² =	29,835	0.68
1	IMPERVIOUS		
	BUILDING AREAS + DRIVEWAY ³	6,890	0.16
	ASSUMED FUTURE CONCRETE FLATWORK ⁴	1,500	0.03
	AC PAVEMENT AREAS (ROADWAY) ⁵	0	0.00
	IMPERVIOUS AREA TOTAL ⁶ =	8,390	0.19
	TOTAL AREA (CHECK) =	38,225	0.88

1. Total = DMA #10 Area Less Bldg + Driveway + Patios & Walkways. . Total pervious area for DMA #10 used in SWQP - Form 3-5, Item 3. 3. Based on Lot Fit Analysis and assumed driveway configurations. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 5. AC pavement within Reserve Court for DMA #10. 6. Total impervious area for DMA #10 used in SWQP - Form 3-5, Item 1

JONI ACE DESCRIPTION	5.	7.10
TOTAL DMA #11 AREA =	61,399	1.41
PERVIOUS		
LANDSCAPE AREAS ¹	44,918	1.03
PERVIOUS AREA TOTAL ² =	44,918	1.03
IMPERVIOUS		
BUILDING AREAS + DRIVEWAY ³	13,481	0.31
ASSUMED FUTURE CONCRETE FLATWORK ⁴	3,000	0.07
AC PAVEMENT AREAS (ROADWAY) ⁵	0	0.00
IMPERVIOUS AREA TOTAL ⁶ =	16,481	0.38
TOTAL AREA (CHECK) =	61,399	1.41
Notes: 1. Total = DMA #11 Area Less Bldg + Driveway + Patios 2. Total pervious area for DMA #11 used in SWQP - Fo 3. Based on Lot Fit Analysis and assumed driveway co 4. Based on 1,500 SF of future flatwork per lot for pat 5. AC pavement within Reserve Court for DMA #11. 6. Total impervious area for DMA #11 used in SWQP -	rm 3-5, Item 3. nfigurations. ios & walkways in ba	ick & side yards.

	ARI	EA
SURFACE DESCRIPTION	SF	AC
TOTAL DMA #12 AREA =	43,792	1.01
PERVIOUS		
LANDSCAPE AREAS ¹	35,129	0.81
PERVIOUS AREA TOTAL ² =	35,129	0.81
IMPERVIOUS		
BUILDING AREAS + DRIVEWAY ³	7,163	0.16
ASSUMED FUTURE CONCRETE FLATWORK ⁴	1,500	0.03
AC PAVEMENT AREAS (ROADWAY) ⁵	0	0.00
IMPERVIOUS AREA TOTAL ⁶ =	8,663	0.20
TOTAL AREA (CHECK) =	43,792	1.01
<u>Notes:</u> 1. Total = DMA #12 Area Less Bldg + Driveway + Patios &	Walkways.	

Total pervious area for DMA #12 used in SWQP - For	orm 3-5, Item 3.	
3. Based on Lot Fit Analysis and assumed driveway co	nfigurations.	
4. Based on 1,500 SF of future flatwork per lot for pat	ios & walkways in b	ack & side yards.
5. AC pavement within Reserve Court for DMA #12.		
6. Total impervious area for DMA #12 used in SWQP -	Form 3-5, Item 1.	
	AF	REA
SURFACE DESCRIPTION	SF	AC
SURFACE DESCRIPTION	SF	AC
SURFACE DESCRIPTION TOTAL DMA #13 AREA =	SF 53,694	1.23

SURFACE DESCRIPTION	SF	AC
TOTAL DMA #13 AREA =	53,694	1.23
PERVIOUS		
LANDSCAPE AREAS ¹	44,101	1.01
PERVIOUS AREA TOTAL ² =	44,101	1.01
IMPERVIOUS		
BUILDING AREAS + DRIVEWAY ³	8,093	0.19
ASSUMED FUTURE CONCRETE FLATWORK ⁴	1,500	0.03
AC PAVEMENT AREAS (ROADWAY) ⁵	0	0.00
IMPERVIOUS AREA TOTAL ⁶ =	9,593	0.22
TOTAL AREA (CHECK) =	53,694	1.23
Notes:	-	1
1. Total = DMA #13 Area Less Bldg + Driveway + Patios &	Walkways.	

6. Total impervious area for DMA #13 used in SWQP	- Form 3-5, Item 1.
5. AC pavement within Reserve Court for DMA #13.	
4. Based on 1,500 SF of future flatwork per lot for pa	itios & walkways in back & side yard
Based on Lot Fit Analysis and assumed driveway co	onfigurations.
2. Total pervious area for DMA #13 used in SWQP - F	orm 3-5, Item 3.
 Total = DMA #13 Area Less Bldg + Driveway + Patio 	os & Walkways.
Notes:	

SURFACE DESCRIPTION	SF	AC
TOTAL DMA #14 AREA =	23,299	0.53
PERVIOUS		
LANDSCAPE AREAS ¹	10,385	0.24
PERVIOUS AREA TOTAL ² =	10,385	0.24
IMPERVIOUS		
BUILDING AREAS + DRIVEWAY ³	0	0.00
ASSUMED FUTURE CONCRETE FLATWORK ⁴	0	0.00
AC PAVEMENT AREAS (ROADWAY) ⁵	12,914	0.30
IMPERVIOUS AREA TOTAL ⁶ =	12,914	0.30
TOTAL AREA (CHECK) =	23,299	0.53
Notes:		
1. Total = DMA #14 Area Less Bldg + Driveway + Patios	& Walkways.	
2. Total pervious area for DMA #14 used in SWQP - Fo	orm 3-5, Item 3.	
3. Based on Lot Fit Analysis and assumed driveway co	_	
4. Based on 1,500 SF of future flatwork per lot for pat		ack & side yards.
5. AC pavement within Reserve Court + Barton Road for	or DMA #14.	
6. Total impervious area for DMA #14 used in SWQP -	Form 3-5, Item 1.	

SF	AC
192,625	4.42
192,625	4.42
192,625	4.42
0	0.00
0	0.00
0	0.00
0	0.00
192,625	4.42
	192,625 192,625 192,625 0 0 0

I. Total = DMA #14 Area Less Bldg + Driveway + Patios & Walkways. 2. Total pervious area for DMA #14 used in SWQP - Form 3-5, Item 3 3. Based on Lot Fit Analysis and assumed driveway configurations. 4. Based on 1,500 SF of future flatwork per lot for patios & walkways in back & side yards. 5. AC pavement within Reserve Court + Barton Road for DMA #14. 5. Total impervious area for DMA #14 used in SWQP - Form 3-5, Item 1

SW Corner of Rocklin Road and Barton Road Loomis, California

APN: 045-330-022

Proposed By:

PREMIER HOMES 8483 Douglas Plaza Dr Granite Bay, CA 95746

TSD ENGINEERING, INC. expect more.

785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701

JULY 16, 2025 3RD SUBMITTAL

EXH-5

The Reserve

PLACER COUNTY, CA

WATER QUALITY - VEGETATED SWALE CALCULATIONS

 Original Date:
 9/30/2024
 Checked By:
 CSF

 Revised Date:
 7/15/2025
 Prepared By:
 TNJ, JM

					WATER Q	JALITY FL	-OW - H	/DRAULICS		
SWALE I.D.	DMA I.D.	DMA AREA ₁	C Value ₂	INTENSITY ₃	Q_4	DEPT SWA		VELOCITY ₆	LENGTH OF SWALE	SWALE RETENTION TIME ₇
		acres		in/hr	cfs	ft	in	ft/s	ft	min
WQ Swale #1	1	3.01	0.44	0.20	0.26	0.3333	4.0	0.26	397	25
WQ Swale #4	4	3.72	0.40	0.20	0.30	0.3333	4.0	0.30	379	21
WQ Swale #5	5	0.46	0.72	0.20	0.07	0.2083	2.5	0.13	351	45
WQ Swale #6	6	0.45	0.70	0.20	0.06	0.1917	2.3	0.12	126	18

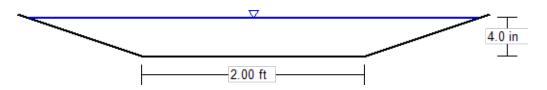
NOTES:

- 1. DMA Area requiring treatment via vegetated swale BMP.
- 2. Composite Runoff Coefficient for subject DMA (See Calulations Page 2).
- 3. Water Quality Flow-Based Intensity per Stormwater Quality Design Manual for Sacramento and South Placer Regions (Roseville), May 2007
- 4. Water Quality Flow Rate \rightarrow Q = CiA
- 5. Water Quality Flow Depth (Reference FlowMaster® Worksheets Appendix G)
- 6. Water Quality Flow Velocity (Reference FlowMaster® Worksheets Appendix G). Velocity ≤ Per Placer County SQDM SDM-6.
- 7. Treatment Contact Time within Swale (Time = Swale Length ÷ Velocity). Time ≥ 10 minutes Per Placer County SQDM SMD-6.

Comp C =	(C1*A1) + (C2*A2)	Where:	C1 =	0.2	\rightarrow	Runoff Coefficient for Landscaped Areas (Pervious)
	A1 + A2	wiicie.	A1 =	2.04	$\stackrel{\checkmark}{\rightarrow}$	Pervious Area for DMA 1 (acres) per Developed DMA SWQ Map - Appendix G
	712 - 712		C2 =	0.95	$\stackrel{'}{\Rightarrow}$	Runoff Coefficient for Impervious Areas
			A2 =	0.97	$\stackrel{\checkmark}{\rightarrow}$	Impervious Area for DMA 1 (acres) per Developed DMA SWQ Map - Appendi
Comp C = _	(0.2*2.7) + (0.95*1.15) 2.7+1.15	\rightarrow	0.44169435			
\ 4						
Comp C =	(C1*A1) + (C2*A2)	Where:	C1 =	0.2	\rightarrow	Runoff Coefficient for Landscaped Areas (Pervious)
	A1 + A2		A1 =	2.75	\rightarrow	Pervious Area for DMA 2 (acres) per Developed DMA SWQ Map - Appendix G
			C2 =	0.95	\rightarrow	Runoff Coefficient for Impervious Areas
			A2 =	0.98	\rightarrow	Impervious Area for DMA 2 (acres) per Developed DMA SWQ Map - Appendi
Comp C = _	(0.2*0.39) + (0.95*0.29) 0.39+0.29	\rightarrow	0.39705094			
\ 5						
Comp C =	(C1*A1) + (C2*A2)	Where:	C1 =	0.2	\rightarrow	Runoff Coefficient for Landscaped Areas (Pervious)
	A1 + A2		A1 =	0.14	\rightarrow	Pervious Area for DMA 5 (acres) per Developed DMA SWQ Map - Appendix G
			C2 =	0.95	\rightarrow	Runoff Coefficient for Impervious Areas
			A2 =	0.32	\rightarrow	Impervious Area for DMA 5 (acres) per Developed DMA SWQ Map - Appendi
Comp C = _	(0.2*1.92) + (0.95*0.71) 1.92+0.71	\rightarrow	0.72173913			
A 6						
Comp C =	(C1*A1) + (C2*A2)	Where:	C1 =	0.2	\rightarrow	Runoff Coefficient for Landscaped Areas (Pervious)
	A1 + A2		A1 =	0.15	\rightarrow	Pervious Area for DMA 6 (acres) per Developed DMA SWQ Map - Appendix G
			C2 =	0.95	\rightarrow	Runoff Coefficient for Impervious Areas
			A2 =	0.3	\rightarrow	Impervious Area for DMA 6 (acres) per Developed DMA SWQ Map - Appendi
Comp C = _	(0.2*0.08) + (0.95*0.09)	\rightarrow	0.7			

Cross Section for Water Quality Swale #1 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.012 ft/ft	
Normal Depth	4.0 in	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.26 cfs	



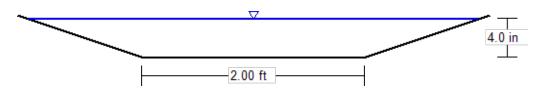
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Worksheet for Water Quality Swale #1 - WQF

Project Description		
Friction Method	Manning	
Solve For	Formula Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.012 ft/ft	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.26 cfs	
Results		
Normal Depth	4.0 in	
Flow Area	1.0 ft ²	
Wetted Perimeter	4.1 ft	
Hydraulic Radius	2.9 in	
Top Width	4.02 ft	
Critical Depth	0.9 in	
Critical Slope	2.238 ft/ft	
Velocity	0.26 ft/s	
Velocity Head	0.00 ft	
Specific Energy	0.34 ft	
Froude Number	0.090	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description		
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	4.0 in	
Critical Depth	0.9 in	
Channel Slope	0.012 ft/ft	
Critical Slope	2.238 ft/ft	

Cross Section for Water Quality Swale #4 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.016 ft/ft	
Normal Depth	4.0 in	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.30 cfs	



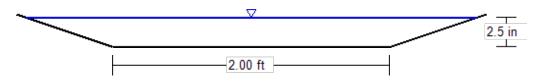


Worksheet for Water Quality Swale #4 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.016 ft/ft	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.30 cfs	
Results		
Normal Depth	4.0 in	
Flow Area	1.0 ft ²	
Wetted Perimeter	4.1 ft	
Hydraulic Radius	3.0 in	
Top Width	4.02 ft	
Critical Depth	1.0 in	
Critical Slope	2.178 ft/ft	
Velocity	0.30 ft/s	
Velocity Head	0.00 ft	
Specific Energy	0.34 ft	
Froude Number	0.104	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		_
Upstream Depth	0.0 in	
Profile Description		
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	4.0 in	
Critical Depth	1.0 in	
Channel Slope	0.016 ft/ft	
Critical Slope	2.178 ft/ft	

Cross Section for Water Quality Swale #5 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.005 ft/ft	
Normal Depth	2.5 in	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.07 cfs	



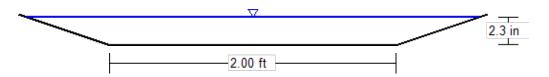


Worksheet for Water Quality Swale #5 - WQF

Project Description		
Friction Method	Manning	
	Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.005 ft/ft	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.07 cfs	
Results		
Normal Depth	2.5 in	
Flow Area	0.6 ft ²	
Wetted Perimeter	3.3 ft	
Hydraulic Radius	2.0 in	
Top Width	3.26 ft	
Critical Depth	0.4 in	
Critical Slope	2.893 ft/ft	
Velocity	0.13 ft/s	
Velocity Head	0.00 ft	
Specific Energy	0.21 ft	
Froude Number	0.054	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description		
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	2.5 in	
Critical Depth	0.4 in	
Channel Slope	0.005 ft/ft	
Critical Slope	2.893 ft/ft	

Cross Section for Water Quality Swale #6 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.005 ft/ft	
Normal Depth	2.3 in	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.06 cfs	





Worksheet for Water Quality Swale #6 - WQF

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.250	
Channel Slope	0.005 ft/ft	
Left Side Slope	3.000 H:V	
Right Side Slope	3.000 H:V	
Bottom Width	2.00 ft	
Discharge	0.06 cfs	
Results		
Normal Depth	2.3 in	
Flow Area	0.5 ft ²	
Wetted Perimeter	3.2 ft	
Hydraulic Radius	1.9 in	
Top Width	3.16 ft	
Critical Depth	0.4 in	
Critical Slope	2.988 ft/ft	
Velocity	0.12 ft/s	
Velocity Head	0.00 ft	
Specific Energy	0.19 ft	
Froude Number	0.054	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description		
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	2.3 in	
Critical Depth	0.4 in	
Channel Slope	0.005 ft/ft	
Critical Slope	2.988 ft/ft	