GEOTECHNICAL ENGINEERING STUDY
FOR
CARMENERE WEST
REVISION 1
3984 Delmar Ave
Loomis, California

Project No. E18269.001
December 2024

YOUNGDAHL
ESTABLISHED 1984

Mima Capitol LLC
4120 Douglas Blvd, #306-175
Granite Bay, California 95746

Project No. E18269.001
Revised 12 December 2024

Attention: Mike Fournier

Subject: **CARMENERE WEST**
3984 Delmar Ave, Loomis, California
GEOTECHNICAL ENGINEERING STUDY – REVISION 1

References:
1. Tentative Map for Delmar Farms, prepared by King Engineering, dated 19 July 2024.
2. Proposal and Contract for Delmar Farms West, prepared by Youngdahl Consulting Group, Inc., dated 11 October 2024 (E18269.001).

Dear Mr. Fournier:

In accordance with your authorization, Youngdahl Consulting Group, Inc. has prepared this geotechnical engineering study for the project site located at 3984 Delmar Ave in Lincoln, California. The purpose of this study was to prepare a site-specific geotechnical report based on new information that can be incorporated into design of the proposed site. To complete this task, our firm completed a subsurface exploration, reviewed the referenced documents, and prepared this report in accordance with the Reference 2 contract.

Based upon our observations, the geotechnical aspects of the site appear to be suitable for support of the proposed structure provided the recommendations presented in this report are incorporated into the design and construction. Geotechnical conditions associated with site development are anticipated to include the potential for perched water conditions, and excavatability of the underlying bedrock materials, the placement of engineered fills, improvement for drainage controls, and the construction of foundations.

Due to the non-uniform nature of soils, other geotechnical issues may become more apparent during grading operations which are not listed above. The descriptions, findings, conclusions, and recommendations provided in this report are formulated as a whole; specific conclusions or recommendations should not be derived or used out of context. Please review the limitations and uniformity of conditions section of this report.

This report has been prepared for the exclusive use of Mima Capitol LLC and their consultants, for specific application to this project, in accordance with generally accepted geotechnical engineering practice. Should you have any questions or require additional information, please contact our office at your convenience.

Very truly yours,
Youngdahl Consulting Group, Inc.

Pavel Fomin
Staff Engineer

Reviewed by:

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GEOTECHNICAL ENGINEERING STUDY
FOR
CARMENERE WEST

1.0 INTRODUCTION

This report presents the results of our geotechnical engineering study performed for the proposed improvements planned to be constructed along Delmar Avenue in Loomis, California. The vicinity map provided on Figure A-1, Appendix A shows the approximate project location.

Project Understanding

We understand that the proposed project consists of the development of a large lot subdivision with 8 lots. The residences are expected to be of one to two-story, wood-framed single-family residences along and associated construction includes asphalt concrete roadways and underground utility services. The structures are expected to be supported by conventional shallow foundations and concrete slab-on-grade floors.

Based on a review of the tentative maps, to achieve the proposed building pad grades, cuts and fills on the order of about 5 to 10 feet are anticipated. In addition, excavations for utilities are anticipated to be around 5 to 15 feet from finished grades.

Background

Based on a review of aerial imagery from Google Earth, the subject property was rural irrigated agricultural land since the earliest imagery from 1985. It is our understanding a portion of the site was used for cattle grazing.

If studies or plans pertaining to the site exist and are not cited as a reference in this report, we should be afforded the opportunity to review and modify our conclusions and recommendations as necessary.

Purpose and Scope

Youngdahl Consulting Group, Inc. has prepared this report to provide geotechnical engineering recommendations and considerations for incorporation into the design and development of the site. The following scope of services were developed and performed for preparation of this report:

- A review of geotechnical and geologic data available to us at the time of our study;
- Performance of a field study consisting of a site reconnaissance and subsurface explorations to observe and characterize the subsurface conditions;
- Laboratory testing on representative samples collected during our field study;
- Evaluation of the data and information obtained from our field study, laboratory testing, and literature review for geotechnical conditions;
- Development of the following geotechnical recommendations and considerations regarding earthwork construction including, existing fills, site preparation and grading, engineered fill criteria, seasonal moisture conditions, compaction equipment, excavation characteristics, slope configuration and grading, and drainage;
- Development of geotechnical design criteria for code-based seismicity, foundations, slabs on grade, retaining walls, and pavements;
- Preparation of this report summarizing our findings, conclusions, and recommendations regarding the above-described information.

2.0 SITE CONDITIONS

The following section describes our findings regarding the site conditions that we observed during our site reconnaissance and subsequent subsurface exploration.

Surface Observations

The project site is located at 3984 Delmar Avenue in Loomis, California and is bounded by Delmar Avenue to the east, a private driveway, a pond, and rural residences to the north, rural residences and train tracks to the west, and a subdivision and rural residences to the south. A drainage channel traverses north to south through the middle of the property. An underground pipe drains the northern off-site pond along the drainage alignment and daylights at the southern portion of the site. Topography at the site consists of gently sloping terrain at varying, typically shallow gradients. At times, the drainage channel banks sloped near vertical from prolonged erosion. At the time of our visit on 23 October 2024, the vegetation at the site consisted of seasonal grasses, shrubs, and mature trees within the proposed development site. Throughout the site, multiple rock outcroppings protrude from the soil surface.

The center of the site has a few old barn structures, anticipated to be demolished during development. Irrigation nozzles were observed through out the site.

Subsurface Conditions

Our field study included a site reconnaissance and subsurface exploration program by representatives of our firm. The exploration program included the excavation of 13 test pits to evaluate the subsurface soil conditions and the depth to rock. The approximate locations of the test pits are presented on Figure A-2, Appendix A.

The subsurface soil conditions at the site appeared to consist of a two-to-three-foot layer of native soils overlying weathered and decomposed granitic rock. Test Pit TP-13 was observed with 3 feet of fill in a medium dense condition. The native soils were observed in a medium dense and dry condition. The underlying bedrock was observed in a soft to hard condition. Excavations to depths on the order of 10 feet were common at our test pits using a Caterpillar 315 tracked excavator. The soil profile observed in our test pits are provided in Figures A-3 to A-22, Appendix A.

Groundwater Conditions

A permanent groundwater table was not encountered at the project site and is expected to be relatively deep. Perched, shallow water was encountered at depths of 7 to 10 feet in TP 6 through TP-8, and TP-13 and has been observed in the region depending on the time of year and annual precipitation. Year-round ponds are common in the region but tend to change elevation with precipitation. The presence of perched water can vary because of many factors such as, the proximity to rock, topographic elevations, and the presence of utility trenches. Some evidence of past repeated exposure to subsurface water may include black staining, clay deposits, and surface markings indicating previous seepage. Based on our experience in the area, water may be perched on the bedrock horizon found beneath the site and could vary through the year with higher concentrations during or following precipitation.

3.0 GEOTECHNICAL SOIL CHARACTERISTICS

The geotechnical soil characteristics presented in this section of the report are based on laboratory testing and observation of samples collected from subsurface soils.

Laboratory Testing
Laboratory testing of the collected samples was directed towards evaluating the physical and engineering properties of the soil underlying the site. A description of the tests performed for this project and the associated test results are presented in Appendix B. In summary, the following tests were performed for the preparation of this report:

Table 1: Laboratory Tests

Laboratory Test	Test Standard	Summary of Results	
Direct Shear	ASTM D3080	TP-4 @ 0-2’	Φ = 34.9°, c = 208 psf (90%RC)
Resistance “R” Value	CTM 301	TP-1 @ 0-3’	55
		TP-12 @ 0-3’	59
Maximum Dry Density	ASTM D1557	TP-4 @ 0-2’	DD = 125.2 pcf, MC = 10.3%
Soil Sieve	ASTM D6913, Method A	TP-12 @ 0-3’	0%>No.4 & 25.0%< No.200
Corrosivity Suite	CA DOT Tests 417, 422 and 643	See Soil Corrosivity Section	

Soil Expansion Potential
The materials encountered in our explorations were generally non-plastic (rock, sand, and non-plastic silt). The non-plastic materials are generally considered to be non-expansive; therefore, we do not anticipate that special design considerations for expansive soils will be necessary for the design or construction of the proposed improvements. If necessary, recommendations can be made based on our observations at the time of construction should expansive soils be encountered at the project site which were not encountered during our study.

Soil Corrosivity
A corrosivity testing suite consisting of soil pH, resistivity, sulfate, and chloride content tests were performed on selected soil samples collected during our site exploration. We are not corrosion specialists and recommend that the results be evaluated by a qualified corrosion expert. The laboratory test results (provided by Sunland Analytical, Inc.) are provided in Appendix B and are summarized in Table 2, below.

Table 2: Corrosivity Summary

Location	Depth (ft)	Soil pH	Minimum Resistivity ohm-cm (x1000)	Chloride (ppm)	Sulfate (ppm)	Caltrans Environment	ACI Environment
TP-4	0-2	5.86	2.68	1.9	101.9	Non-Corrosive	S0 (Not a Concern)
TP-9	5-10	5.62	16.08	1.2	4.3	Non-Corrosive	S0 (Not a Concern)

According to Caltrans Corrosion Guidelines Version 3.2, May 2021, the test results appear to indicate a non-corrosive environment. According to the 2022 California Building Code Section 1904.1 and ACI 318-19 Table 19.3.1.1, the test results indicate the onsite soils have a negligible potential for sulfide attack of concrete. Accordingly, Type I/II Portland cement is appropriate for use in concrete construction. A certified corrosion engineer should be consulted to review the above tests and site conditions in order to develop specific mitigation recommendations if metallic pipes or structural elements are designed to be in contact with or buried in soil.

4.0 GEOLOGY AND SEISMICITY

The geologic portion of this report includes a review of geologic data pertinent to the site based on an interpretation of our observations of the surface exposures and our observations in our exploratory test pits excavated during the field study.

Geologic Conditions

According to the Geologic map of the Sacramento deposits of the Sacramento quadrangle, California (Wagner, 1981), the subject property and vicinity are underlain by Mesozoic plutonic dioritic rocks of the Penryn Pluton Formation (Mzg). The dioritic rocks are consistent with our observations and experience.

Naturally Occurring Asbestos

Asbestos is classified by the EPA as a known human carcinogen. Naturally occurring asbestos (NOA) has been identified as a potential health hazard. The California Geological Survey published a map in 2008 that indicates the likelihood for NOA in Placer County. This project has been identified as being within an area least likely to contain naturally occurring asbestos (NOA). Disclosure is required for properties containing asbestos.

Seismicity

Our evaluation of seismicity for the project site included reviewing existing fault maps and obtaining seismic design parameters from the USGS online calculators and databases. For the purpose of this study, we used a latitude and longitude of 38.812907, -121.222883 to identify the project site.

Alquist-Priolo Regulatory Faults

Based upon the records currently available from the California Department of Conservation, the project site is not located within an Alquist-Priolo Regulatory Review Zone and there are no known faults located at the subject site. We do not anticipate special design or construction requirements for faulting at this project site.

Code Based Seismic Criteria

Based upon the subsurface conditions encountered during our study and our experience in the area, the site should be classified as Site Class C. The structural engineer should review the conditions of the exception and the final choice of design parameters remains the purview of the project structural engineer.

Table 3: Seismic Design Parameters*

	Reference	Seismic Parameter	Recommended Value
ASCE 7-16	Table 20.3-1	Site Class	C
	Figure 22-7	Maximum Considered Earthquake Geometric Mean (MCEC) PGA	0.185g
	Table 11.8-1	Site Coefficient F_{PGA}	1.215
	Equation 11.8-1	$PGA_M = F_{PGA}\ PGA$	0.225g
2022 CBC	Figure 1613.2.1(1)	Short-Period MCE at 0.2s, S_S	0.433g
	Figure 1613.2.1(3)	1.0s Period MCE, S_1	0.218g
	Table 1613.2.3(1)	Site Coefficient, F_a	1.300
	Table 1613.2.3(2)	Site Coefficient, F_v	1.500
	Equation 16-20	Adjusted MCE Spectral Response Parameters, $S_{MS} = F_aS_s$	0.563g
	Equation 16-21	Adjusted MCE Spectral Response Parameters, $S_{M1} = F_vS_1$	0.327g
	Equation 16-22	Design Spectral Acceleration Parameters, $S_{DS} = ⅔S_{MS}$	0.375g
	Equation 16-23	Design Spectral Acceleration Parameters, $S_{D1} = ⅔S_{M1}$	0.218g
	Table 1613.2.5(1)	Seismic Design Category (Short Period), Occupancy I to III	C
	Table 1613.2.5(1)	Seismic Design Category (Short Period), Occupancy IV	D
	Table 1613.2.5(2)	Seismic Design Category (1-Sec Period), Occupancy I to IV	D

*Based on the online calculator available at https://earthquake.usgs.gov/ws/designmaps/

Earthquake Induced Liquefaction, Settlement, and Surface Rupture Potential
Liquefaction is the sudden loss of soil shear strength and sudden increase in porewater pressure caused by shear strains, as could result from an earthquake. Research has shown that saturated, loose to medium-dense sands with a silt content less than about 25 percent and located within the top 40 feet are most susceptible to liquefaction and surface rupture/lateral spreading.

Due to the absence of permanently elevated groundwater table, the relatively low seismicity of the area and the relatively shallow depth to rock, the potential for seismically induced damage due to liquefaction, surface ruptures, and settlement is considered low. For the above-mentioned reasons mitigation for these potential hazards is not considered necessary for the development of this project.

Static and Seismically Induced Slope Instability
The existing slopes on the project site were generally observed to have adequate vegetation on the slope face, appropriate drainage away from the slope face, and no apparent tension cracks or slump blocks in the slope face or at the head of the slope. The creek area exhibited some steep banks which could have minor localized instability. However, these are not expected to be near structures or would be addressed during grading. No other indications of slope instability such as seeps or springs were observed. Additionally, due to the absence of permanently elevated groundwater table, the relatively low seismicity of the area, and the relatively shallow depth to bedrock, the potential for seismically induced slope instability for the existing slopes is considered low.

5.0 DISCUSSION AND CONCLUSIONS
Based upon the results of our field explorations, findings, and analysis described above, it is our opinion that construction of the proposed improvements is feasible from a geotechnical standpoint, provided the recommendations contained in this report are incorporated into the design plans, specifications, and implemented during construction. The native soils, once processed and compacted as recommended below, may be considered "engineered" and suitable for support of the planned improvements.

Geotechnical Considerations for Development

The project site is generally comprised of soil over shallow rock which is considered suitable for support of the proposed improvements. Based on our evaluation, we anticipate that the earthwork operations my be subject to similar conditions as other projects in the region. This includes potential excavations into the underlying hard rock, perched water, and excess water entering utility trenches. The potential for encountering perched water is expected to increase where excavations are proposed to establish lower grades. The intentional implementation of subdrainage should be anticipated in some areas. The contractor should be prepared to pump and control seepage during grading the installation of underground utilities.

Based on our experience with neighboring developments some blasting operations may be necessary to achieve the anticipated target grading. The contractor should be prepared to accommodate this potential.

6.0 SITE GRADING AND EARTHWORK IMPROVEMENTS

Excavation Characteristics

Based on our experience with the bedrock conditions on the project site, we anticipate that the underlying bedrock material can likely be excavated to depths of several feet using dozers equipped with rippers. We expect that the upper, weathered portion of the rock, will require use of a Caterpillar D9 equipped with a single or multiple shank rippers, or similar equipment.

Where hard rock cuts in fractured rock are proposed, the orientation and direction of excavation/ripping will likely play a large role in the rippability of the material. When hard rock is encountered, we should be contacted to provide additional recommendations prior to performing an alternative such as blasting.

Utility trenches will likely encounter hard rock excavation conditions especially in deeper cut areas. Utility contractors should be prepared to use special rock trenching equipment such as large excavators (Komatsu PC400 or CAT 345 or equivalent). Blasting to achieve utility line grades, especially in planned cut areas, cannot be precluded. Water inflow into any excavation approaching the hard rock surface is likely to be experienced and has been observed on other nearby developments even in the summer and fall months. Pre-ripping during mass grading may be beneficial and should be considered with the Geotechnical Engineer prior to, or during mass grading.

Soil Moisture Considerations

The compaction of soil to a desired relative compaction is dependent on conditioning the soil to a target range of moisture content. Moisture contents that are excessively dry or wet could limit the ability of the contractor to compact soils to the requirements for engineered fill. When dry, moisture should be added to the soil and the soils blended to improve consistency. Wet soil will need to be dried to become compactable. Generally, this includes blending and working the soil to avoid trapping moisture below a dryer surficial crust. Other options are available to reduce the time involved (e.g., cement treatment) but typically have higher costs and require more evaluation prior to implementation.

The largest contributor to excessive soil moisture is generally precipitation and seepage during the rainy season. In recognition of this, we suggest that consideration be given to the seasonal limitations and costs of winter grading operations on the site. Special attention should be given regarding the drainage of the project site. If the project is expected to work through the wet season, the contractor should install appropriate temporary drainage systems at the construction site and should minimize traffic over exposed subgrades due to the moisture-sensitive nature of the on-site soils. During wet weather operations, the soil should be graded to drain and should be sealed by rubber tire rolling to minimize water infiltration.

Site Preparation

Preparation of the project site should involve demolition operation, site drainage controls, dust control, clearing and stripping, overexcavation and recompaction of loose native soils, and exposed grade compaction considerations. The following paragraphs state our geotechnical comments and recommendations concerning site preparation.

Demolition

As part of the demolition operation, structural improvement, or site improvement elements (including underground utilities) should be exhumed and removed from the site. In addition, any underground storage tanks, abandoned wells, or other utilities not intended for reuse should be removed or backfilled in accordance with the appropriate regulations.

Concrete and asphalt separated from the other debris, and adequately broken down in particle size, may be mixed thoroughly with soil and placed as engineered fill as described below. If this option is exercised, a representative from our firm should be contacted to observe the adequacy of grading operations associated with the breaking and mixing of these elements.

Site Drainage Controls

We recommend that initial site preparation involve intercepting and diverting any potential sources of surface or near-surface water within the construction zones. Because the selection of an appropriate drainage system will depend on the water quantity, season, weather conditions, construction sequence, and methods used by the contractor, final decisions regarding drainage systems are best made in the field at the time of construction. All drainage and/or water diversion performed for the site should be in accordance with the Clean Water Act and applicable Storm Water Pollution Prevention Plan.

Dust Control

Dust control provisions should be provided for as required by the local jurisdiction's grading ordinance (i.e., water truck or other adequate water supply during grading). Dust control is the purview of the grading contractor.

Clearing and Stripping of Organic Materials

Clearing and stripping operations should include the removal of all organic laden materials including trees, bushes, root balls, root systems, and any soft or loose soil generated by the removal operations. Short or mowed dry grasses may be pulverized and lost within fill materials provided no concentrated pockets of organics result. It is the responsibility of the grading contractor to remove excess organics from the fill materials. **No more than 2 percent of organic material, by weight, should be allowed within the fill materials at any given location.** Preserved trees may require tree root protection which should be addressed on an individual basis by a qualified arborist.

Our recommendations are based on limited windows into the surface and interpretations thereof; therefore, a representative of our firm should be present during site clearing operations to identify the location and depth of potential fills or loose soils, some of which may not have been found during our evaluation. We should also be present to observe removal of deleterious materials, and to identify any existing site conditions which may require mitigation or further recommendations prior to site development.

Overexcavation and Recompaction of Undocumented Fill and/or Loose Native Soils
Following general site clearing, all existing undocumented fill, old ponds, and loose or saturated native soils within the development footprint should be overexcavated down to firm native materials and backfilled with engineered fill as detailed in the engineered fill section below. Any depressions extending below final grade resulting from the removal of fill materials or other deleterious materials should be properly prepared as discussed below and backfilled with engineered fill.

Lots 6, 7, and 8 appear to have had more trees and an orchard historically. While we found a shallow depth to rock, it may be possible that the soil was disturbed to plant trees or root balls may be present. Lot 1 appears to have similar conditions prior to 1952. The contractor should be prepared to overexcavate 1 to 2 feet as conditions require.

Exposed Grade Compaction
Exposed soil grades following initial site preparation activities and overexcavation operations should be scarified to a minimum depth of 8 inches and compacted to the requirements for engineered fill. Generally, where rock conditions are exposed, no scarification should be necessary; however, these surfaces should be moisture conditioned and compacted to mitigate disturbance resulting from site preparation. Prior to placing fill, the exposed grades should be in a firm and unyielding state. Any localized zones of soft or pumping soils observed within the exposed grade should either be scarified and recompacted or be overexcavated and replaced with engineered fill as detailed in the engineered fill section below.

Engineered Fill Criteria
All materials placed as fills on the site should be placed as “Engineered Fill" which is observed, tested, and compacted as described in the following paragraphs.

Suitability of Onsite Materials
We expect that soil generated from excavations on the site, excluding deleterious material, may be used as engineered fill provided the material does not exceed the maximum size specifications listed below.

Rock fragments or boulders exceeding 24 inches in maximum dimension should not be placed within the upper five feet of site grades or utility corridors. The upper two feet of the site grades and within the zone of proposed underground facilities should consist of predominantly rocks and rock fragments less than 12 inches in maximum dimension. Boulders over 24 inches in maximum dimension should be placed within the deeper portions of fill embankments below a depth of 5 feet and a minimum of 5 feet from the finish slope face. The individual boulders should be spaced such that compaction of finer rock and soil materials between the boulders can be achieved with the equipment being used for compaction. Materials placed between the boulders should consist of predominantly soil and rock less than 12 inches in maximum dimension. The soil/rock mixture should be thoroughly mixed and placed between the boulders so as to preclude nesting or the formation of voids. Should insufficient deep fill areas exist for oversize rock disposal, the contractor should either dispose of the excess materials to an offsite location or mechanically reduce the rocks to less than 12 inches.

Compaction Equipment
Where a significant quantity of rock materials that will comprise the fills on the project site, a Caterpillar 825 steel-wheel compactor or approved equivalent should be employed as a minimum to facilitate breakdown of oversize bedrock materials and generation of soil fines during the fill placement process. If the quantity of rock fragments in the fills preclude traditional compaction

testing, then the proposed fills should be compacted using method specifications as indicated below.

In focused or isolated areas where significant rock quantities will not be present, we anticipate that a large vibratory padded drum compactor or approved equivalent will be capable of achieving the compaction requirements for engineered fill provided the soil is placed and compacted within 0 to 3 percent of the optimum moisture content as determined by the ASTM D1557 test method and in lifts not greater than 12 inches in uncompacted thickness. The use of handheld equipment such as jumping jack or plate vibration compactors may require thinner lifts of 6 inches or less to achieve the desired relative compaction parameters.

Fill Placement and Compaction

Engineered fills should be placed in thin horizontal lifts not to exceed 12 inches in uncompacted thickness. If the contractor can achieve the recommended relative compaction using thicker lifts, the method may be judged acceptable based on field verification by a representative of our firm using standard density testing procedures. Lightweight compaction equipment may require thinner lifts to achieve the recommended relative compaction. Fills should have a maximum particle size of 12 inches unless approved by our firm.

The relative compaction of engineered fills is based on the maximum density and optimum moisture determined through the ASTM D1557 test method. Depending on the moisture condition of the soils, the engineered fills may require moisture conditioning to be within a suitable compaction range.

Table 4: Recommended Relative Compaction

Fill Materials	Relative Compaction	Method
Engineered Fill	90 percent	ASTM D1557
Utility Trench Backfill*	95 percent	ASTM D1557
Subgrade	95 percent	ASTM D1557
Aggregate Baserock Grade	95 percent	ASTM D1557
Asphalt Concrete Pavement	92 to 96 percent	ASTM D2041 or CTM 309

* Unless otherwise noted by the governing agency.

Our firm should be requested for consultation, observation, and testing for the earthwork operations prior to the placement of any fills. Fill soil compaction should be evaluated by means of in-place density tests performed during fill placement so that adequacy of soil compaction efforts may be evaluated as earthwork progresses. If performed, method specification methods will likely include the excavation of test pits within the fill materials to observe and document that a uniform over-optimum moisture condition, and absence of large and/or concentrated voids has been achieved prior to additional fill placement.

Method Specification

Soils exceeding 30 percent rock by mass may be considered non-testable by conventional methods. The materials may be placed as engineered fill if placed in accordance with the following method specification during full time observation by a representative of our firm.

Soils should be moisture conditioned and compacted in place by a minimum of six completely covering passes with a Caterpillar 825, or approved equivalent. The compactor's last three passes should be at 90 degrees to the initial passes. Engineered fill should be constructed in lifts not exceeding 12 inches in uncompacted thickness, moisture conditioned and compacted in accordance with the above specification. Additional passes as deemed necessary during fill placement to achieve the desired condition based upon field conditions may be recommended.

Import Materials
The recommendations presented in this report are based on the assumption that the import materials will be similar to the materials present at the project site. High quality materials are preferred for import; however, these materials can be more dependent on source availability. Import material should be approved by our firm prior to transporting it to the project site.

Material for this project should consist of a material with the geotechnical characteristics presented below. If these requirements are not met, additional testing and evaluation may be necessary to determine the appropriate design parameters for foundations, pavement, and other improvements.

Table 5: Select Import Criteria

Behavior Property	Reference Document	Recommendation
Direct Shear Strength	ASTM D3080	≥ 34° when compacted
Plasticity Index	ASTM D4318	≤ 12
Expansion Index	ASTM D4829	≤ 20
Sieve Analysis	ASTM D1140	Not more than 30% Passing the No. 200 sieve
Maximum Particle Size	ASTM D1140	12”
Resistance Value	CTM 301	≥ 30

Underground Improvements
Trench Excavation
Trenches or excavations in soil should be shored or sloped back in accordance with current Cal/OSHA regulations prior to persons entering them. Where clay rind in combination with moist conditions is encountered in fractured bedrock, the project engineering geologist should be consulted for appropriate mitigation measures. The potential use of a shield to protect workers cannot be precluded. Refer to the Excavation Characteristics section of Site Grading and Improvements of this report for anticipated excavation conditions.

Backfill Materials
Backfill materials for utilities should conform to the requirements of the local jurisdiction. It should be realized that permeable backfill materials will likely carry water at some time in the future.

When backfilling within structural footprints, compacted low permeability materials are recommended to be used a minimum of 5 feet beyond the structural footprint to minimize moisture intrusion. If a permeable material is used as backfill within this zone, subdrainage mitigation may be required. In addition, if the structure is oriented below the roadway and associated utilities, grout cutoffs and/or plug and drains around all utility penetrations are useful to keep moisture out from underneath the structure.

A common problem occurs on sites graded with large equipment and rocky fill materials where the excavated spoils from the site utilities are too rocky to place as engineered fill back in the trench with the common compaction practices employed by the subcontractors installing these utilities. We recommend that, where excavated soils are too rocky to place and compact to a tight condition with low void space, these materials be replaced with a proper import material for compaction.

Backfill Compaction

Backfill compaction should conform to the requirements of the local jurisdiction or to the recommendations of this report, whichever is greater. Where backfill compaction is not specified by the local jurisdiction, the backfill should be compacted to achieve the minimum relative compactions specified above.

Due to the perched water conditions and water commonly encountered in the region and observed at the site, the contractor should be prepared to provide crushed rock backfill wrapped with a geotextile filter fabric (e.g., Mirafi 140N or equivalent) or other approval methods in accordance with the City of Lincoln.

Exposure to Water

The configuration of a trench increases the likelihood that the trench may be exposed to or retain water. The presence of water can adversely impact the performance of the trench by increasing the potential for the transmission of water to undesired outlets and settlement, even when compacted to the requirements of engineered fill. The contractor should consider these conditions when managing water during interim and post construction periods. This topic is discussed further in the Drainage section of this report.

Slope Configuration and Grading

The project site is proposed to have cuts and fill with a maximum slope orientation of 2H:1V (Horizontal:Vertical). Generally, a cut slope orientation of 2H:1V is considered stable with the material types encountered on the site. A fill slope constructed at the same orientation is considered stable if compacted to the engineered fill recommendations as stated in the recommendations section of this report. All slopes should have appropriate drainage and vegetation measures to minimize erosion of slope soils.

Placement of Fills on Slopes

Placement of fill material on natural slopes should be stabilized by means of keyways and benches. Where the slope of the original ground equals or exceeds 5H:1V, a keyway should be constructed at the base of the fill. The keyway should consist of a trench excavated to a depth of at least 2 feet into firm, competent materials. The keyway trench should be at least 10 feet wide or as designated by our firm based on the conditions at the time of construction. Benches should be cut into the original slope as the filling operation proceeds. Each bench should consist of a level surface excavated at least 6 feet horizontally into firm soils or 4 feet horizontally into rock. The rise between successive benches should not exceed 36 inches. The need for subdrainage should be evaluated at the time of construction. Refer to Figure C-1 in Appendix C for typical keyway and bench construction.

Slope Face Compaction

All slope fills should be laterally overbuilt and cut back such that the required compaction is achieved at the proposed finish slope face. As a less preferable alternative, the slope face could be track walked or compacted with a wheel. If this second alternative is used, additional slope maintenance may be necessary.

Slope Drainage

Surface drainage should not be allowed to flow uncontrolled over any slope face. Adequate surface drainage control should be designed by the project civil engineer in accordance with the latest applicable edition of the CBC. All slopes should have appropriate drainage and vegetation measures to minimize erosion of slope soils.

7.0 DESIGN RECOMMENDATIONS

The contents of this section include recommendations for foundations, slabs-on-grade, retaining walls, pavements, and drainage.

Shallow Conventional Foundations

Shallow conventional foundation systems are considered suitable for construction of the planned improvements, provided that the site is prepared in accordance with the recommendations discussed in Section 6.0 of this report.

The provided values do not constitute a structural design of foundations which should be performed by the structural engineer. In addition to the provided recommendations, foundation design and construction should conform to applicable sections of the 2022 California Building Code.

Foundation Capacities

The foundation bearing and lateral capacities are presented in the table below. The allowable bearing capacity is for support of dead plus live loads based on the foundation configuration presented in this report. The allowable capacity may be increased by 1/3 for short-term wind and seismic loads. Lateral forces on structures may be resisted by passive pressure acting against the sides of shallow footings and/or friction between the foundation bearing material and the bottom of the footing. Section 1806.3 of the 2022 CBC allows for the combination of the friction factor and passive resistance value to lateral resistance. Consideration should be given to ignoring passive resistance where soils could be disturbed later or within 6 feet horizontally of the slope face.

Table 6: Foundation Capacities

Soil Type	Design Condition	Design Value	Applied Factor of Safety
Engineered Fill or Firm Native Soil	Allowable Bearing Capacity	2,500 psf	3.0
	Allowable Friction Factor*	0.45	1.5
	Allowable Passive Resistance	280 psf/ft	1.5
* Friction Factor is calculated as tan(ϕ)			

Foundation Settlement

A total settlement of less than 1 inch is anticipated; a differential settlement of 0.5 inches in 25 feet is anticipated where foundations are bearing on like materials. The settlement criteria are based upon the assumption that foundations will be sized and loaded in accordance with the recommendations in this report.

Foundation Configuration

Conventional shallow foundations should be a minimum of 12 inches wide and founded a minimum of 12 inches below the lowest adjacent soil grade for one-and-two-story structures. Isolated pad foundations should be a minimum of 24 inches in plan dimension. A grade beam, having the same depth as the continuous footings, should also be cast across the vehicle openings of the residence garage.

Foundation reinforcement should be provided by the structural engineer. The reinforcement schedule should account for typical construction issues such as load consideration, concrete cracking, and the presence of isolated irregularities. At a minimum, we recommend that continuous footing foundations be reinforced with two No. 4 reinforcing bars, one located near the bottom of the footing and one near the top of the stem wall.

Foundation Influence Line and Slope Setback
All footings should be founded below an imaginary 2H:1V plane projected up from the bottoms of adjacent footings and/or parallel utility trenches, or to a depth that achieves a minimum horizontal clearance of 6 feet from the outside toe of the footings to the slope face, whichever requires a deeper excavation.

Subgrade Conditions
Footings should never be cast atop soft, loose, organic, slough, debris, nor atop subgrades covered by ice or standing water. A representative of our firm should be retained to observe all subgrades during footing excavations and prior to concrete placement so that a determination as to the adequacy of subgrade preparation can be made.

Shallow Footing / Stemwall Backfill
All footing/stemwall backfill soil should be compacted to the criteria for engineered fill as recommended in Section 6.0 of this report.

Slab-on-Grade Construction
It is our opinion that soil-supported slab-on-grade floors could be used for the main floor of the structure, contingent on proper subgrade preparation. Often the geotechnical issues regarding the use of slab-on-grade floors include proper soil support and subgrade preparation, proper transfer of loads through the slab underlayment materials to the subgrade soils, and the anticipated presence or absence of moisture at or above the subgrade level. We offer the following comments and recommendations concerning support of slab-on-grade floors. The slab design (concrete mix design, curing procedures, reinforcement, joint spacing, moisture protection, and underlayment materials) is the purview of the project Structural Engineer.

Slab Subgrade Preparation
All subgrades proposed to support slab-on-grade floors should be prepared and compacted to the requirements of engineered fill as discussed in Section 6.0 of this report.

Slab Underlayment
As a minimum for slab support conditions, the slab should be underlain by a minimum 4-inch-thick crushed rock layer that is covered by a minimum 10-mil thick moisture retarding plastic membrane. The membrane may only be functional when it is above the vapor sources. The bottom of the crushed rock layer should be above the exterior grade to act as a capillary break and not a reservoir, unless it is provided with an underdrain system. The slab design and underlayment should be in accordance with ASTM E1643 and E1745.

An optional 1-inch blotter sand layer placed above the plastic membrane, is sometimes used to aid in curing of the concrete. Although historically common, this blotter layer is not currently included in slabs designed according to the 2022 Green Building Code. When omitted, special wet curing procedures will be necessary. If installed, the blotter layer can become a reservoir for excessive moisture if inclement weather occurs prior to pouring the slab, excessive water collects in it from the concrete pour, or an external source of water enters above or bypasses the membrane.

Our experience has shown that vapor transmission through concrete is controlled through proper concrete mix design. As such, proper control of moisture vapor transmission should be considered in the design of the slab as provided by the project architect, structural or civil engineer. It should be noted that placement of the recommended plastic membrane, proper mix design, and proper slab underlayment and detailing per ASTM E1643 and E1745 will not provide a waterproof condition. If a waterproof condition is desired, we recommend that a waterproofing expert be consulted for slab design.

Slab Thickness and Reinforcement

Geotechnical reports have historically provided minimums for slab thickness and reinforcement for general crack control. The concrete mix design and construction practices can additionally have a large impact on concrete crack control. All concrete should be anticipated to crack. As such, these minimums should not be considered to be standalone items to address crack control, but are suggested to be considered in the slab design methodology.

In order to help control the growth of cracks in interior concrete from becoming significant, we suggest the following minimums. Interior concrete slabs-on-grade not subject to heavy loads, should be a minimum of 4-inches thick and reinforced. A minimum of No. 3 deformed reinforcing bars placed at 24 inches on center both ways, at the center of the structural section is suggested. Joint spacing should be provided by the structural engineer. Troweled joints recovered with paste during finishing or “wet sawn” joints should be considered every 10 feet on center. Expansion joint felt should be provided to separate floating slabs from foundations and at least at every third joint. Cracks will tend to occur at recurrent corners, curved or triangular areas and at points of fixity. Trim bars can be utilized at right angle to the predicted crack extending 40 bar diameters past the predicted crack on each side.

Vertical Deflections

Soil-supported slab-on-grade floors can deflect downward when vertical loads are applied, due to elastic compression of the subgrade. For preliminary design of concrete floors, a modulus of subgrade reaction of k = 150 psi per inch would be applicable for engineered fills.

Exterior Flatwork

Exterior concrete flatwork is recommended to have a 4-inch-thick rock cushion. This could consist of vibroplate compacted crushed rock or compacted ¾-inch aggregate baserock. If exterior flatwork concrete is against the floor slab edge without a moisture separator it may transfer moisture to the floor slab. Expansion joint felt should be provided to separate exterior flatwork from foundations and at least at every third joint. Contraction / groove joints should be provided to a depth of at least 1/4 of the slab thickness and at a spacing of less than 30 times the slab thickness for unreinforced flatwork, dividing the slab into nearly square sections. Cracks will tend to occur at recurrent corners, curved or triangular areas and at points of fixity. Trim bars can be utilized at right angle to the predicted crack extending 40 bar diameters past the predicted crack on each side.

Retaining Walls

Our design recommendations and comments regarding retaining walls for the project site are discussed below. *Retaining wall foundations should be designed in accordance with the Shallow Conventional Foundations section above.*

Retaining Wall Lateral Pressures

Based on our observations and testing, the retaining wall should be designed to resist lateral pressure exerted from a soil media having an equivalent fluid weight provided in the table below.

The values presented below are not factored and are for conditions when firm native soil or engineered fill is used within the zone behind the wall defined as twice the height of the retaining wall. Additionally, the values do not account for the friction of the backfill on the retaining wall which may or may not be present depending on the wall materials and construction.

The lateral pressures presented in the table below include recommendations for earthquake loading which is required for structures to be designed in Seismic Design Categories D, E, or F per Section 1803.5.12.1 of the 2022 California Building Code. The lateral pressures presented have been calculated using the Mononobe-Okabe Method derived from Wood (1973) and modified by Whitman et al. (1991). The values are intended to be used as the multiplier for uniformly distributed loads and the parameter "H" is the total height of the wall including the footing but excluding any key, if used.

Table 7: Retaining Wall Pressures (Drained)

<table>
<tr><th>Wall Type</th><th>Wall Slope Configuration</th><th>Equivalent Fluid Weight (pcf)</th><th>Surcharge Load (psf)*</th><th>Lateral Pressure Coefficient</th><th colspan="2">Earthquake Loading (plf)</th></tr>
<tr><td rowspan="2">Free Cantilever</td><td>Flat</td><td>37</td><td>per stuctural</td><td>0.28</td><td>3H²</td><td rowspan="3">Applied 0.6H above the base of the wall</td></tr>
<tr><td>2H:1V</td><td>55</td><td>per structural</td><td>0.42</td><td rowspan="2">15H²</td></tr>
<tr><td>Restrained*</td><td>Flat</td><td>58</td><td>per stuctural</td><td>0.44</td></tr>
</table>

* The surcharge loads should be applied as uniform loads over the full height of the walls as follows: Surcharge Load (psf) = (q) (K), where q = surcharge in psf, and K = coefficient of lateral pressure. Final design is the purview of the project structural engineer.

** Restrained conditions shall be defined as walls which are structurally connected to prevent flexible yielding, or rigid wall configurations (i.e., walls with numerous turning points) which prevent the yielding necessary to reduce the driving pressures from an at-rest state to an active state.

Table 8: Retaining Wall Pressures (Undrained)

<table>
<tr><th>Wall Type</th><th>Wall Slope Configuration</th><th>Equivalent Fluid Weight (pcf)</th><th>Surcharge Load (psf)*</th><th>Lateral Pressure Coefficient</th><th colspan="2">Earthquake Loading (plf)</th></tr>
<tr><td rowspan="2">Free Cantilever</td><td>Flat</td><td>80</td><td>per stuctural</td><td>0.28</td><td rowspan="3">15H²</td><td rowspan="3">Applied 0.6H above the base of the wall</td></tr>
<tr><td>2H:1V</td><td>89</td><td>per structural</td><td>0.42</td></tr>
<tr><td>Restrained*</td><td>Flat</td><td>91</td><td>per stuctural</td><td>0.44</td></tr>
</table>

* The surcharge loads should be applied as uniform loads over the full height of the walls as follows: Surcharge Load (psf) = (q) (K), where q = surcharge in psf, and K = coefficient of lateral pressure. Final design is the purview of the project structural engineer.

** Restrained conditions shall be defined as walls which are structurally connected to prevent flexible yielding, or rigid wall configurations (i.e., walls with numerous turning points) which prevent the yielding necessary to reduce the driving pressures from an at-rest state to an active state.

<u>Generalized Design Values</u>

Some software and design methods do not use the equivalent fluid weight method presented above; instead, they use design soil properties for a given soil condition such as the internal friction angle, cohesion, and bulk unit weight. Generally, this occurs for keyed or interlocking non-mortared walls such as segmental block (Basalite, Keystone, Allan Block, etc.) or rockery walls. When this occurs, the following soil parameters would be applicable for design with the onsite native materials in a firm condition or for engineered fills. The seismic coefficient is considered to be ½ of the adjusted peak ground acceleration for the site conditions is given in Section 4.0 of this report. Some software allows for the extension of the Mononobe-Okabe Method beyond the conventional limitations and, if the method is applied, could calculate seismic values significantly higher than those provided by the multiplier method provided above.

Table 9: Generalized Design Parameters

Internal Angle of Friction	Cohesion	Bulk Unit Weight	Seismic Coefficient, Kh
34°	0 psf	130 pcf	0.113g

Wall Drainage
The criteria presented above is based on fully drained conditions as detailed in the attached Figure C-2, Appendix C. For these conditions, we recommend that a blanket of filter material be placed behind all proposed walls. Permeable materials are specified in Section 68 of the California Department of Transportation Standard Specifications, current edition. The filter material should conform to Class 1, Type B permeable material in combination with a filter fabric to separate the open graded gravel/rock from the surrounding soils. Generally, a clean ¾ inch crushed rock should be acceptable. Consistent with Caltrans Standards, when Class 2 permeable materials are used, the filter fabric may be omitted unless otherwise designed.

The blanket of filter material should be a minimum of 12-inches thick and should extend from the bottom of the wall to within 12 inches of the ground surface. The top 12 inches of wall backfill should consist of a compacted soil cap. A filter fabric having specifications equal to or greater than those for Mirafi 140N should be placed between the gravel filter material and the surrounding soils to reduce the potential for infiltration of soil into the gravel. A 4-inch diameter drain pipe should be installed near the bottom of the filter blanket with perforations facing down. The drainpipe should be underlain by at least 4 inches of filter-type material. An adequate gradient should be provided along the top of the foundation to discharge water that collects behind the retaining wall to a controlled discharge system.

The configuration of a long retaining wall generally does not allow for a positive drainage gradient within the perforated drain pipe behind the wall since the wall footing is generally flat with no gradient for drainage. Where this condition is present, to maintain a positive drainage behind the walls, we recommend that the wall drains be provided with a discharge to an appropriate non-erosive outlet a maximum of 50 feet on center. **In addition, if the wall drain outlets are temporarily stubbed out in front of the walls for future connection during building construction, it is imperative that the outlets be routed into the tight pipe area drainage system and not buried and rendered ineffective.**

Asphalt Concrete Pavement Design
We understand that asphalt pavements will be used for the associated roadways. The following comments and recommendations are given for pavement design and construction purposes. All pavement construction and materials used should conform to applicable sections of the latest edition of the California Department of Transportation Standard Specifications.

Relative Compaction
The asphalt concrete pavement section should be constructed to achieve the minimum relative compactions specified in Section 6.0 of this report. Deviation from the following table should be reviewed by the governing agency when the pavements are to be constructed within their right-of-way. Final acceptance of the constructed pavement section is the purview of the governing agency or owner of the site.

Subgrade Stability
All subgrades and aggregate base should be proof-rolled with a full water truck or equivalent immediately before paving, in order to evaluate their condition.

Subgrade stability has been an issue at other sites with granitic soils following the start of rain events. Measures such as cement-treatment and geogrid reinforcement have been successful in similar conditions. If instability of the subgrade is anticipated or encountered, we should be requested to provide additional recommendations.

Subgrade Resistance Value

Critical features that govern the durability of a pavement section include the stability of the subgrade; the presence or absence of moisture, free water, and organics; the fines content of the subgrade soils; the traffic volume; and the frequency of use by heavy vehicles. Soil conditions can be defined by a soil resistance value, or “R-Value,” and traffic conditions can be defined by a Traffic Index (TI).

Laboratory testing was performed on a bulk sample considered to be representative of the materials expected to be exposed at subgrade. The tested soils had an R-Value of 55 and 59. To account for the variability of materials, have used and R-Value of 40 for the pavement sections this report.

Design values provided are based upon properly drained subgrade conditions. Although the R-Value design to some degree accounts for wet soil conditions, proper surface and landscape drainage design is integral in performance of adjacent street sections with respect to stability and degradation of the asphalt. If clay soils are encountered and cannot be sufficiently blended with non-expansive soils, we should review pavement subgrades to determine the appropriateness of the provided sections, and provide additional pavement design recommendations as field conditions dictate. Even minor clay constituents will greatly reduce the design R-Value.

Due to the redistribution of materials that occurs during grading operations, we should review pavement subgrades to determine the appropriateness of the provided sections.

Section Thickness

The recommended design thicknesses presented in the following table were calculated in accordance with the methods presented in the Sixth Edition of the California Department of Transportation Highway Design Manual. A varying range of traffic indices are provided for use by the project Civil Engineer for roadway design.

Table 10: Asphalt Pavement Section Recommendations

Design Traffic Indices	Alternative Pavement Sections (Inches)	
	Asphalt Concrete *	Aggregate Base **
5.0	3.0	4.0
5.5	3.0 3.5	5.0 4.0
6.0	3.0 3.5	6.5 5.5
6.5	3.5 4.0	7.0 6.0
7.0	4.0 4.5	7.0 6.0
8.0	4.5 5.0	9.0 8.0
9.0	5.5 6.0	9.5 9.0
10.0	5.0 6.0 7.0	13.0 11.5 9.5

* Asphalt Concrete: must meet specifications for Caltrans Hot Mix Asphalt Concrete
** Aggregate Base: must meet specifications for Caltrans Class II Aggregate Base (R-Value = minimum 78)

Portland Cement Concrete Pavement Design

We understand that Portland cement concrete pavements may be considered for various aspects of the parking and drive access areas. The American Concrete Institute (ACI) Concrete Pavement Design method (ACI 330R-08) was used for design of the exterior concrete (rigid) pavements at the site.

<u>Relative Compaction</u>

The asphalt concrete pavement section should be constructed to achieve the minimum relative compactions specified in Section 6.0 of this report. Deviation from the following table should be reviewed by the governing agency when the pavements are to be constructed within their right-of-way. Final acceptance of the constructed pavement section is the purview of the governing agency.

<u>Subgrade Stability</u>

All subgrades and aggregate base should be proof-rolled with a full water truck or equivalent immediately before paving, in order to evaluate their condition.

Subgrade stability has been an issue at other sites with granitic soils following the start of rain events. Measures such as cement-treatment and geogrid reinforcement have been successful in similar conditions. If instability of the subgrade is anticipated or encountered, we should be requested to provide additional recommendations.

<u>Soil Design Parameters</u>

The pavement thicknesses were evaluated based on the soil design parameters provided in the following table.

Table 11: Soil Parameters

Subgrade Soil Description	k, Modulus of Subgrade Reaction*	Base Course
Silty SAND	200 pci	6 inches

* Based on an R-Value of 40 as recommended above and correlated to a k-Value recommended by ACI 330R.

<u>Section Thickness</u>
Based on the subgrade soil parameters shown in the above table, the recommended concrete thicknesses for various traffic descriptions are presented in the table below. The recommended thicknesses provided below assume the use of plain (non-reinforced) concrete pavements.

Table 12: Concrete Pavement Section Recommendations

Category	ADTT*	Pavement Traffic Description	Thickness (inches)	
			3000 psi**	4000 psi**
A	1	Car parking areas and access lanes Autos, pickups, and panel trucks only	4.0	4.0
A	10		5.0	4.5
B	25	Shopping center entrance and service lanes Bus parking areas and interior lanes Single-unit truck parking areas and interior lanes	5.5	5.0
B	300		6.0	5.5
C	100	Roadway Entrances and Exterior Lanes	6.0	6.0
C	300		6.5	6.0
C	700		6.5	6.5

* Average Daily Truck Traffic
** 28-day concrete compressive strength

<u>Jointing and Reinforcement</u>
From a geotechnical perspective, contraction joints should be placed in accordance with the American Concrete Institute (ACI) recommendations which include providing a joint spacing about 30 times the slab thickness up to a maximum of 10 feet. The joint patterns should also divide the slab into nearly square panels. If increased joint spacing is desired, reinforcing steel should be installed within the pavement in accordance with ACI recommendations. Final determination of steel reinforcement configurations (if used within the pavements) remains the purview of the Project Structural Engineer.

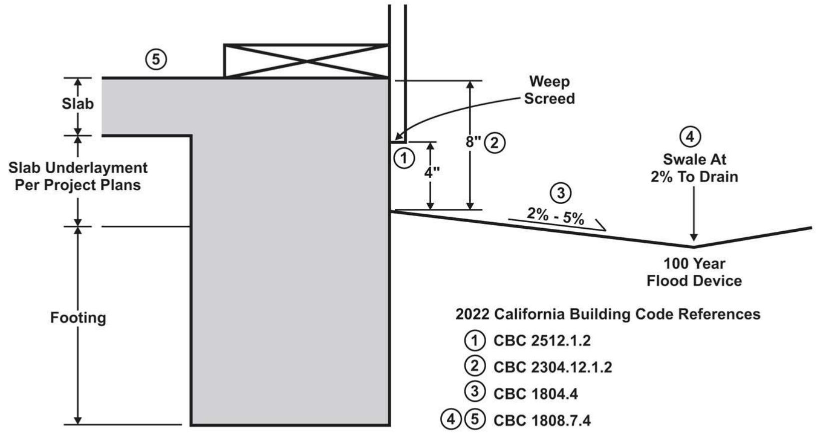
Drainage
In order to maintain the engineering strength characteristics of the soil presented for use in this report, maintenance of the site will need to be performed. This maintenance generally includes, but is not limited to, proper drainage and control of surface and subsurface water which could affect structural support and fill integrity. A difficulty exists in determining which areas are prone to the negative impacts resulting from high moisture conditions due to the diverse nature of potential sources of water; some of which are outlined in the paragraph below. We suggest that measures be installed to minimize exposure to the adverse effects of moisture, but this will not guarantee that excessive moisture conditions will not affect the structure.

Some of the diverse sources of moisture could include water from landscape irrigation, annual rainfall, offsite construction activities, runoff from impermeable surfaces, collected and channeled water, and water perched in the subsurface soils. Some of these sources can be controlled through drainage features installed either by the owner or contractor. Others may not become evident until they, or the effects of the presence of excessive moisture, are visually observed on the property.

Some measures that can be employed to minimize the buildup of moisture include, but are not limited to proper backfill materials and compaction of utility trenches within the footprint of the proposed structures; grout plugs at foundation penetrations; collection and channeling of drained water from impermeable surfaces (i.e. roofs, concrete or asphalt paved areas); installation of subdrain/cut-off drain provisions; utilization of low flow irrigation systems; education to the proposed owners of proper design and maintenance of landscaping and drainage facilities that they or their landscaper installs.

Drainage Adjacent to Buildings

All grades should provide rapid removal of surface water runoff; ponding water should not be allowed on building pads or adjacent to foundations or other structural improvements (during and following construction). All soils placed against foundations during finish grading should be compacted to minimize water infiltration. Finish and landscape grading should include positive drainage away from all foundations. Section 1808.7.4 of the 2022 California Building Code (CBC) states that for graded soil sites, the top of any exterior foundation shall extend above the elevation of the street gutter at the point of discharge or the inlet of an approved drainage device a minimum of 12 inches plus 2 percent. If overland flow is not achieved adjacent to buildings, the drainage device should be designed to accept flows from a 100-year event. Grades directly adjacent to foundations should be no closer than 8 inches from the top of the slab (CBC 2304.12.1.2), and weep screeds are to be placed a minimum of 4 inches clear of soil grades and 2 inches clear of concrete or other hard surfacing (CBC 2512.1.2). From this point, surface grades should slope a minimum of 2 percent away from all foundations for at least 5 feet but preferably 10 feet, and then 2 percent along a drainage swale to the outlet (CBC 1804.4). Downspouts should be tight piped via an area drain network and discharged to an appropriate non-erosive outlet away from all foundations.



**Typical 2022 California Building Code
Drainage Requirements**

The above referenced elements pertaining to drainage of the proposed structures is provided as general acknowledgement of the California Building Code requirements, restated and graphically illustrated for ease of understanding. Surface drainage design is the purview of the Project Architect/Civil Engineer. Review of drainage design and implementation adjacent to the building envelopes is recommended as performance of these improvements is crucial to the performance of the foundation and construction of rigid improvements.

Building Pad Subdrainage

It has been our experience that building pads constructed with cuts and fills into weathered bedrock generally have an increased potential for moisture related issues related to water perched on the bedrock horizon and/or present in the fractures of the bedrock as well as moisture transmission through utility trenches. Reduction of potential moisture related issues could be addressed by the construction of subdrains in addition to the drainage provisions provided in the 2022 CBC. Typical subdrain construction would include a 3 feet deep trench (or depth required to intercept the bottom of utility trenches) constructed as detailed on Figure C-3, Appendix C. The water collected in the subdrain pipe would be directed to an appropriate non-erosive outlet.

As noted in the previous discussions, the moisture conditions may not manifest until after the site is developed. As such, any recommendations for the subdrain orientation and location to mitigate the moisture conditions can be provided on an as requested basis as the conditions arise. We recommend that the developer notify future lot owners of this potential.

Subsurface Water within Utilities

Water can become perched on the relatively impermeable rock horizons and eventually inundate utility trench backfill. The variable support condition between native soils and compacted trench backfills materials, coupled with prolonged water exposure can lead to subsidence of trench backfill materials, especially if bridging of trench backfill occurs during placement or natural jetting of soils into voids around pipes occurs. Joint utility trenches are generally more susceptible to the jetting issues due to the quantity of pipe placed in the trench. Recommendations to reduce the risk associated with this condition may be provided based on observed field conditions.

Following site development, additional water sources (i.e., landscape watering, downspouts) are generally present. The presence of low permeability materials can prohibit rapid dispersion of surface and subsurface water drainage.

Utility trenches can become collection points for subsurface water and typically provide a conduit for water distribution. Provisions may be necessary to mitigate adverse effects of perched water conditions. Where this condition arises, we recommend plug and drains within the utility trenches (Figure C-4, Appendix C) to collect and convey water to the storm drain system or other approved outlet. Temporary dewatering measures may be necessary and could include the installation of submersible pumps and/or point wells. ***As the observed site conditions dictate, representatives from our firm, the contractor, and the civil engineer should coordinate the locations of plug and drains.***

Roadway or Parking Area Landscaping Drainage
Prolonged water seepage into pavement sections can result in softening of subgrade soils and subsequent pavement distress. It is anticipated that heavy landscape watering could enter and pond within the aggregate base section as it permeates through the aggregate base under the sidewalks and/or curbs. Prolonged seepage within the pavement section could cause distress to pavements in heavy traffic areas. Some measures that can be employed to minimize the saturation of the subgrade and aggregate base materials include, but are not limited to, construction of cut-off drains or moisture barriers alongside the roadway adjacent to the roadway interface, construction of subdrains within landscape medians and installation of plug and drain systems within utility trenches. Due to the elusive and discontinuous nature of drainage related issues, a risk-based approach should be determined by the developer based on consultation and discussions with the design professionals and the amount of protection of facilities that the developer may want to provide against potential moisture related issues.

Post Construction
All drainage related issues may not become known until after construction and landscaping are complete. Therefore, some mitigation measures may be necessary following site development. Landscape watering is typically the largest source of water infiltration into the subgrade. Given the soil conditions on site, excessive or even normal landscape watering could contribute to moisture related problems and/or cause distress to foundations and slabs, pavements, and underground utilities, as well as creating a nuisance where seepage occurs.

8.0 DESIGN REVIEW AND CONSTRUCTION MONITORING

Geotechnical engineering can be affected by natural variability of soils and, as with many projects, the contents of this report could be used and interpreted by many design professionals for the application and development of their plans. For these reasons, we recommend that our firm provide support through plan reviews and construction monitoring to aid in the production of a successful project.

Plan Review

The design plans and specifications should be reviewed and accepted by Youngdahl Consulting Group, Inc. prior to contract bidding. A review should be performed to determine whether the recommendations contained within this report are still applicable and/or are properly interpreted and incorporated into the project plans and specifications. Modifications to the recommendations provided in this report or to the design may be necessary at the time of our review based on the proposed plans.

Construction Monitoring

Construction monitoring is a continuation of geotechnical engineering to confirm or enhance the findings and recommendations provided in this report. It is essential that our representative be involved with all grading activities in order for us to provide supplemental recommendations as field conditions dictate. Youngdahl Consulting Group, Inc. should be notified at least two working days before site clearing or grading operations commence, and should observe the stripping of deleterious material, overexcavation of soft soils and existing fills (if present), and provide consultation, observation, and testing services to the grading contractor in the field. At a minimum, Youngdahl Consulting Group, Inc. should be retained to provide services listed in Table 13 below.

The recommendations included in this report have been based in part on assumptions about strata variations that may be tested only during earthwork. Accordingly, these recommendations should not be applied in the field unless Youngdahl Consulting Group, Inc. is retained to perform construction observation and thereby provide a complete professional geotechnical engineering service through the observational method. Youngdahl Consulting Group, Inc. cannot assume responsibility or liability for the adequacy of its recommendations when they are used in the field without Youngdahl Consulting Group, Inc. being retained to observe construction.

Post Construction Drainage Monitoring
Due to the elusive nature of subsurface water, the alteration of water features for development, and the introduction of new water sources, all drainage related issues may not become known until after construction and landscaping are complete. Youngdahl Consulting Group, Inc. can provide consultation services upon request that relate to proper design and installation of drainage features during and following site development.

9.0 LIMITATIONS AND UNIFORMITY OF CONDITIONS

1. This report has been prepared for the exclusive use of the addressee of this report for specific application to this project. The addressee may provide their consultants authorized use of this report. Youngdahl Consulting Group, Inc. has endeavored to comply with generally accepted geotechnical engineering practice common to the local area. Youngdahl Consulting Group, Inc. makes no other warranty, expressed or implied.

2. As of the present date, the findings of this report are valid for the property studied. With the passage of time, changes in the conditions of a property can occur whether they be due to natural processes or to the works of man on this or adjacent properties. Legislation or the broadening of knowledge may result in changes in applicable standards. Changes outside of our control may cause this report to be invalid, wholly or partially. Therefore, this report should not be relied upon after a period of three years without our review nor should it be used or is it applicable for any properties other than those studied.

3. Section [A] 107.3.4 of the 2022 California Building Code states that, in regard to the design professional in responsible charge, the building official shall be notified in writing by the owner if the registered design professional in responsible charge is changed or is unable to continue to perform the duties.

   WARNING: Do not apply any of this report's conclusions or recommendations if the nature, design, or location of the facilities is changed. If changes are contemplated, Youngdahl Consulting Group, Inc. must review them to assess their impact on this report's applicability. Also note that Youngdahl Consulting Group, Inc. is not responsible for any claims, damages, or liability associated with any other party's interpretation of this report's subsurface data or reuse of this report's subsurface data or engineering analyses without the express written authorization of Youngdahl Consulting Group, Inc.

4. The analyses and recommendations contained in this report are based on limited windows into the subsurface conditions and data obtained from subsurface exploration. The methods

used indicate subsurface conditions only at the specific locations where samples were obtained, only at the time they were obtained, and only to the depths penetrated. Samples cannot be relied on to accurately reflect the strata variations that usually exist between sampling locations. Should any variations or undesirable conditions be encountered during the development of the site, Youngdahl Consulting Group, Inc. will provide supplemental recommendations as dictated by the field conditions.

Table 13: Checklist of Recommended Services

Item Description		Recommended	Not Anticipated
1	Provide foundation design parameters	Included	
2	Review grading plans and specifications	✓	
3	Review foundation plans and specifications	✓	
4	Observe and provide recommendations regarding demolition	✓	
5	Observe and provide recommendations regarding site stripping	✓	
6	Observe and provide recommendations on moisture conditioning removal, and/or recompaction of unsuitable existing soils	✓	
7	Observe and provide recommendations on the installation of subdrain facilities	✓	
8	Observe and provide testing services on fill areas and/or imported fill materials	✓	
9	Review as-graded plans and provide additional foundation recommendations, if necessary	✓	
10	Observe and provide compaction tests on storm drains, water lines and utility trenches	✓	
11	Observe foundation excavations and provide supplemental recommendations, if necessary, prior to placing concrete	✓	
12	Observe and provide moisture conditioning recommendations for foundation areas and slab-on-grade areas prior to placing concrete		✓
13	Provide design parameters for retaining walls	Included	
14	Provide finish grading and drainage recommendations	Included	
15	Provide geologic observations and recommendations for keyway excavations and cut slopes during grading	✓	
16	Excavate and recompact all test pits within structural areas	✓	

APPENDIX A

Field Study

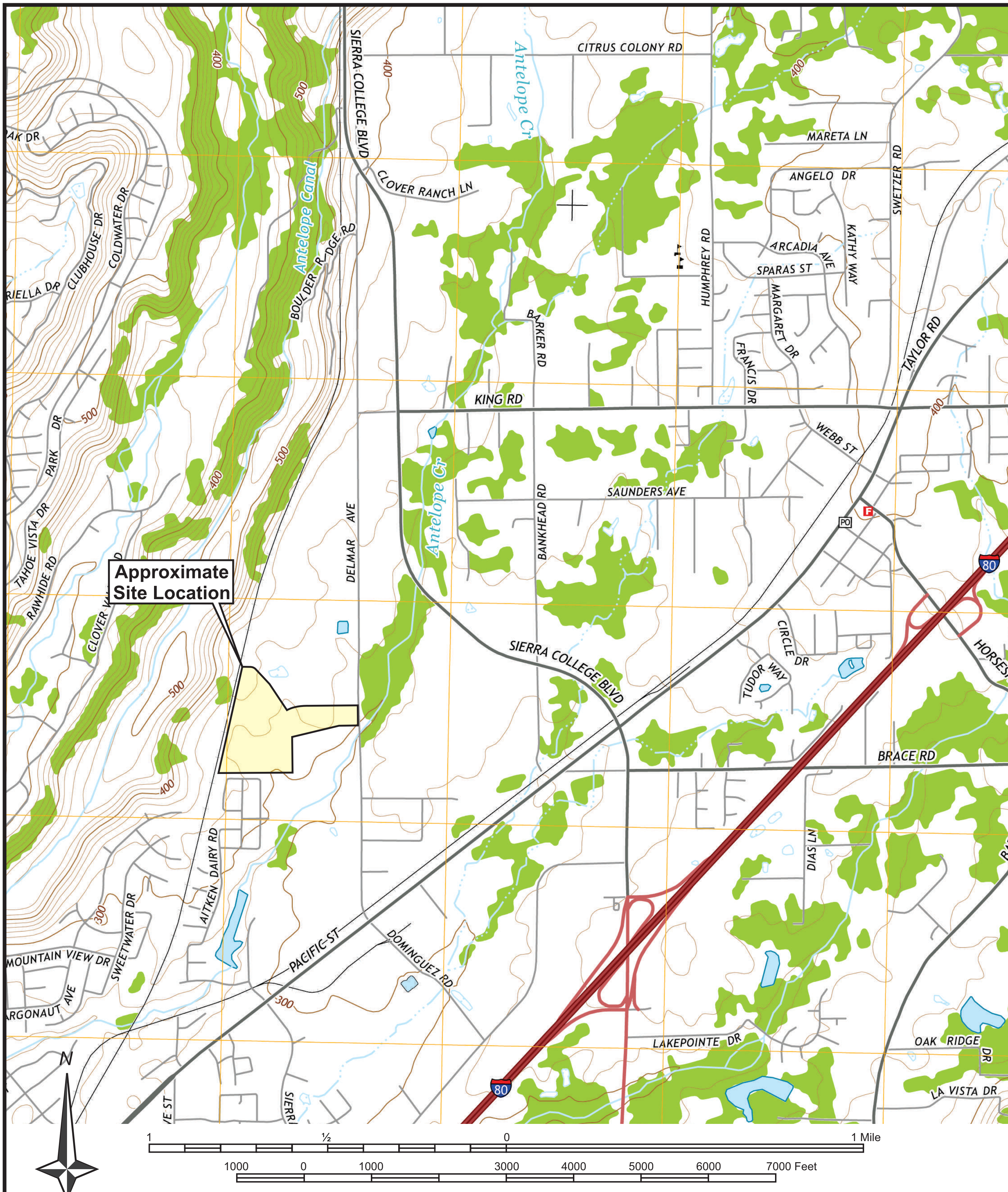
Vicinity Map
Site Plan
Logs of Exploratory Test Pits
Soil Classification Chart and Log Explanation

Introduction
The contents of this appendix shall be integrated with the Geotechnical Engineering Study of which it is a part. They shall not be used in whole or in part as a sole source for information or recommendations regarding the subject site.

Our field study included a site reconnaissance by a Youngdahl Consulting Group, Inc. representative followed by a subsurface exploration program conducted on 23 October 2024, which included the excavation of 13 test pits under their direction at the approximate locations shown on Figure A-2, this Appendix. Excavation of the test pits was accomplished with a Caterpillar 315 Excavator equipped with a 24-inch-wide bucket. The bulk and bag samples collected from the test pits were returned to our laboratory for further examination and testing.

The Exploratory Test Pit Logs describe the vertical sequence of soils and materials encountered in each test pit, based primarily on our field classifications and supported by our subsequent laboratory examination and testing. Where a soil contact was observed to be gradual, our logs indicate the average contact depth. Our logs also graphically indicate the sample type, sample number, and approximate depth of each soil sample obtained from the test pits.

The soils encountered were logged during excavation and provide the basis for the "Logs of Test Pits", Figures A-3 through A-15, this Appendix. These logs show a graphic representation of the soil profile, the location, and depths at which samples were collected.



Scale: 1:24,000

BASE MAP REFERENCE: *U.S.G.S. 7.5 Minute Topographic Series, Rocklin Quadrangle, Dated 2021*

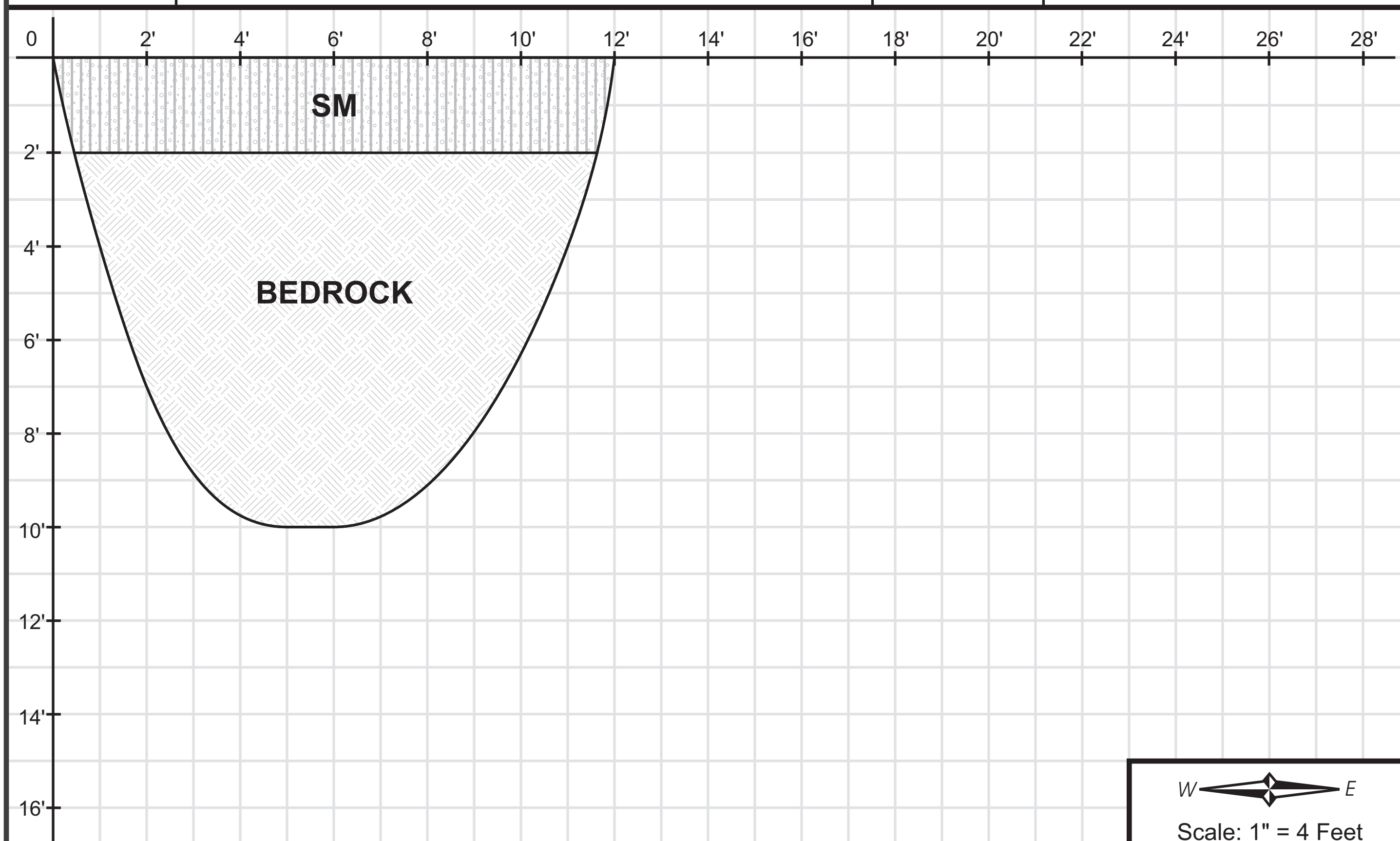
YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**VICINITY MAP** **Carmenere West** Loomis, California	**FIGURE** **A-1**

REFERENCE: *Vesting Tentative Map Overview, Delmar Farms, King Engineering, Sheet 1 of 7, Dated July 2024; Google Earth, Aerial Data Dated 11/23/2023*

YOUNGDAHL CONSULTING GROUP, INC. Celebrating 40 Years of Service	Project No.: E18269.001 December 2024	**SITE PLAN** **Carmenere West** Loomis, California	**FIGURE** **A-2**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812972° / W 121.220203°**		Pit No. **TP-1**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **95°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2'	Brown silty **SAND (SM)**, medium dense, dry	**TP-1** @ 0-3'	**TP-4 @ 0-2'** R-Value = 55
@ 2' - 10'	Red brown to brown granitic **BEDROCK**, completely to highly weathered, soft, slightly moist		
	Test pit terminated at 10' No free groundwater encountered No caving noted		

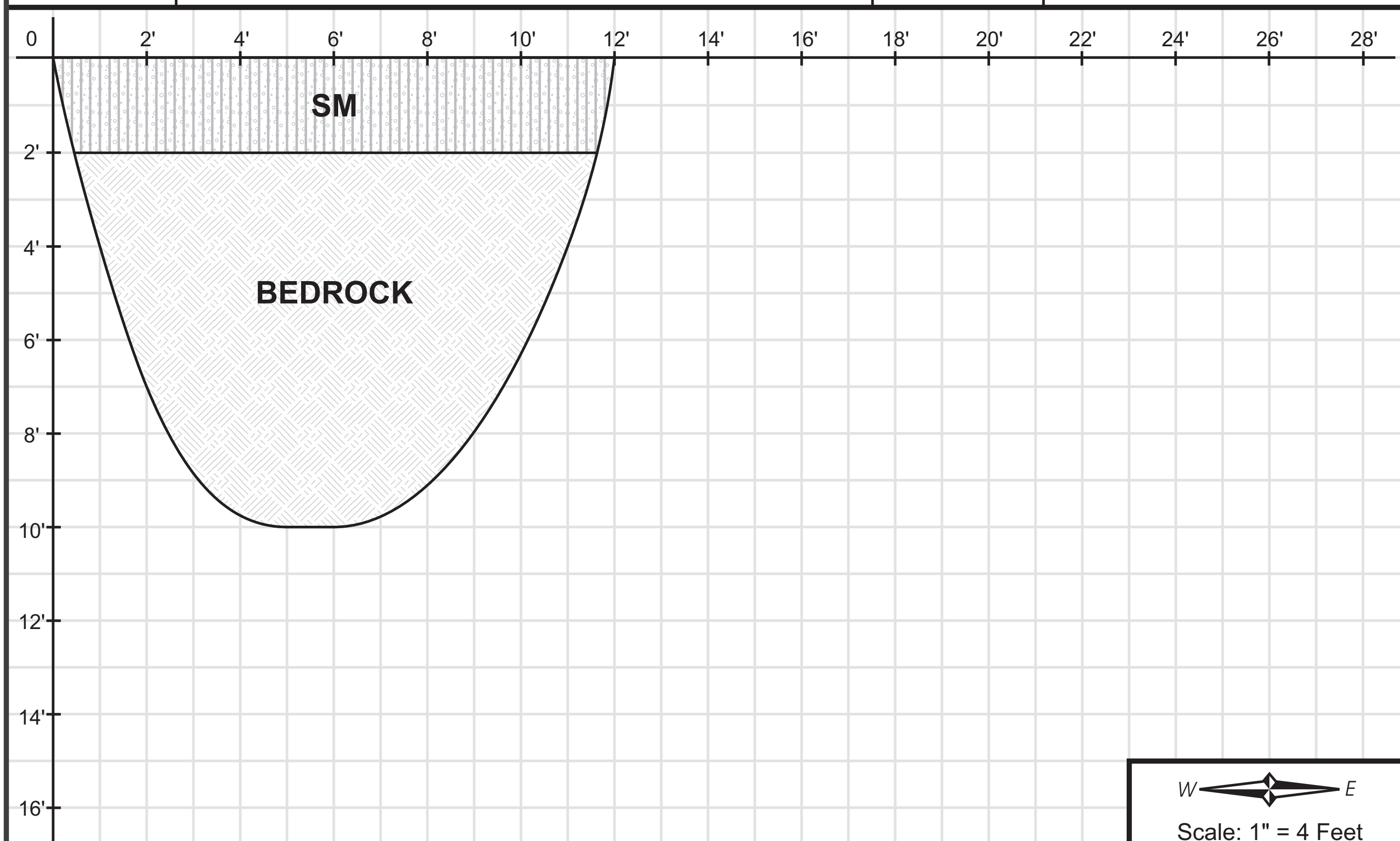


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE A-3**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812997° / W 121.221628°**		Pit No.
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **90°**	Elevation: **~**	**TP-2**

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2'	Brown silty **SAND (SM)**, medium dense, dry		
@ 2' - 4'	Red brown to brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 4' - 10'	*Grades brown to light grey, soft to moderately hard*		
	Test pit terminated at 10' No free groundwater encountered No caving noted		

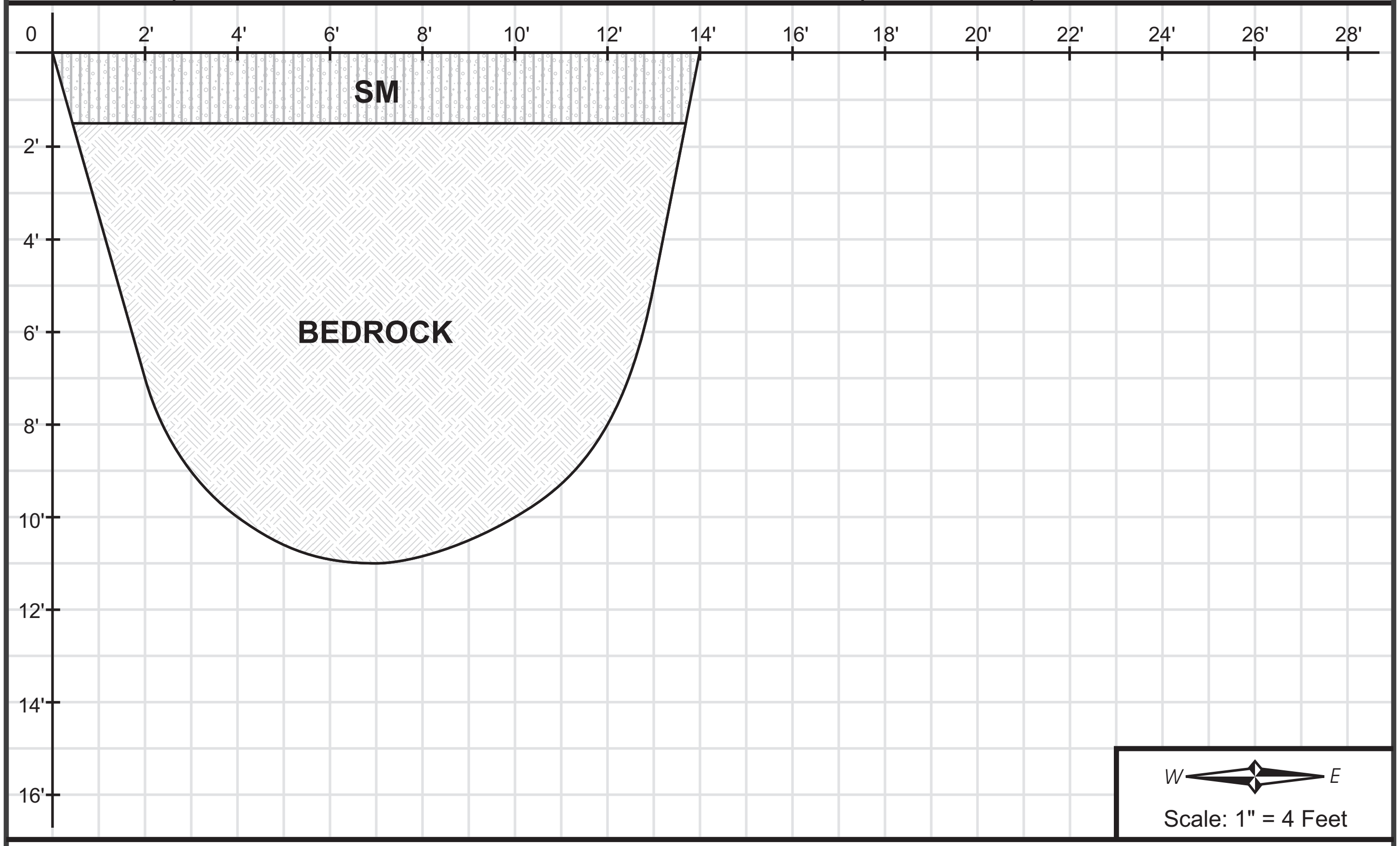


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. — Celebrating 40 Years of Service	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE** **A-4**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812263° / W 121.221251°**		Pit No. **TP-3**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **110°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 1.5'	Brown silty **SAND (SM)**, medium dense, dry		
@ 1.5' - 5'	Red brown to brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 5' - 11'	*Grades red brown, light grey, and mottled black, soft to moderately hard*		
	Test pit terminated at 11' No free groundwater encountered No caving noted		

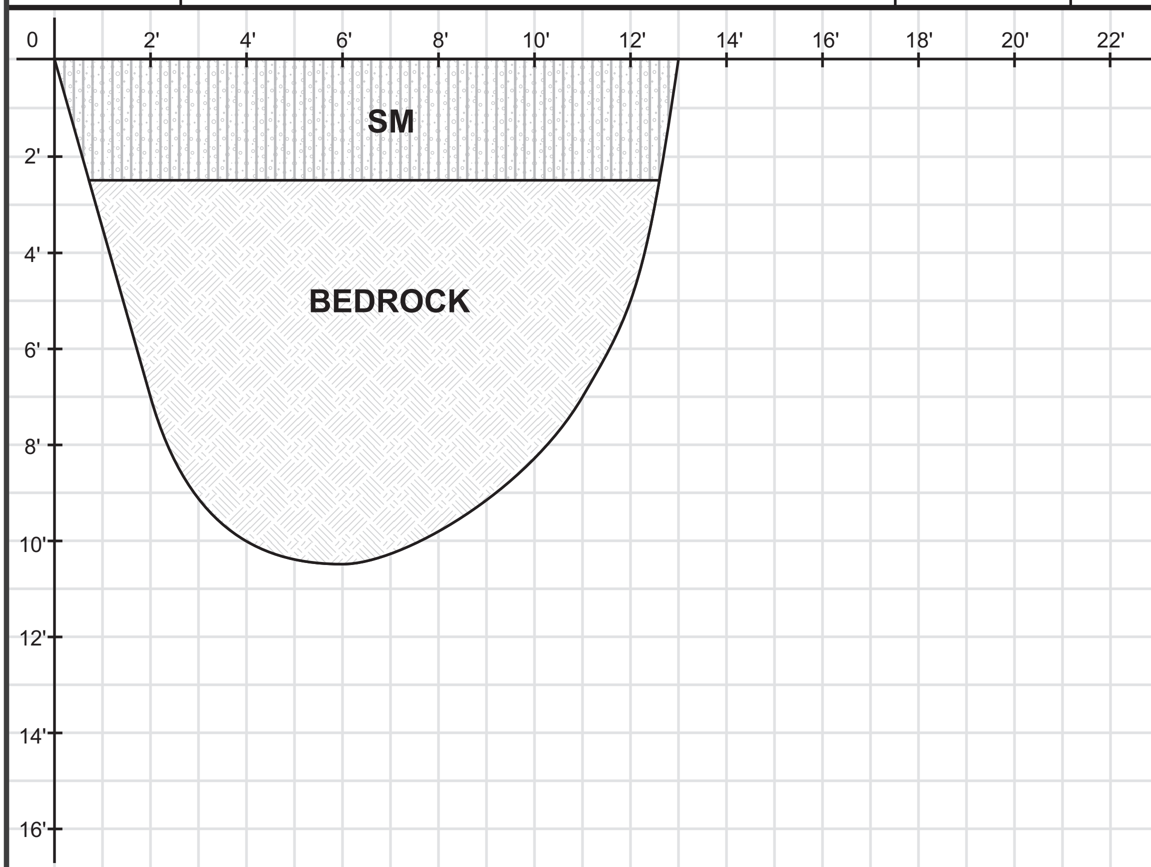


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE A-5**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812053° / W 121.222393°**		Pit No. **TP-4**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **80°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2.5'	Brown silty **SAND (SM)**, medium dense, dry	**TP-4** @ 0-2'	**TP-4 @ 0-2'** ϕ = 34.9°, c = 208 psf DDmax = 125.2 pcf MCopt = 10.3% Corrosivity Suite
@ 2.5' - 5'	Red brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 5' - 10.5'	*Grades red brown and light grey, soft to moderately hard*		
	Test pit terminated at 10.5' No free groundwater encountered No caving noted		



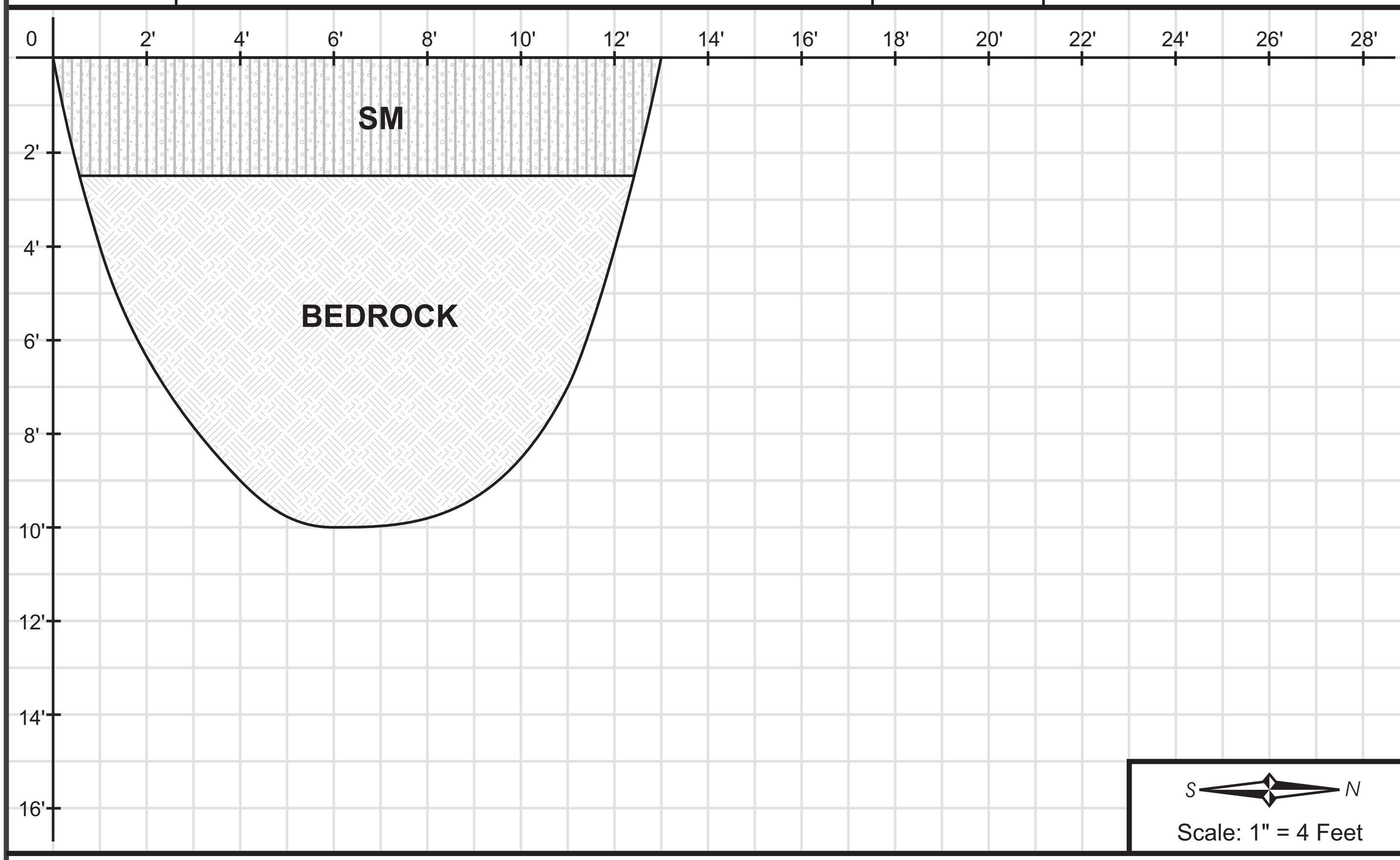
Scale: 1" = 4 Feet

Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001	**EXPLORATORY TEST PIT LOG**	**FIGURE**
	December 2024	**Carmenere West** Loomis, California	**A-6**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.811655° / W 121.223451°**		Pit No. **TP-5**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **0°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 3'	Brown silty **SAND (SM)**, medium dense, dry		
@ 3' - 5'	Red brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 5' - 10'	*Grades red brown and light grey, soft to moderately hard*		
	Test pit terminated at 10' No free groundwater encountered No caving noted		

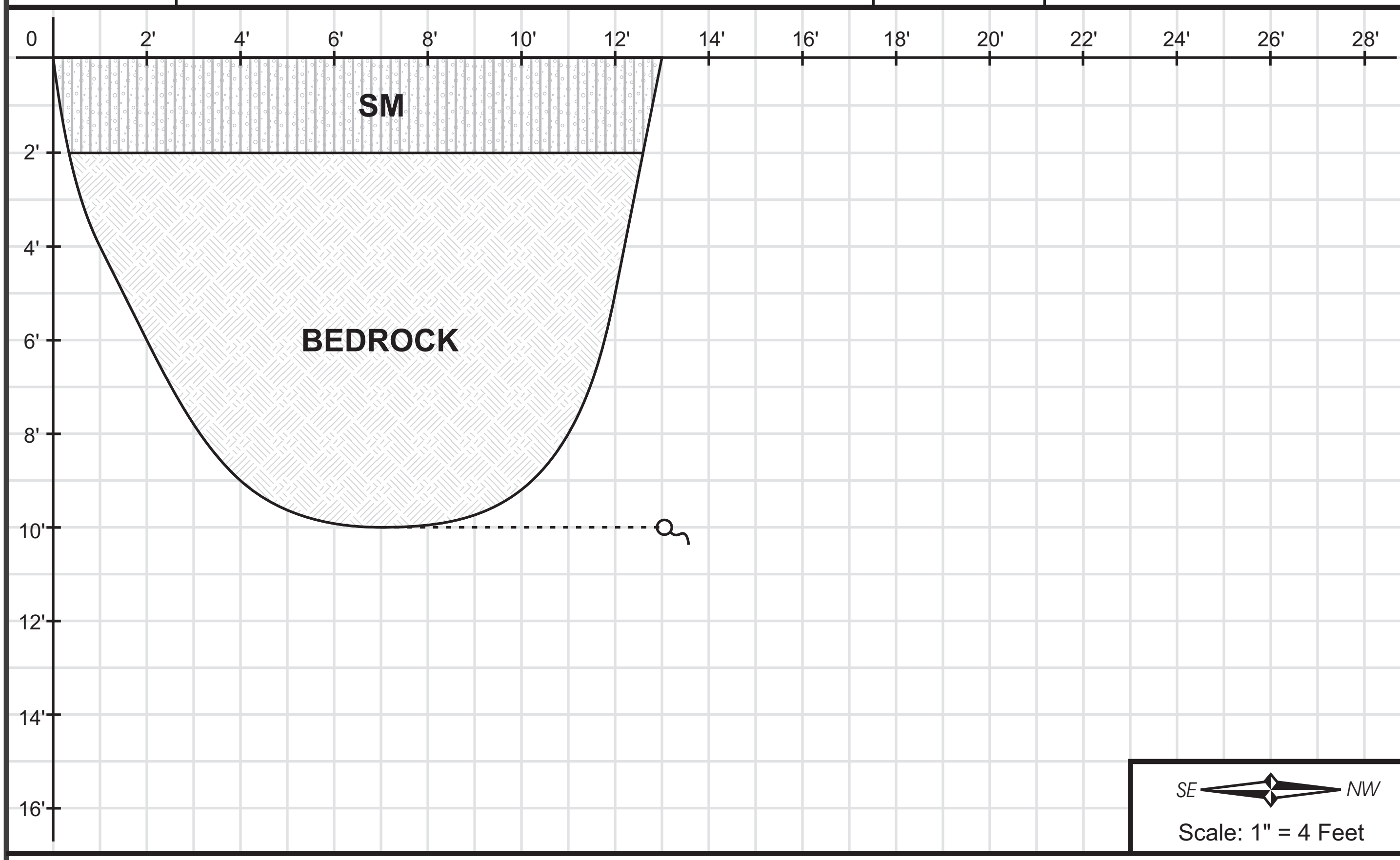


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. — Celebrating 40 Years of Service	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE A-7**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.810711° / W 121.223867°**		Pit No. **TP-6**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **340°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2'	Brown silty **SAND (SM)**, medium dense, dry		
@ 2' - 6'	Red brown to brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 6' - 10'	*Grades brown and light grey, soft to moderately hard, moist*		
@ 10'	*Grades hard, wet*		
	Test pit terminated at 10' (practical refusal) Seepage encountered at 10' No caving noted		

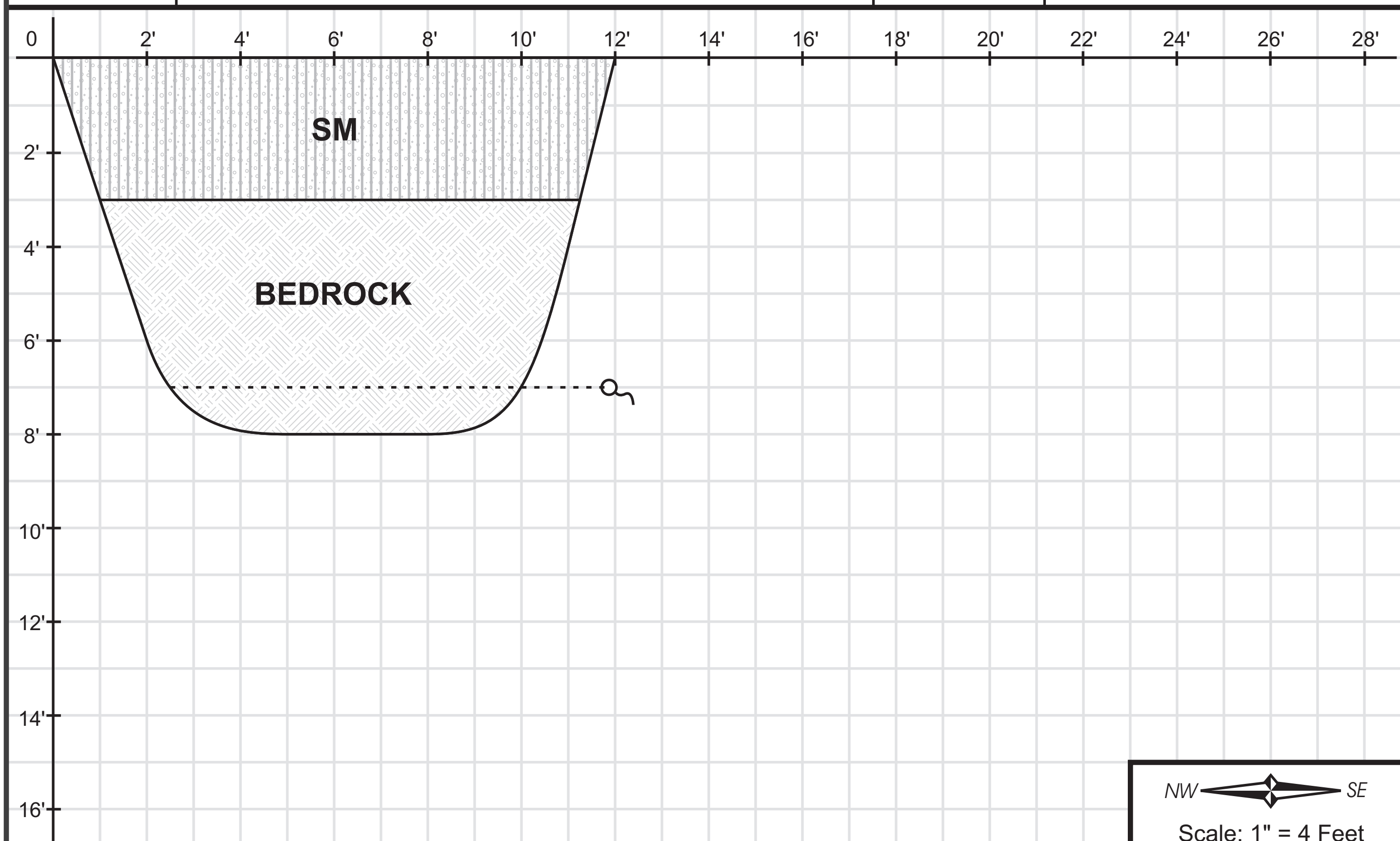


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

40 YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE** **A-8**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.810739° / W 121.225708°**		Pit No. **TP-7**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **320°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 3'	Brown silty **SAND (SM)**, medium dense, dry		
@ 3' - 7'	Brown and grey granitic **BEDROCK**, highly weathered, soft, moist		
@ 7' - 8'	*Grades soft to moderately hard, wet*		
	Test pit terminated at 8' Seepage encountered at 7' Caving conditions at 6-7'		

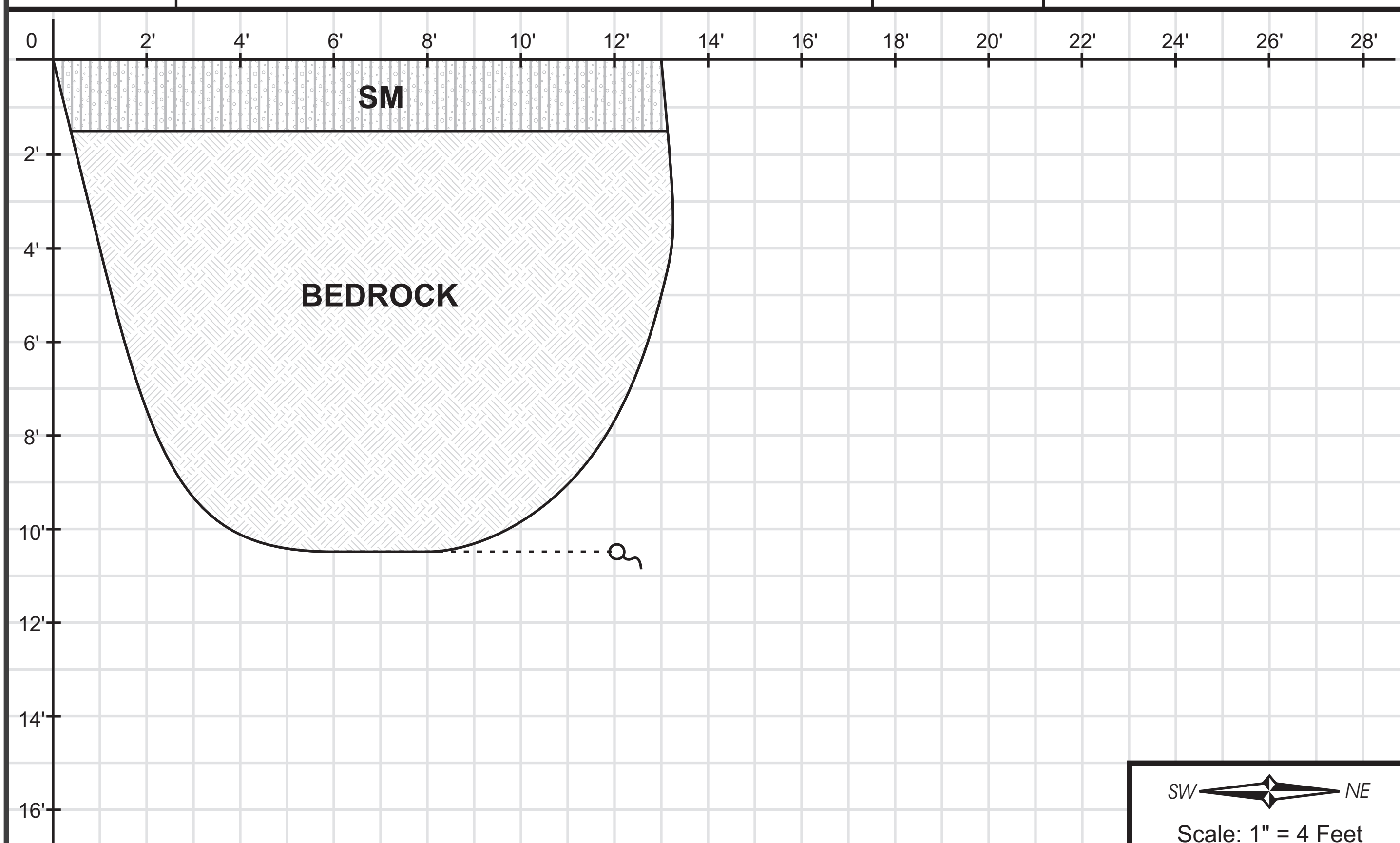


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE A-9**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.811995° / W 121.225416°**		Pit No. **TP-8**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **55°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 1.5'	Brown silty **SAND (SM)**, medium dense, dry	**TP-8** @ 0-2'	
@ 1.5' - 5'	Red brown and brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 5' - 10.5'	*Grades brown and grey soft to moderately hard, moist*		
@ 10.5'	*Grades wet*		
	Test pit terminated at 10.5' Seepage encountered at 10.5' (minor) No caving noted		

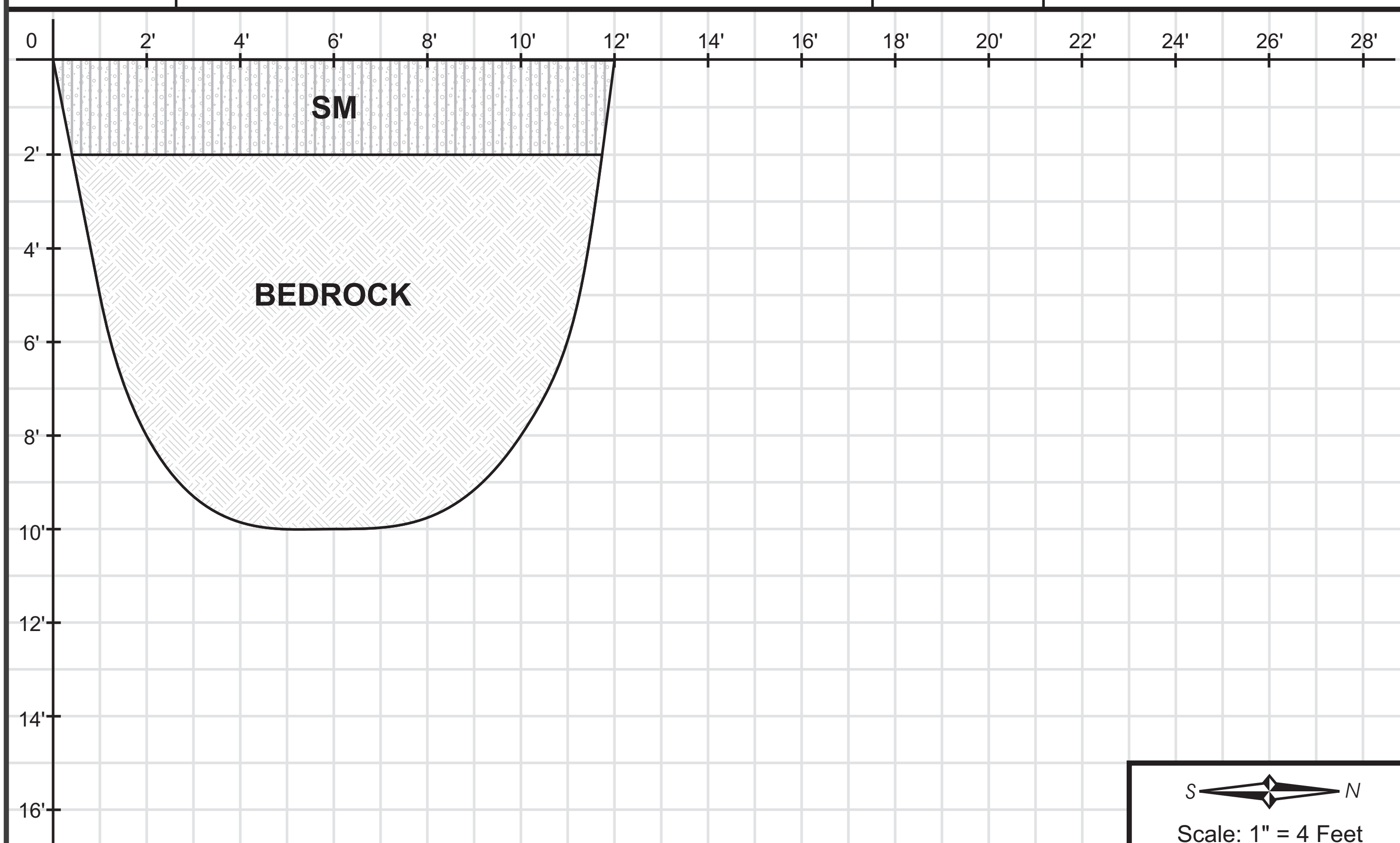


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

40 YOUNGDAHL CONSULTING GROUP, INC. Celebrating 40 Years of Service	Project No.: E18269.001	**EXPLORATORY TEST PIT LOG**	**FIGURE**
	December 2024	**Carmenere West** Loomis, California	**A-10**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.811610° / W 121.224717°**		Pit No. **TP-9**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **5°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2'	Brown silty **SAND (SM)**, medium dense, dry		**<u>TP-9 @ 5-10'</u>** Corrosivity Suite
@ 2' - 5'	Red brown and brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 5' - 10'	*Grades brown and light grey, soft to moderately hard*	**TP-9** @ 5-10'	
	Test pit terminated at 10' No free groundwater encountered No caving noted		

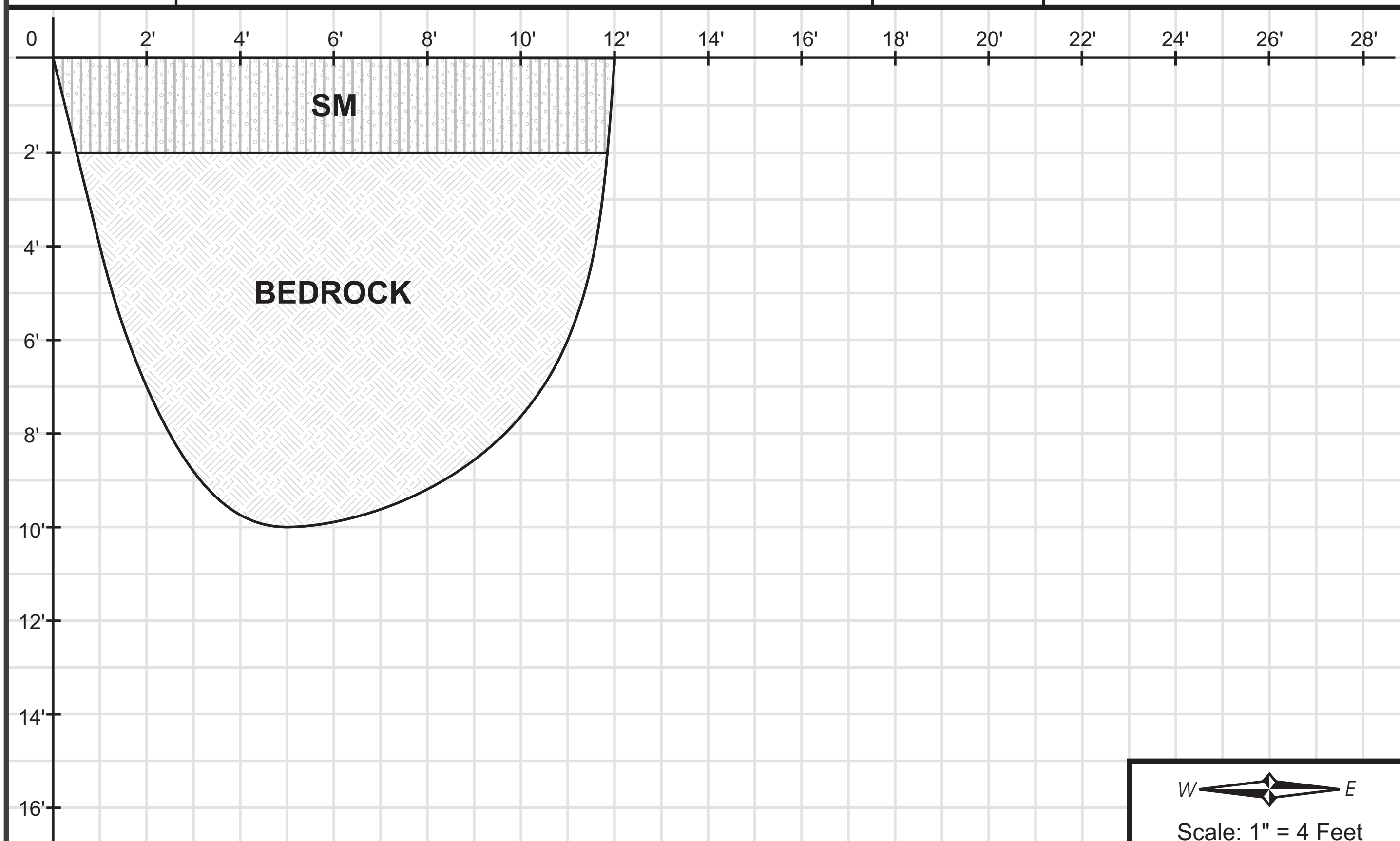


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE** **A-11**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.813934° / W 121.224723°**	Pit No. **TP-10**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **105°** / Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 2'	Brown silty **SAND (SM)**, medium dense, dry		
@ 2' - 6'	Red brown and brown granitic **BEDROCK**, highly weathered, soft, moist		
@ 6' - 10'	*Grades brown and light grey, soft to moderately hard*		
	Test pit terminated at 10' No free groundwater encountered No caving noted		

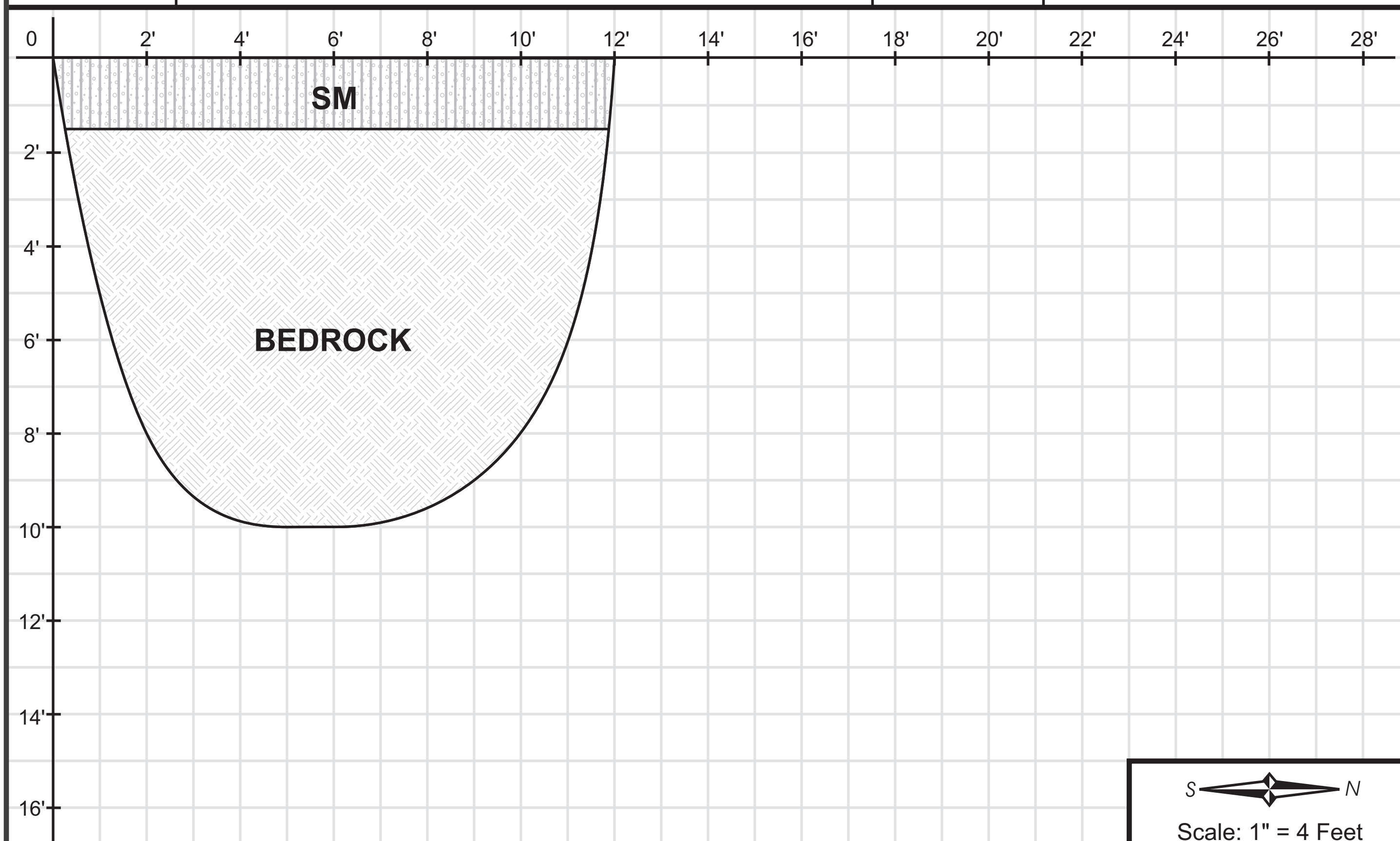


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	**FIGURE A-12**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.813128° / W 121.225418°**		Pit No. **TP-11**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **15°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 1.5'	Brown to red brown silty **SAND (SM)**, medium dense, dry		
@ 1.5' - 6'	Red brown and brown granitic **BEDROCK**, highly weathered, soft, slightly moist		
@ 6' - 10'	*Grades brown, light grey, and red brown, soft to moderately hard, slightly moist to moist*		
	Test pit terminated at 10' No free groundwater encountered No caving noted		

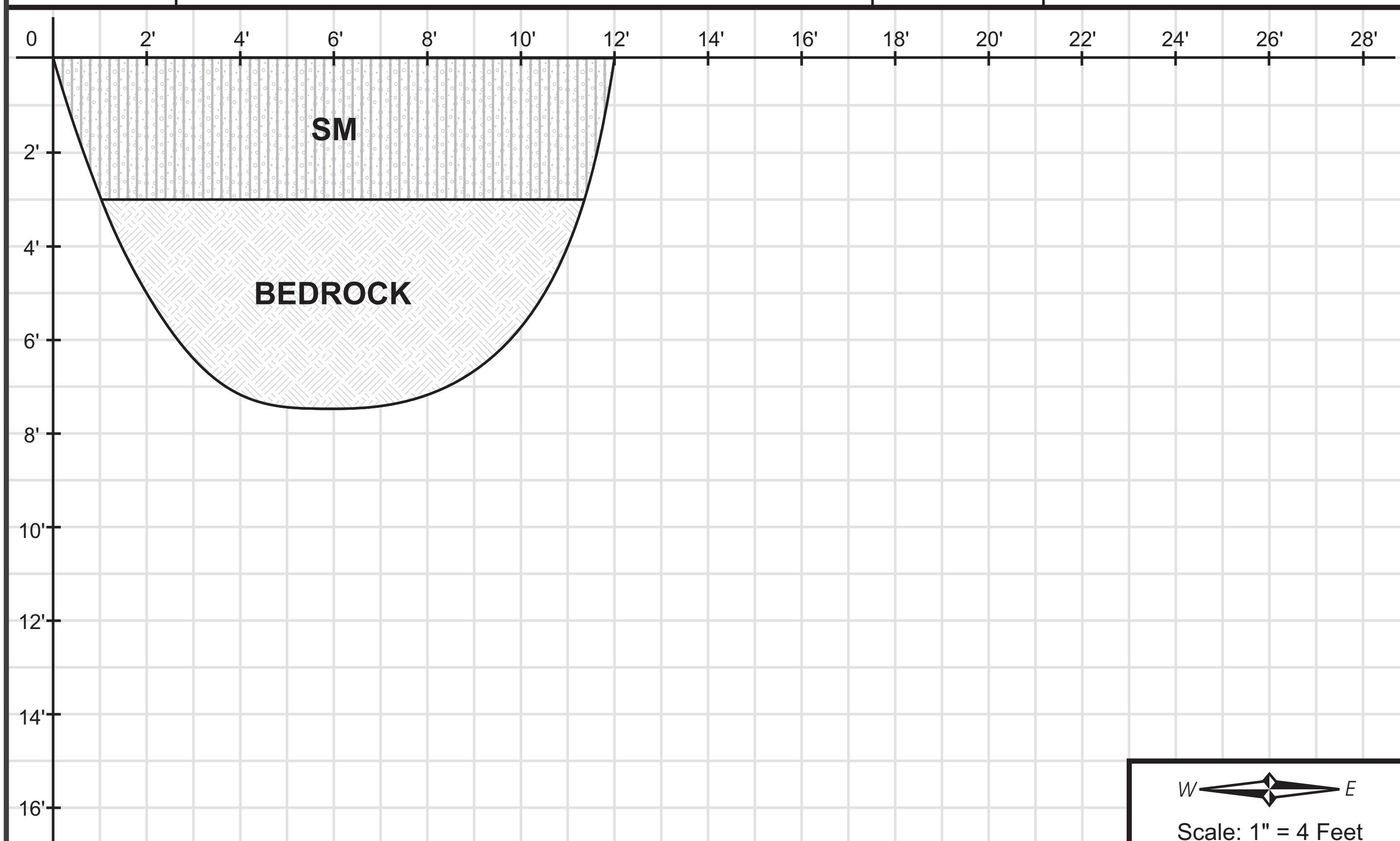


Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**EXPLORATORY TEST PIT LOG** **Carmenere West** Loomis, California	FIGURE **A-13**

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812545° / W 121.224014°**		Pit No. **TP-12**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **305°**	Elevation: **~**	

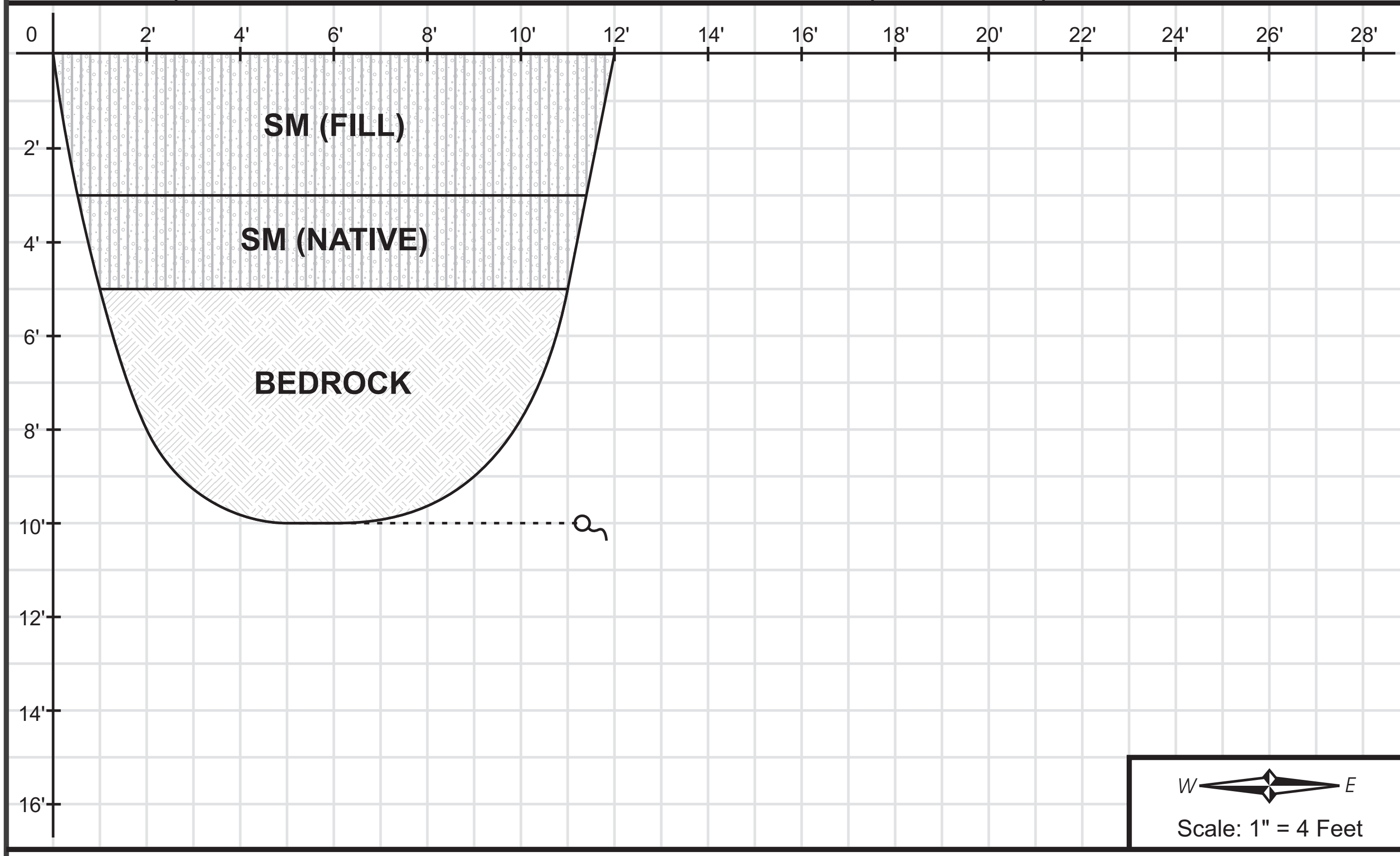
Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 3'	Brown silty **SAND (SM)**, medium dense, moist	**TP-12** @ 0-3'	**TP-13 @ 0-3'** R-Value = 59 0% #4 25.0% < #200
@ 3' - 7'	Red brown and brown granitic **BEDROCK**, highly weathered, soft to moderately hard, moist (decomposed)		
@ 7' - 7.5'	*Grades light grey, hard*		
	Test pit terminated at 7.5' (practical refusal) No free groundwater encountered No caving noted		



Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

Logged By: **PF**	Date: **23 October 2024**	Lat / Lon: **N 38.812907° / W 121.222883°**		Pit No. **TP-13**
Equipment: **CAT 315 EX with 24" Bucket**		Pit Orientation: **80°**	Elevation: **~**	

Depth (Feet)	Geotechnical Description & Unified Soil Classification	Sample	Tests & Comments
@ 0' - 3'	Brown silty **SAND (SM)** with gravel and cobble, medium dense, slightly moist (FILL)		
@ 3' - 5'	Brown silty **SAND (SM)**, medium dense, moist (NATIVE)		
@ 5' - 8'	Red brown and brown granitic **BEDROCK**, highly weathered, soft to moderately hard, moist (decomposed)		
@ 8' - 10'	*Grades light grey, hard*		
	Test pit terminated at 10' Seepage encountered at 10' (minor) No caving noted		



Note: The test pit log indicates subsurface conditions only at the specific location and time noted. Subsurface conditions, including groundwater levels, at other locations of the subject site may differ significantly from conditions which, in the opinion of Youngdahl Consulting Group, Inc., exist at the sampling locations, Note, too, that the passage of time may affect conditions at the sampling locations.

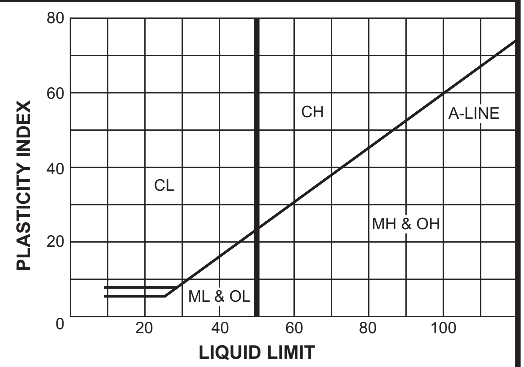
YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001	**EXPLORATORY TEST PIT LOG**	**FIGURE**
	December 2024	**Carmenere West** Loomis, California	**A-15**

UNIFIED SOIL CLASSIFICATION SYSTEMS

MAJOR DIVISION			SYMBOLS	TYPICAL NAMES
COARSE GRAINED SOILS Over 50% > #200 sieve	GRAVELS Over 50% > #4 sieve	Clean **GRAVELS** With Little Or No Fines	GW	Well graded **GRAVELS**, **GRAVEL-SAND** mixtures
			GP	Poorly graded **GRAVELS**, **GRAVEL-SAND** mixtures
		GRAVELS With Over 12% Fines	GM	Silty **GRAVELS**, poorly graded **GRAVEL-SAND-SILT** mixtures
			GC	Clayey **GRAVELS**, poorly graded **GRAVEL-SAND-CLAY** mixtures
	SANDS Over 50% < #4 sieve	Clean **SANDS** With Little Or No Fines	SW	Well graded **SANDS**, gravelly **SANDS**
			SP	Poorly graded **SANDS**, gravelly **SANDS**
		SANDS With Over 12% Fines	SM	Silty **SANDS**, poorly graded **SAND-SILT** mixtures
			SC	Clayey **SANDS**, poorly graded **SAND-CLAY** mixtures
FINE GRAINED SOILS Over 50% < #200 sieve	**SILTS & CLAYS** Liquid Limit < 50		ML	Inorganic **SILTS**, silty or clayey fine **SANDS**, or clayey **SILTS** with plasticity
			CL	Inorganic **CLAYS** of low to medium plasticity, gravelly, sandy, or silty **CLAYS**, lean **CLAYS**
			OL	Organic **CLAYS** and organic silty **CLAYS** of low plasticity
	SILTS & CLAYS Liquid Limit > 50		MH	Inorganic **SILTS**, micaceous or diamacious fine sandy or silty soils, elastic **SILTS**
			CH	Inorganic **CLAYS** of high plasticity, fat **CLAYS**
			OH	Organic **CLAYS** of medium to high plasticity, organic **SILTS**
HIGHLY ORGANIC CLAYS			PT	PEAT & other highly organic soils

PLASTICITY CHART

USED FOR CLASSIFICATION OF FINE GRAINED SOILS



SAMPLE DRIVING RECORD

BLOWS PER FOOT	DESCRIPTION
25	25 Blows drove sampler 12 inches, after initial 6 inches of seating
50/7"	50 Blows drove sampler 7 inches, after initial 6 inches of seating
50/3"	50 Blows drove sampler 3 inches during or after initial 6 inches of seating

Note: To avoid damage to sampling tools, driving is limited to 50 blows per 6 inches during or after seating interval.

SOIL GRAIN SIZE

U.S. STANDARD SIEVE		6"		3"	¾"	4	10	40	200		
	BOULDER		COBBLE	GRAVEL		SAND			SILT		CLAY
				COARSE	FINE	COARSE	MEDIUM	FINE			
SOIL GRAIN SIZE IN MILLIMETERS		150		75	19	4.75	2.0	.425	0.075	0.002	

KEY TO PIT & BORING SYMBOLS

Standard Penetration test

2.5" O.D. Modified California Sampler

3" O.D. Modified California Sampler

Shelby Tube Sampler

2.5" Hand Driven Liner

Bulk Sample

Water Level At Time Of Drilling

Water Level After Time Of Drilling

P Perched Water

KEY TO PIT & BORING SYMBOLS

Joint

Foliation

Water Seepage

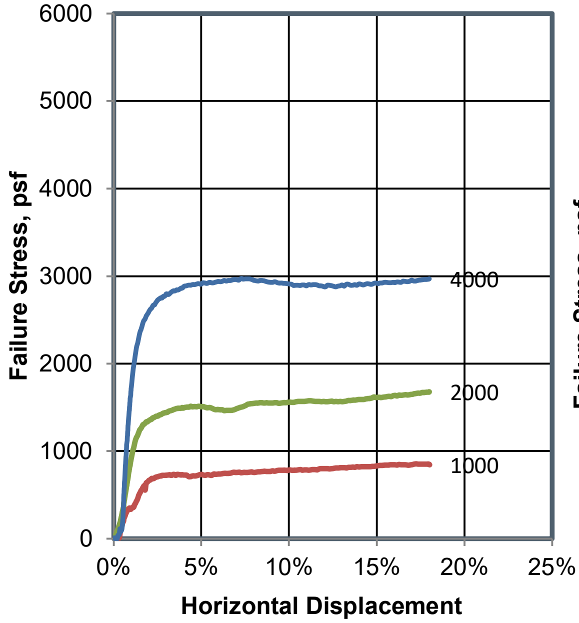
NFWE	No Free Water Encountered
FWE	Free Water Encountered
REF	Sampling Refusal
DD	Dry Density (pcf)
MC	Moisture Content (%)
LL	Liquid Limit
PI	Plasticity Index
PP	Pocket Penetrometer
UCC	Unconfined Compression (ASTM D2166)
TVS	Pocket Torvane Shear
EI	Expansion Index (ASTM D4829)
Su	Undrained Shear Strength

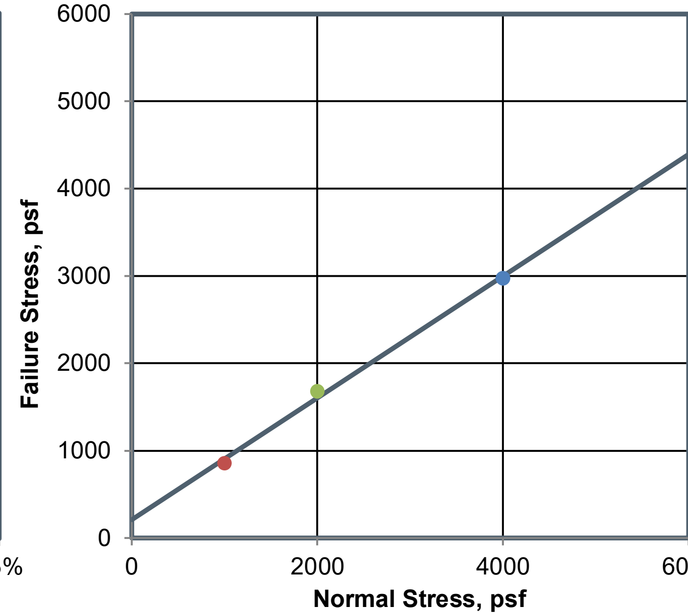
APPENDIX B

Laboratory Testing

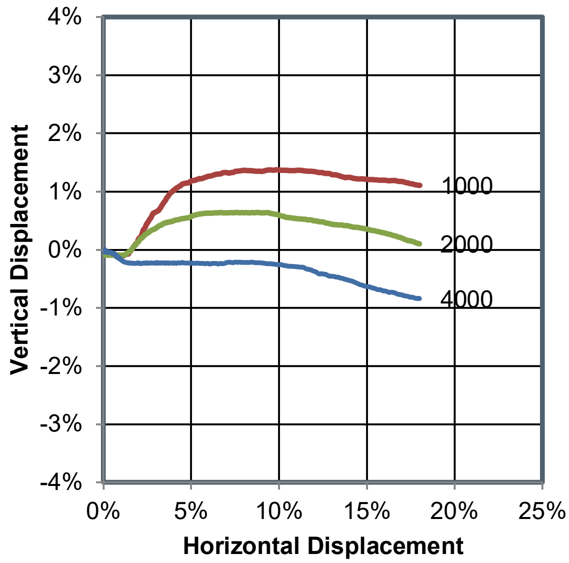
Direct Shear Test
R-Value Test
Modified Proctor Test
Sieve Analysis Test
Corrosivity Tests

Direct Shear Test of Soils Under Consolidated Drained Conditions, ASTM D3080





Direct Shearbox Results
Friction Angle
34.9°
Cohesion
208 psf



	Test No.	1	2	3
Initial	Wet Density, pcf	124.3	124.3	124.3
	Dry Density, pcf	112.7	112.7	112.7
	Moisture Content, %	10.3	10.3	10.3
	Diameter, in	2.50	2.50	2.50
	Height, in	1.00	1.00	1.00
Pre Shear	Wet Density, pcf	132.2	131.9	131.6
	Dry Density, pcf	113.4	113.6	114.0
	Moisture Content, %*	16.6	16.1	15.5
	Diameter, in	2.50	2.50	2.50
	Height, in	0.99	0.99	0.99
Normal Stress, psf		1000	2000	4000
Failure Stress, psf		856	1678	2974
Failure Strain, %		17.24	18.00	7.27
Rate, in/min			0.002	

*Based on post shear moisture content

Sample Type: Remolded to 90% RC

Material Description: **Brown Silty SAND**

Source: Native

Notes: Gravel removed from test sample

Sample No./Depth: TP-4 @ 0-2'		USCS Class.	Liquid Limit	Plasticity Index	% Greater than No. 4	% Less than No. 200
Date Sampled: 10/23/2024	Date Test Started: 11/1/2024				1	

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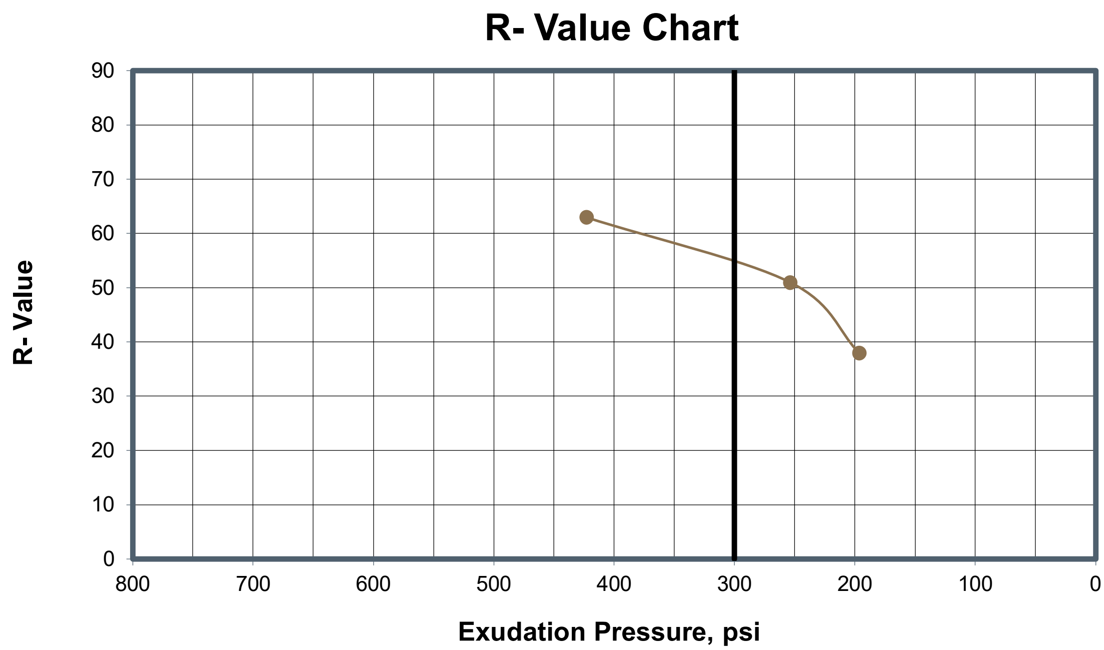
Project: **Carmenere West GES**

Project No.: **E18269.001**

Reviewed By: DN | Date: 11/4/2024

Figure B-1

Resistance "R" Value of Soil and Soil-Aggregate Mixtures, CTM 301



Test Specimen No.:	1	2	3
Moisture Content at Test, %	8.6	9.7	10.7
Dry Density at Test, pcf	129.4	126.7	125.9
Expansion Pressure, psf	65	0	0
Exudation Pressure, psi	423	254	197
Resistance "R" Value	63	51	38
"R" Value at 300 psi Exudation Pressure			**55**

Material Description: **Brown Silty Fine SAND**

Source: TP-1 @ 0-3'

Notes:

Sample No./Depth:	RV-1			USCS Class.	Liquid Limit	Plasticity Index	% Greater than No. 4	% Less than No. 200
Date Sampled:	10/23/2024	Date Test Started:	11/11/2024				0	

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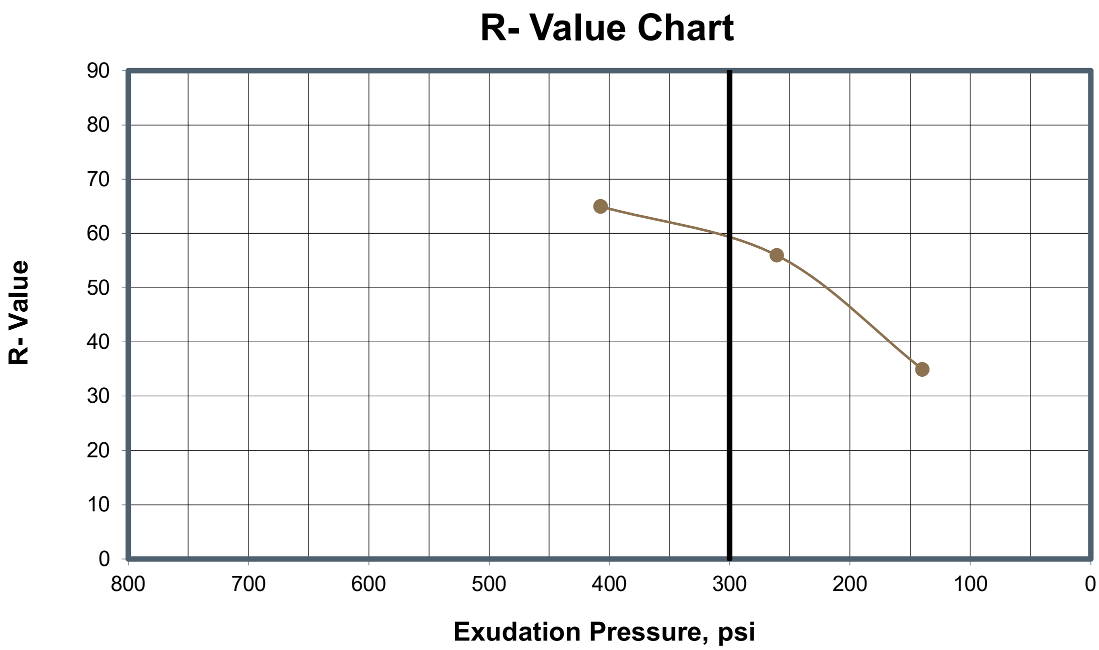
Project:	**Carmenere West**			
Project No.:	**E18269.001**			Figure
Reviewed By:	JLC	Date:	11/12/2024	B-2

Resistance "R" Value of Soil and Soil-Aggregate Mixtures, CTM 301



Test Specimen No.:	1	2	3
Moisture Content at Test, %	9.9	11.0	12.0
Dry Density at Test, pcf	126.3	124.4	123.6
Expansion Pressure, psf	0	0	0
Exudation Pressure, psi	407	261	140
Resistance "R" Value	65	56	35
"R" Value at 300 psi Exudation Pressure			**59**

Material Description: **Brown Silty Fine SAND**

Source: TP-12 @ 0-3'

Notes:

Sample No./Depth:	RV-2			USCS Class.	Liquid Limit	Plasticity Index	% Greater than No. 4	% Less than No. 200
Date Sampled:	10/23/2024	Date Test Started:	11/11/2024				0	

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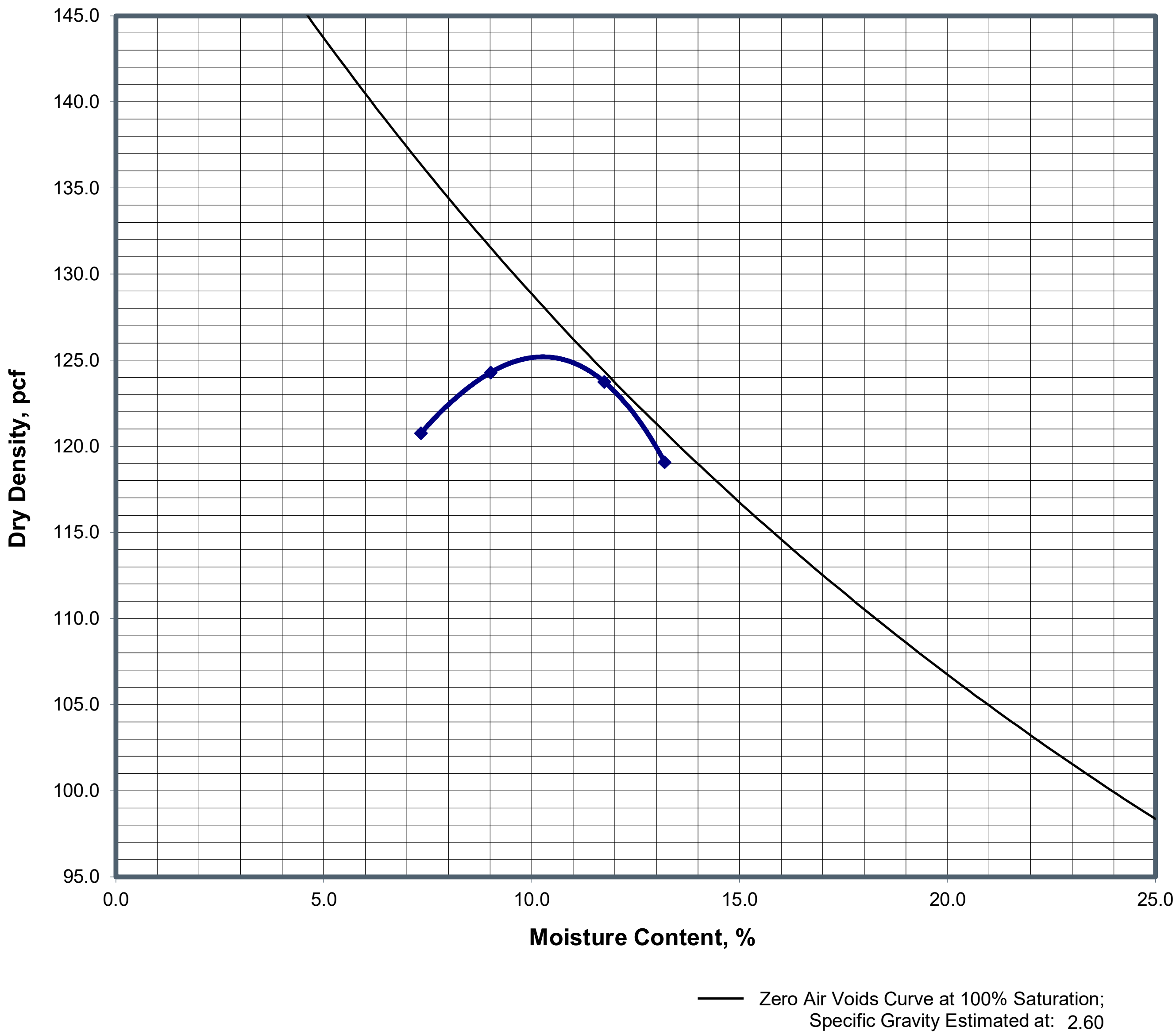
Project:	**Carmenere West**			
Project No.:	**E18269.001**			Figure
Reviewed By:	JLC	Date:	11/12/2024	B-3

Laboratory Compaction Characteristics of Soil
Using Modified Effort (56,000 lf-lbf/ft3), ASTM D1557, Method A



Maximum Dry Density, pcf:	**125.2**	Optimum Moisture Content, %:	**10.3**		
Material Description:	**Brown Silty SAND**				
Source:	Native				
Notes:	TP-4 @ 0-2'				
Sample No./Depth:	Curve 1	USCS Class.	Liquid Limit	Plasticity Index	% Greater than No. 4 :	% Less than No. 200
Date Sampled: 10/23/2024	Date Test Started: 10/29/2024				1	

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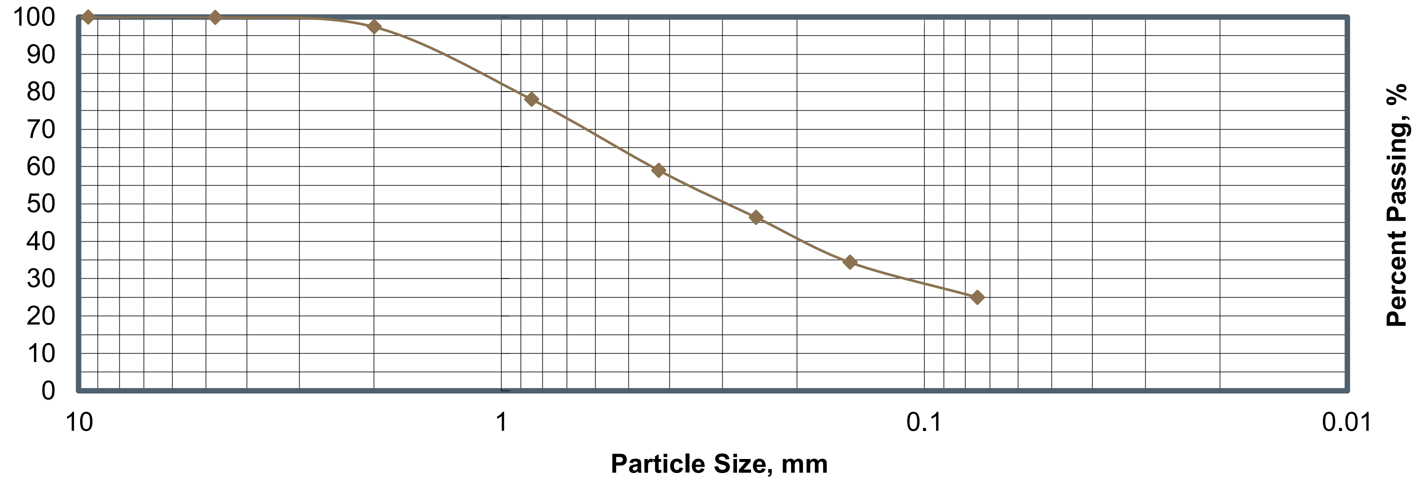
Project:	**Carmenere West GES**		
Project No.:	**E18269.001**		Figure
Reviewed By: DN	Date: 11/4/2024		B-4

Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis, ASTM D6913, Method A



U.S. Standard Sieve Size	Combined % Passing
3/8 Inch (9.5 mm)	100
No. 4 (4.75 mm)	100
No. 10 (2 mm)	97
No. 20 (850 µm)	78
No. 40 (425 µm)	59
No. 60 (250 µm)	46
No.100 (150 µm)	34
No. 200 (75 µm)	25.0

Material Description: **Brown Silty Fine SAND**

Source:

Notes:

Sample No./Depth:	TP-12 @ 0-3'			USCS Class.	Liquid Limit	Plasticity Index	% Greater than No. 4	% Less than No. 200
Date Sampled:	10/23/2024	Date Test Started:	11/11/2024				0	25.0

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Project:	**Carmenere West GES**			Figure
Project No.:	**E18269.001**			
Reviewed By:	DN	Date:	11/14/2024	B-5

Sunland Analytical
11419 Sunrise Gold Circle, #10
Rancho Cordova, CA 95742
(916) 852-8557

Date Reported 11/06/2024
Date Submitted 10/30/2024

To: Jeffry Cannon
Youngdahl Consulting Group
1234 Glenhaven Ct.
El Dorado Hills, CA 95630

From: Gene Oliphant, Ph.D. \ Ty Bui
General Manager \ Lab Manager

The reported analysis was requested for the following location:
Location : E18269.001 Site ID : TP-4@0-2FT.
Thank you for your business.

* For future reference to this analysis please use SUN # 93436-193299.

EVALUATION FOR SOIL CORROSION

Soil pH	5.86		
Minimum Resistivity	2.68 ohm-cm (x1000)		
Chloride	1.9 ppm	00.00019	%
Sulfate	101.9 ppm	00.01019	%

METHODS
pH and Min.Resistivity CA DOT Test #643
Sulfate CA DOT Test #417, Chloride CA DOT Test #422m

Sunland Analytical
11419 Sunrise Gold Circle, #10
Rancho Cordova, CA 95742
(916) 852-8557

Date Reported 11/06/2024
Date Submitted 10/30/2024

To: Jeffry Cannon
Youngdahl Consulting Group
1234 Glenhaven Ct.
El Dorado Hills, CA 95630

From: Gene Oliphant, Ph.D. \ Ty Bui
General Manager \ Lab Manager

The reported analysis was requested for the following location:
Location : E18269.001 Site ID : TP-9@5-10FT.
Thank you for your business.

* For future reference to this analysis please use SUN # 93436-193300.

EVALUATION FOR SOIL CORROSION

Soil pH	5.62		
Minimum Resistivity	16.08	ohm-cm (x1000)	
Chloride	1.2 ppm	00.00012	%
Sulfate	4.3 ppm	00.00043	%

METHODS
pH and Min.Resistivity CA DOT Test #643
Sulfate CA DOT Test #417, Chloride CA DOT Test #422m

APPENDIX C

Details

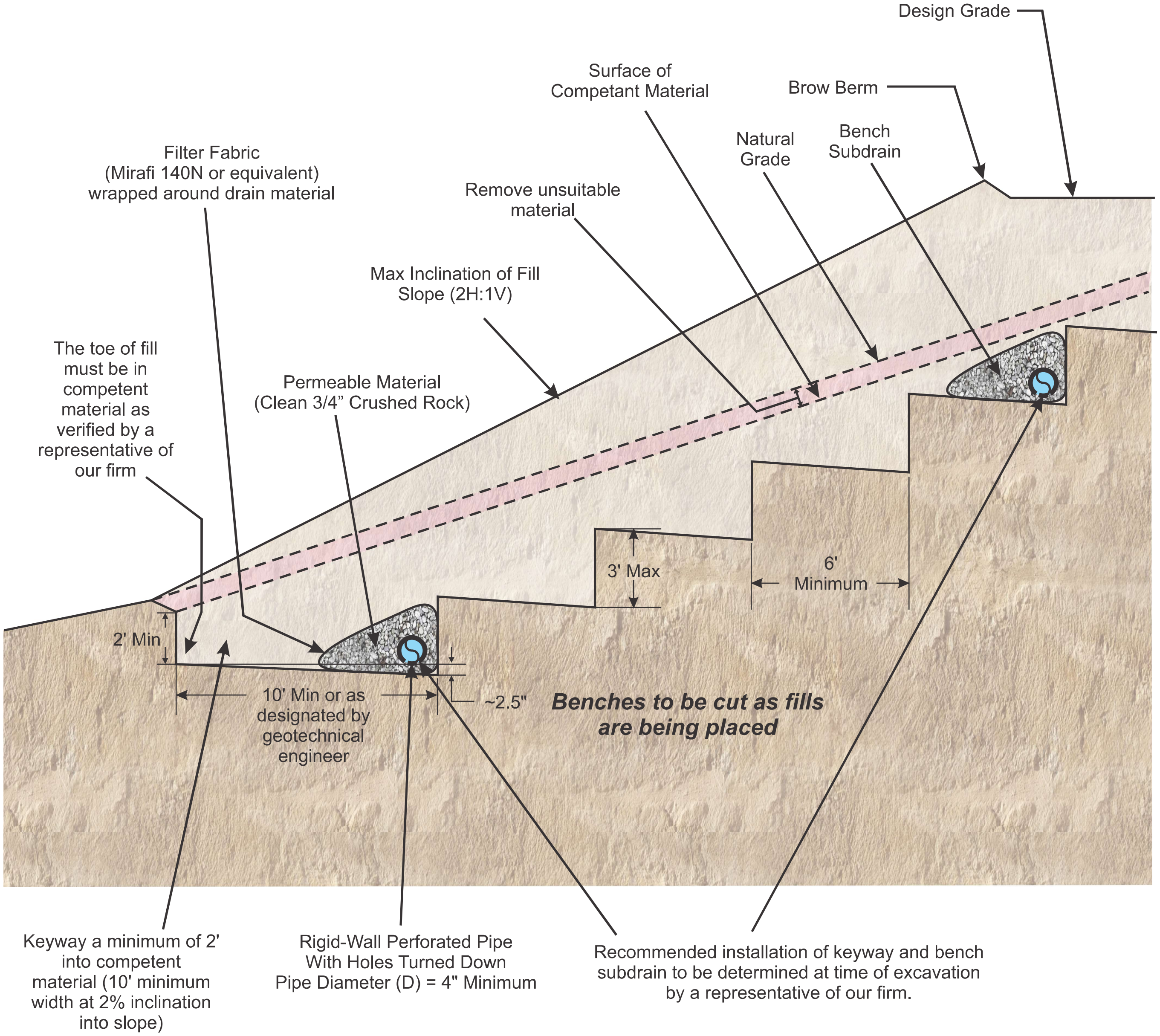
Keyway and Bench with Drain
Site Wall Drainage
Subdrain
Plug and Drain

PLACEMENT OF FILL ON NATURAL SLOPES

(Typical Cross Section)

All keyways should be observed and approved prior to placement of fill.

A keyway is required by CBC for fills on natural slopes of 5H:1V or steeper.



Notes:

1. Slope trench and rigid-wall pipes at least 1% gradient to drain to an appropriate outfall area away from residence.
2. Use sweeps for directional changes in pipe flow **(do not use 90°elbows)**.
3. Provide periodic clean-outs.

Not to Scale

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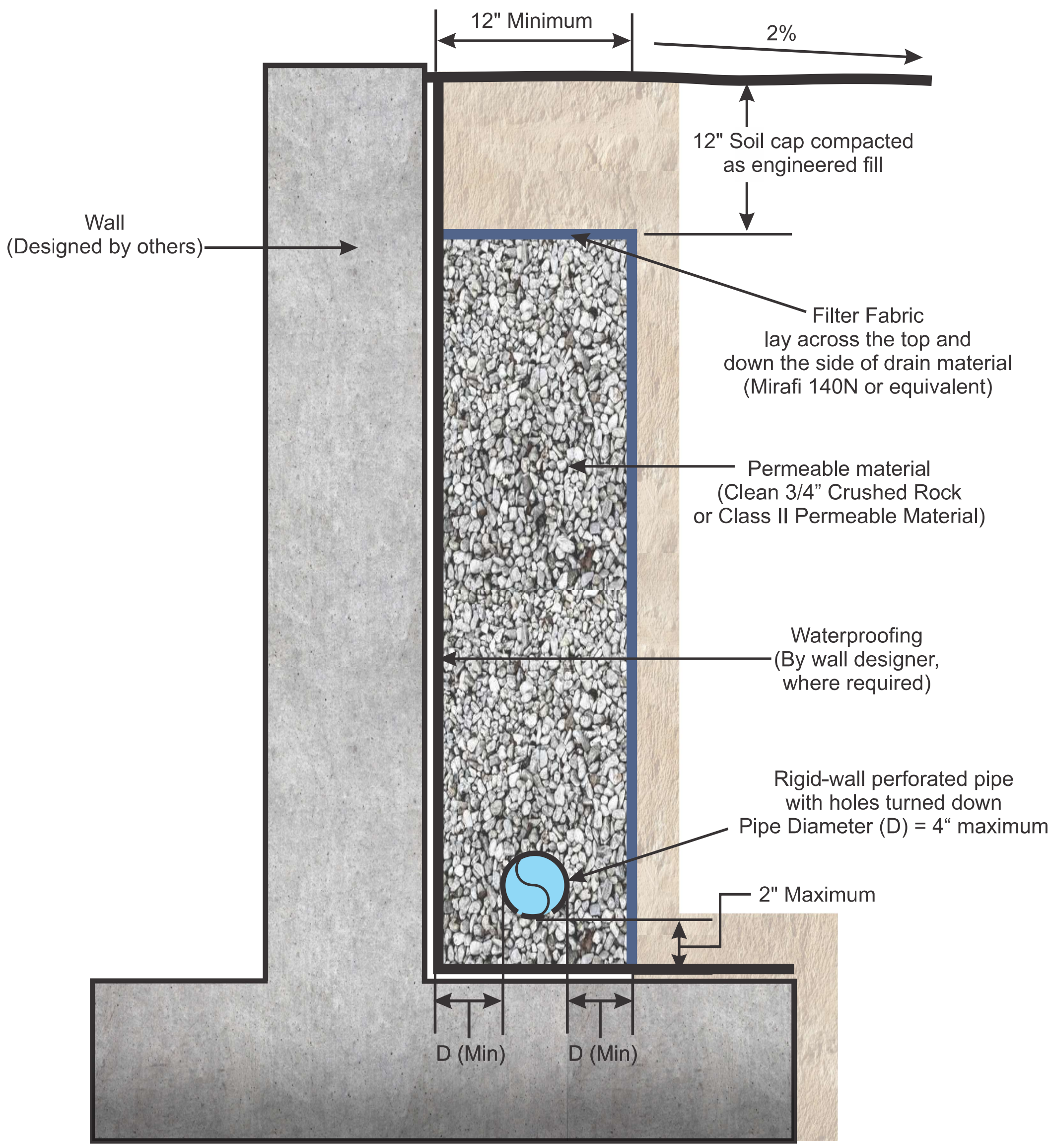
Project No.: E18269.001	KEYWAY AND BENCH DETAIL	FIGURE
December 2024	**Carmenere West** Loomis, California	**C-1**

Site Retaining Wall With Perforated Pipe Backdrain

(Typical Cross Section)



Notes:

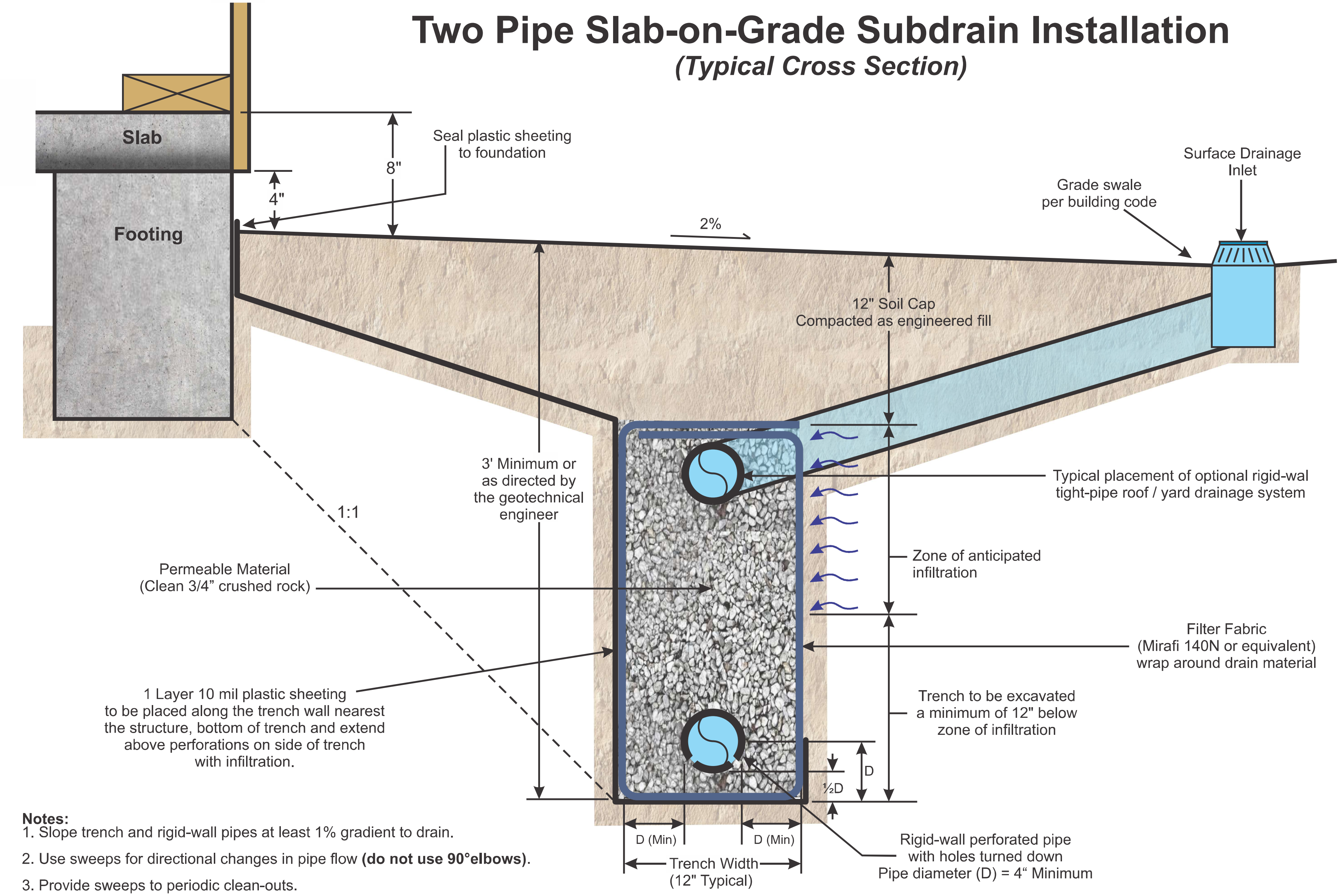
1. Slope trench and rigid-wall pipes at least 1% gradient to drain to an appropriate outfall area away from residence.
2. Use sweeps for directional changes in pipe flow **(do not use 90°elbows)**.
3. Provide periodic clean-outs.
4. If Class II Permeable Material is used, installation of filter fabric may be waived.

Not to Scale

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**SITE RETAINING WALL BACKDRAIN DETAIL** **Carnemere West** Loomis, California	**FIGURE C-2**

Two Pipe Slab-on-Grade Subdrain Installation

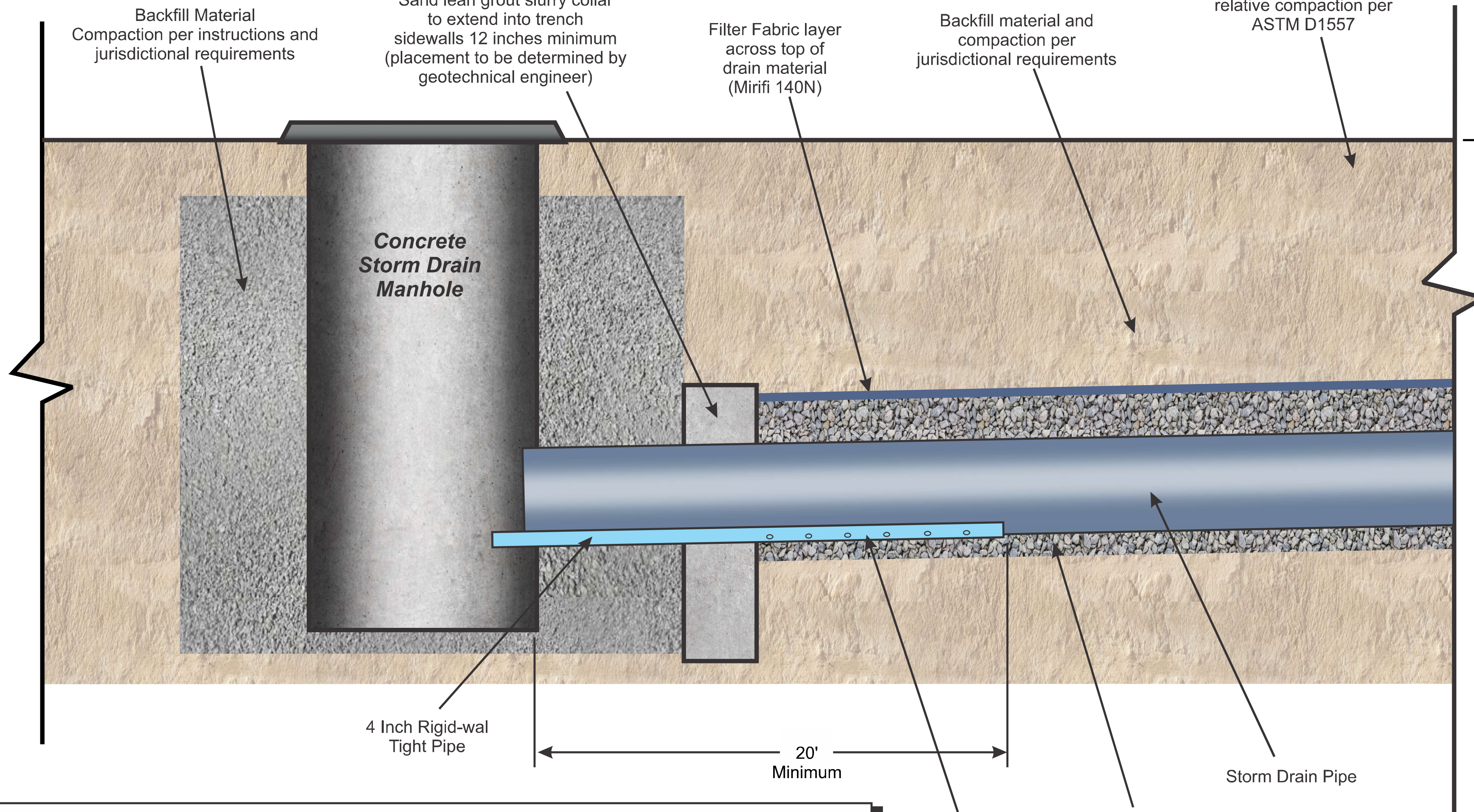
(Typical Cross Section)

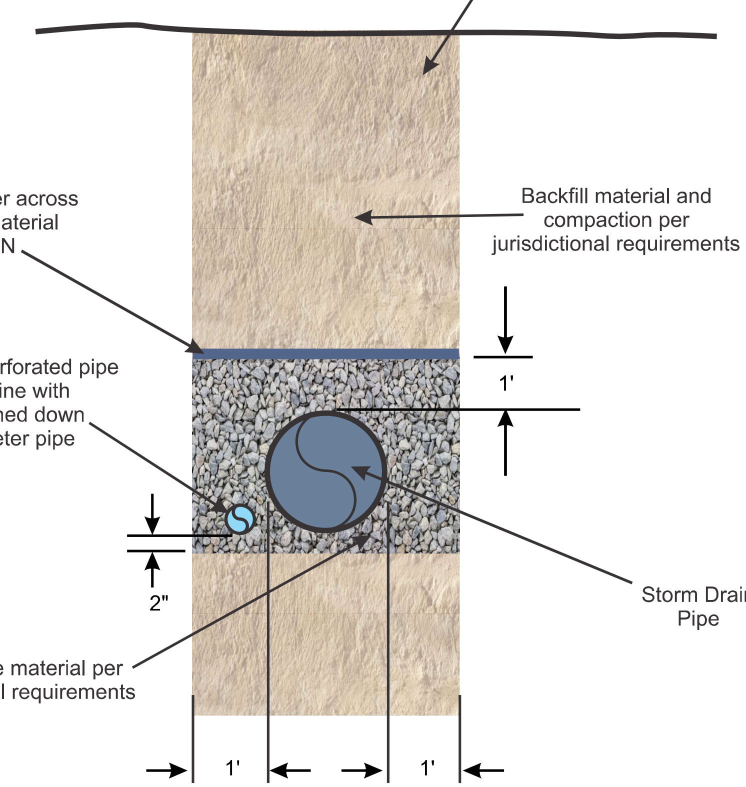


Notes:
1. Slope trench and rigid-wall pipes at least 1% gradient to drain.
2. Use sweeps for directional changes in pipe flow **(do not use 90°elbows)**.
3. Provide sweeps to periodic clean-outs.

Not to Scale

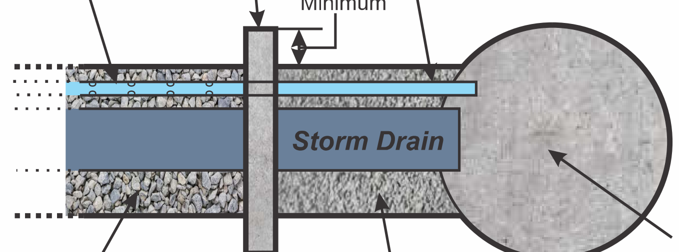
YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**SUBDRAIN DETAIL** **Carmenere West** Loomis, California	**FIGURE** **C-3**





Grout Collar Cut-Off Subdrain Detail (Typical)

Plan View



Notes:
1. Slope trench and rigid-wall pipes at least 1% gradient to drain.
2. Use sweeps for directional changes in pipe flow **(do not use 90°elbows)**.
3. Provide periodic clean-outs.
4. Slurry collar to extend into trench sidewalls 12 inches minimum and extend to top of pipe envelope.

Not to Scale

YOUNGDAHL CONSULTING GROUP, INC. *Celebrating 40 Years of Service*	Project No.: E18269.001 **December 2024**	**STORM DRAIN PLUG AND DRAIN DETAIL** **Carmenere West** Loomis, California	**FIGURE** **C-4**