

Madeira Beach Town Center, LLC Mesh Architecture



Madeira Beach, Florida November 4, 2016

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AERIAL VIEW FROM EAST





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Watershed Science

& Engineering, LLC





# Index

8

Project Description	
Special Area Plan	3
Project Data	4

**Perspective View** 

**Aerial View of Madeira Beach** 7 **Aerial View of Madeira Beach Town Center** 

**Overall Site Plan** 

**Perspective View** 9-17

18 **Public Access Site Plan** 

**Overall Parcel Plan** 19

20 **Marina Site Plan** 

21 **Parcel 1 Design Guidelines** 

22 **Dock Master Elevations** 

23 **Aerial View** 

24 Condo A Site Plan

25 **Parcel 2 Design Guidelines** 

26-27 Condo A Elevations

Condo B Site Plan 28 29

**Parcel 3 Design Guidelines** 

30-31 **Condo B Elevations** 

**Hotel A Site Plan** 32 33

**Parcel 4 Design Guidelines** 

34-35 **Hotel A Elevations** 

**Hotel B Site Plan** 

37 **Parcel 5 Design Guidelines** 

Hotel B Elevations 38-39

41 **Appendix** 

42 Survey of East Property **Survey of West Property** 43

44-47 Civil Site Plan and Data

**Traffic Analysis** 48-69

Madeira Beach Town Center

MADEIRA BEACH, FLORIDA

36

#### PROJECT DESCRIPTION

The Madeira Beach Town Center project is envisioned to be just that, a new gathering place for the local community. The master plan is organized around two new civic places. Madeira Way has been reconceived and redesigned to be a two block long pedestrian-oriented street lined with shops and cafes. At the intersection of Gulf Boulevard and 150th Avenue is Madeira Plaza, a new half-acre town square designed to host events and activities, large and small throughout the day, week and year. In addition to these two spaces a pedestrian/bike trail has been extended from Madeira Way to Causeway Park. Also an elevated crosswalk is proposed to provide safe passage across Gulf Boulevard.

The 6.7-acre mixed-use project consists of two properties, both with frontage onto Madeira Way. All of the existing buildings will be demolished as new construction proceeds. Distributed across the two properties are five new buildings. Two new 6-story condominium buildings, totaling 80 units, front onto 150th Avenue. Parking is located on the first two floors with four floors of residential units above. These buildings are two floors less than Boca Vista across the street. The existing 43-slip marina will be completely redeveloped. A new one or two-story dockmaster building with up to 8,000 square feet will be built at the northeast end of the site to service the marina.

North of Madeira Way a new suites hotel with up to 148 rooms will be developed. This building will have eight floors. On the first floor will be the hotel lobby, up to 5,000 square feet of retail and restaurant space and the entry to the parking garage. The second and third floors will contain parking and above will be five floors of hotel rooms. The building has been designed to reinforce the pedestrian character of Madeira Way.

Across the street, on the triangular block, will be a new full-service hotel with up to 168 rooms plus 10 condominiums. This building will also have eight floors, four floors less than the Ocean Sands Condominiums located across Gulf Boulevard. On the first floor will be the hotel lobby, up to 40,000 square feet of retail and restaurant space and the parking garage entry with covered drop-off for the hotel. In addition, a pedestrian arcade through the building will connect Madeira Way to the public plaza. The parking garage will be on the second and third floors. The hotel amenities are located on the fourth floor with direct access to a large roof terrace. The hotel rooms comprise the 5th through 7th floors and the condominiums are located on the top floor.

Automobile traffic has been carefully considered. The traffic signal at the intersection of 150th Avenue and Madeira Way is proposed to be removed. This will reduce congestion on 150th Avenue by allowing for a smoother flow of traffic. Currently between Causeway Park and Madeira Way there are eight curb cuts on the north side of 150th Avenue. These will all be eliminated except for two, one for the marina and one for the two new condominium buildings. This too will reduce traffic conflicts.

As previously mentioned Madeira Way will be completely re-built as a two-lane, two-way pedestrian oriented street with parallel parking and generous sidewalks. It will no longer function as a shortcut by through traffic due to its slow speed and because vehicles will no longer be allowed to make a left turn at the east end of the street onto 150th Avenue. On the triangular block, the existing curb cuts onto 150th Avenue and Gulf Boulevard will be eliminated. All vehicular access for the two hotels and their accessory retail space will be via Madeira Way, thereby minimizing conflicts with the two major streets.

The parking needs for the proposed new development have also been thoughtfully designed. Consistent with the Madeira Beach Town Center Special Area Plan parking has been designed to improve pedestrian flow throughout the project and minimize conflicts with vehicular traffic. Enough parking has been provided to slightly exceed the city's required standards; and except for a small surface parking lot servicing the marina all other parking has been consolidated into four parking garages. This minimizes the land area occupied by automobile parking and allows more area to be devoted to landscaped green spaces and lively public spaces.

#### SPECIAL AREA PLAN OBJECTIVES

As described below, special consideration has been given to meeting the Objectives of the Madeira Beach Town Center Special Area Plan as this proposal was developed.

Create a unique sense of place for the Town Center, and create a sense of arrival for those entering the area.

The design emphasis has been on creating distinctive pedestrian oriented places including a redeveloped pedestrian-oriented Madeira Way, a one-half acre public plaza at the intersection of Gulf Boulevard and 150th Avenue and a landscaped green space fronting the marina. Special care has been taken to create scenic gateways and views when entering the district from the east, south and north.

Promote a wide variety of uses to create an activity center for both local residents and tourists.

This is a true mixed-use project with a balance of residences, tourist accommodations, retail, restaurant, entertainment and recreational uses. And all of these uses are connected via sidewalks allowing for safe, convenient access across the district.

Set a standard for urban design so that new development and redevelopment in the Town Center contributes to the public realm.

As a result of this project two major public spaces will either be greatly improved (Madeira Way) or newly created (Madeira Plaza). This project proposes a carefully calibrated balance between new private development and public enhancement. These improvements will be codified through the Development Agreement as well as the Design Guidelines contained within this document.

Promote redevelopment of older properties in a manner that contributes to the quality of urban design in the Town Center.

Although the majority of the property is currently developed, it is at a density and pattern that does not create a sense of place for the community. The goal of the proposed project is to redevelop the area into a memorable destination for both locals and visitors.

Increase the number of temporary lodging units and maintain existing residential units in the Town Center that have the quality characteristics included in the Special Area Plan.

The proposal will add 316 hotel rooms and 90 dwelling units to the core of the Town Center. All will have thoughtful architectural design that reflect the relaxed, subtropical character of Madeira Beach while simultaneously working together to create a coherent character for the Town Center.

Improve pedestrian and bicycling access to all major destinations with the Town Center, including the parks, the beach, retail properties and civic destinations.

Numerous improvements are proposed to enhance pedestrian and bicycle access. A 12-foot-wide combined pedestrian/bicycle trail will connect Madeira Way to Causeway Park. Madeira Way will be redeveloped as a pedestrian oriented shopping street. A new public plaza will be located at the intersection of Gulf Boulevard and 150th Avenue with a direct pedestrian access to Madeira Way. An elevated crosswalk is proposed to provide safe, pedestrian access across Gulf Boulevard.

Increase connections and access to parks, ensuring that views of the Gulf and Boca Ciega Bay are preserved.

A new pedestrian/bicycle trail will connect Causeway Park back to the Town Center. A new public space will be created at the intersection of Gulf Boulevard and 150th Avenue. The new condominium buildings have been separated by 80 feet. And there is a 60 foot opening between Condominium B and Hotel A. The marina property is 345 feet long and is designed to appear as an extension of the Causeway Park. Although sometimes controversial, allowing for taller buildings in waterfront districts reduces the size of the structure's footprint and results in better views overall to and from neighboring properties. It also allows a greater percentage of the site to be dedicated to green space.

Develop parking and access strategies that help to make the most efficient use of scarce land and contribute to the quality of the public realm in the Town Center.

The site plan proposes to eliminate all curb cuts along 150th Avenue and Gulf Boulevard except for two access points, one for the marina and one for the two condominium buildings. All vehicular access for the two hotels will be via Madeira Way. Except for a small, 42 space, surface parking lot serving the marina, all other parking for the development will be located within four parking structures at the base of the new buildings. This allows for a significant increase in the amount of landscaped green space and lively public areas.

## **PROJECT DATA**

	Parcel 1	Parcel 2	Parcel 3	Parcel 4	Parcel 5	Totals
Land Use	Marina	Condo A	Condo B	Hotel A	Hotel B	
Land Area	60,258 s.f.	40,059 s.f.	48,791 s.f.	50,066 s.f.	92,511 s.f.	291,685 s.f.
Parcel Acreage	1.383 ac.	0.920 ac.	1.120 ac.	1.149 ac.	2.124 ac.	6.696 ac.
Causeway District Zoning	1.111 ac.	0.309 ac.				1.420 ac.
Commercial Core District Zoning	0.272 ac.	0.611 ac.	1.120 ac.	1.149 ac.	2.124 ac.	5.276 ac.
Hotel - Density Allowed (Commercial Core)						60 rooms/ac.
Hotel - Rooms Allowed (Commercial Core)						317 rooms
Hotel - Density Proposed (Commercial Core)						59.9 rooms/ac.
Hotel - Rooms Proposed (Commercial Core)				148 rooms	168 rooms	316 rooms
Condos - Density Allowed (Commercial Core)						15 units/ac.
Condos - Units Allowed (Commercial Core)					1	79 units
Condos - Density Proposed (Commercial Core)						11.1 units/ac.
Condos - Units Proposed (Commercial Core)		20 units	44 units		10 units	74 units
Condos - Density Allowed (Causeway)						15 units/ac.
Condos - Units Allowed (Causeway)						21 units
Condos - Density Proposed (Causeway)						11.3 units/ac.
Condos - Units Proposed (Causeway)		16 units				16 units
Condos - Units Proposed (Total)		36 units	44 units		10 units	90 units
Retail Space (Commercial Core)		14		2,000 s.f.	28,000 s.f.	30,000 s.f.
Restaurant Space (Commercial Core)				3,000 s.f.	12,000 s.f.	15,000 s.f.
Commercial Area - Proposed (Commercial Core)				5,000 s.f.	40,000 s.f.	45,000 s.f.
Commercial Density - Allowed (Commercial Core)						1.2 FAR
Commercial Area - Allowed (Commercial Core)						275,798 s.f.
Retail Space (Causeway)	7,000 s.f.					7,000 s.f.
Restaurant Space (Causeway)	1,000 s.f.					1,000 s.f.
Commercial Area - Proposed (Causeway)	8,000 s.f.					8,000 s.f.
Commercial Density - Allowed (Causeway)						0.5500 FAR
Commercial Area - Allowed (Causeway)						34,020 s.f.
Commercial Area - Proposed (Total)						53,000 s.f.
Boat Slips - Allowed						43 slips
Boat Slips - Proposed	43 slips					43 slips
Parking - Minimum Required	40 spaces	72 spaces	88 spaces	175 spaces	326 spaces	701 spaces
Parking - Proposed	42 spaces	77 spaces	93 spaces	175 spaces	326 spaces	713 spaces

#### Notes

2. In the Causeway District, 76% of the allowable density is allocated to residential use and 24% is allocated to commercial use.

<sup>1.</sup> Parking Ratios - Residential: 2 spaces per unit / Hotel: 1 space per room / Retail: 3 spaces per 2,000 s.f. / Restaurant: 1 space per 4 seats (125 s.f.) / Marina: 1 space per 2 slips



## **CITY OF MADEIRA BEACH**

he city of Madeira Beach is strategically located midway along the 30 plus miles of Pinellas peaches. It's a small town with a big waterfront over two miles of gulf beaches and many more miles of frontage onto Boca Ciega Bay. With 4,300 esidents in its one square mile, Madeira Beach salmost completely built out with relatively little new development in the last 15 years. A new Courtyard Marriott hotel opened last year on the mainland, but no new hotels have been built on the island since the 1970's. Although the residential nousing stock is reasonably stable, much of the commercial property consists of older onestory buildings with large, surface parking lots.



GOOGLE EARTH IMAGE

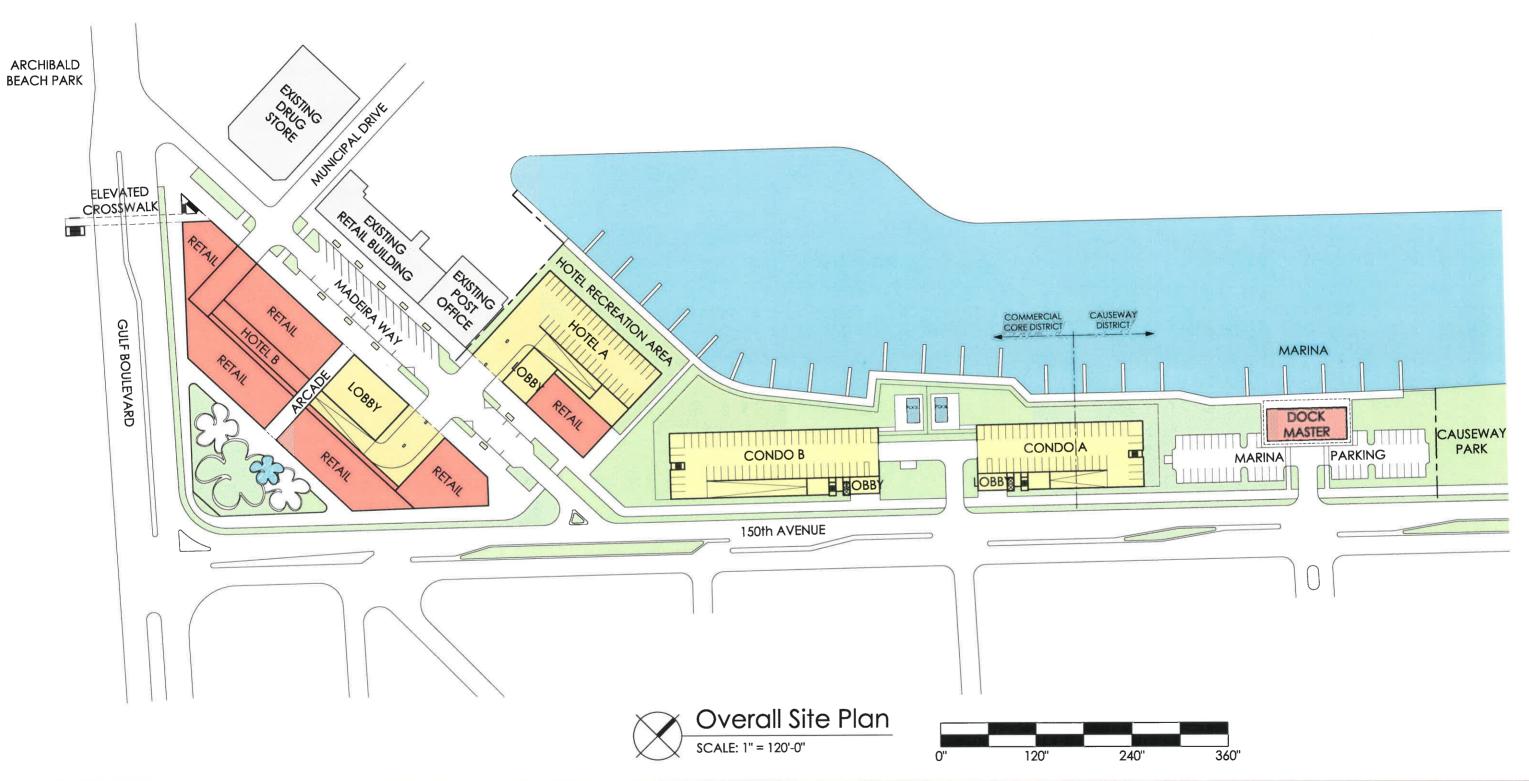


GOOGLE EARTH IMAGE

#### MADEIRA BEACH TOWN CENTER

As described within the city's Special Area Plan, the overall Town Center district is approximately 80 acres in size. The subject property's 6.7 acres is located right in the heart of the district. This property is significantly under-developed containing only 63,000 square feet of commercial space across multiple parcels. With very little green space the majority of the land consists of surface parking lots. However, as noted within the plan the location is superb. Serviced by two major arterial roadways, Gulf Boulevard and 150th Avenue, the property enjoys over 1,200 feet of frontage on the intercoastal waterway and is within walking distance of the Gulfbeaches, the town civic center, public parks and a drug store and grocery store.











VIEW OF MADEIRA WAY LOOKING EAST

Madeira Beach Town Center





VIEW OF MADEIRA WAY LOOKING WEST

Madeira Beach Town Center



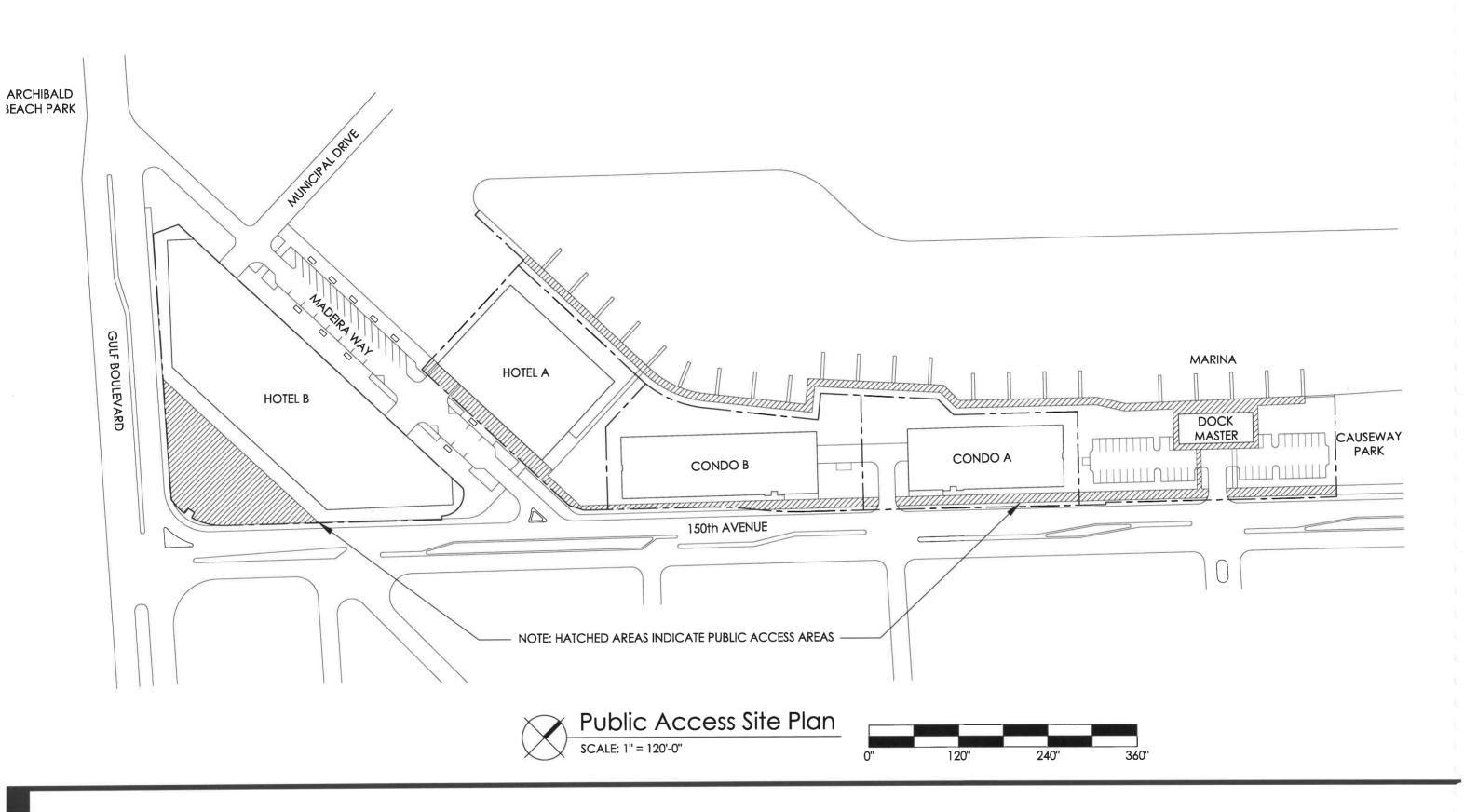


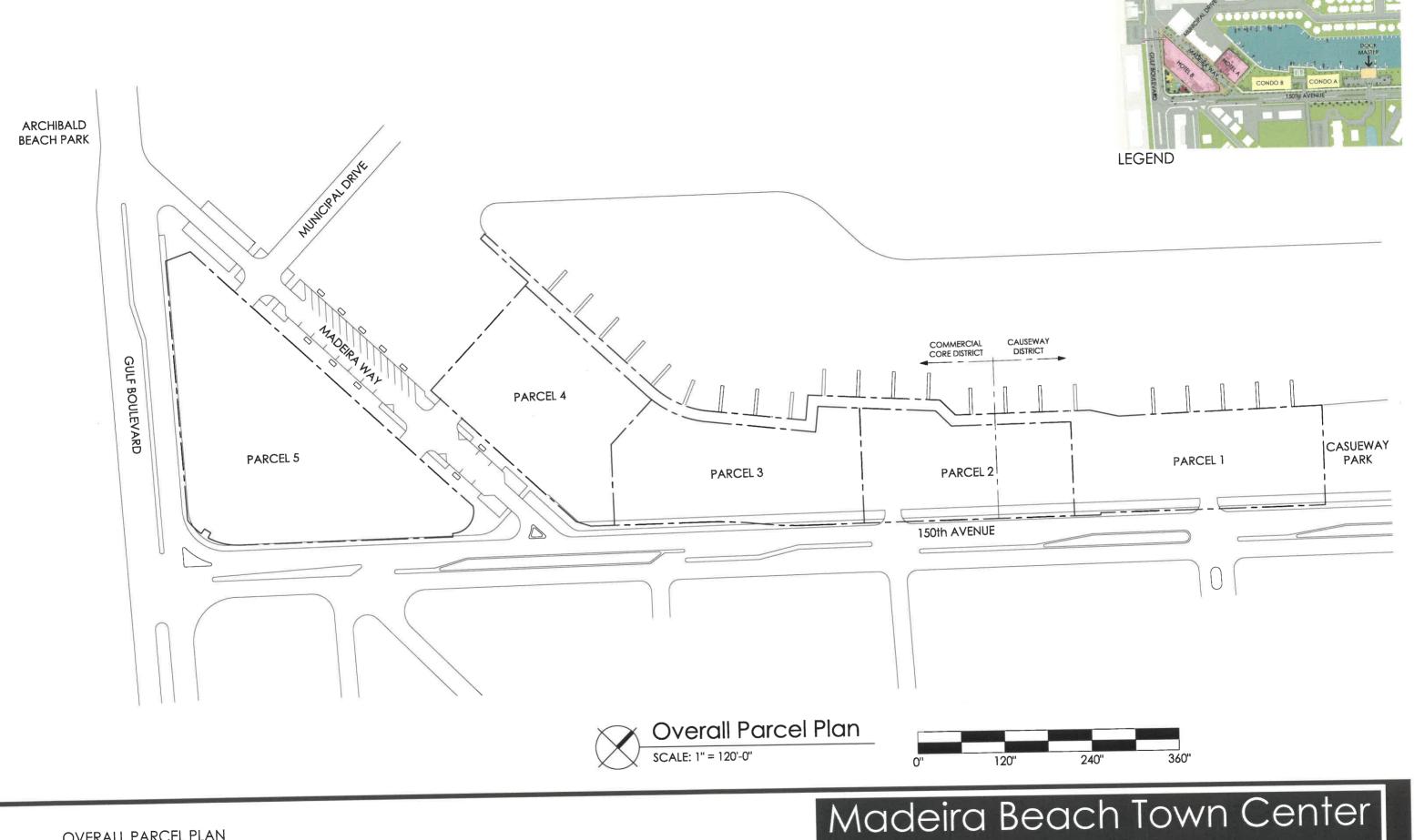
VIEW OF MADEIRA PLAZA

Madeira Beach Town Center





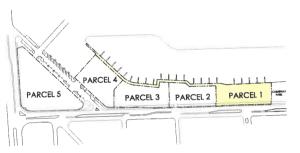




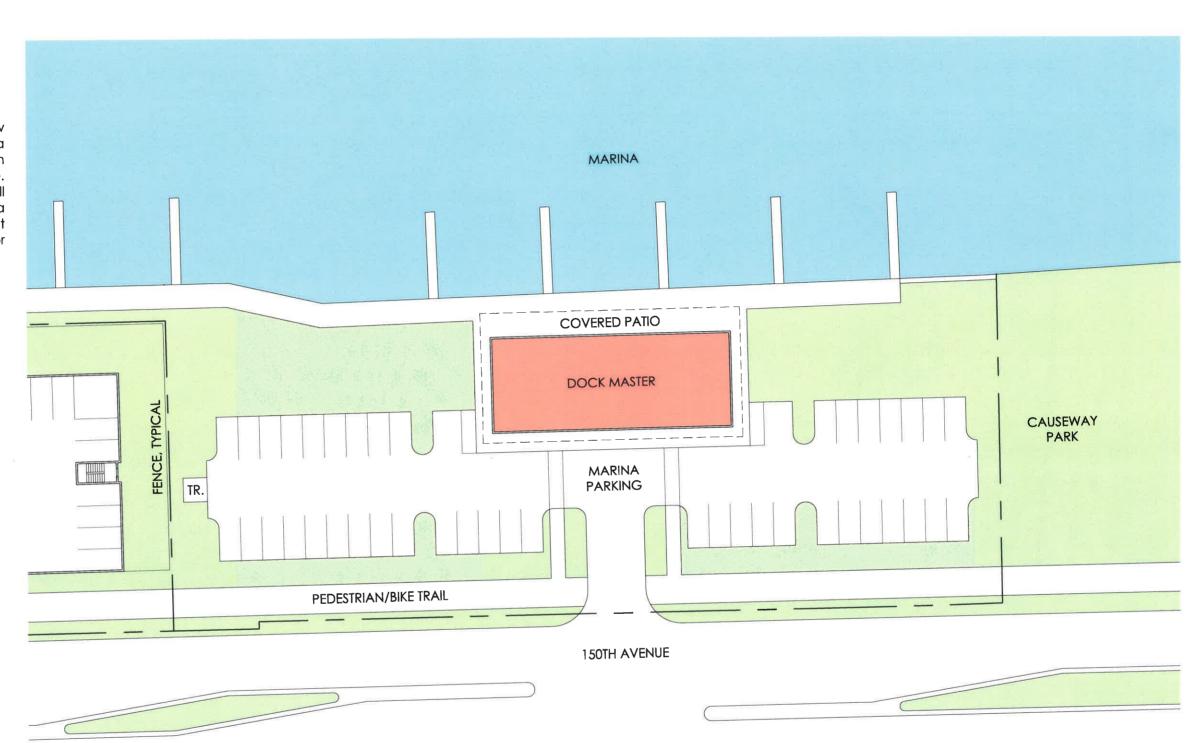
#### **MARINA**

# ,000 S.F. COMMERCIAL FLOORS 2 PARKING SPACES

ne existing 43 slip marina will be rebuilt with new eawall and docks. Serving the marina will be a ne or two story Dockmaster building located on xis with the new entry drive off of 150th Avenue. ne Dockmaster structure may include a small afé or snack stand. Adjacent to the building is a 2-space landscaped parking lot. The waterfront long the marina will be publicly accessible for redestrians.

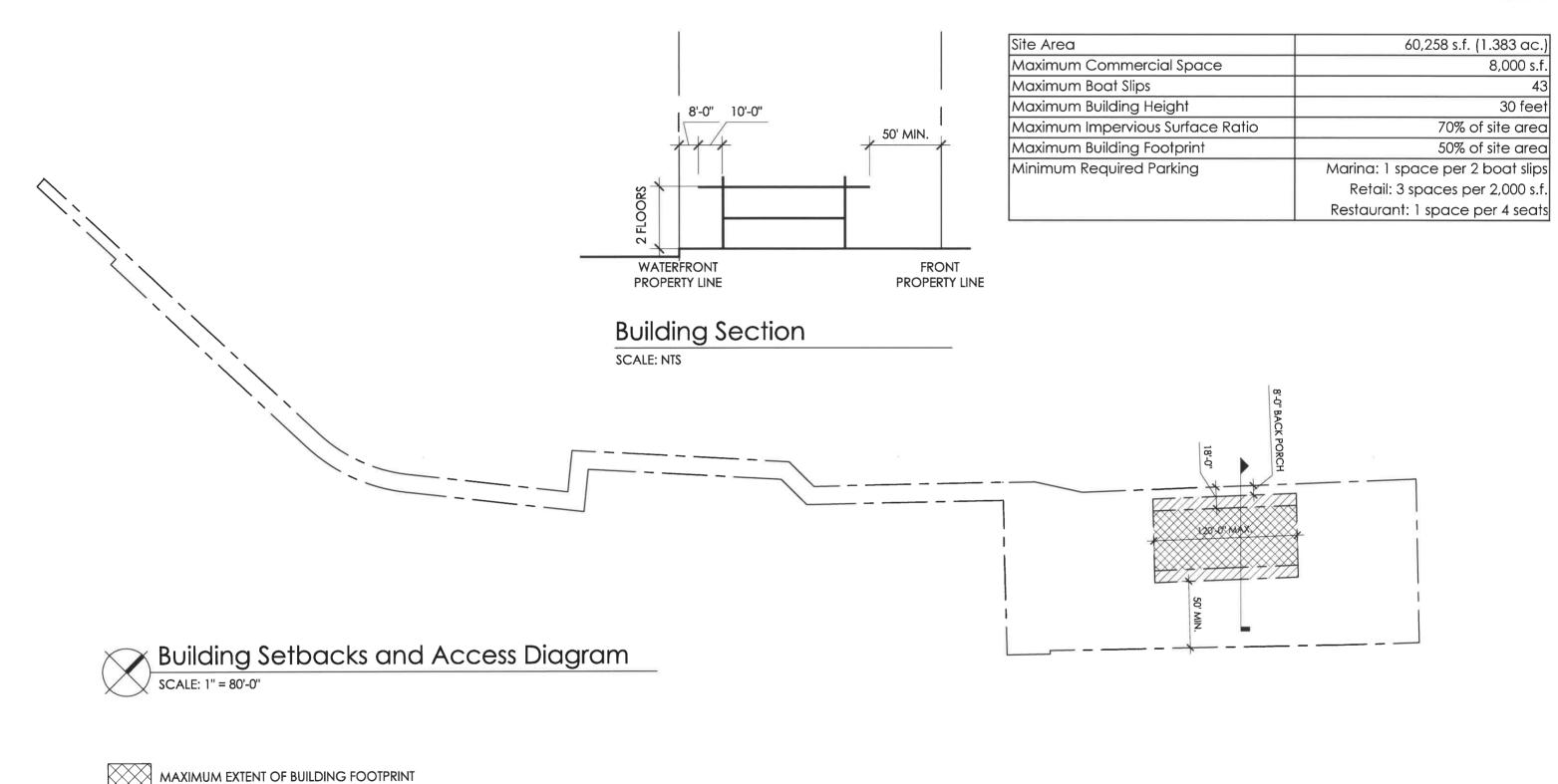


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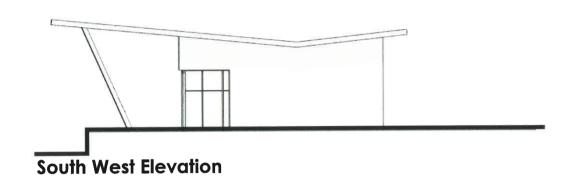


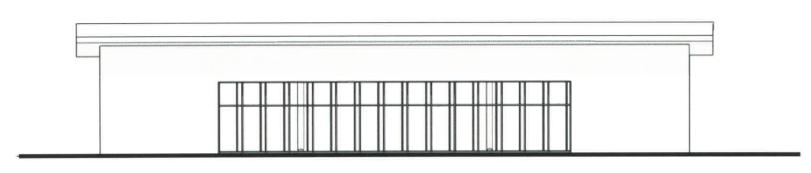


#### PARCEL 1

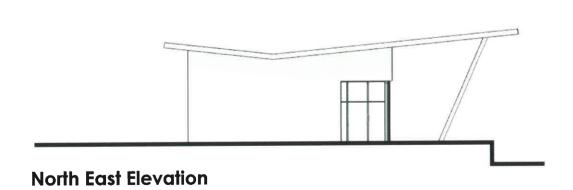


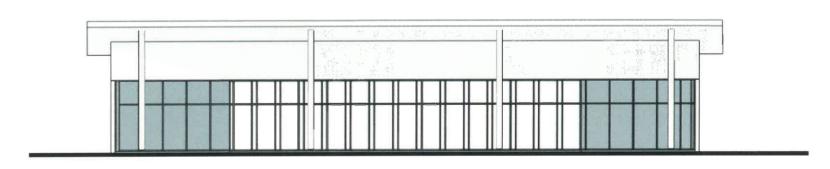
MAXIMUM EXTENT OF PORCH





**South East Elevation** 





**North West Elevation** 



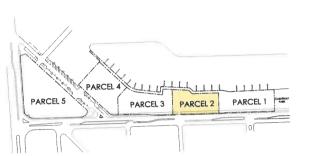
AERIAL VIEW

Madeira Beach Town Center

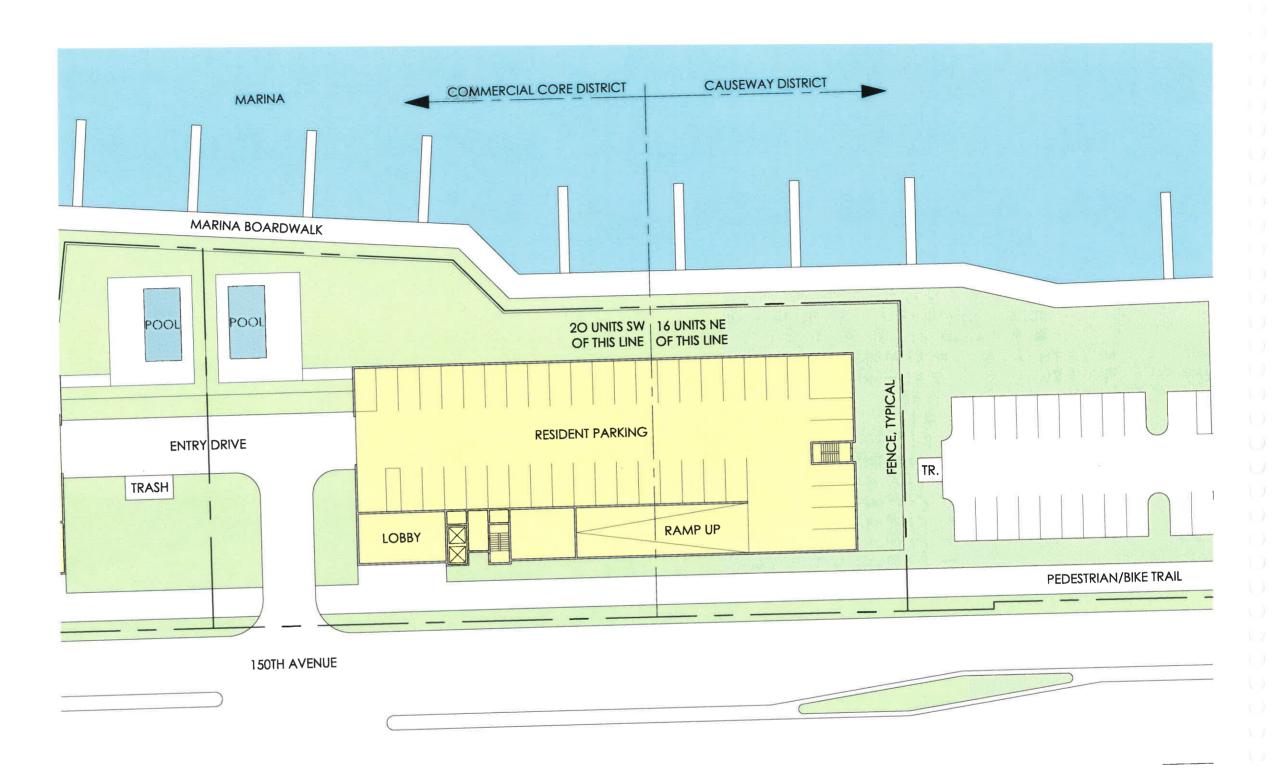
### **CONDO A**

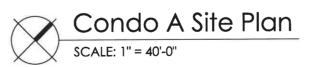
#### 6 CONDOMINIUM UNITS FLOORS 7 PARKING SPACES

nis building will have up to 36 dwelling units veraging approximately 1,600 square feet ach. The building's first floor lobby faces 150th venue to provide a visible front door. Parking is scated on the lower two floors with two spaces er unit plus a few extra for visitors. Above are four oors of units. The building is designed to take full idvantage of the water views in all directions. A par amenity area includes a swimming pool.



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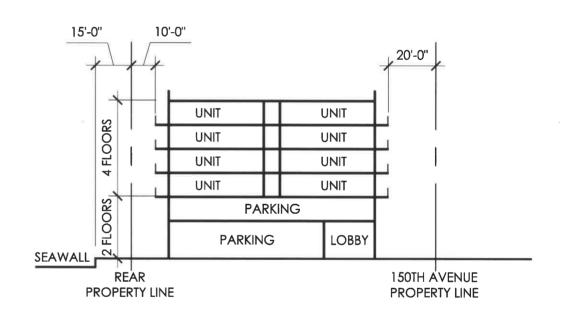


# **SEAWALL** PROPERTY LINE (TYP.) BUILDING SETBACK (TYP.) **VEHICULAR** ACCESS 40'-0" 150th AVENUE PEDESTRIAN ACCESS

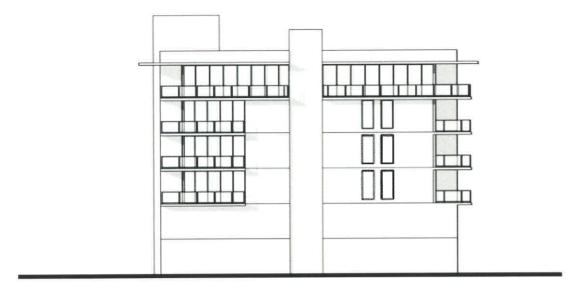


## PARCEL 2

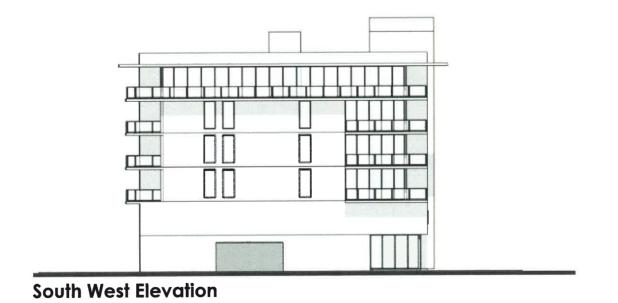
Site Area	40,059 s.f. (0.920 ac.)
Maximum Dwelling Units	36 units
Maximum Building Height	70 feet
Maximum Impervious Surface Ratio	70% of site area
Maximum Building Footprint	50% of site area
Minimum Required Parking	Residential: 2 spaces per unit



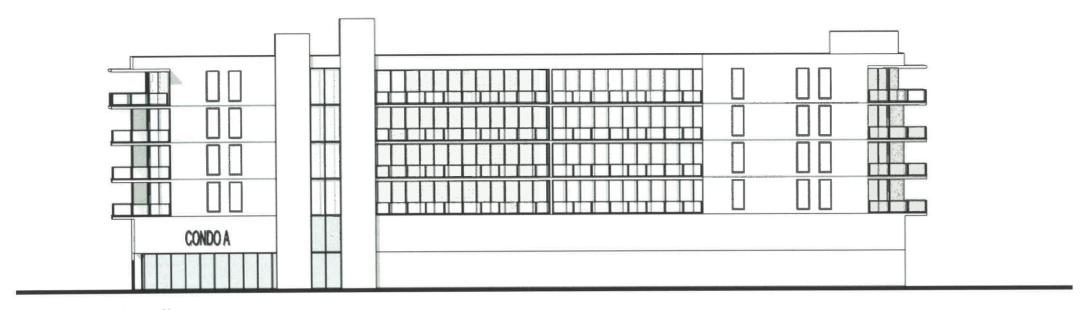
**Building Section** SCALE: NTS



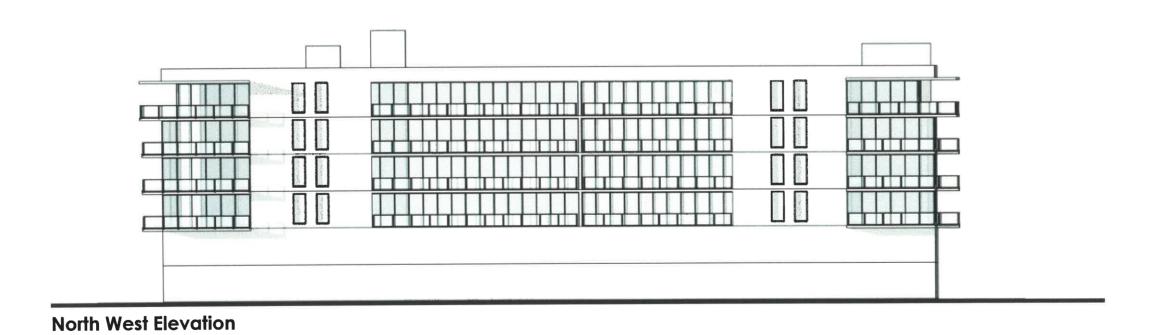
North East Elevation



CONDO A ELEVATIONS



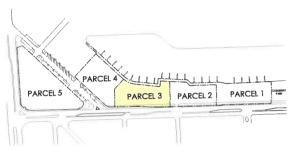
**South East Elevation** 



### **:ONDO B**

#### 4 CONDOMINIUM UNITS FLOORS 3 PARKING SPACES

nis building will have up to 44 dwelling units veraging approximately 1,600 square feet ach. The building's first floor lobby faces 150th venue to provide a visible front door. Parking is cated on the lower two floors with two spaces er unit plus a few extra for visitors. Above are four pors of units. The building is designed to take full dvantage of the water views in all directions. A par amenity area includes a swimming pool.



**EGEND** 





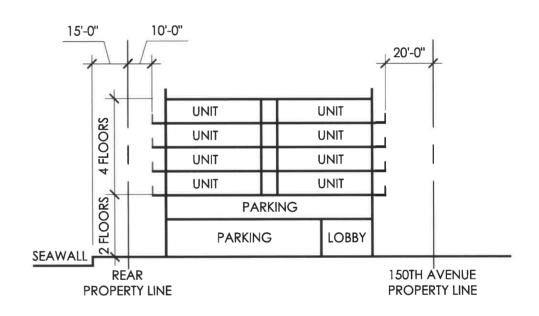
# 10'-0" SEAWALL PROPERTY LINE (TYP.) 20'-0" VEHICULAR ACCESS BUILDING SETBACK (TYP.) 15'-0" PEDESTRIAN ACCESS 150th AVENUE



MAXIMUM EXTENT OF BUILDING FOOTPRINT

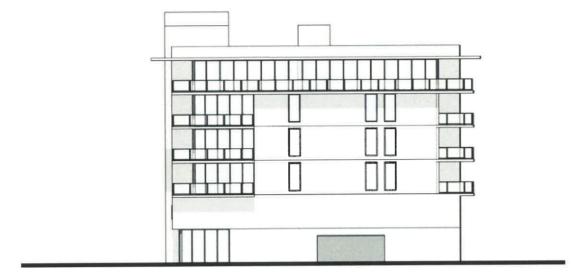
PARCEL 3

Site Area	48,791 s.f. (1.12 ac.)
Maximum Dwelling Units	44 units
Maximum Building Height	70 feet
Maximum Impervious Surface Ratio	70% of site area
Maximum Building Footprint	50% of site area
Minimum Required Parking	Residential: 2 spaces per unit

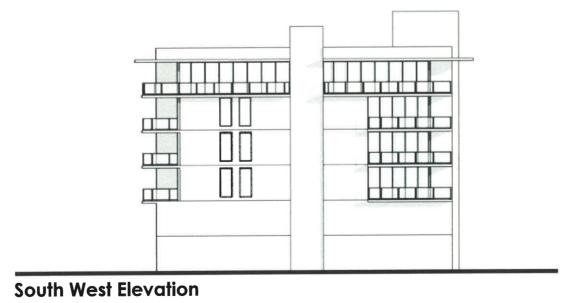


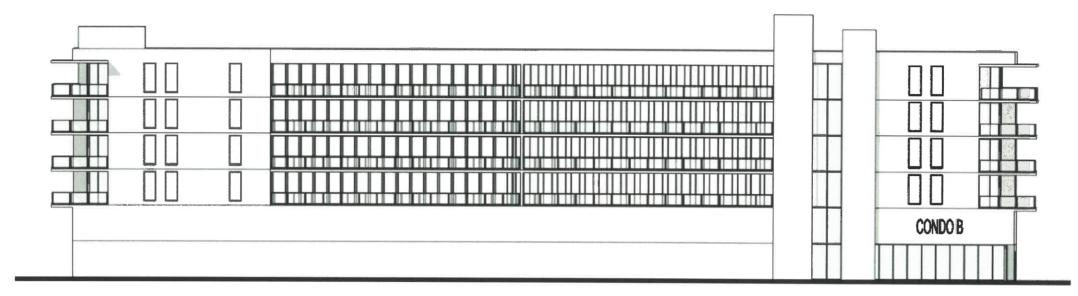
**Building Section** 

SCALE: NTS

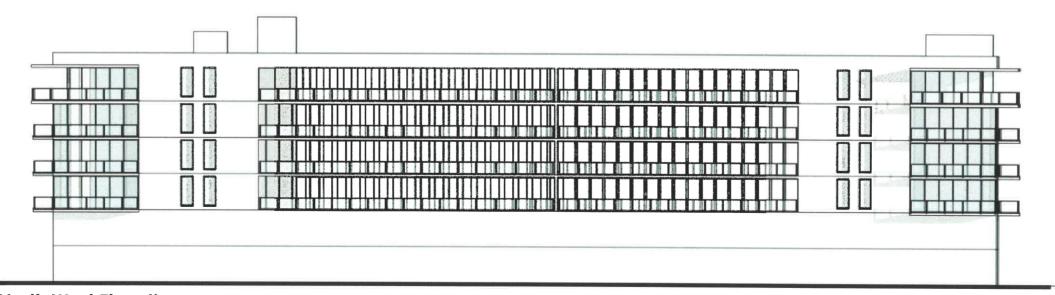


**North East Elevation** 





**South East Elevation** 

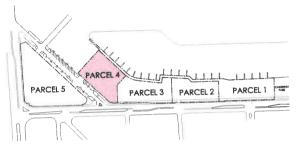


**North West Elevation** 

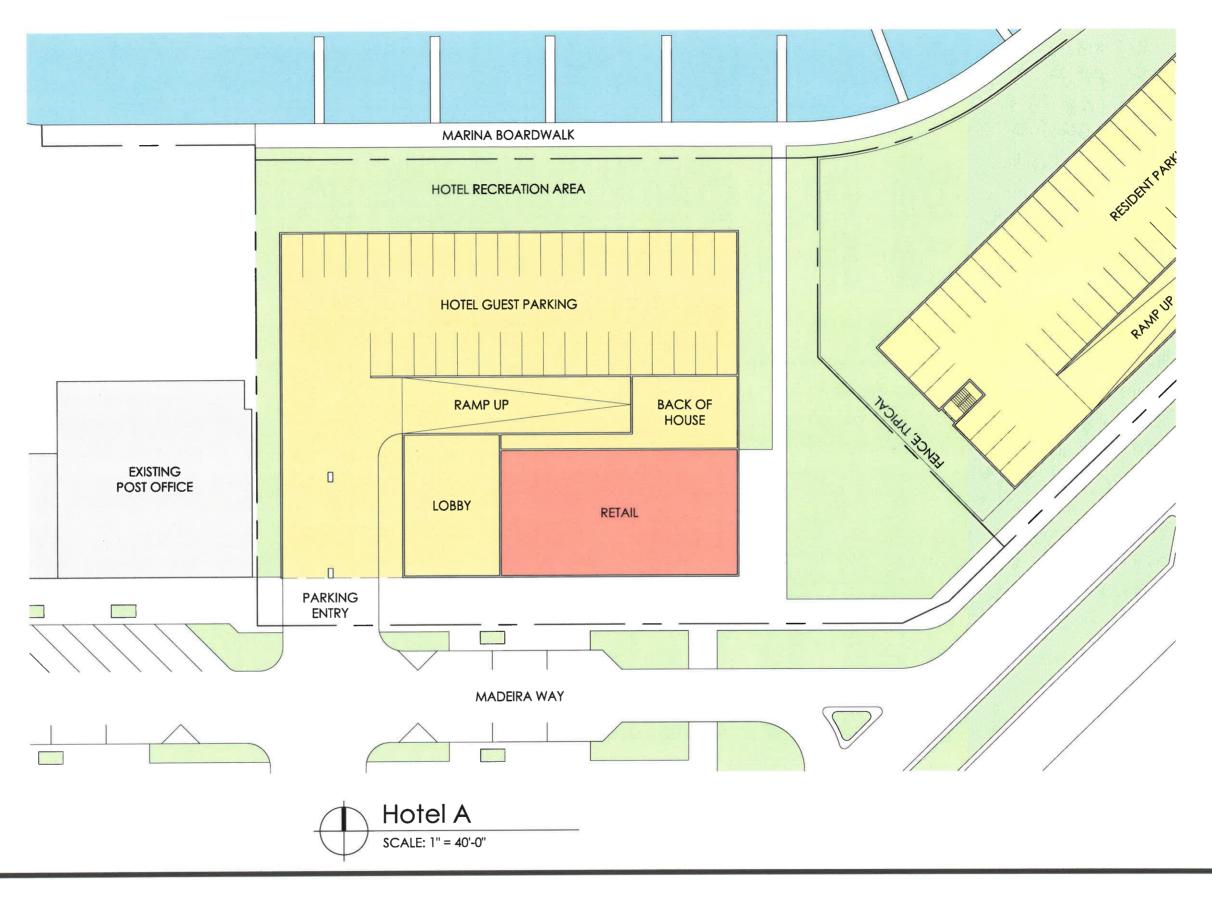
## **IOTEL A**

#### 48 HOTEL ROOMS ,000 S.F. COMMERCIAL FLOORS 75 PARKING SPACES

nis is programmed to be a suites hotel with up to 48 rooms. The first floor of the building contains ne lobby, up to 5,000 square feet of retail and estaurant space, parking and support and ervice spaces. Above are two additional floors of parking. The 4th floor will have hotel amenities pening onto a roof terrace as well as some hotel poms. The remainder of the hotel rooms will be no floors 5 through 8. The design of the building einforces the pedestrian nature of Madeira Vay by its close relationship to the sidewalk and pround level active uses.

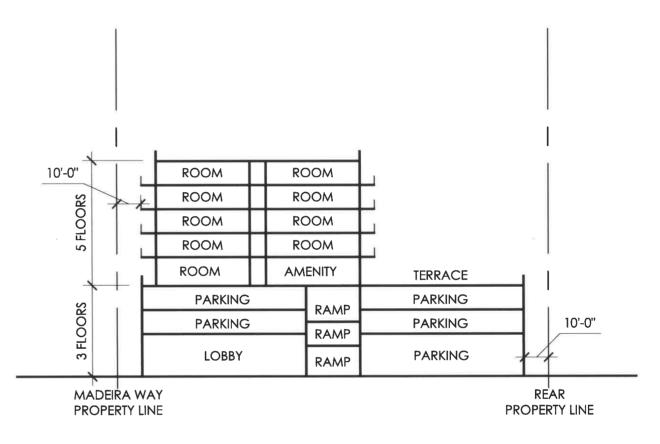


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#### PARCEL 4

Site Area	50,066 s.f. (1.149 ac.)
Maximum Hotel Rooms	148
Maximum Commercial Space	5,000 s.f.
Maximum Building Height	90 feet
Maximum Impervious Surface Ratio	85% of site area
Maximum Building Footprint	70% of site area
Minimum Required Parking	Hotel: 1 space per room
	Retail: 3 spaces per 2,000 s.f.
	Restaurant: 1 space per 4 seats





PARKING GARAGE ACCESS	0,0,
And Sets and District Contraction of the Contractio	

Building Setbacks and Access Diagram SCALE: 1" = 60'-0"

MAXIMUM EXTENT OF BUILDING FOOTPRINT



**North Elevation** 



**South Elevation** 



**East Elevation** 

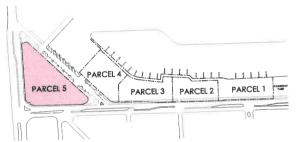


West Elevation

#### **IOTEL B**

# 68 HOTEL ROOMS 0 CONDOMINIUM UNITS 0,000 S.F. COMMERCIAL FLOORS 26 PARKING SPACES

nis is a true mixed-use block designed to be a ecognizable landmark at the heart of Madeira each. It combines 40,000 square feet of retail nd restaurant space on the ground floor with full-service hotel with up to 168 rooms above s well as 10 condominium units. All sides of ne first floor are lined with active uses fronting nto Madeira Way to the north and a new ublic plaza to the south. These two spaces are onnected by a mid-block pedestrian arcade nrough the building. The parking garage is ocated on the 2nd and 3rd floors. The 4th floor is ledicated to the hotel's amenity spaces (fitness enter, spa, meeting rooms, business center, tc.). These open onto a generous roof terrace vith swimming pool, spa and bar. Hotel rooms ire located on the 5th through 7th floors, with 10 condominiums on the 8th floor. All are oriented o maximize water views.

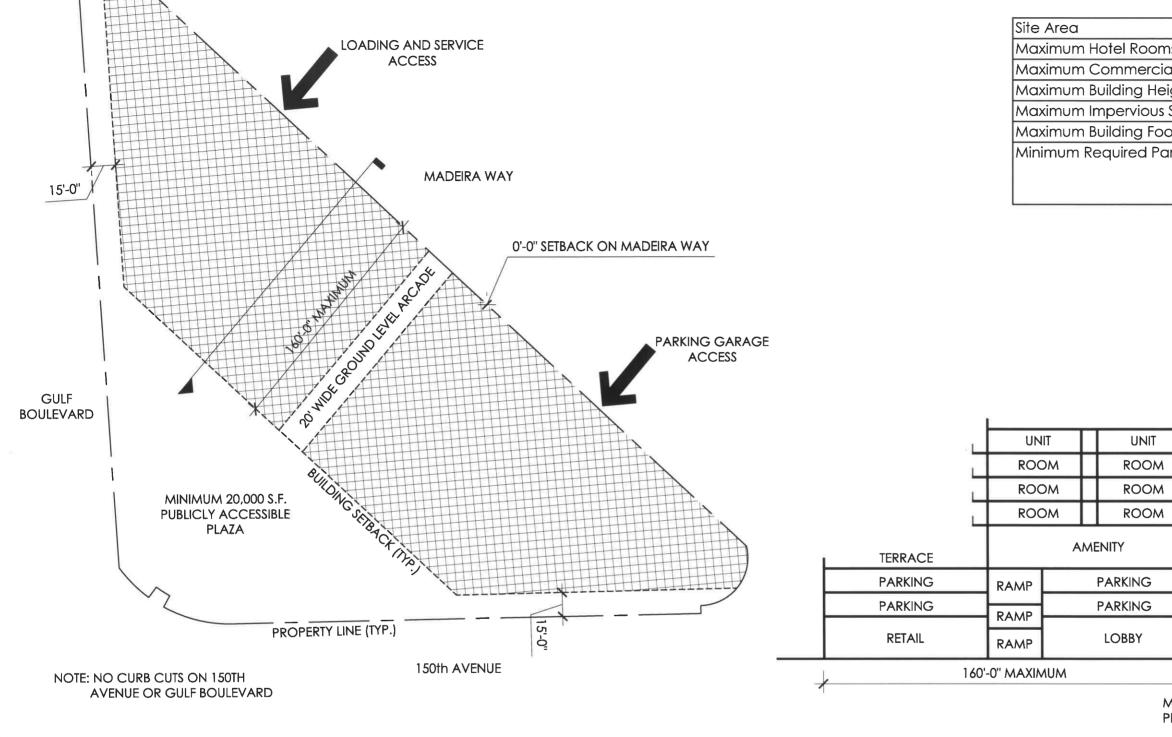


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#### PARCEL 5

Site Area	92,511 s.f. (2.124 ac.)
Maximum Hotel Rooms	168
Maximum Commercial Space	40,000 s.f.
Maximum Building Height	99 feet
Maximum Impervious Surface Ratio	85% of site area
Maximum Building Footprint	70% of site area
Minimum Required Parking	Hotel: 1 space per room
	Retail: 3 spaces per 2,000 s.f.
	Restaurant: 1 space per 4 seats



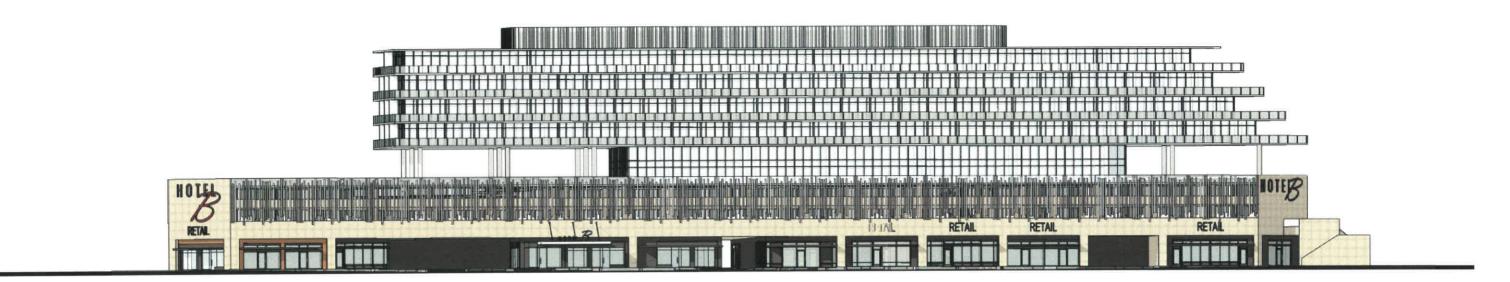
MADEIRA WAY PROPERTY LINE

Building Setbacks and Access Diagram SCALE: 1" = 60'-0"

**Building Section** SCALE: NTS

MAXIMUM EXTENT OF BUILDING FOOTPRINT

Madeira Beach Town Center MADEIRA BEACH, FLORIDA 37



#### **North Elevation**

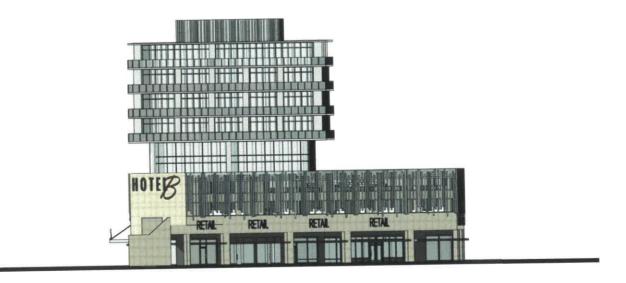


#### **South Elevation**

HOTEL B ELEVATIONS



**East Elevation** 



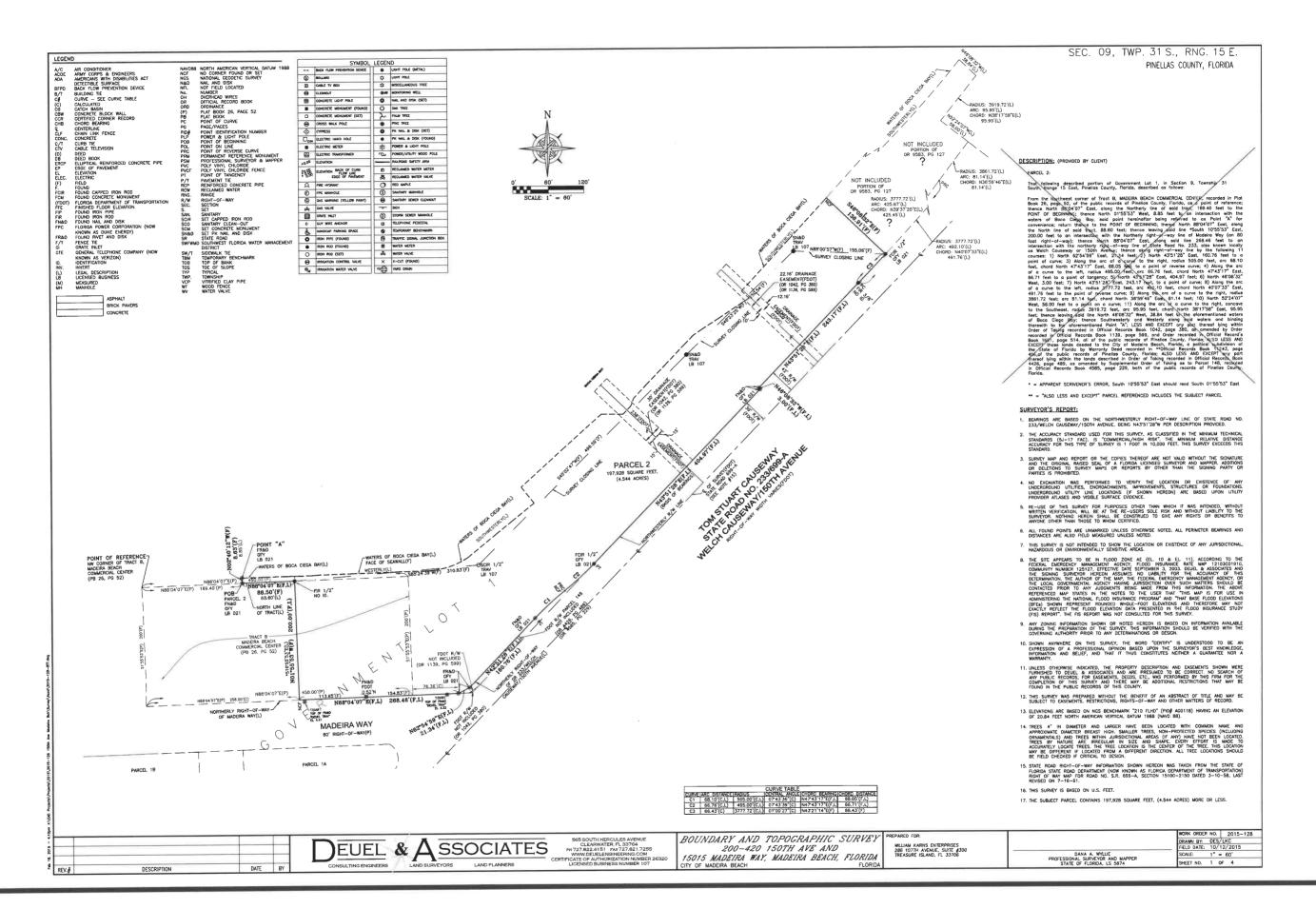
**West Elevation** 

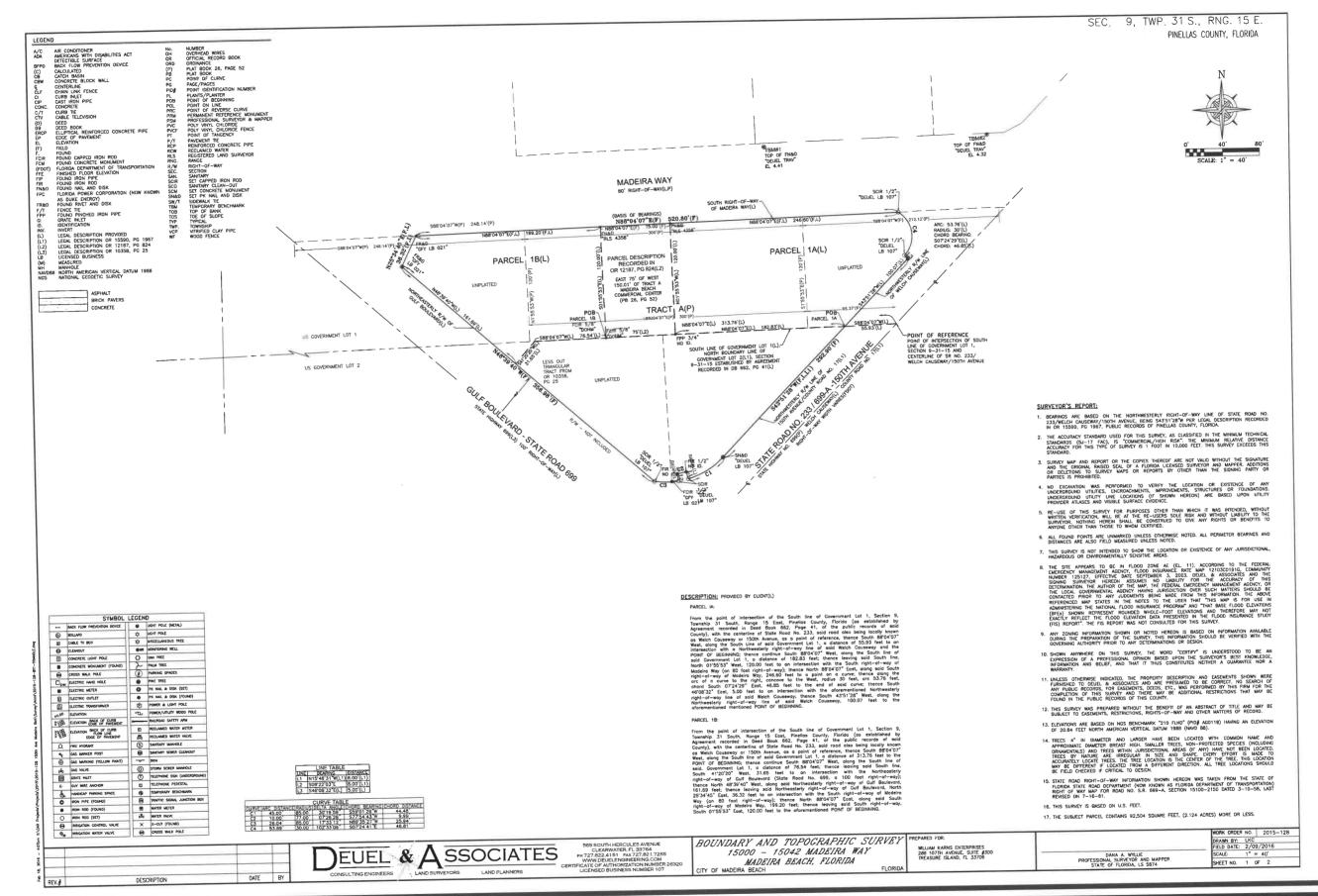
# APPENDIX SECTION

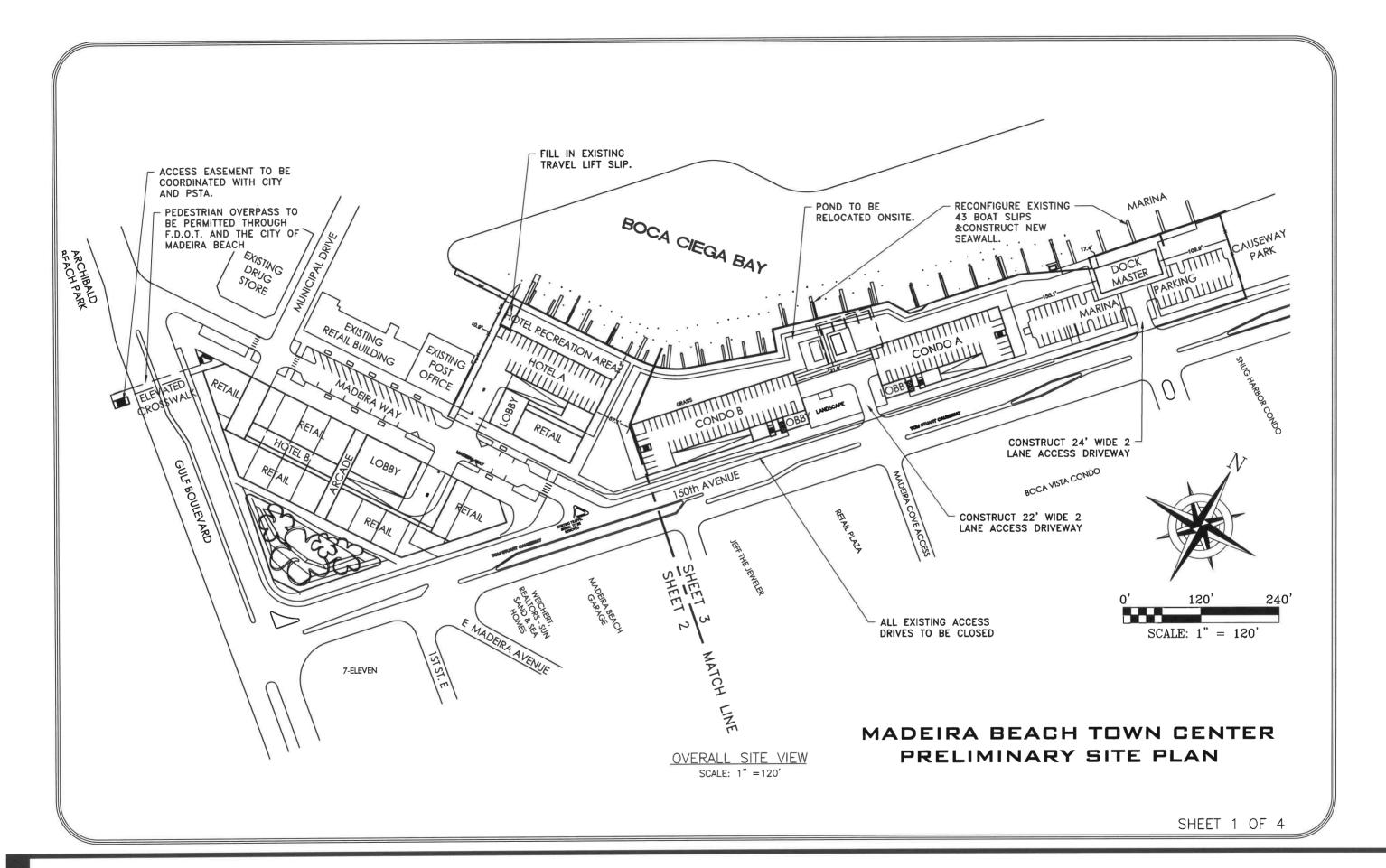
Surveys Civil Site Plan and Data **Traffic Analysis** 

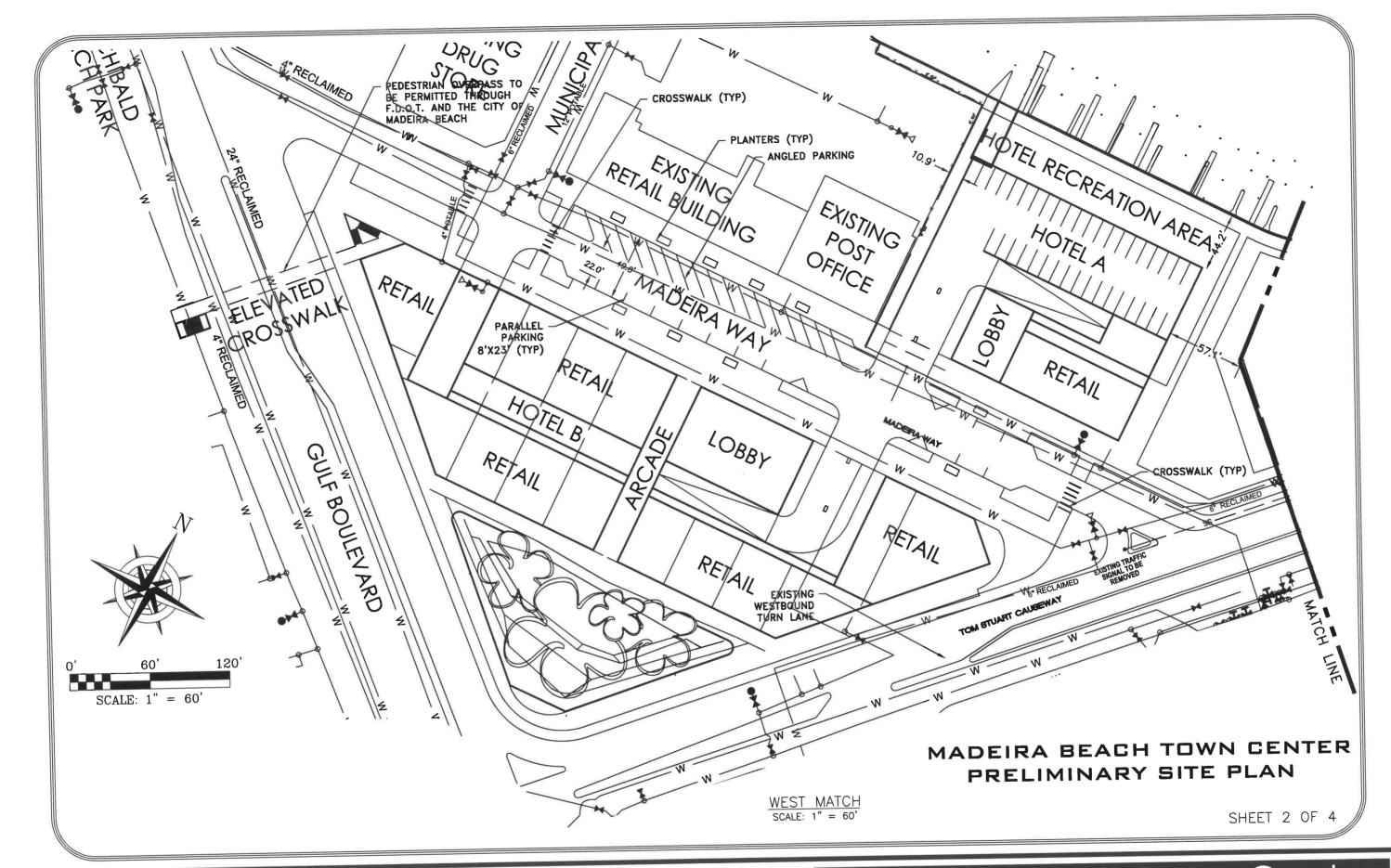
Madeira Beach Town Center

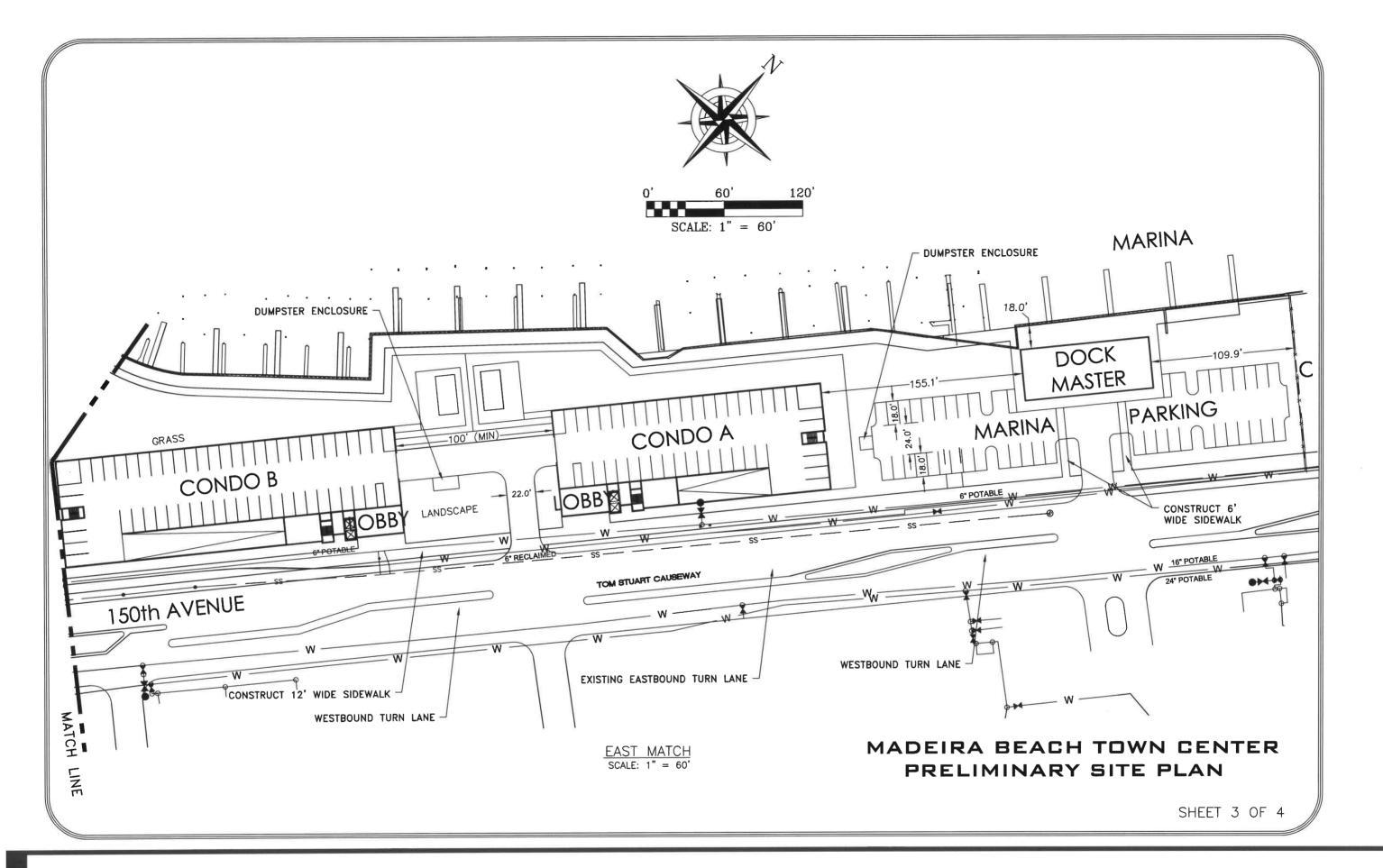
MADEIRA BEACH, FLORIDA 41











CIVIL SITE PLAN AND DATA

### SITE DATA

#### HOTEL A

PARKING REQUIRED: 1 SP/ROOM = 148 ROOMS: 148 3 SP/2.000 SF = 3RETAIL SPACE: 2,000 SF 1 SP/4 SEATS = RESTAURANT SPACE: 3,000 SF TOTAL REQUIRED =

HOTEL B

PARKING PROVIDED: 210 SPACES

PARKING PROVIDED: 400 SPACES

PARKING REQUIRED: 1 SP/ROOM = 168**ROOMS: 168** 3 SP/2,000 SF = 42RETAIL SPACE: 28,000 SF 1 SP/4 SEATS = RESTAURANT SPACE: 12,000 SF

CONDO A

PARKING REQUIRED: 2 SP/UNIT = 72UNITS: 36 PARKING PROVIDED: 77 SPACES TOTAL REQUIRED = 72

CONDO B

PARKING REQUIRED: 2 SP/UNIT = 88 UNITS: 44 PARKING PROVIDED: 93 SPACES TOTAL REQUIRED = 88

MARINA

PARKING REQUIRED: 3 SP/2,000 SF = 6

TOTAL REQUIRED =

RETAIL SPACE: 7,000 SF 1 SP/4 SEATS = RESTAURANT SPACE: 1,000 SF

PARKING PROVIDED: 42 SPACES

DOCK SLIPS: 43

TOTAL REQUIRED = 40

OVERALL PROJECT SUMMARY & SITE DATA: EXISTING

= 62,904 SF(21.7%) 130,729 SF(45.0%) TOTAL BUILDING AREA 81,171 SF(27.9%) = 173.003 SF(59.5%)TOTAL ASPHALT/CONC. = 235,907 SF(81.2%) 211,900 SF(72.9%) TOTAL IMPERVIOUS AREA = 54,525 SF(18.8%) 78,532 SF(27.4%) TOTAL GREEN AREA 290,432 SF (6.67 AC) TOTAL PROJECT AREA

OVERALL SITE IMPERVIOUS SURFACE RATIO: 0.73

### MADEIRA BEACH TOWN CENTER PRELIMINARY SITE PLAN

PROPOSED

SHEET 4 OF 4

FOOT PERMIT TRAFFIC ANALYSIS
FOR
MADEIRA BEACH TOWN CENTER
150<sup>TH</sup> AVENUE / MADEIRA WAY
MADEIRA BEACH, FL

PREPARED FOR:
MADEIRA BEACH TOWN CENTER, LLC

PREPARED BY:
GULF COAST CONSULTING, INC.
REVISED NOVEMBER 2016
PROJECT # 16-006

#### TABLE OF CONTENTS

I. Introduction

II. EXISTING CONDITIONS

III. FUTURE CONDITIONS WITH DEVELOPMENT

IV. CONCLUSIONS AND RECOMMENDATIONS

Robert Pergolizzi, AICP/PTP

Octavio Cabrera, P.E. FL. Reg. #14663

Octavio Cabrera

NOV 0 1 2016

FL P.E. No. 14663

#### INTRODUCTION

The applicant proposes to improve its property located on the north side of Tom Stuart Causeway (SR 666 / 150th Avenue) at Madeira Way and the west side of Madeira Way in the City of Madeira Beach (See Figure 1) The property is currently developed with various retail uses, a marina, and surface parking lots with multiple driveways to Tom Stuart Causeway as well as Madeira Way. Since the June 14, 2016 Madeira Beach City Commission approval, the applicant has revised the plan to reduce the height and intensity of the development to address neighbor concerns. The applicant now intends to redevelop the property with a 148 room hotel (Hotel "A"), a 168 room hotel (Hotel "B"), each containing ancillary ground floor retail space, and 90 condominium units in multiple buildings, with associated parking. Of the 90 condominium units, 80 will be accessed directly from 150th Avenue (Condo "A" and Condo "B") and 10 units will be included in Hotel "B" which is accessed from Madeira Way. Relocation of the existing marina access is also proposed.

The potential improvements include consolidation of access points, removal of the traffic signal at Madeira Way, access connection with a turn lane at the median opening serving the site and Madeira Cove Condominiums, and modifying turn lanes within 150th Avenue. This traffic analysis was prepared to evaluate the traffic impacts at the driveways and to aid in driveway/ turn lane design. A pre-application meeting was held with FDOT in February 2016.

#### **EXISTING CONDITIONS**

The Tom Stuart Causeway (SR 666) is a four-lane divided arterial roadway with a posted speed of 40 MPH (45 MPH design speed) and is controlled by traffic signals at Madeira Way and Gulf Boulevard to the west. SR 666 is an Access Class 7 roadway per FDOT Rule 14-97, with a minimum driveway spacing requirement of 125 feet, and a full median opening spacing of 660 feet. Existing conditions were established by obtaining PM peak period (4-6 PM) intersection turning movement counts at multiple locations shown below on February 2, 2016. Weather conditions were excellent with a high temperature of 79 degrees and sunshine.

Gulf Boulevard / Madeira Way (signal) Tom Stuart Causeway (SR 666/150th Ave.) / Gulf Boulevard (signal) Tom Stuart Causeway (SR 666/150<sup>th</sup> Ave)/ Madeira Way (signal) Tom Stuart Causeway (SR 666/150<sup>th</sup> Ave.)/ Directional Median opening serving retail plaza Tom Stuart Causeway (SR 666/150th Ave.) / Full Median opening serving Madeira Cove

Tom Stuart Causeway (SR 666/150th Ave.) / Full Median opening serving Boca Vista

These counts were seasonally adjusted to peak season equivalents using FDOT seasonal adjustment factors. Intersection analysis was performed using the SYNCHRO and HCS software. The existing (2016) peak hour traffic volumes are shown in Figure 2, the intersection operations are shown below in Table 1 and the SYNCHRO and HCS printouts are included in Appendix A.

TRAFFIC SIGNAL **PROJECT** LOCATION SNUG HARBOR CONDOS **BOCA VISTA CONDOS** MADEIRA COVE CONDOS PROJECT NO: PROJECT LOCATION - MADEIRA BEACH TOWN CENTER SITE 16-006 FIGURE: Gulf Coast Consulting, Inc. 01/2016 Land Development Consulting DRAWN BY: MKC

2

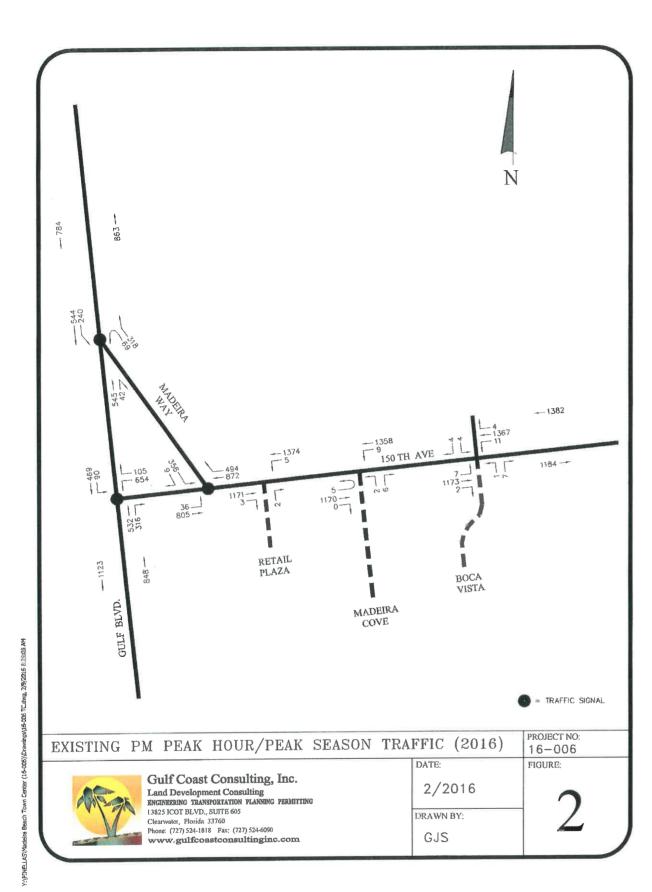


Table 1 - Existing Intersection Conditions (2016)

Intersection Location	Type	PM Peak Hour LOS	Ave. Delay (sec/veh)	
Gulf Blvd / Madeira Way	Signal	A	9.1	
150th Ave / Gulf Blvd.	Signal	В	16.5	
150 Ave / Madeira Way	Signal	В	17.5	
150th Ave / Directional Opening Retail	Unsignalized	B/B	12.6 / 13.5	
150th Avenue / Madeira Cove	Unsignalized	B/C	12.2 / 17.1	
150th Avenue / Boca Vista	Unsignalized	B/C	12.7 / 17.4	

B/C = LOS of SR 666 left turn / LOS of side street approach

Tom Stuart Causeway (SR 666/150<sup>th</sup> Avenue) is a 4-lane divided arterial roadway with a poste speed of 40 MPH and is controlled by traffic signals at Madeira Way and Gulf Boulevard. These traffic signals are closely spaced and do not meet FDOT signal spacing criteria. Based on the adjusted traffic counts, roadway segment volumes were calculated and analyzed using FDOT Generalized Capacity Tables. The adjacent segment of SR 666 carries 2,566 vehicles during the PM peak hour which represents LOS C on a 4-lane divided roadway.

Gulf Boulevard (SR 699) is a 4-lane divided arterial roadway with a posted speed of 35 MPH and is controlled by traffic signals at Madeira Way and 150<sup>th</sup> Avenue. Gulf Boulevard is considered a Class II arterial due to the lower posted speed. Gulf Boulevard north of Madeira Way carries 1,647 vehicles during the PM peak hour which represents LOS D conditions. Gulf Boulevard south of 150<sup>th</sup> Avenue carries 1,971 vehicles during the PM peak hour which represents LOS D conditions.

#### III. FUTURE CONDITIONS WITH DEVELOPMENT

Traffic impacts from the recently proposed Holiday Isle (Holton) project on the south side of Tom Stuart Causeway east of this property was included as background traffic to evaluate the cumulative effects of both redevelopments. The Holiday Isle project has also been downsized and is now expected to generate 3,092 daily trips with 248 trips occurring during the PM peak hour. Of these 173 are expected to travel to/from the mainland over the drawbridge, and 75 are expected to impact the study area for Madeira Beach Town Center. This traffic volume was added to the existing counts to consider the impact of Holiday Isle.

Trip generation estimates of the additional traffic caused by the proposed Madeira Beach Town Center development were made using ITE Trip Generation, 9<sup>th</sup> Edition rates.

Table 2 – Trip Generation Estimates

Land Use	Amount	ITE LUC	Daily Trips	PM Peak (in/out)
High-Rise Condo "A"	36 units	232	150	14 (8/6)
High-Rise Condo "B"	44 units	232	184	16 (10/6)
Hotel "A"	148 rooms	310	1209	89 (45/44)
Hotel "B" + 10 condos	168 rooms/10	310/232	1415	105 (54/51)
Total			2,958	224 (117/107)

The additional traffic caused by the development is expected to be 2,958 daily trips of which 224 would occur during the PM peak hour (117 entering/107 exiting). This represents a 24% reduction in trip generation as compare to the previously approved development levels. This would classify as a Category "D" permit application with FDOT. Based on pre-application meetings with FDOT, substantial access management improvements are proposed. These include removing the traffic signal at Madeira Way, and constructing a turn lane at the Madeira Cove median opening which will also serve the project condominiums. In addition, converting Madeira Way into right-in/right out access at the 150<sup>th</sup> Avenue intersection, lengthening left turn lanes, and providing an eastbound left turn lane at the project access aligning with Madeira Cove is proposed. These changes will create U-turns and a redistribution of existing traffic. This will also provide median opening separation to better conform to FDOT access management criteria.

Project traffic was distributed to the surrounding roadway system based on the following percentages.

10% north on Gulf Boulevard (SR 699) 20% south on Gulf Boulevard (SR 699) 70% east on SR 666 (Tom Stuart Causeway)

Project generated traffic is shown in Figure 3.

The intersections were reanalyzed considering the project traffic, median opening modifications, the removal of a traffic signal and revised access points. The expected future traffic volumes are shown in Figure 4, intersection conditions are shown in Table 3, and the SYNCHRO and HCS printouts are included in Appendix B.

Table 3 - Future Intersection Conditions

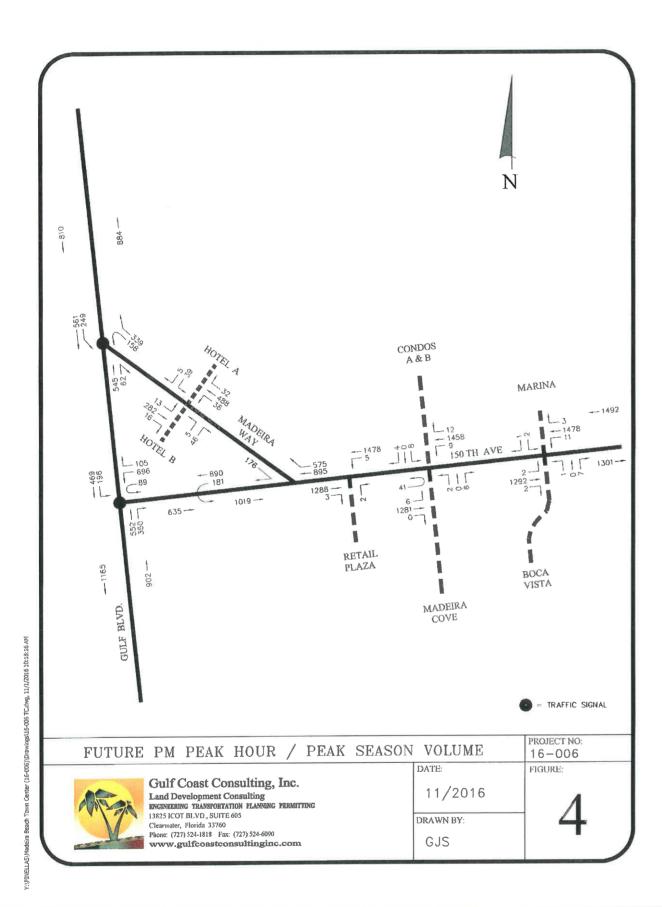
Intersection Location	Туре	PM Peak Hour LOS	Ave. Delay (sec/veh)	
Gulf Blvd / Madeira Way	Signal	В	10.3	
150 <sup>th</sup> Ave / Gulf Blvd.	Signal	С	20.7	
150 Ave / Gall Bivd. 150 Ave / Madeira Way (RIRO)	Unsignalized	C* (SBRT)	18.2	
150 <sup>th</sup> Ave / Directional Opening Retail	Unsignalized	B/B*	13.6/15.6	
150 th Avenue / Madeira Cove	Unsignalized	C/E*	16.2/40.9	
150 Avenue / Boca Vista	Unsignalized	C/E*	15.4/43.0	
Madeira Way / Hotel Driveways	Unsignalized	A/D*	8.5/25.5	

B/C = LOS of SR 666 left turn / LOS of side street approach

The adjacent segment of SR 666 would continue to operate at LOS C with volume increasing to 2,793 vehicles during the PM peak hour. This represents an acceptable level of service.

The segment of Gulf Boulevard north of Madeira Way would carry 1,694 vehicles during the PM peak hour which represents LOS D conditions. Gulf Boulevard south of 150th Avenue would carry 2,067 vehicles during the PM peak hour which represents LOS D conditions.

70% 150 TH AVE +74-RETAIL BOCA MADEIRA = TRAFFIC SIGNAL PROJECT NO PROJECT ADDED TRAFFIC 16-006 FIGURE: DATE: Gulf Coast Consulting, Inc. 10/2016 Land Development Consulting ENGINEERING TRANSPORTATION PLANNIN 13825 ICOT BLVD . SUITE 605 DRAWN BY Phone: (727) 524-1818 Fax: (727) 524-6090 GJS



#### IV. CONCLUSIONS AND RECOMMENDATIONS

The proposed development of this property to contain condominiums, and hotels with ancillary ground floor retail space is expected to generate 2,958 daily trips and an additional 224 PM peak hour trips. With the impacts of the proposed development all affected intersections and roadway segments would continue to operate at acceptable levels of service.

The following access management improvements are recommended:

Tom Stuart Causeway (150<sup>th</sup> Avenue) / Madeira Way – Remove traffic signal, install raised median in 150<sup>th</sup> Avenue, permit right turn only into/from Madeira Way.

Tom Stuart Causeway (150<sup>th</sup> Avenue)/ Median Opening at 1<sup>st</sup> Street E. – Extend the westbound left turn lane past Madeira Way to contain 150 feet of queue storage plus 185 feet of deceleration distance per FDOT Index # 301 for a 45 MPH design speed.

Tom Stuart Causeway (150<sup>th</sup> Avenue) / Madeira Cove Median Opening — Construct an eastbound left turn lane into Madeira Beach Town Center access. Construct driveway with 2 exiting lanes to separate left and right turns. The eastbound left turn lane should include 50 feet of queue storage plus 185 feet of deceleration distance per FDOT Index # 301. Due to distance constraints a design exception for deceleration distance may be needed from FDOT. The westbound left turn lane should contain 50 feet queue storage plus 185 feet deceleration distance per FDOT Index #301. This requires lengthening the existing left turn lane and removal of landscaping area.

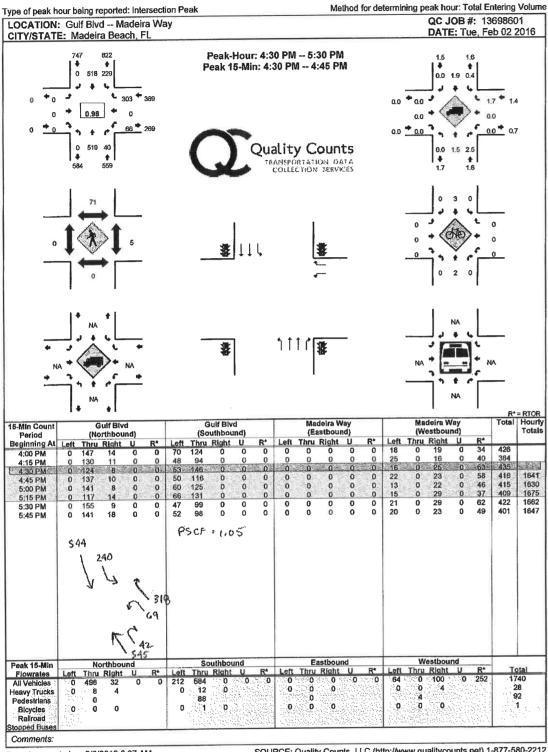
Tom Stuart Causeway / Boca Vista / Marina Median Opening — Extend the westbound left turn lane to include 50 feet of queue storage plus 185 feet of deceleration distance per FDOT Index #301. This requires lengthening the existing left turn lane and removal of landscaping area. Provide eastbound left turn lane into the marina access with 50-feet queue storage and 185 feet deceleration distance. Due to distance constraints a design exception from FDOT may be needed.

APPENDIX A

2014 Peak Season Factor Category Report - Report Type: ALL Category: 1500 PINELLAS COUNTWIDE

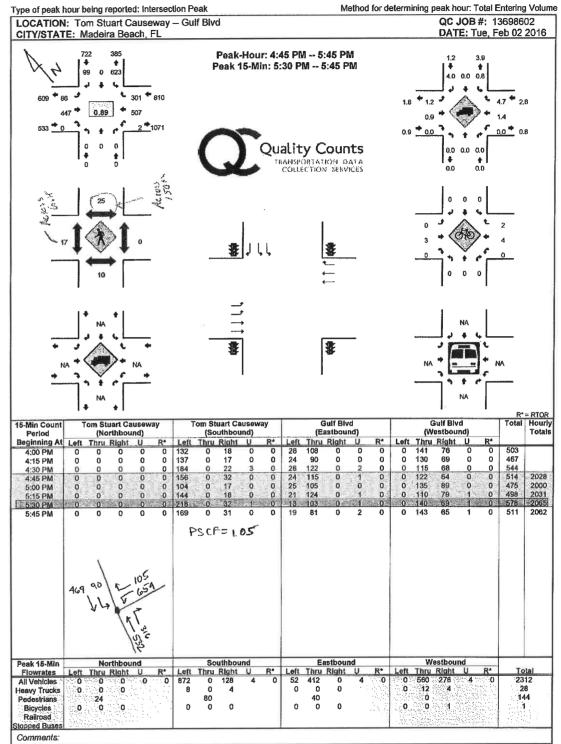
Week	Dates	SF	MOCE: 0.95 [PSCF]
1	01/01/2014 - 01/04/2014	1.03	1.08
2	01/05/2014 - 01/11/2014	1.05	1.11
3	01/12/2014 - 01/18/2014	1.07	1.13
4	01/19/2014 - 01/25/2014	1.05	1.11
5	01/26/2014 - 02/01/2014	1.03	1.08
6	02/02/2014 - 02/08/2014	1.00	[1.05] TMC Courds
7	02/09/2014 - 02/15/2014	0.98	1.03
* 8	02/16/2014 - 02/22/2014	0.96	1.01
* 9	02/23/2014 - 03/01/2014	0.95	1.00
*10	03/02/2014 - 03/08/2014	0.95	1.00
*11	03/09/2014 - 03/15/2014	0.94	0.99
*12	03/16/2014 - 03/22/2014	0.93	0.98
*13	03/23/2014 - 03/29/2014	0.93	0.98
*14	03/30/2014 - 04/05/2014	0.94	0.99
*15	04/06/2014 - 04/12/2014	0.94	0.99
*16	04/13/2014 - 04/19/2014	0.94	0.99
*17	04/20/2014 - 04/26/2014	0.95	1.00
*18	04/27/2014 - 05/03/2014	0.96	1.01
*19	05/04/2014 - 05/10/2014	0.97	1.02
*20	05/11/2014 - 05/17/2014	0.98	1.03
21	05/18/2014 - 05/24/2014	0.99	1.04
22	05/25/2014 - 05/31/2014	0.99	1.04
23	06/01/2014 - 06/07/2014	0.99	1.04
24	06/08/2014 ~ 06/14/2014	0.99	1.04
25	06/15/2014 - 06/21/2014	0.99	1.04
26	06/22/2014 - 06/28/2014	1.00	1.05
27	06/29/2014 - 07/05/2014	1.00	.1.05
28	07/06/2014 ~ 07/12/2014	1.00	1.05
29	07/13/2014 - 07/19/2014	1.01	1.06
30	07/20/2014 - 07/26/2014	1.01	1.06
31	07/27/2014 - 08/02/2014	1.01	1.06
32	08/03/2014 - 08/09/2014	1.02	1.07
33	08/10/2014 - 08/16/2014	1.02	1.07
34	08/17/2014 - 08/23/2014	1.02	1.07
35	08/24/2014 - 08/30/2014	1.04	1.09
36	08/31/2014 - 09/06/2014	1.05	1.11
37	09/07/2014 - 09/13/2014	1.06	1.12
38	09/14/2014 - 09/20/2014	1.07	1.13
39	09/21/2014 - 09/27/2014	1.06	1.12
40	09/28/2014 - 10/04/2014	1.06	1.12
41	10/05/2014 - 10/11/2014	1.05	1.11
42	10/12/2014 - 10/18/2014	1.05	1.11
43	10/19/2014 ~ 10/25/2014	1.05	1.11
44	10/26/2014 - 11/01/2014	1.05	1.11
45	11/02/2014 - 11/08/2014	1.06	1.12
46	11/09/2014 - 11/15/2014	1.06	1.12
47	11/16/2014 - 11/22/2014	1.06	1.12
48	11/23/2014 - 11/29/2014	1.06	1.12
49	11/30/2014 - 12/06/2014	1.05	1.11
50	12/07/2014 - 12/13/2014	1.04	1.09
51	12/14/2014 - 12/20/2014	1.03	1.08
52	12/21/2014 - 12/27/2014	1.05	1.11
53	12/28/2014 - 12/31/2014	1.07	1.13

\* Peak Season



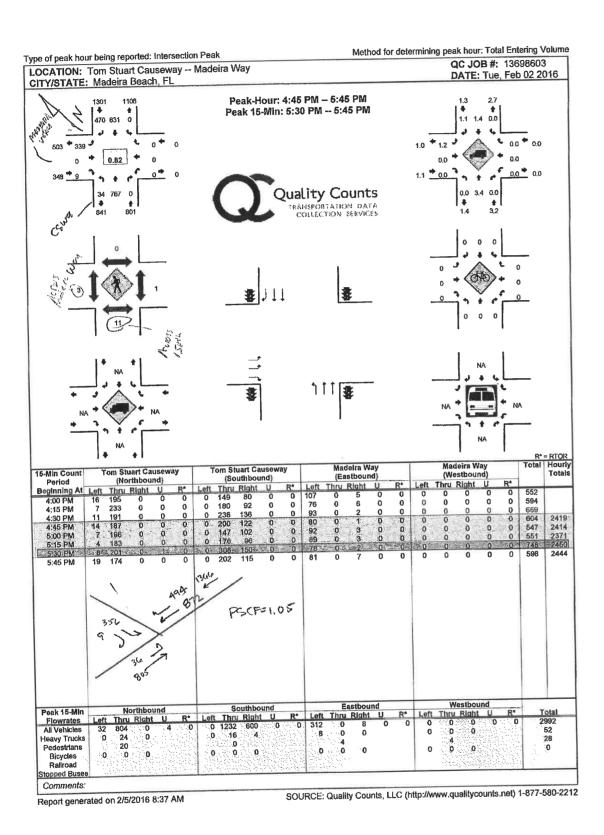
Report generated on 2/5/2016 8:37 AM

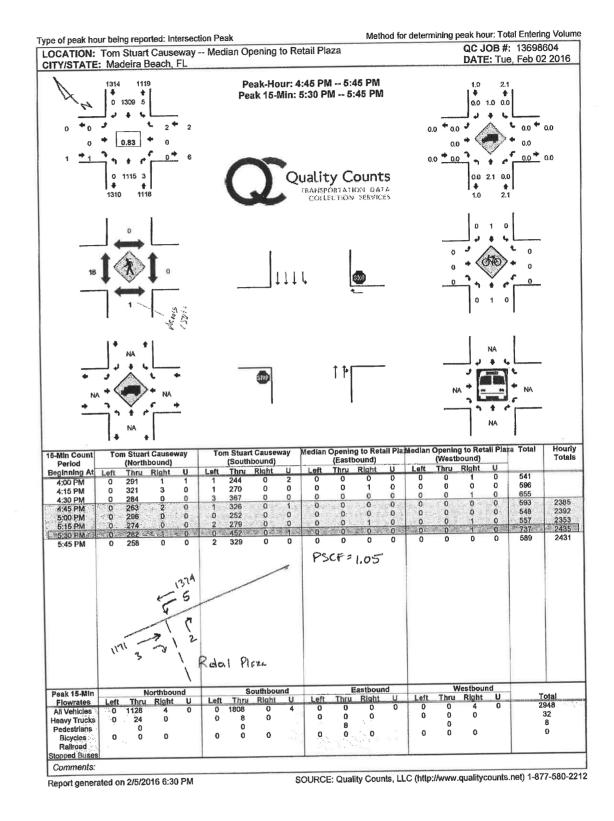
SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212

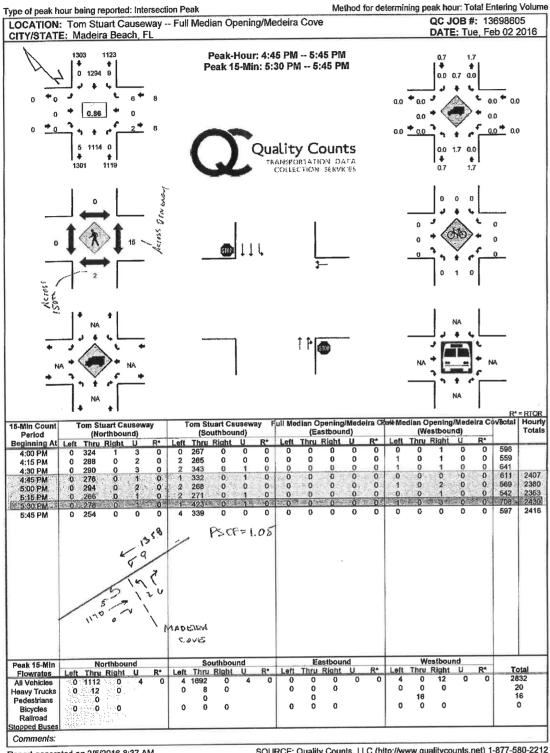


Report generated on 2/5/2016 8:37 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212

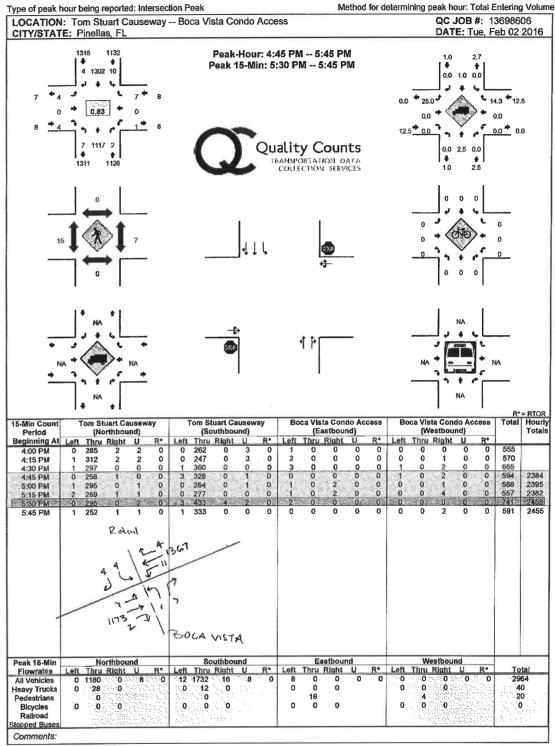






Report generated on 2/5/2016 8:37 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212



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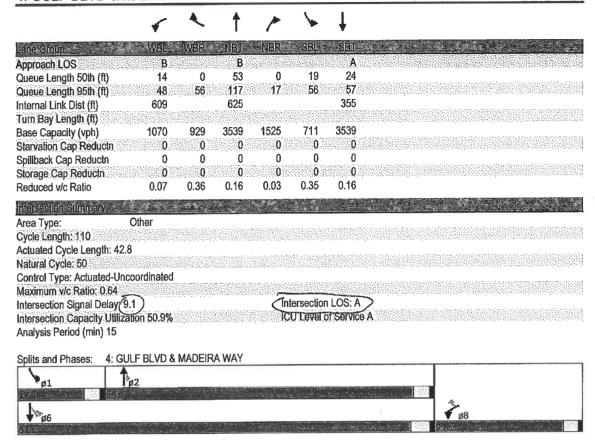
SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212

	1	*	<b>†</b>	-	-	ı İ
		teams 45	CAL	LF BLW	· au	F BLVO.
Lane Group	Wille.	WBR	MBIL	NER	SIBLE	SIBIT
Lane Configurations	7	7	<b>^</b>	7	ħ	<b>^</b>
Volume (vph)	69	318	545	42	240	544
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	0.95	1,00	1.00	0.95
Ped Bike Factor	0.99	0.83		0.96	0.92	Sec. 10 (10 (10 (10 (10 (10 (10 (10 (10 (10
Fit		0.850		0.850		
Fit Protected	0.950				0.950	
Satd. Flow (prot)	1770	1583	3539	1583	1770	3539
Flt Permitted	0.950				0.294	
Satd. Flow (perm)	1751	1321	3539	1525	503	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		331		44		
Link Speed (mph)	30		35			35
Link Distance (ft)	689		705			435
Travel Time (s)	15.7		13.7			8.5
	10.7	76		5	76	
Confl. Peds. (#/hr)	ef and district a feature and a second	0.96	0.96	0.96	0.96	0.96
Peak Hour Factor	0.96	331	568	0.90 44	250	567
Adj. Flow (vph)	72	<b>ુ</b> ડડા	200	44	ZUU	UU/
Shared Lane Traffic (%)		686 <b>882</b> 8	FAA	4866688 <b>5</b> 7	OFO	E07
Lane Group Flow (vph)	_72	_331	568	44	250	567
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		<u> </u>	6
Permitted Phases		8		2	6	والمراجع والمحاجبة والمحاجبة والمحاجبة
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	20.0	20.0	20.0	20.0	8.0	20.5
Total Split (s)	29.0	29.0	64.0	64.0	17.0	81.0
Total Split (%)	26.4%	26.4%	58.2%	58.2%	15.5%	73.6%
Maximum Green (s)	25.0	25.0	60,0	60.0	13.0	76.5
	3.0	3.0	3.0	3.0	3.0	3.5
Yellow Time (s)	3.0 1.0	1.0	1,0	1.0	1.0	1.0
All-Red Time (s)	Take the state of	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	0.0		4.0	4,0	4.0	4.5
Total Lost Time (s)	4.0	4.0				
Lead/Lag	alegadolas ales	Nagolaenen	Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	• •
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min
Walk Time (s)	5.0	5.0	5.0	5.0	outedate consist	5.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0		11.0
Pedestrian Calls (#/hr)	0	0	0	0		0
Act Effct Green (s)	7.9	7.9	13.7	13.7	26.6	26,1
Actuated g/C Ratio	0.18	0.18	0.32	0.32	0.62	0.61
v/c Ratio	0,22	0,64	0.50	0.09	0.44	0.26
Control Delay	18.2	9.4	14.0	5.1	6.3	4.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
	18.2	9.4	14.0	5.1	6.3	4.3
Total Delay	10.2 B	9.4 A	14.0 B	A.I	A A	Ä
LOS		A.			salada (Salada)	4.9
Approach Delay	11.0		13.3			4.0

GULF BLVD / MADEIRA WAY EXISTING 10/17/2016 EXISTING CONDITIONS 2016 PM PEAK Synchro 8 Report Page 1

Lanes, Volumes, Timings 4: GULF BLVD & MADEIRA WAY

10/17/2016



GULF BLVD / MADEIRA WAY EXISTING 10/17/2016 EXISTING CONDITIONS 2016 PM PEAK

Synchro 8 Report Page 2 Prot

8.0

NA

1 6

4.0 4.0

16.0 73.0

20.0

NA Perm

4.0

51.8% 51.8% 14.5% 66.4%

53.0 53.0 12.0 69.0

20.0

**5**7.0 **5**7.0

2

4.0

20.0

8 2 2

Yellow Time (s)	4.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1,0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	4.0	4.0	4.0	4.0	
Lead/Lag		Lag	Lag	Lead	tes topotostes and	2-17-4
Lead-Lag Optimize?		Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	e ves
Recall Mode	None	Min	Min	None	Min	
Walk Time (s)	5.0	5.0	5.0		5.0	
Flash Dont Walk (s)	11.0	11.0	11.0		11.0	
Pedestrian Calls (#/hr)	0	0	0	a construction and a fe	0	
Act Effct Green (s)	22.9	20.2	20.2	7.9	29.0	
Actuated g/C Ratio	0.37	0.33	0.33	0.13	0.47	
v/c Ratio	0.68	0.52	0.49	0.23	0.32	
Control Delay	20.5	20.2	5.1	30.4	10.8	

Prot

21.0

37.0

33.6%

32.0

GULF BLVD / SR 666 EXISTING 10/17/2016 EXISTING CONDITIONS 2016 PM PEAK

Synchro 8 Report Page 1

	1	< t	1	-	1	
Lane Group	WHE V	VBR INB'		- SBL	Signal :	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	20.5	20.2	5.1	30.4	10.8	
LOS	C	C	Α	С	В	
Approach Delay	20.5	14.5			13.9	
Approach LOS	С	В			В	
Queue Length 50th (ft)	138	97	0	18	57	esterno dura electrica se se o cultura se montro de cesa se montro destendo motiva e metalolica no cultura del
Queue Length 95th (ft)	237	174	53	47	110	
Internal Link Dist (ft)	696	426	era era angles er ares e	rangan sa sabar	531	
Turn Bay Length (ft)	400		400	300		
Base Capacity (vph)	1886	2936	1289	714	3377	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	Autorit tereteli, liste eleksepitalis eritereret reskapelitat elaste tessa terra et filotoris te
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.45	0.20	0.28	0.14	0.16	
nterseolon Summary 🕒 🛎	. •	3.87				
aca typo	Other		a manaza a sa	manara.aana	saturate entendende transcention and account	
Cycle Length: 110						
Actuated Cycle Length: 61.6	) 	en e	eteration and engage	es et e to eta e es el etc.	our was to be a committee on	
Natural Cycle; 50						
Control Type: Actuated-Unc	oordinated	distriction and trade to starting	anduktina sitat	EN FORFACCOAS	edeletia diakerania	
Maximum v/c Ratio: 0.68 🚄						
Intersection Signal Delay:(16		accessores			LOS: B	
Intersection Capacity Utiliza	tion 51.3%		IC	CU Level	of Service A	
Analysis Period (min) 15						
som the Add	1 C DI V/D @ 05	OCCUPATION !				
Splits and Phases: 3: GU	LF BLVD & SF	( 000/15UIH				
pi Tp:	2					
165 578			e a			
		The state of the s				7 /
w n6						<b>▼</b> ø8

Lanes, Volumes, Timings

3: GULF BLVD & SR 666/150TH

GULF BLVD / SR 666 EXISTING 10/17/2016 EXISTING CONDITIONS 2016 PM PEAK

Synchro 8 Report Page 2

Turn Type

Protected Phases Permitted Phases

Detector Phase Switch Phase

Minimum Initial (s)

Minimum Split (s)

Maximum Green (s)

Total Split (s)

Total Split (%)

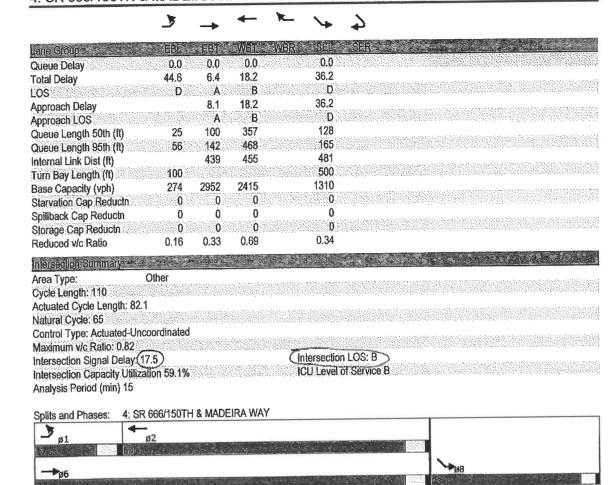
752	**************************************	## 872 1900 0.95 0.99 0.946	494 1900 250 0 0.95	356 1900 500 1 25 0.97	9 1900 0 0
36 900 100 1 25 1.00 1.00 950 752	805 1900 0.95	.872 1900 0.95 0.99	1900 250 0	356 1900 500 1 25 0.97	1900 0 0
36 900 100 1 25 1.00 1.00 950 752	805 1900 0.95	.872 1900 0.95 0.99	1900 250 0	356 1900 500 1 25 0.97	1900 0 0
900 100 1 25 1.00 1.00 950 752	1900	0.95 0.99	250 0	500 1 25 0.97	0 0
100 1 25 1.00 1.00 950 752	0.95	0.99	0	1 25 0.97	0
1 25 1.00 1.00 950 752		0.99		25 0.97	
25 1.00 1.00 950 752		0.99	0.95	0.97	0.95
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1.00 .950 .752				A 2 1 4 - A 2 - A	
.950 752		0.946		0.99	
752				0.996	
752				0.954	
	3505	3344	0	3462	
				0.954	
	3505	3344	0	3427	0
			Yes		Yes
A CONTRACTOR CONTRACTOR	2012/04/2014 19:00/	144		2	
	40	40		30	
***************************************	519	535		561	
	8.8	9.1		12.8	
14			3	3	14
	0.82	0,82	0.82	0.82	0.82
		1%	1%	1%	1%
44		1063	602	434	11
91945 Care.					
44	982	1665	0	445	0
		NA		Prot	
1	6	2		8	
	6	2		8	
27,-127,554,57					
4.0	4.0	4.0		4.0	
	21.0	21.0		20.0	
	77.0	60.0			
		54.5%			
		55.0		29.0	
4.0	4.0	4.0		3.0	
1.0	1.0	1.0			
0.0	0.0	0.0			
5.0	5.0	5.0		4.0	
Lead		Lag			
Yes		Yes			
3.0	3.0	3.0		3.0	
None	Min	Min		None	
	5.0	5.0		5.0	
	11.0	11.0			
	0	0		0	
7.9	55,6	48.5			
0.10	0.68	0.59	r responsable to the term	0.21	
0.26	0.41	0,82			
44.6	6.4	18.2		36.2	•
	14 0.82 3% 44 Prot 1 4.0 9.0 17.0 15.5% 12.0 4.0 0.0 5.0 Lead Yes 3.0 None	40 519 8.8 14 0.82 0.82 3% 3% 44 982 Prot NA 1 6 1 6 4.0 4.0 9.0 21.0 17.0 77.0 12.0 72.0 4.0 4.0 1.0 1.0 0.0 0.0 5.0 5.0 Lead Yes 3.0 3.0 None Min 5.0 0.10 0.68 0.26 0.41	1752 3505 3344  950  1744 3505 3344  40 40 40  519 535  8.8 9.1  14  0.82 0.82 0.82 3% 3% 1%  44 982 1063  44 982 1063  44 982 1065  Prot NA NA  1 6 2  1 6 2  1 6 2  1 7.0 77.0 60.0  15.5% 70.0% 54.5%  12.0 72.0 55.0  4.0 4.0 4.0 4.0  1.0 1.0 1.0 1.0  1.0 1.0 1.0  1.0 0.0 0.0  5.0 5.0  Lead Lag Yes 3.0 3.0 3.0  None Min Min 5.0 5.0  11,0 11.0 0 0 7.9 55.6 48.5  0.10 0.68 0.59 0.26 0.41 0.82	1752   3505   3344   0     950       1744   3505   3344   0     144     40   40     519   535     8.8   9.1     14     3     0.82   0.82   0.82   0.82     3%   3%   1%   1%     44   982   1063   602     44   982   1063   602     44   982   1065   0     Prot   NA   NA     1   6   2     1   6   2     4.0   4.0   4.0     9.0   21.0   21.0     17.0   77.0   60.0     15.5%   70.0%   54.5%     12.0   72.0   55.0     4.0   4.0   4.0     1.0   1.0   1.0     0.0   0.0   0.0     5.0   5.0   5.0     Lead   Lag     Yes   Yes     3.0   3.0   3.0     None   Min   Min     5.0   5.0     11.0   11.0     0   0     7.9   55.6   48.5     0.10   0.68   0.59     0.26   0.41   0.82	1752   3505   3344   0   3462     950

SR 666 / MADEIRA WAY EXISTING 10/17/2016 EXISTING CONDITIONS 2016 - PM PEAK

Synchro 8 Report Page 1

Lanes, Volumes, Timings 4: SR 666/150TH & MADEIRA WAY

10/17/2016



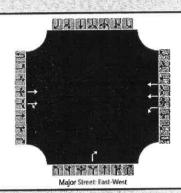
SR 666 / MADEIRA WAY EXISTING 10/17/2016 EXISTING CONDITIONS 2016 - PM PEAK

Synchro 8 Report Page 2

Madeira Beach Town Center

HCS 2010 Two-Way Stop Control Summary Report								
General Information								
Analyst	RP	Intersection	SR 666 / RETAIL PLAZA ACCESS					
Agency/Co.	GCC	Jurisdiction	FDOT					
Date Performed	10/17/2016	East/West Street	SR 666					
Analysis Year	2016 EXISTING	North/South Street	RETAIL PLAZA ACCESS					
Time Analyzed	PM PEAK	Peak Hour Factor	0.83					
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25					
Project Description	MADEIRA BEACH TOWN CTR							

#### Lanes



		The state of the s	
	4	A AL	
Vehicle Vo	lumes and	s Adius	tments

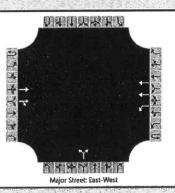
Approach		Easth	ound			West	oound			North	bound			South	bound	
Movement	U	L	T	R	U	%.f	T	R	U	L	T	R	U	L	T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	1	2	0		0	0	1	Mil	0	0	0
Configuration			Т	TR		L	T					R				
Volume (veh/h)			1171	3	Market St.	5	1374					2	Jan 1	4. T.		
Percent Heavy Vehicles						1						1				
Proportion Time Blocked	N. T.	Will.	Man.	###	有關		Trans.			William.	188	1933		and.		100
Right Turn Channelized		١	io			N	lo			ı	lo			N	lo	
Median Type								Left +	Thru							
Median Storage									2							
Delay, Queue Length, and	Level	of Se	ervice		*											
Flow Rate (veh/h)						6						2				
Capacity	LEARS.	Will.	TATES	14.000	MISS											
cupucity			= 000	3.00		483	W.	10.13		12000		380	Real Control	Hilli	Y.S.	Yau
v/c Ratio			100	V-125.2	V., V.	0.01		10.13				380	in the second			Vall
	15.35			SAS	***					KIY.		Santa Service		VALUE OF THE PARTY		Valle Va Valle Va Valle Va Valle Va Va Va Va Va Va Va Va Va Va Va Va Va
v/c Ratio	10.850 8.861 8.861			343	NAME OF THE PARTY	0.01	)					0.01				Valle Va Valle Valle Va Valle Va Valle Va Valle Va Va Valle Va Va Ve Va Va Va Va Ve Va Ve Va Ve Va Ve Va Ve Va V Va V Va V V Va V Ve
v/c Ratio 95% Queue Length	B, A, 60 S, 61 S,					0.01	)					0.01				
v/c Ratio 95% Queue Length Control Delay (s/veh)						0.01 0,0 12.6 B	)				4.5	0.01 0.0 14.5				

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HCS 2010™ TWSC Version 6.80 SR666RETAILX.xtw RETUIL PLAZA Generated: 10/17/2016 11:35:52 AM

#### HCS 2010 Two-Way Stop Control Summary Report Site Information **General Information** Intersection SR 666 / MADEIRA COVE ACCESS GCC Jurisdiction FDOT Agency/Co. East/West Street SR 666 10/17/16 MADEIRA COVE ACCESS 2016 EXISTING North/South Street Analysis Year Peak Hour Factor 0.86 PM PEAK Time Analyzed 0.25 East-West Analysis Time Period (hrs) Intersection Orientation MADEIRA BEACH TOWN CTR Project Description

#### Lanes



#### Vehicle Volumes and Adjustments

Approach		Eastl	bnucc			West	bound			North	bound			South	bound	
Movement	U	L	I	R	U	्रा	T	R	U	1	T	R	U	L	T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	1	2	0		0	0	0		0	0	0
Configuration			Т	TR		L	T				LR					
Volume (veh/h)	365		1170	0		9	1358			2	1100	6		-1111	N.	
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked			NA.	triin.		N. S.			12.		Mary		HAF		3.54	this.
Right Turn Channelized		ı	No			٨	10			N	lo			N	No	
Median Type								Left +	Thru							
Median Storage								;	2							
Delay, Queue Length, and	Leve	of S	ervice													
Flow Rate (veh/h)						10					9					
Capacity		Will				507		THE	THE STATE OF		308	N. S. S.		1330	Time.	
v/c Ratio						0.02					0.03			CONTRACTOR IN		
95% Queue Length	A			MAN		0.1			dia.		0,1	şilliği	111117		Marin	
Control Delay (s/veh)					1	12.2					17.1					

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Level of Service (LOS)

Approach Delay (s/veh)

Approach LOS

HCS 2010™ TWSC Version 6.80 SR666MADCOVEX.xtw

0.1

MADEIRA COVE

17.1

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General Information							Site I	nforn	nation							
Analyst	RP	(Exhau)	and a			V-1-1-1-1-1	Interse	ection			SR 666	6 / BOCA	A VISTA	ACCESS		or said
Agency/Co.	GCC			Kalina	: Nation		Jurisdi	ction		\$4.E	FDOT					
Date Performed	10/17/	2016					East/V	Vest Stre	et		SR 666	5				
Analysis Year	2016 E	XISTING					North	/South S	treet		BOCA	VISTA A	ACCESS			
Time Analyzed	PM PE	AK					Peak H	lour Fac	tor		0.83					
Intersection Orientation	East-W	est					Analys	is Time	Period (h	ırs)	0.25		Series.			
Project Description	MADE	RA BEAG	CH TOW	N CTR												
Lanes																
					AN CASE LANCES	T Street: Ea	-	<u>, 1</u>								
and the second s	US NY LA	184, D.F.		w r d Yr	Wajo	Stiect, Lo	STEELS I						-4-16-5			
Vehicle Volumes and Adju	stmei	and of the			Wajo				I	North	bound		ı	South	bound	√ · · · · · · · · · · · · · · · · · · ·
Approach		Eastb	and the second			West	oound	R	U		bound	R		South	bound	R
Approach  Movement	U	Eastb	ा	R	U	Westt	oound T	R	U		W.	<b>R</b> 9	, u	10000000	5.25.85.2	-
Approach  Movement  Priority	υ 10	Eastb L 1	T 2	3	U 4U	Westb	oound T	R 6	U		Versity (1)		u.	i L	Wi N	1
Approach  Movement  Priority  Number of Lanes	U	Eastb L 1	ा	3	U	Westt	oound T	6	U	7	8	9	u.	10	11	1
Approach  Movement  Priority  Number of Lanes  Configuration	υ 10	Eastb  L  1  0  LT	2 2	3 0 TR	U 4U	Westh	oound T 5	6 0	U	7	8 1	9	, u	10	11 1	17
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (vely/h)	υ 10	Eastb L 1 0 LT 7	T 2	3	U 4U	Westh L 4 1	oound T 5 7	6 0 TR	U	7 0	8 1 LTR	9		10 0	11 1 1 LTR	R 12 0
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles	υ 10	Eastb  L  1  0  LT	2 2	3 0 TR	U 4U	Westh  4  1  L  11	oound T 5 7	6 0 TR	U	7 0	8 1 LTR 0	9	u u	10 0	11 1 1 LTR 0	17
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked	υ 10	Eastb  L  1  0  LT  7	2 2	3 0 TR	U 4U	Westh  L  4  1  L  11  1	oound T 5 7	6 0 TR	U	7 0 1	8 1 LTR 0	9		10 0 4 1	11 1 1 LTR 0	12 0
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized	υ 10	Eastb  L  1  0  LT  7	2 2 2 1173	3 0 TR	U 4U	Westh  L  4  1  L  11  1	5 2 T 1367	6 0 TR 4	U U	7 0 1	8 1 LTR 0 1	9		10 0 4 1	11 1 1 LTR 0 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked	υ 10	Eastb  L  1  0  LT  7	2 2 2 1173	3 0 TR	U 4U	Westh  L  4  1  L  11  1	5 2 T 1367	6 0 TR 4		7 0 1	8 1 LTR 0 1	9		10 0 4 1	11 1 1 LTR 0 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage	1U 0	Eastb  L  1  0  LT  7  1	2 2 1173	3 0 TR 2	U 4U	Westh  L  4  1  L  11  1	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 LTR 0 1	9	u u	10 0 4 1	11 1 1 LTR 0 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	1U 0	Eastb  L  1  0  LT  7  1	2 2 1173	3 0 TR 2	U 4U	Westh  L  4  1  L  11  1	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 LTR 0 1 1	9		10 0 4 1	11 1 1 LTR 0 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	1U 0	Eastb L 1 0 LT 7 1	2 2 1173	3 0 TR 2	U 4U	Westh L 4 1 L 11 1	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 ILTR O ILTR	9		10 0 4 1	11 1 1 LTR 0 1 1 No	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	1U 0	Eastb  L  1  0  LT  7  1	2 2 1173	3 0 TR 2	U 4U	Westk  4  1  L  11  1	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 LTR 0 1 1	9		10 0 4 1	11 1 1 LTR 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity	1U 0	Eastb L 1 0 LT 7 1 N N 8 396	2 2 1173	3 0 TR 2	U 4U	Westt  4  1  1  1  1  13  483	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 1 LTR 0 1 1 Sec. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	9		10 0 4 1	11 1 1 LYR 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio	1U 0	Eastb L 1 0 LT 7 1 1 of So 8 396 0.02	2 2 1173	3 0 TR 2	U 4U	Westh  L  4  1  1  1  1  1  1  0.03	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 LTR 0 1 1 9 299 0.03	9		10 0 4 1	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length	1U 0	Eastb L 1 0 LT 7 1 1 of Se 8 396 0.02 0.1	2 2 1173	3 0 TR 2	U 4U	Westb  4  1  1  1  1  13  483  0.03	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	9 299 0.03	9		10 0 4 1	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length  Control Delay (s/veh)	1U 0	Eastb L 1 0 LT 7 1 1 of S 8 396 0.02 0.1 14.3 B	2 2 1173	3 0 TR 2	U 4U	Westh  4  1  1  1  13  483  0.03  0.1  12.7  8	5 2 T 1367	6 0 TR 4	+ Thru	7 0 1 1	8 1 1 LTR 0 1 1 9 299 0.03 0.1 17.4	9		10 0 11	10 138 0.07 0.2 33.0	100

TRAFFIC ANALYSIS

#### Generalized Peak Hour Two-Way Volumes for Florida's Urbanized Areas<sup>1</sup>

TABLE 4

INTERRUPTED FLOW FACILITIES UNINTERRUPTED FLOW FACILITIES FREEWAYS STATE SIGNALIZED ARTERIALS Class I (40 mph or higher posted speed limit) Median 5,540 4,120 6,700 7,190 Median Lanes 6,130 8,370 10,060 11,100 1,510 Undivided 2 13,390 15,010 8,230 11,100 3,580 Divided 3,420 16.840 18,930 10.330 14,040 Divided 5,250 5,390 22,030 14,450 18,880 22,860 7,090 Divided Freeway Adjustments Class II (35 mph or slower posted speed limit) Auxiliary Lanes Median D Present in Both Directions Metering 2 [2,920] 3,040 1.310 Divided 4,500 4,590 2,090 6 Divided 6,130 2,880 6,060 Non-State Signalized Roadway Adjustments (Alter corresponding state volumes by the indicated percent.) Non-State Signalized Roadways - 10% Median & Turn Lane Adjustments UNINTERRUPTED FLOW HIGHWAYS Exclusive Exclusive Lanes Median CD Left Lanes Right Lanes Factors Undivided 770 1,530 2,170 2,990 +5% Divided Divided 3,300 4,660 5,900 6,530 -20% Undivided No No Divided 4,950 6,990 8,840 9,790 Multi Undivided Yes -5% -25% Multi Undivided Νo Nο Yes Uninterrupted Flow Highway Adjustments Median Exclusive left lanes Adjustment factors One-Way Facility Adjustment Divided Yes +5% Multiply the corresponding two-directional Undivided Undivided -25% Values shown are presented as peak hour two-way volumes for levels of service and are for the automobile/truek modes unless specifically stated. This table does not constitute a standard and should be used only for general planning applications. The computer models from which this table is derived should be used for arone specific plauning applications. The table and deriving computer models should not be used for corridor or intersection design, where more refined techniques exist. Calculations are based on plauning applications of the Highway Capacity Mannaj and the Transii.

Capacity and Quality of Service Munual. BICYCLE MODE<sup>2</sup> (Multiply motorized vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service Paved Shoulder/Bicycle Lane Coverage 680 260 1,770 0-49% I level of service for the brownle and nedestrian modes in this table is based on number 1,770 >1,770 50-84% 190 600 85-100% 830 1,770 >1,770 Buses per hour shown are only for the peak hour in the single direction of the higher traffic PEDESTRIAN MODE<sup>2</sup> (Multiply motorized vehicle volumes shown below by number of Cannot be schieved using table input value defaults. \*\* Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached. For the bicycle mode, the level of service letter grade (including F) is not achievable because there is no maximum vehicle volume threshold using table input Sidewalk Coverage 250 850 0-49% 50-84% 150 780 1,420 1,560 >1,770 960 85-100% BUS MODE (Scheduled Fixed Route)3 (Buses in peak hour in peak direction) ≥3 ≥2 ≥4 >4

2012 FDOT QUALITY/LEVEL OF SERVICE HANDBOOK TABLES

MADEIRA BEACH, FLORIDA 61

APPENDIX B

# High-Rise Residential Condominium/Townhouse (232)

Average Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Number of Studies: 4

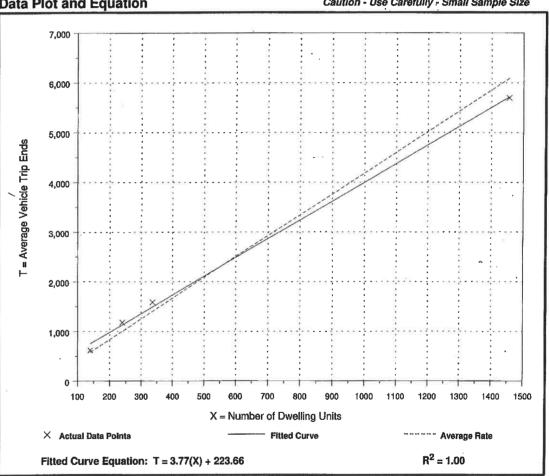
Avg. Number of Dwelling Units: 543
Directional Distribution: 50% entering, 50% exiting

#### Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
4.18	3.91 - 4.93	2.08

#### **Data Plot and Equation**

Caution - Use Carefully - Small Sample Size



Trip Generation, 9th Edition • Institute of Transportation Engineers

# High-Rise Residential Condominium/Townhouse (232)

Average Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

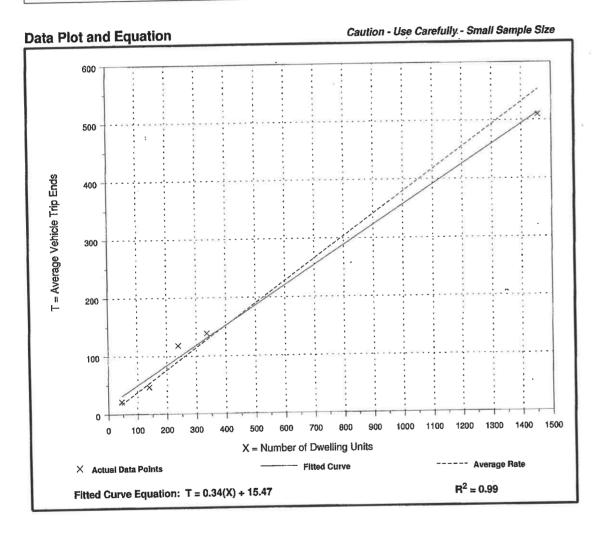
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 5
Avg. Number of Dwelling Units: 444

Directional Distribution: 62% entering, 38% exiting

#### Trip Generation per Dwelling Unit

Range of Rates	Standard Deviation
0.34 - 0.49	0.62



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429

### Hotel

(310)

Average Vehicle Trip Ends vs: Rooms

On a: Weekday

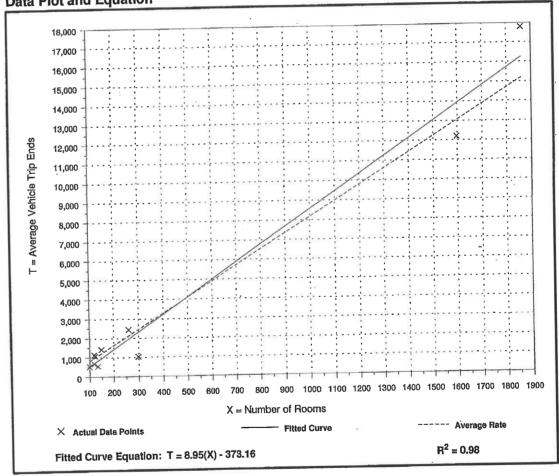
Number of Studies: 10 Average Number of Rooms: 476

Directional Distribution: 50% entering, 50% exiting

#### Trip Generation per Room

ilb douglasses has see		
Average Rate	Range of Rates	Standard Deviation
8.17	3,47 - 9.58	3.38
8.17	0.11	

**Data Plot and Equation** 



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013

# **Hotel** (310)

Average Vehicle Trip Ends vs: Rooms

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

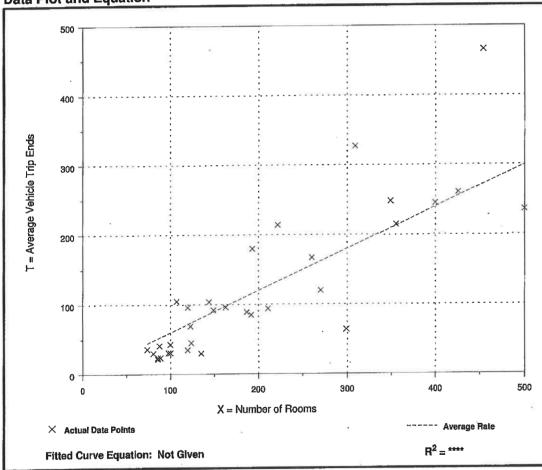
Number of Studies: 33 Average Number of Rooms: 200

Directional Distribution: 51% entering, 49% exiting

#### **Trip Generation per Room**

The delicitation for the		
Average Rate	Range of Rates	Standard Deviation
0.60	0.21 - 1.06	0.81

#### **Data Plot and Equation**



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615

## Marina

(420)

Average Vehicle Trip Ends vs: Berths
On a: Weekday

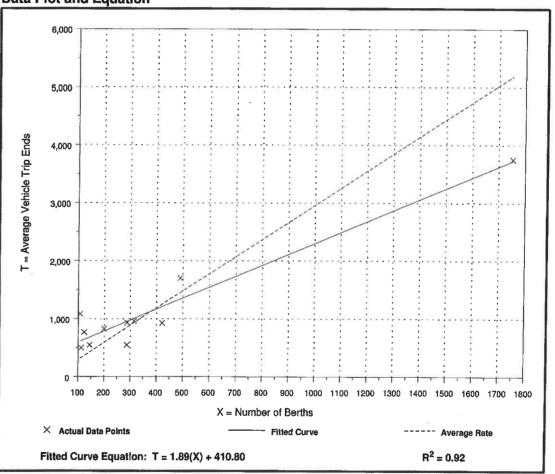
Number of Studies: 11 Average Number of Berths: 386

Directional Distribution: 50% entering, 50% exiting

#### Trip Generation per Berth

Average Rate	Range of Rates	Standard Deviation
2.96	1.91 - 10.04	2.26

**Data Plot and Equation** 



Trip Generation, 9th Edition • Institute of Transportation Engineers

TRAFFIC ANALYSIS

### Marina

(420)

Average Vehicle Trip Ends vs: Berths

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 2 Average Number of Berths: 362

Directional Distribution: 60% entering, 40% exiting

#### Trip Generation per Berth

Average Rate	Range of Rates	Standard Deviation
0.19	0.17 - 0.21	*

# Caution - Use Carefully - Small Sample Size **Data Plot and Equation** 310 X = Number of Berths × Actual Data Points $R^2 = *****$ Fitted Curve Equation: Not given

Trip Generation, 9th Edition • Institute of Transportation Engineers

TRAFFIC ANALYSIS

Lanes, Volumes, Timings 4: GULF BLVD & MADEIRA WAY

10/17/2016

	•		Ť,	~	-	1
ane Group	Mabel WBL	DA WAY	av Net	LF BLV	SBI	F BLVV.
	ኝ	7	ተተ	7	ħ	ተተ
ane Configurations	-1 158	339	545	62	249	561
/olume (vph)	1900	1900	1900	1900	1900	1900
deal Flow (vphpl)	1,00	1,00	0.95	1.00	1,00	0.95
ane Util, Factor	0.99	0.83	0,50	0.96	0.92	0.00
Ped Bike Factor	0.88	0.850	VERSESSE	0.850		
it.	۸ ۵۲۸	UCO.U		0.000	0.950	
It Protected	0.950	4500	3539	1583	1770	3539
Satd. Flow (prot)	1770	1583	งงงข	1000	0.286	0003
It Permitted	0.950	4004	3539	1525	489	3539
Satd, Flow (perm)	1751	1321	აეეაყ		409	องงอ
Right Turn on Red		Yes		Yes		0000000000
Satd, Flow (RTOR)		353	or.	65		35
Link Speed (mph)	30	veresteriterit	35	SERVER SERVER		
Link Distance (ft)	689		705			435
Travel Time (s)	15.7	s coccessor 12 129	13.7	Negosobii a	sassas <b>a k</b> io	8.5
Confl. Peds. (#/hr)	5	76		5	76	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	165	353	568	65	259	584
Shared Lane Traffic (%)						e per un exposuration de la com-
Lane Group Flow (vph)	165	353	568	65	259	584
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases	anterior more and an	8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase		a a a a a a a a a a a a a a a a a a a	and the second s			
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	20.0	20.0	20.0	20.0	8.0	20.5
Total Split (s)	29.0	29.0	64.0	64.0	17.0	81.0
	26.4%	26.4%	58.2%	58.2%	15.5%	73.6%
Total Split (%)	25.0	25.0	60.0	60.0	13.0	76,5
Maximum Green (s)			3.0	3.0	3.0	3.5
Yellow Time (s)	3.0	3.0	3.0 1.0	3.0 1,0	1.0	1.0
All-Red Time (s)	1.0	1,0		0.0	0.0	0.0
Lost Time Adjust (s)	0.0	0.0	0.0		4.0	
Total Lost Time (s)	4.0	4,0	4.0	4.0		4.5
Lead/Lag	geremberskings	NACONALISM	Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min
Walk Time (s)	5.0	5.0	5.0	5.0	the terrest and the	5.0
Flash Dont Walk (s)	11.0	11,0	11.0	11.0		11.0
Pedestrian Calls (#/hr)	0	0	0	0		0
Act Effct Green (s)	10.2	10.2	14.6	14.6	28.3	27.8
Actuated g/C Ratio	0.22	0.22	0.31	0.31	0.60	
v/c Ratio	0.43	0.63	0.51	0.12	0,46	
Control Delay	21.3	8.4	15.5	5.0	7.4	
Queue Delay	0.0		0.0	0.0		
	21.3	8.4	15.5	5.0	7.4	
Total Delay		1 1 4 4 1 5 5 5 5 5 5 5 TO 10 10 10 10 10 10 10 10 10 10 10 10 10	10.0 B	A.	Ā	Ä
LOS	C 12.5	Α	14.4	setemente.		5.8
Approach Delay	12.5		14.4	Tarris and the		0.0

GULF BLVD / MADEIRA WAY WITH PROJECT 10/17/2016 FUTURE CONDITIONS WITH PROJECTS - PM PEAK

ame Group

Lane Configurations Volume (vph)

Ideal Flow (vphpl)

Storage Length (ft)

Storage Lanes

Taper Length (ft)

Lane Util. Factor

Ped Bike Factor

Act Effct Green (s)

Actuated g/C Ratio

v/c Ratio

Control Delay

SR 660/1501-

400

0

1900

400

25

0.91

785 105 552 350

0.97 0.95 0.95 1.00

1900 1900 1900

	1	*	<b>†</b>	1	1	<b>↓</b>	
ane Group	was:	WBR=	NBTr	Meir	SBL	SBT* No. 10 To See Sec. 12 No. 25	
Approach LOS	В		В			Α	
Queue Length 50th (ft)	37	0	62	0	25	31	Nacasta (Nacasta (Na
Queue Length 95th (ft)	99	59	124	21	64	66	
nternal Link Dist (ft)	609	00000000000000000000000000000000000000	625		04046484646	<b>355</b>	
Furn Bay Length (ft)						000	
Base Capacity (vph)	979	888	3523	1518	664	3539	\$4000000000000
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0.00	0	
Reduced v/c Ratio	0.17	0.40	0.16	0.04	0.39	0.17	elegendoskung* i dio es
ntersection Summary							
	Other	cantorno navoran	Charles Addition		en (1.12.20.00.40.4		
Cycle Length: 110							
	3		loksini secinto	eranen erana	90005050		85958445933
Actuated Cycle Length: 46.6	- Brodenski MAANSSANS						
Natural Cycle: 50							
Natural Cycle: 50 Control Type: Actuated-Und	coordinated						
Natural Cycle: 50 Control Type: Actuated-Unc Maximum:v/c Ratio: 0.63 z	coordinated			<i></i>	la-nant-a	100.0	
Natural Cycle: 50 Control Type: Actuated-Unc Maximum v/c Ratio: 0.63 Intersection Signal Delay: 1	coordinated					LOS: B	
Natural Cycle: 50 Control Type: Actuated-Unc Maximum v/c Ratlo: 0.63 Intersection Signal Delay(1 Intersection Capacity Utiliza	coordinated					LOS: B) Service A	
Natural Cycle: 50 Control Type: Actuated-Unc Maximum v/c Ratio: 0.63 Intersection Signal Delay: 1	coordinated						
Natural Cycle: 50 Control Type: Actuated-Unc Maximum v/c Ratlo: 0.63 Intersection Signal Delay(1 Intersection Capacity Utiliza Analysis Period (min) 15	coordinated 0.3 110n 51.8%	MADEIR	A WAY				
Natural Cycle: 50 Control Type: Actuated-Unc Vlaximum V/c: Ratio: 0.63 Intersection Signal Delay 1 Intersection Capacity Utiliza Analysis Period (min) 15  Splits and Phases: 4: GU	coordinated 0.3 0.3 0.51:8% LF BLVD &	MADEIR	A WAY				
Natural Cycle: 50 Control Type: Actuated-Unc Vlaximum V/c: Ratio: 0.63 Intersection Signal Delay 1 Intersection Capacity Utiliza Analysis Period (min) 15  Splits and Phases: 4: GU	coordinated 0.3 110n 51.8%	MADEIR	A WAY				
Natural Cycle: 50 Control Type: Actuated-Unc Maximum v/c Ratlo: 0.63 Intersection Signal Delay Intersection Capacity Utiliza Analysis Period (min) 15  Splits and Phases: 4: GU	coordinated 0.3 0.3 0.51:8% LF BLVD &	MADEIR	A WAY				

plits and Phases:	4; GULF BLVD & M	MADEIRA WAY			
V <sub>est</sub>	102				
			Challes Constant		
W 45		×		1	ø8

I bu billo i doloi	and the second second	-24	erenining to	2000	frankritani	The state of the s		
Frt	0.982			0.850			and the second second	
Fit Protected	0.958				0.950			
Satd. Flow (prot)	3368	0	3505	1568	3400	3505	terresistant at a factor of	
FIt Permitted	0.958				0.950			
Satd. Flow (perm)	3113	0	3505	1470	3192	3505	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Right Turn on Red		Yes		Yes				
Satd. Flow (RTOR)	14			393				
Link Speed (mph)	40		35			35		
Link Distance (ft)	776		506			611		
Travel Time (s)	13.2		9.9			11.9		
Confl. Peds. (#/hr)	25	42		25	42			
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89		
Heavy Vehicles (%)	1%	1%	3%	3%	3%	3%		
Adj. Flow (vph)	882	118	620	393	220	527		
Shared Lane Traffic (%)		and the first of Paris Paris		~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~				
Lane Group Flow (vph)	1000	0	620	393	220	527		
Turn Type	Prot		NA	Perm	Prot	NA		
Protected Phases	8		2		1	6		
Permitted Phases	elektrick en	Marie Standard Standard Confession		2				
Detector Phase	8		2	2	1	6		10000
Switch Phase	A CONTRACTOR OF A STATE OF A STAT		en en jaron en					
Minimum Initial (s)	4.0		4.0	4.0	4.0	4.0		
Minimum Split (s)	21.0		20.0	20.0	8.0	20.0		•
Total Split (s)	37.0		57.0	57,0	16.0	73.0		1000
Total Split (%)	33.6%	ene anna anna	51.8%	51.8%	14.5%	66.4%		
Maximum Green (s)	32.0		53.0	53.0	12.0	69.0		
Yellow Time (s)	4.0		3.0	3.0	3.0	3.0	- en en entretate en entretate	
All-Red Time (s)	1.0		1,0	1.0	1.0	1.0		
Lost Time Adjust (s)	0.0	acceptancy (A	0.0	0.0	0.0	0.0		
Total Lost Time (s)	5.0		4.0	4.0	4.0	4.0		
Lead/Lag			Lag	Lag	Lead		endiana dianggraphy	
Lead-Lag Optimize?			Yes	Yes	Yes			
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0	and a second	•
Recall Mode	None		Min	Min	None	Min		
Walk Time (s)	5.0		5.0	5.0	SOUNDINGS:	5.0	an akpanyananan	
Flash Dont Walk (s)	3.0 11.0		11.0	11.0		11.0		
Pedestrian Calls (#/hr)	0		0	0	e e en	0	ne se de digenda de la dela Pelifiq	-1-7
Pedestrian Cans (#/III)	24.6		210	21.0	10.1	ี 36 กั		

10.1 36.0

0.47

0.32

13.0

0.13

36.2

GUF BLVO

**↑↑** 469

**ችች** 196

2

25

0.94

400 300

1900 1900

0.97 0.95

GILF BLUD

0.94

GULF BLVD / SR 666 WITH PROJECTS 10/17/2016 FUTURE CONDITIONS WITH PROJECTS - PM PEAK

0.29

26.6

21.9 21.9

0.29

0,62 0.56 0.49

5.8

31.6

0.72

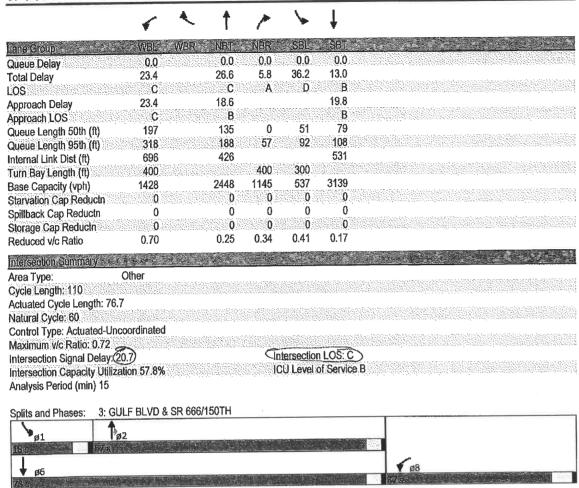
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Synchro 8 Report Page 1

Synchro 8 Report

Lanes, Volumes, Timings 3: GULF BLVD & SR 666/150TH

10/17/2016



GULF BLVD / SR 666 WITH PROJECTS 10/17/2016 FUTURE CONDITIONS WITH PROJECTS - PM PEAK

Synchro 8 Report

General Information							Site I	nforn	natior	)						
Analyst	RP					acatiolistics	Interse	ection			SR 66	6 / MAD	EIRA WA	ΑY		
Agency/Co.	GCC	Georgia.		Çanti			Jurisd	iction	us s		FDOT	· Dalib				
Date Performed	10/17	/16					East/V	Vest Stre	eet		SR 66	6				
Analysis Year	FUTU	RE WITH	PROJEC	TS	ana,	!N\!!	North	/South S	treet		MAD	EIRA WA	Y (RIRO)	terill.	By Art	
Time Analyzed	PM P	AK			Q-101111-1-1-1		Peak I	lour Fac	tor	-	0.82					
Intersection Orientation	East-V	Vest					Analy	sis Time	Period (l	hrs)	0.25		YHAN.			
Project Description	MADE	IRA BEA	CH TOW	/N CTR												
Lanes					, DIAI	J			White courses in which a carbon parties a parties on the content of the content o		*					
Vehicle Volumes and Adju	ıstme	nts	3			Street: Ea										
Approach		Easth	ound			Westl	oound			North	bound	40		South	bound	
Movement	U		T	R	U	WLW.	T	R	IJ	A LA	J	R	U		T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	2	1	1,44,14	0	0	0		0	0	1
Configuration			Т				T	R								R
Comigaradon			1019		東藍		895	575				N. S.	Taken.			17
Volume (veh/h)	Allela															1
Carrier Carrier Williams Commission Commission									l			L		ļ		
Volume (veh/h)						No.	180	WW		A KUM	WENN!				Spirite	110
Volume (veh/h) Percent Heavy Vehicles		1	iniini No				lo	11.0.1.1.1 11.0.1.1.1	25.00	1	lo lo		N. S.	1	No	
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked		1	No			N	lo	Left -	Thru		No			1	lo	
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized		•	No			7	lo		Thru	l constant	No.		A LOS	1	No.	
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Tum Channelized  Median Type	Leve						lo				No			<b>N</b>	No	
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage	Leve					N 1	lo			ı	No			<b>N</b>	No.	21
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Tum Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)	Leve						lo				No				No.	21 48
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity							lo				No.				No	-
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio							lo lo				No.				No	48
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length											No				No.	48 0.4 2.
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length  Control Delay (s/veh)											No.				No	48 0.4 2.
Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length		l of \$i	ervice								No No				No	48

	HCS	201	.0 Tw	o-W	ay St	op C	ontr	ol Su	ımma	ary R	epoi	t 💮				
General Information							l Balancia de la		natio							
Analyst	RP						Inters	ection			SR 66	6 / RET	AIL PLAZ	A ACCES	S	
Agency/Co.	GCC		907	11110			Juriso		ei, vi	ri not	FDO	r	Ville V			- N
Date Performed	10/17	/2016					East/\	West Str	eet		SR 66	56				
Analysis Year	2017				general.		5.275,475	/South S		i i i	1,15,25		A ACCESS	s ARA		
Time Analyzed	PM PE						_	Hour Fac			0.83					
Intersection Orientation	East-V	5 5.5	Maria.	MARK	SAMA				Period (	hrs)	0.25	V.V.				
Project Description			ACH TOV	VN CTR -	FUTURE	WITH F		-					-			
						- 77		* F - F								
Lanes			122				· · · · · · · · · · · · · · · · · · ·		ř.							
						7		5								
Vehicle Volumes and Adju	istmei	n <b>ts</b>			emily/st streets	r Street Ea	Sport at resist on a real									
Vehicle Volumes and Adju	ıstme		bound		emily/st streets	r Street: Ea	Sport at resist on a real			North	bound	- 101 114-		South	bound	
	istme			R	emily/st streets	r Street: Ea	ast-West	R	Ü	North	bound	R	<b>1</b>	South	bound T	R
Approach		Eastl	bound		Majo	r Street: Ea	ost-West					R 9	U			
Approach  Movement	Ü	Eastl	bound	R	Majo	r Street Ea	bound	R A		L	T.	-	U	NL 1	ा	12
Approach  Movement  Priority	U 10	Eastl	bound T	R 3	Majo U 4U	West	bound T	R 6		7	T 8	9	U	10	11	12
Approach  Movement  Priority  Number of Lanes	U 10	Eastl	bound T	R 3	Majo U 4U	Westl	bound T	R 6		7	T 8	9	U	10	11	R 12
Approach  Movement  Priority  Number of Lanes  Configuration	U 10	Eastl	bound T 2 T	R 3 0 TR	Majo U 4U	Westl	bound T 5 2	R 6		7	T 8	9 1 R	U	10	11	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)	U 10	Eastl	bound T 2 T	R 3 0 TR	Majo U 4U	Westl L 4 1 L	bound T 5 2 T	R 6		7	T 8	9 1 R	U	10	11	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles	U 10	Eastl L 1 0	bound T 2 T	R 3 0 TR	Majo U 4U	West L 4 1 L 5	bound T 5 2 T	R 6		7 0	T 8	9 1 R	U	10 0	11	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked	U 10	Eastl L 1 0	2 2 T 1288	R 3 0 TR	Majo U 4U	West L 4 1 L 5	bound T 5 2 T 1478	6 0		7 0	8 0	9 1 R	. U	10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized	U 10	Eastl L 1 0	2 2 T 1288	R 3 0 TR	Majo U 4U	West L 4 1 L 5	bound T 5 2 T 1478	6 0 Left 1		7 0	8 0	9 1 R	U	10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage	U 10 0	Eastl L 1 0	Dound T 2 2 7 1288	R 3 0 TR 3	Majo U 4U	West L 4 1 L 5	bound T 5 2 T 1478	6 0 Left 1	Thru	7 0	8 0	9 1 R		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (vely/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	U 10 0	Eastl L 1 0	Dound T 2 2 7 1288	R 3 0 TR 3	Majo U 4U	West L 4 1 L 5	bound T 5 2 T 1478	6 0 Left 1	Thru	7 0	8 0	9 1 R	U	10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	1U 0	Eastl L 1 0	Dound T 2 2 7 1288	R 3 0 TR 3	Majo U 4U	Westl L 4 1 L 5	bound T 5 2 T 1478	6 0 Left 1	Thru	7 0	8 0	9 1 R 2 1		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Tum Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity	U 10 0	Eastly 1 1 0 0 of Si	Dound T 2 2 7 1288	R 3 0 TR 3	Wajo	Westl L 4 1 L 5 1	bound T 5 2 T 1478	R 6 0	Thru	7	8 0	9 1 R 2 1 1		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (vely/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio	1U 0	Eastly 1 1 0 0 of Si	Dound T 2 2 7 1288	R 3 0 TR 3	Wajo	West L 4 1 1 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	bound T 5 2 T 1478	R 6 0	Thru	7	8 0	9 1 R 2 1		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length	1U 0	Eastly 1 1 0 0 of Si	oound T 2 T 1288	R 3 0 TR 3	Wajo	West L 4 1 L 5 1 N N N N N N N N N N N N N N N N N N	bound T 5 2 T 1478	R 6 0	Thru	7	8 8 0 8 80	9 1 R 2 1 1 2 341 0.01		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12
Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, and Flow Rate (veh/h) Capacity v/c Ratio	1U 0	Eastly 1 1 0 0 of Si	oound T 2 T 1288	R 3 0 TR 3	Wajo	Westl L 4 1 1 5 1 1 6 426 0.01	bound T 5 2 T 1478	R 6 0	Thru	7	8 8 0 8 80	9 1 R 2 1 1 2 341 0.01 0.0		10 0	1 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12

HCS 2010™ TWSC Version 6.80 SR666RETAILWP.xtw

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Generated: 10/17/2016 2:53:47 PM

General Information							Site	Infor	matio	n						
Analyst	RP	20.3	MINOR DANGERS		10, 04,2000 mg/s	. (	Inters	ection			SR 6	66 / MAI	DEIRA CO	OVE ACC	ESS	- 20,000,000,000
Agency/Co.	GCC	NO BUILD	a Aprilla	ant int	H.V.	200	Juriso	liction	e j	g 1997	FDO	<b>7</b> . 1000			V. DA	
Date Performed	10/17	7/16					East/	West Str	reet		SR 6	66				
Analysis Year	2017						North	ı/South	Street	Marin (	MAE	DEIRA CC	VE ACCE	SS		
Time Analyzed	PM P	EAK					Peak	Hour Fa	ctor		0.86					
Intersection Orientation	East-	West					Analy	sis Time	Period (	hrs)	0.25	THE S				
Project Description	MAD	EIRA BE	ACH TOV	VN CTR	- FUTUR	E WITH I	PROJECT	S								
Lanes																
						*										
				v ex i		or Street: E					Ng: 30					
/ehicle Volumes and Adju	ıstme	SAL NOVE				SALU or Street: Ea	ast-West									
Approach		Eastl	pound	· 2	Majo	Y Co or Street: Ea West	ast-West bound				bound			1000000	nbound	
Approach Movement	\U .	Eastl L	ii i	R	Majo	West	bound	R	υ	NE X	T	R	U	ar w	J	+-
Approach  Movement  Priority	υ 10	Eastl	<b>T</b>	3	Majo U 4U	West	bound T	6		7	T	9	T.U.S.	10	11	12
Approach  Movement  Priority  Number of Lanes	\U .	Eastl	2 2	3	Majo	West 4	bound 5	6	A.U	NE X	8 1		U	1 10 0	J	R 12
Approach  Movement  Priority  Number of Lanes  Configuration	1U 0	Eastl	2 2 T	3 0 TR	Wajc	West  4	bound T 5 2	6 0 TR		7 0	8 1 LTR	9	J. U.	10 0 LT	11 11	12 1 R
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)	υ 10	Eastl  1  1  1  47	2 2	3	Majo U 4U	Westing August 1	bound 5	6		7 0	8 1 LTR	9 0	U	1 10 0 LT 8	11 1 1	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles	1U 0	Eastl	2 2 T	3 0 TR	Wajc	West  4	bound T 5 2	6 0 TR		7 0	8 1 LTR	9	J J	10 0 LT	11 11	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked	1U 0	Eastl	2 2 T 1281	3 0 TR	Wajc	West  4 1 L 9	bound T 5 2 T 1458	6 0 TR		7 0 2 1	8 1 LTR 0 0	9 0	U	10 0 LT 8	11 11 0 0	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized	1U 0	Eastl	2 2 T	3 0 TR	Wajc	West  4 1 L 9	bound T 5 2	6 0 TR 12		7 0 2 1	8 1 LTR	9 0		10 0 LT 8	11 1 1	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type	1U 0	Eastl	2 2 T 1281	3 0 TR	Wajc	West  4 1 L 9	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	8 1 LTR 0 0	9 0	U	10 0 LT 8	11 11 0 0	12
Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage	1U 0	Eastl  1  1  1  L  47  0	7 2 2 T 1281	3 0 TR 0	Wajc	West  4 1 L 9	bound T 5 2 T 1458	6 0 TR 12		7 0 2 1	8 1 LTR 0 0	9 0		10 0 LT 8	11 11 0 0	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and	1U 0	Eastl  1  1  1  47  0	7 2 2 T 1281	3 0 TR 0	Wajc	West  4  1  L  9	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	1 LTR 0 0	9 0	U	10 0 LT 8	11 11 0 0	12 1 R 4
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)	1U 0	Eastl  1  1  47  0  of St	T 2 2 T 1281	3 0 TR 0	Wajc	West  4  1  1  1  10	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	1 LTR 0 0 No	9 0 1		10 0 LT 8 0	11 11 0 0	12 1 R 4 0
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity	1U 0	Eastl  1  1  47  0  of Se  55  377	7 2 2 T 1281	3 0 TR 0	Wajc	West L 4 1 1 L 9 1 1 1 0 452	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	8 1 1 1 TR 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	9 0		11 10 0 UT 8 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	11 11 0 0	12 1 R 4 0
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio	1U 0	Eastl  1  1  1  47  0  of Se  55  377  0.15	T 2 2 T 1281	3 0 TR 0	Wajc	West  4 1 1 1 10 452 0.02	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	1 LTR 0 0 No	9 0 1		10 0 LT 8 0	11 11 0 0	12 1 1 1 8 4 4 0
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, and Flow Rate (veh/h) Capacity v/c Ratio 95% Queue Length	1U 0	Eastl  1  1  47  0  of St  55  377  0.15	T 2 2 T 1281	3 0 TR 0	Wajc	West L 4 1 1 L 9 1 1 1 0 452	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	9 208 0.04	9 0 1		10 0 10 8 0 17 8 9 8 82 0,11	11 11 0 0	12 1 R 4 0
Approach  Movement  Priority  Number of Lanes  Configuration  Volume (veh/h)  Percent Heavy Vehicles  Proportion Time Blocked  Right Turn Channelized  Median Type  Median Storage  Delay, Queue Length, and  Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length  Control Delay (s/veh)	1U 0	Eastl  1  1  47  0  Of Se  55  377  0.15  16.2	T 2 2 T 1281	3 0 TR 0	Wajc	West 1 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	bound T 5 2 T 1458	6 0 TR 12	Thru	7 0 2 1	9 208 0.04 0.1	9 0 1		9 82 0.11	11 11 0 0	12 1 1 R 4 0 0
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, and Flow Rate (veh/h) Capacity v/c Ratio 95% Queue Length	1U 0	Eastl  1  1  1  L  47  0  55  377  0.15  0.5  16.2	T 2 2 T 1281	3 0 TR 0	Wajc	West  1  4  1  1  10  452  0.02  13.1  8	bound T 5 2 T 1458	6 0 TR 12	Thru	2 1	9 208 0.04 0.1 23.1	9 0 6 1		9 82 0,11 0,4 54.2	11 11 0 0	12 1 1 R 4 0

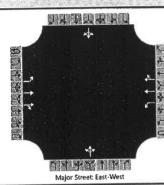
TRAFFIC ANALYSIS

Approach LOS

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	HCS 2010 Two-Way Stop Control Summary Report							
General Information		Site Information						
Analyst	RP	Intersection	SR 666 / BOCA VISTA ACCESS					
Agency/Co.	GCC	Jurisdiction	FDOT					
Date Performed	11/1/16	East/West Street	SR 666					
Analysis Year	2017	North/South Street	BOCA VISTA ACCESS/MARINA					
Time Analyzed	PM PEAK	Peak Hour Factor	0.83					
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25					
Project Description	MADEIRA BEACH TOWN CTR - FUTURE WITH	PROJECTS						

#### Lanes



Approach		Eastb	ound			Westb	ound			North	ound			South	bound	
Movement	U	L	τ	R	U		T	R	Ü	L	T	R	U	L	I	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0	Nation	0	1	0
Configuration		l.	Т	TR		L	T	TR			LTR				LTR	1,000,00
Volume (veh/h)		2	1292	2		11	1478	3		1	0	7		2	0	1
Percent Heavy Vehicles		1				1				1	1	1		1	1	1
Proportion Time Blocked			Sixxii	TAR			NIN.	23333				10.50		300000 3000000000000000000000000000000	MEX. TO	20.00
										N				1	No	
Right Turn Channelized		١	lo			- 1	lo				iO	and the Se	33.43.43.43.43.43		er salt de teren en de	45,000
Right Turn Channelized  Median Type	4000 10000	1	lo				10	Left	Thru							
Laboration and a state of the s		1	lo				10	_	Thru 2							
Median Type	Leve						10		_							
Median Type Median Storage	Leve					13	10		2						3	
Median Type Median Storage <b>Delay, Queue Length, and</b>	Leve	) of Si							2						3 98	
Median Type  Median Storage  Delay, Queue Length, and Flow Rate (veh/h)	Leve	l of S				13	10		2		9 271 0.03				3 98 0.03	
Median Type  Median Storage  Delay, Queue Length, and Flow Rate (veh/h)  Capacity	Leve	2 348				13 425			2		9 271				3 98 0.03	
Median Type  Median Storage  Delay, Queue Length, and Flow Rate (veh/h)  Capacity  v/c Ratio	Leve	2 348 0.01				13 425 0.03			2		9 271 0.03 0.1 18.8		\(\frac{1}{2}\)		3 98 0.03 0.1 43.0	
Median Type  Median Storage  Delay, Queue Length, and Flow Rate (veh/h)  Capacity  v/c Ratio  95% Queue Length	Leve	2 348 0.01				13 425 0.03 0.1			2		9 271 0.03 0.1				3 98 0.03	

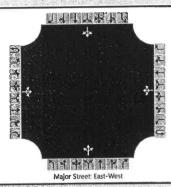
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#### HCS 2010 Two-Way Stop Control Summary Report **Site Information General Information** MADEIRA WAY/HOTEL DRIVEWAYS Intersection Analyst MADEIRA BCH Jurisdiction GCC Agency/Co. MADEIRA WAY East/West Street 10/17/16 Date Performed HOTEL DRIVEWAYS North/South Street **FUTURE WITH PROJECTS** Analysis Year 0.95 Peak Hour Factor PM PEAK HOUR Time Analyzed 0.25 Analysis Time Period (hrs) Intersection Orientation MADEIRA BEACH TOWN CTR Project Description

#### Lanes



Vehicle Volumes and Adju	stmei	nts														
Approach	Million	Eastb	ound			Westb	ound			Northb	ound			South	bound	
Movement	U	i.	NT.	R	U	J.D.	<b>T</b> .	R	U	11.	TA	R	U	à L	<b>I</b>	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0	W.	0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)	12531	13	282	16	1130	38	488	32		5	0	46	A sales	39	0	5
Percent Heavy Vehicles		1				1				1	1	1		1	1	1
Proportion Time Blocked						Kin		10.00				WAY.		N. S.		MAR
Right Turn Channelized		N	lo			N	lo			N	0			N	lo .	
Median Type								Undi	vided							
Median Storage											- nick links	20,000,000,000	nel en træ selnett.	Spile Spinger Facilities (Spile)	and the grade and the	
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate (veh/h)		14		T	Γ	40					53				46	
Capacity	1,510	1032	183	Water	W.	1258	YES		100		610	HAN	Villa b		224	N. S.
v/c Ratio		0.01				0.03					0.09				0.21	
95% Queue Length		0,0			ATAX ATAXA	0.1		35.4	133	HAN	0.3			ANA.	0.7	
Control Delay (s/veh)		8.5	1			8.0					11.5				25.2	
Level of Service (LOS)		A	1)	11.0	li li li li	Α		TO BE		19.00	В	100			59	(Alaka
Approach Delay (s/veh)		(	0.5			(	).9			1:	1.5			/ 2	5.2	\
4899474 4010 2223	1 10 10		e v	TO THE	1/32	rigitaly o	12,300-1	- 37 7 8	1011 12	7	В		1		D	1

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## **END OF DOCUMENT**

Madeira Beach Town Center

MADEIRA BEACH, FLORIDA 69

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