

# Project Conditions Hydraulic Evaluation of the Corte Madera Creek Levees

Stetson Engineers Inc.

October 11, 2019

## 1. Introduction

The 10/7/2019 technical memorandum entitled “Existing Conditions Hydraulic Evaluation of the Corte Madera Creek Levees” documented Stetson’s hydraulic evaluation of the Corte Madera levees under existing conditions. After examining the 100-year riverine and coastal flood inundation extents with consideration to sources of floodwater and future sea level rise (SLR), Stetson identified two potential alignments of flood barriers. Alignment #1 would fully utilize flood barriers to combat flooding while Alignment #2 would primarily use existing earthen channel levees that would be raised and tidal gates. The two alignments were documented and described in the 10/7/2019 technical memorandum.

Figure 1a shows the Alignment #2 flood barriers and tidal gates. A3GEO reviewed the flood barrier alignments in the field and selected the Alignment #2 flood barriers and tidal gates in the upper portion of the Corte Madera Creek earthen channel (the label-highlighted flood barriers in Figure 1b) as the recommended project for further hydraulic evaluation, as these flood barriers and tidal gates were considered most feasible in terms of constructability and cost (e.g., less private property impacts and required private property coordination, etc.). These flood barriers (with stationing) and tidal gates are also shown in Figure 2. The proposed flood barriers include:

- Existing levee – Hillview Gardens levee (length: 2,925 ft);
- Existing levee – Kentfield Gardens levee (length: 3,995 ft);
- College Ct flood barrier (length: 1,595 ft);
- Bon Air Road Complex flood barrier (length: 560 ft);
- South Eliseo flood barrier (length: 2,225 ft);
- Wolfe Grade flood barrier #1 (length: 460 ft);
- Wolfe Grade flood barrier #2 (length: 235 ft); and
- Wolfe Grade flood barrier #3 (length: 1,575 ft).

Three segments of the Wolfe Grade flood barrier were considered due to the two bridges between them. Floodwalls and wingwalls would be needed at the bridges to connect with the Wolfe Grade flood barriers.

In addition to the above proposed flood barriers and tidal gates, under the County direction, the project condition also considered the earthen channel dredging and the preferred Alternative J measures for the upstream concrete channel and Unit 4 under the U.S. Army Corps of Engineers’ (USACE) Corte Madera Creek Flood Risk Management Project.

Figure 3 shows the dredging extent scenarios from the Stilling Basin down to the Larkspur Creek confluence (green + blue + yellow dashed lines; Phase 1 + 2 + 3). Laterally the dredged channel cross sections in the earthen channel followed the typical cross section shown in Figure 4, with the same width and same side slope (1V: 6H) as the USACE original design channel (but to varying depths along the reach). The simulated water surface elevation (WSE) profiles under

existing levee conditions are also shown in Figure 3. Under the County direction, the Phase 1 + 2 dredging scenario was considered in the project condition analysis. Figure 5a shows the water surface reduction benefit by the Phase 1 + 2 dredging. Figure 5b shows the zoom-in water surface reduction benefit.

The preferred Alternative J measures for the upstream concrete channel and Unit 4 under the U.S. Army Corps of Engineers' (USACE) Corte Madera Creek Flood Risk Management Project are shown in Figure 6, which were described in the Draft EIR for the USACE Corte Madera Creek Flood Risk Management Project that was released in October 2018.

This technical memorandum documents the results of hydraulic evaluation of the Corte Madera Creek levees/flood barriers under project conditions.

## 2. Model Simulations of Project Conditions

The levee assessment will seek to evaluate the adequacy of the heights and freeboards of the levees to contain the design flood events. Since the levees/flood barriers must contain both riverine floods and coastal floods, it follows that the assessment will need to analyze both *riverine* flood conditions and *coastal* flood conditions, independently, since simultaneous flooding from both sources is improbable. The model results for the two flood conditions will be overlaid to determine which flood condition “controls” and where.

Table 1a is a summary of formulated four scenarios for the 100-year riverine flood (Scenarios #1, #2, #3, and #4) and Table 1b is a summary of formulated four scenarios for the 100-year coastal flood (Scenarios #5, #6, #7, and #8). Coastal floods in the San Francisco Bay area are typically accompanied by storms, so some measure of riverine contribution should be accounted for. For the purpose of this levee evaluation, a 10-year storm event to accompany the 100-year coastal flood was assumed in the 100-year coastal flood analysis. The inclusion of a 10-year storm event with the 100-year coastal flood is considered a reasonable combination<sup>1</sup>; however, it is not required by FEMA for its 100-year coastal floodplain mapping.

The model simulations of project conditions used the updated HEC-RAS 1D/2D unsteady-flow hydraulic model that was prepared for the simulations of existing conditions. In the model simulations of project conditions, unlimited heights of the levees/flood barriers were assumed and, thus, there would be no overtopping.

The State of California Sea-Level Rise Guidance/2018 Update (California Natural Resources Agency 2017) provides a science-based methodology for state and local governments to analyze and assess the risks associated with sea level rise and incorporate sea level rise into their planning, permitting, and investment decisions. For the purpose of this study, the future sea level rise of 1.1 ft by 2050 under the “67% probability, high emissions” scenario (see Table 1 of the California Sea-Level Rise Guidance) was selected for the sea level rise modeling analysis.

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<sup>1</sup> For example, the 1/27/1983 coastal flood event, which has the highest recorded peak tide observed at the San Francisco Bay tidal gage station (NOAA #9414290) over its 150-year period of record, was accompanied by a 5 - 10 year storm event.

Figure 7 compares the simulated inundation extents between the 100-year riverine flood and the 100-year coastal flood under the without dredging and without SLR conditions (Scenario #1 vs. Scenario #5).

Figure 8 compares the simulated inundation extents between the 100-year riverine flood and the 100-year coastal flood under the without dredging and with SLR conditions (Scenario #2 vs. Scenario #6).

Figure 9 compares the simulated inundation extents between the 100-year riverine flood and the 100-year coastal flood under the with dredging and without SLR conditions (Scenario #3 vs. Scenario #7).

Figure 10 compares the simulated inundation extents between the 100-year riverine flood and the 100-year coastal flood under the with dredging and with SLR conditions (Scenario #4 vs. Scenario #8).

Figure 11 compares the simulated 100-year riverine inundation extents between the without SLR and the with SLR conditions under the without dredging condition (Scenario #1 vs. Scenario #2).

Figure 12 compares the simulated 100-year coastal inundation extents between the without SLR and the with SLR under the without dredging condition (Scenario #5 vs. Scenario #6).

Figure 13 compares the simulated 100-year riverine inundation extents between the without dredging and the with dredging conditions under the without SLR condition (Scenario #1 vs. Scenario #3).

Figure 14 compares the simulated 100-year riverine inundation extents between the without dredging and the with dredging conditions under the with SLR condition (Scenario #2 vs. Scenario #4).

Figure 15 shows the simulated riverine 100-year WSE profiles for Scenarios #1 to #4 and Figure 16 shows the simulated coastal 100-year WSE profiles for Scenarios #5 to #8.

Figure 17 compares the simulated WSE profiles between the riverine 100-year flood and the coastal 100-year flood under the without dredge and without SLR conditions (Scenario #1 vs. Scenario #5).

Figure 18 compares the simulated WSE profiles between the riverine 100-year flood and the coastal 100-year flood under the without dredge and with SLR conditions (Scenario #2 vs. Scenario #6).

Figure 19 compares the simulated WSE profiles between the riverine 100-year flood and the coastal 100-year flood under the with dredge and without SLR conditions (Scenario #3 vs. Scenario #7).

Figure 20 compares the simulated WSE profiles between the riverine 100-year flood and the coastal 100-year flood under the with dredge and with SLR conditions (Scenario #4 vs. Scenario #8).

## Findings

- The levees/flood barriers would be effective to protect the properties behind the levees/flood barriers from both the 100-year riverine and coastal floods, except the interior area behind the existing Hillview Gardens levee and the proposed Bon Air Rd Complex flood barrier (see Figures 7 – 10). Examination of the 100-year riverine flood WSE results in the Corte Madera Creek channel and the ground elevations in this interior area indicates that the flood water in this interior area could be drained into the Corte Madera Creek channel by the storm drain pipes. The 100-year coastal flood inundation in this area, which is the result from the assumed 10-year storm flow accompanying the 100-year coastal flood, would not be drained out due to too high tailwater elevations in the Corte Madera Creek channel. A pump station would be needed to drain the 10-year storm flow during a 100-year coastal flood.
- The flood WSE along the Corte Madera Creek earthen channel is dominated by the 100-year coastal flood (see Figures 17 – 20).
- With the levees/flood barriers in place, the benefit of dredging during a 100-year riverine flood would be insignificant in terms of reduction in flood inundation (see Figures 13 and 14).
- The benefit of dredging during a 100-year coastal flood would be minimal (see Figure 16).

### **3. Freeboard Assessment**

Using the results of the hydraulic analysis for project conditions and the FEMA's applicable freeboard requirements, a freeboard assessment was conducted for the proposed levees/flood barriers. Per FEMA CFS65.10, the following requirements will be applicable:

#### Riverine Flood

- A minimum freeboard of three feet above the water-surface level of the base flood;
- An additional one foot above the minimum is required within 100 feet upstream and downstream of structures (such as bridges) riverward of the levee or wherever the flow is constricted;
- An additional one-half foot above the minimum at the upstream end of the levee is also required, tapering to not less than the minimum at the downstream end of the levee;

#### Coastal Flood

- For coastal levees, the freeboard must be established at one foot above the height of the 1-percent wave or maximum wave runoff (whichever is greater) associated with the 100-year stillwater surge elevation at the site.

For the purpose of this analysis, a freeboard of 3 ft for riverine flood and a freeboard of 1 ft for coastal flood were used. This is reasonable because there are no structures/bridges except the high

Bon Air Bridge<sup>2</sup> and the large HWY 101 Bridge, and there would be little wave effect in the study reach due to its small wind fetch and far away location from the bay. Wave runup is normally considered for coastal shorelines, not for tide-affected stream reaches.

Refer to Figure 2 for levee/flood barrier stationing that was used for the freeboard assessment. The ground elevations for the levees/flood barriers used the 2010 LiDAR data except for the Wolfe Grade flood barrier #3, for which the Stetson field-surveyed flood threshold along the Wolfe Grade Creek (see Figure 29) was used.

Figures 21a, 21b, 21c, and 21d graphically present the freeboard assessment for the existing Hillview Gardens levee (Scenarios #1 vs. #5, #2 vs. #6, #3 vs. #7, and #4 vs. #8, respectively).

Figures 22a, 22b, 22c, and 22d graphically present the freeboard assessment for the existing Kentfield Gardens levee.

Figures 23a, 23b, 23c, and 23d graphically present the freeboard assessment for the proposed College Ct flood barrier.

Figures 24a, 24b, 24c, and 24d graphically present the freeboard assessment for the proposed Bon Air Rd Complex flood barrier.

Figures 25a, 25b, 25c, and 25d graphically present the freeboard assessment for the proposed South Eliseo flood barrier.

Figures 26a, 26b, 26c, and 26d graphically present the freeboard assessment for the proposed Wolfe Grade flood barrier #1.

Figures 27a, 27b, 27c, and 27d graphically present the freeboard assessment for the proposed Wolfe Grade flood barrier #2.

Figures 28a, 28b, 28c, and 28d graphically present the freeboard assessment for the proposed Wolfe Grade flood barrier #3. Note that the ground elevations along the Wolfe Grade flood barrier #3 used the Stetson field-surveyed flood threshold along the Wolfe Grade Creek (see Figure 29).

The freeboard assessment of the levees/flood barriers is summarized in Table 2a for the 100-year riverine flood and in Table 2b for the 100-year coastal flood.

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<sup>2</sup> In this modeling, the old Bon Air Bridge was used and was not updated to the proposed new bridge with construction currently being underway by the City of Larkspur. Previous modeling for the new bridge design indicated that the new bridge and the old bridge would have almost the same hydraulics and little backwater effects during the 100-year riverine flood. In other words, using either the old Bon Air Bridge or the new bridge in the modeling would have almost the same WSE profiles for the levee evaluation.

#### 4. Interior Drainage Analysis

As shown in Figures 7 – 10, the interior area behind the existing Hillview Gardens levee and the proposed Bon Air Rd Complex flood barrier would be flooded during the 100-year riverine and coastal floods, with the 100-year coastal flood having more/higher inundation. Examination of the 100-year riverine flood WSE results in the Corte Madera Creek channel and the ground elevations in this interior area indicates that the flood water in this interior area could be drained into the Corte Madera Creek channel by the storm drain pipes. The 100-year coastal flood inundation in this area, which is the result from the assumed 10-year storm flow (primarily from the King Mountain Creek) accompanying the 100-year coastal flood, would not be drained out due to too high tailwater elevations in the Corte Madera Creek channel. A pump station would be needed to drain the 10-year storm flow during a 100-year coastal flood<sup>3</sup>. The pump station being designed by the City of Larkspur for the Bon Air Road just south of the Hillview Subdivision would provide some relief for the flooding in this interior area during a 100-year coastal flood.

The 10-year storm flow has an estimated peak discharge of approximately 110 cfs. It is estimated that a pump station with a capacity of about 50,000 gpm would be needed. The pump intake could be located at the King Mountain Creek discharge marsh near the existing Hillview Gardens levee. The construction cost of the pump station is estimated to be approximately \$4.5 million.

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<sup>3</sup> Any interior stormwater runoff accompanying the 100-year coastal flood would be ponded behind the improved/raised Hillview Gardens levee due to too high tailwater elevations in the Corte Madera Creek channel. If there is no stormwater runoff during a 100-year coastal flood, there would be no flood inundation behind the improved/raised Hillview Gardens levee that is designed to prevent overtopping from the Corte Madera Creek channel.

**Table 1a 100-Year Riverine Modeling**

Scenario	Channel Bed Condition	Creek Improvements	Upstream Inflow	Downstream Tidal Boundary Condition
Scenario #1	Existing Condition (2018)	<ul style="list-style-type: none"> <li>USACE’s Alternative J</li> <li>Proposed Lower CMC Flood Barriers and Tidal Gates</li> </ul>	Q100	Current MHHW (5.9 ft NAVD88)
Scenario #2				Future MHHW (7.0 ft NAVD88) with SLR (1.1 ft) by 2050
Scenario #3	Phase 1 + 2 Dredging			Current MHHW (5.9 ft NAVD88)
Scenario #4				Future MHHW (7.0 ft NAVD88) with SLR (1.1 ft) by 2050

**Table 1b 100-Year Coastal Modeling**

Scenario	Channel Bed Condition	Creek Improvements	Upstream Inflow	Downstream Tidal Boundary Condition
Scenario #5	Existing Condition (2018)	<ul style="list-style-type: none"> <li>USACE’s Alternative J</li> <li>Proposed Lower CMC Flood Barriers and Tidal Gates</li> </ul>	Q10	Current 100-Yr Tide (10 ft NAVD88)
Scenario #6				Future 100-Yr Tide (11.1 ft NAVD88) with SLR (1.1 ft) by 2050
Scenario #7	Phase 1 + 2 Dredging			Current 100-Yr Tide (10 ft NAVD88)
Scenario #8				Future 100-Yr Tide (11.1 ft NAVD88) with SLR (1.1 ft) by 2050

1) Source of current 100-year tide: FEMA effective 2017 FIS.

2) Source of sea level rise (SLR): California Natural Resources Agency: *State of California Sea-Level Rise Guidance, 2018 Update*.

**Table 2a Levee/Flood Barrier Height and Freeboard Assessment – Riverine 100-Year Flood**

<b>Levee/ Flood Barrier</b>	<b>Deficiency</b>	<b>Scenario #1</b> (Riverine 100-yr, no dredge, no SLR)	<b>Scenario #2</b> (Riverine 100-yr, no dredge, with SLR)	<b>Scenario #3</b> (Riverine 100-yr, with dredge, no SLR)	<b>Scenario #4</b> (Riverine 100-yr, with dredge, with SLR)
<b>Existing Levee – Hillview Gardens Levee</b>	Height Deficiency	Small portion; up to 2 ft	Small portion; up to 2.5 ft	Small portion; up to 1.2 ft	Small portion; up to 1.8 ft
	Freeboard Deficiency	Most portion; up to 5 ft	Most portion; up to 5.5 ft	Most portion; up to 4.2 ft	Most portion; up to 4.8 ft
<b>Existing Levee – Kentfield Gardens Levee</b>	Height Deficiency	None	Small portion; up to 0.2 ft	None	Small portion; up to 0.4 ft
	Freeboard Deficiency	Most portion; up to 2.3 ft	Most portion; up to 3.2 ft	Most portion; up to 2.9 ft	Most portion; up to 3.4 ft
<b>College Ct Flood Barrier</b>	Height Deficiency	Entire reach; up to 4.5 ft	Entire reach; up to 4.7 ft	Entire reach; up to 4.0 ft	Entire reach; up to 4.2 ft
	Freeboard Deficiency	Entire reach; up to 7.5 ft	Entire reach; up to 7.7 ft	Entire reach; up to 7.0 ft	Entire reach; up to 7.2 ft
<b>Bon Air Rd Complex Flood Barrier</b>	Height Deficiency	None	Small portion; up to 0.3 ft	None	Most portion; up to 0.3 ft
	Freeboard Deficiency	Entire reach; up to 2.5 ft	Small portion; up to 3.3 ft	Entire reach; up to 2.5 ft	Small portion; up to 3.3 ft
<b>South Eliseo Flood Barrier</b>	Height Deficiency	Small portion; up to 0.2 ft	Half portion; up to 1.1 ft	Small portion; up to 0.2 ft	Half portion; up to 1.1 ft
	Freeboard Deficiency	Most portion; up to 3.2 ft	Most portion; up to 4.1 ft	Most portion; up to 3.2 ft	Most portion; up to 4.1 ft
<b>Wolfe Grade Flood Barrier #1</b>	Height Deficiency	Small portion; up to 0.6 ft	1/3 portion; up to 1.0 ft	Small portion; up to 0.2 ft	Small portion; up to 0.7 ft
	Freeboard Deficiency	Entire reach; up to 3.6 ft	Entire reach; up to 4.0 ft	Entire reach; up to 3.2 ft	Entire reach; up to 3.7 ft
<b>Wolfe Grade Flood Barrier #2</b>	Height Deficiency	Most portion; up to 0.9 ft	Most portion; up to 1.3 ft	Most portion; up to 0.6 ft	Most portion; up to 1.0 ft
	Freeboard Deficiency	Entire reach; up to 3.9 ft	Entire reach; up to 4.3 ft	Entire reach; up to 3.6 ft	Entire reach; up to 4.0 ft
<b>Wolfe Grade Flood Barrier #3</b>	Height Deficiency	Small portion; up to 0.7 ft	Half portion; up to 1.2 ft	Small portion; up to 0.3 ft	Half portion; up to 0.8 ft
	Freeboard Deficiency	Entire reach; up to 3.7 ft	Entire reach; up to 4.2 ft	Entire reach; up to 3.3 ft	Entire reach; up to 3.8 ft

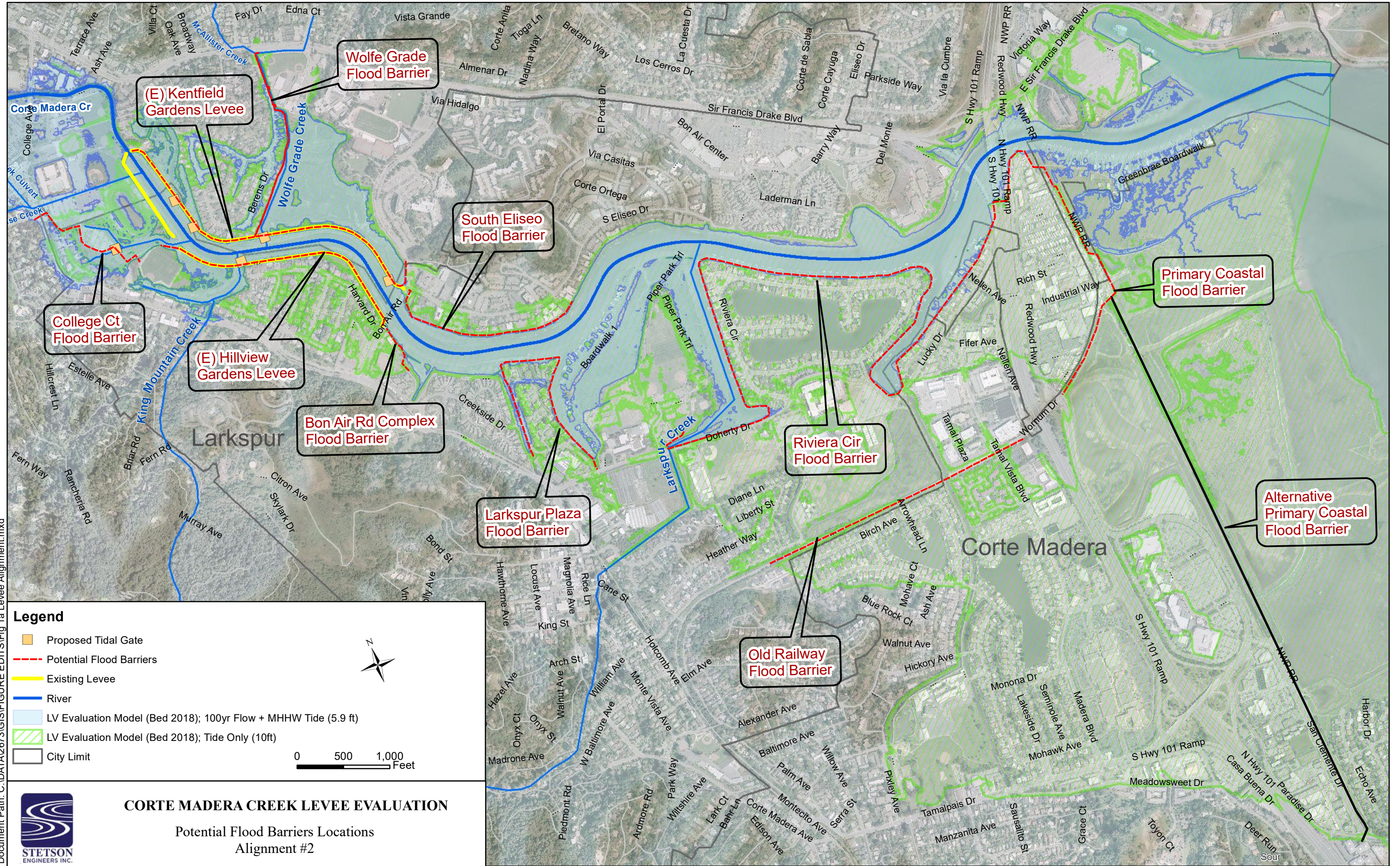
Elevation datum: ft NAVD88; Required freeboard for riverine flood: 3 ft.



**Table 2b Levee/Flood Barrier Height and Freeboard Assessment – Coastal 100-Year Flood**

<b>Levee/ Flood Barrier</b>	<b>Deficiency</b>	<b>Scenario #5</b> (Coastal 100-yr, no dredge, no SLR)	<b>Scenario #6</b> (Coastal 100-yr, no dredge, with SLR)	<b>Scenario #7</b> (Coastal 100-yr, with dredge, no SLR)	<b>Scenario #8</b> (Coastal 100-yr, with dredge, with SLR)
<b>Existing Levee – Hillview Gardens Levee</b>	Height Deficiency	1/3 portion; up to 3.5 ft	Most portion; up to 4.5 ft	Small portion; up to 3.2 ft	Most portion; up to 4.2 ft
	Freeboard Deficiency	Most portion; up to 4.5 ft	Entire reach; up to 5.5 ft	Most portion; up to 4.2 ft	Entire reach; up to 5.2 ft
<b>Existing Levee – Kentfield Gardens Levee</b>	Height Deficiency	1/4 portion; up to 2 ft	Most portion; up to 3 ft	1/4 portion; up to 2 ft	Most portion; up to 3 ft
	Freeboard Deficiency	Most portion; up to 3 ft	Entire reach; up to 4 ft	Most portion; up to 3 ft	Entire reach; up to 4 ft
<b>College Ct Flood Barrier</b>	Height Deficiency	Most portion; up to 4.8 ft	Entire reach; up to 5.8 ft	Most portion; up to 4.5 ft	Entire reach; up to 5.5 ft
	Freeboard Deficiency	Entire reach; up to 5.8 ft	Entire reach; up to 6.8 ft	Entire reach; up to 5.5 ft	Entire reach; up to 6.5 ft
<b>Bon Air Rd Complex Flood Barrier</b>	Height Deficiency	Most portion; up to 2 ft	Most portion; up to 3 ft	Most portion; up to 2 ft	Most portion; up to 3 ft
	Freeboard Deficiency	Most portion; up to 3 ft	Entire reach; up to 4 ft	Most portion; up to 3 ft	Entire reach; up to 4 ft
<b>South Eliseo Flood Barrier</b>	Height Deficiency	Most portion; up to 3.4 ft	Entire reach; up to 4.4 ft	Most portion; up to 3.4 ft	Entire reach; up to 4.4 ft
	Freeboard Deficiency	Entire reach; up to 4.4 ft	Entire reach; up to 5.4 ft	Entire reach; up to 4.4 ft	Entire reach; up to 5.4 ft
<b>Wolfe Grade Flood Barrier #1</b>	Height Deficiency	Most portion; up to 2.1 ft	Entire reach; up to 3.1 ft	Most portion; up to 1.9 ft	Entire reach; up to 2.9 ft
	Freeboard Deficiency	Entire reach; up to 3.1 ft	Entire reach; up to 4.1 ft	Entire reach; up to 2.9 ft	Entire reach; up to 3.9 ft
<b>Wolfe Grade Flood Barrier #2</b>	Height Deficiency	Entire reach; up to 2.6 ft	Entire reach; up to 3.5 ft	Entire reach; up to 2.3 ft	Entire reach; up to 3.3 ft
	Freeboard Deficiency	Entire reach; up to 3.6 ft	Entire reach; up to 4.5 ft	Entire reach; up to 3.3 ft	Entire reach; up to 4.3 ft
<b>Wolfe Grade Flood Barrier #3</b>	Height Deficiency	Most portion; up to 2.2 ft	Entire reach; up to 3.1 ft	Most portion; up to 2.1 ft	Entire reach; up to 3.1 ft
	Freeboard Deficiency	Entire reach; up to 3.2 ft	Entire reach; up to 4.1 ft	Entire reach; up to 3.1 ft	Entire reach; up to 4.1 ft

Elevation datum: ft NAVD88. Required freeboard for coastal flood: 1 ft.



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**Legend**

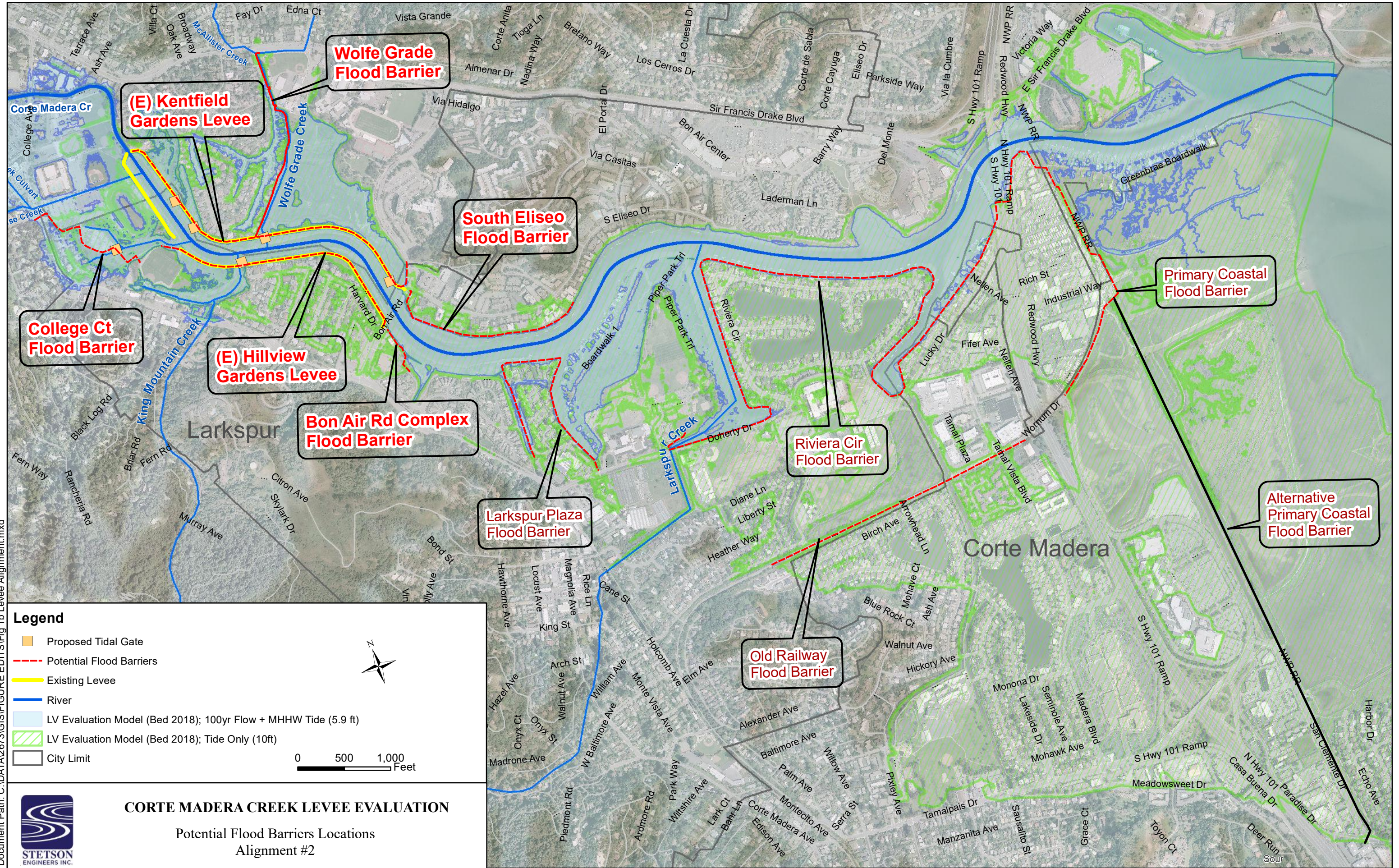
- Proposed Tidal Gate
- Potential Flood Barriers
- Existing Levee
- River
- LV Evaluation Model (Bed 2018); 100yr Flow + MHHW Tide (5.9 ft)
- LV Evaluation Model (Bed 2018); Tide Only (10ft)
- City Limit

0 500 1,000 Feet

N

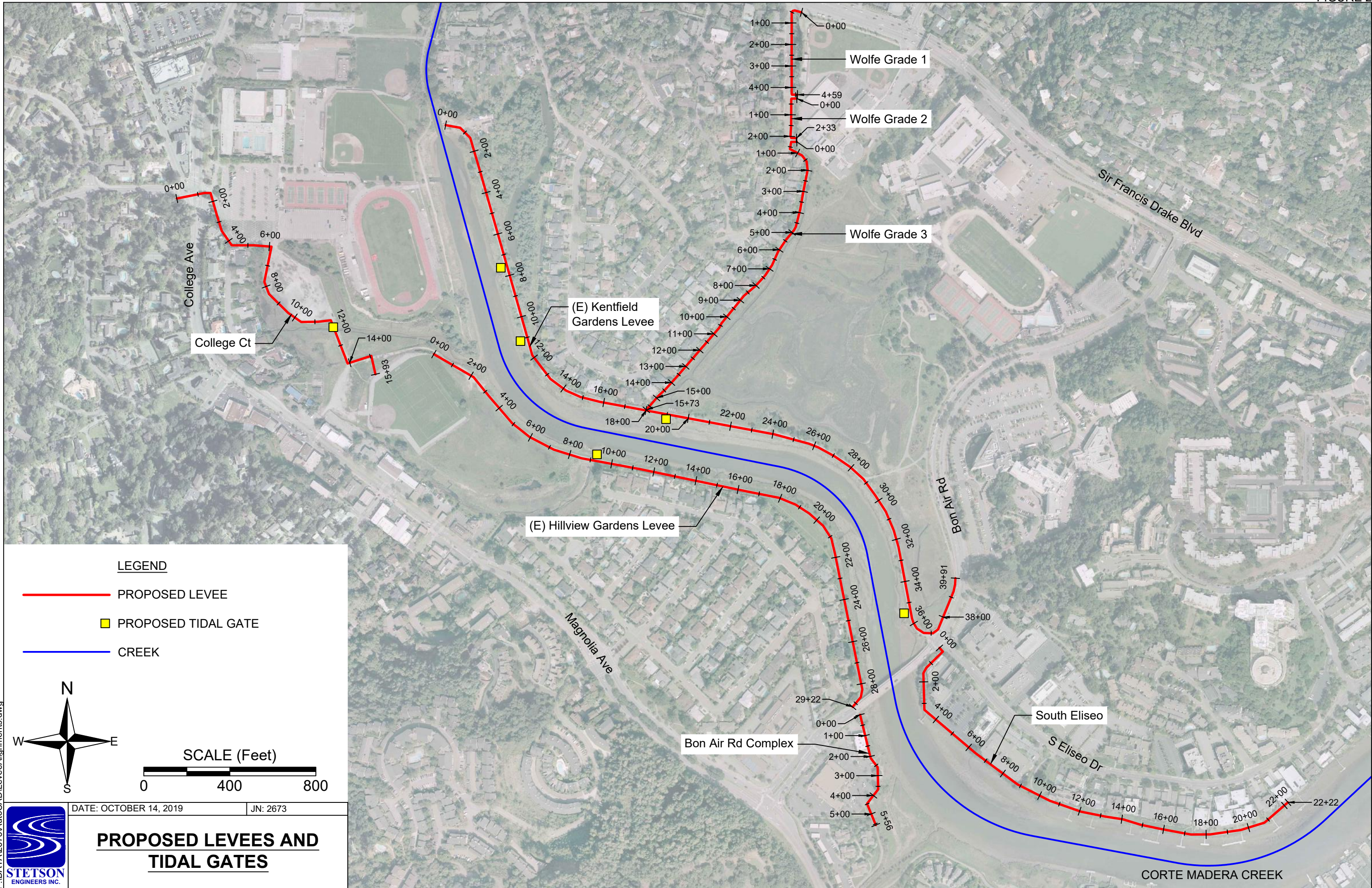


**CORTE MADERA CREEK LEVEE EVALUATION**  
 Potential Flood Barriers Locations  
 Alignment #2

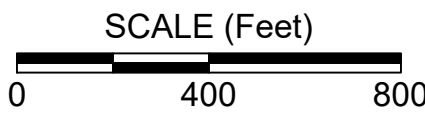
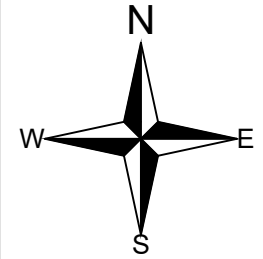


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DATE: OCTOBER 14, 2019 | JN: 2673



### PROPOSED LEVEES AND TIDAL GATES

Figure 3

### HEC-RAS Simulated 100-Year WSE of Localized Dredging of Corte Madera Creek Earthen Channel and Estimated Dredging Volume

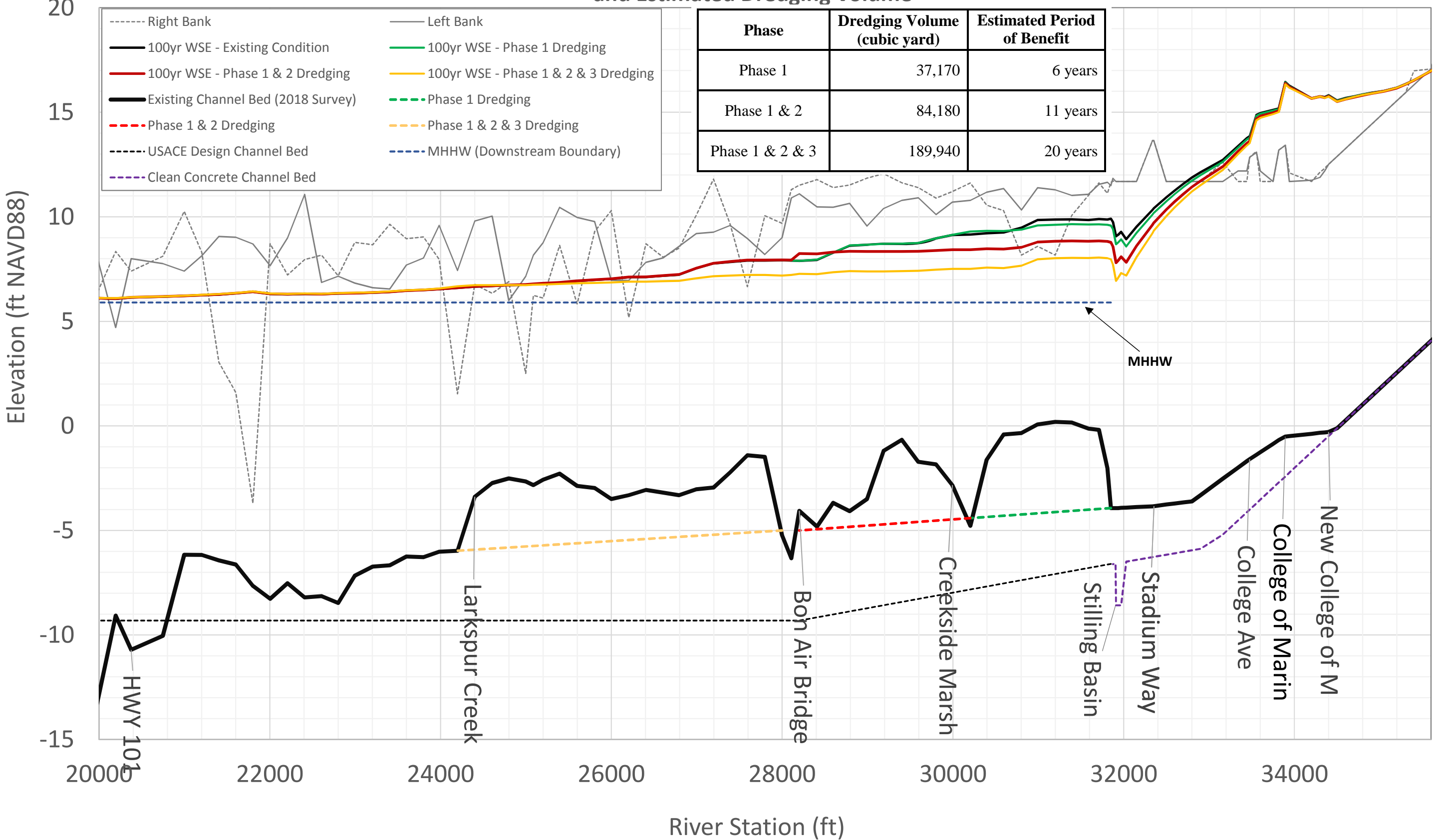
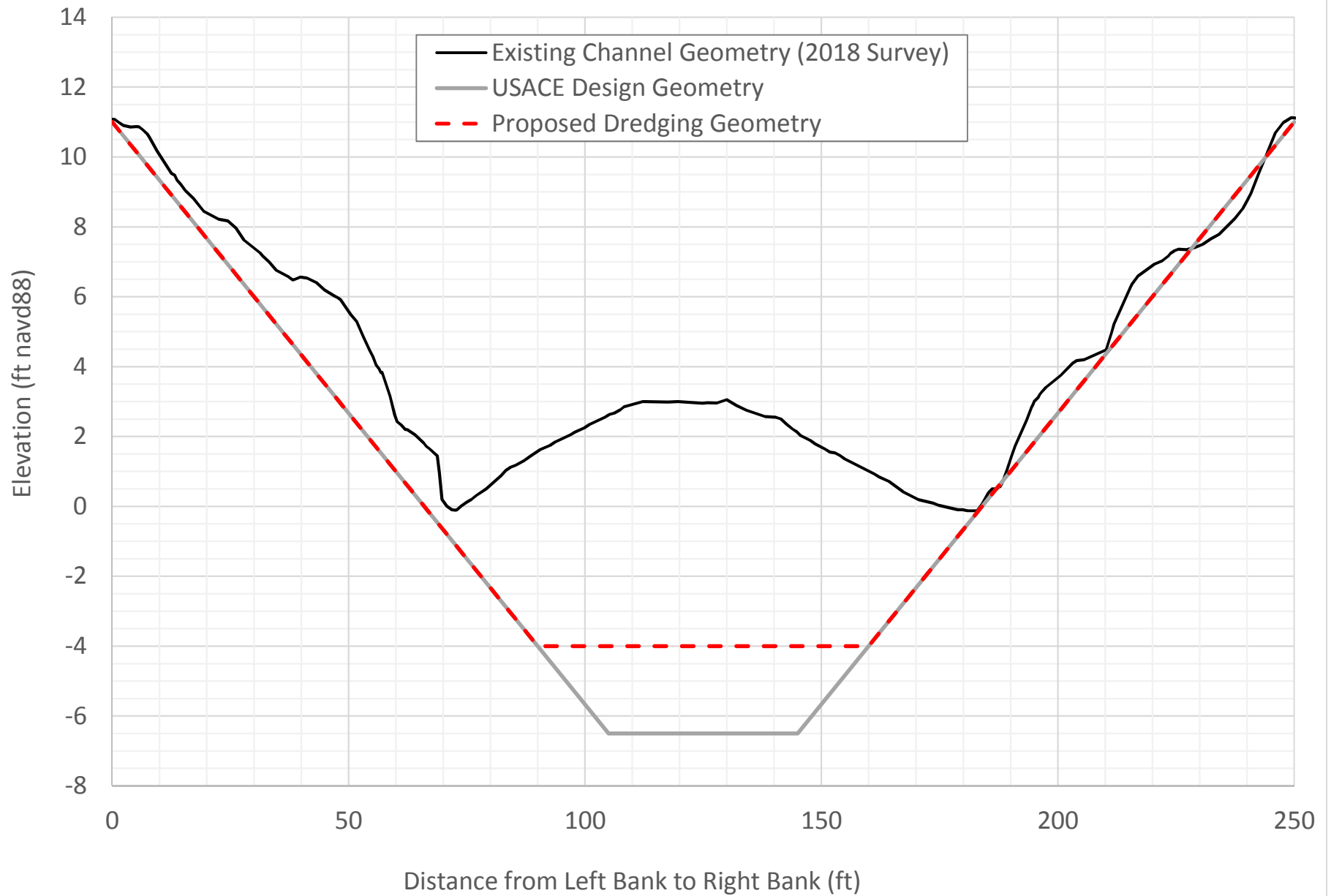
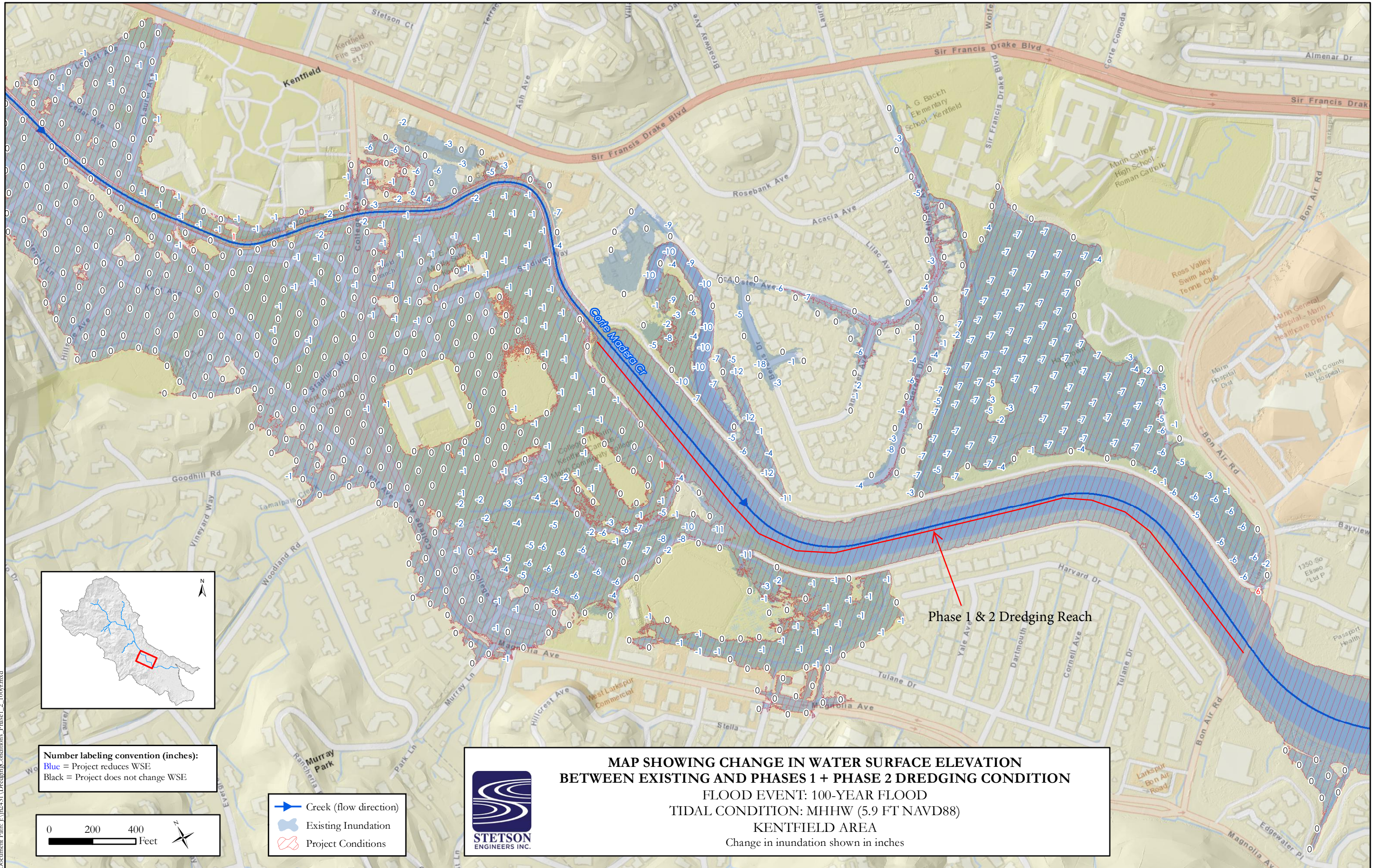


Figure 4 Proposed Earthen Channel Dredging at Typical Cross-Section (XS 31587)



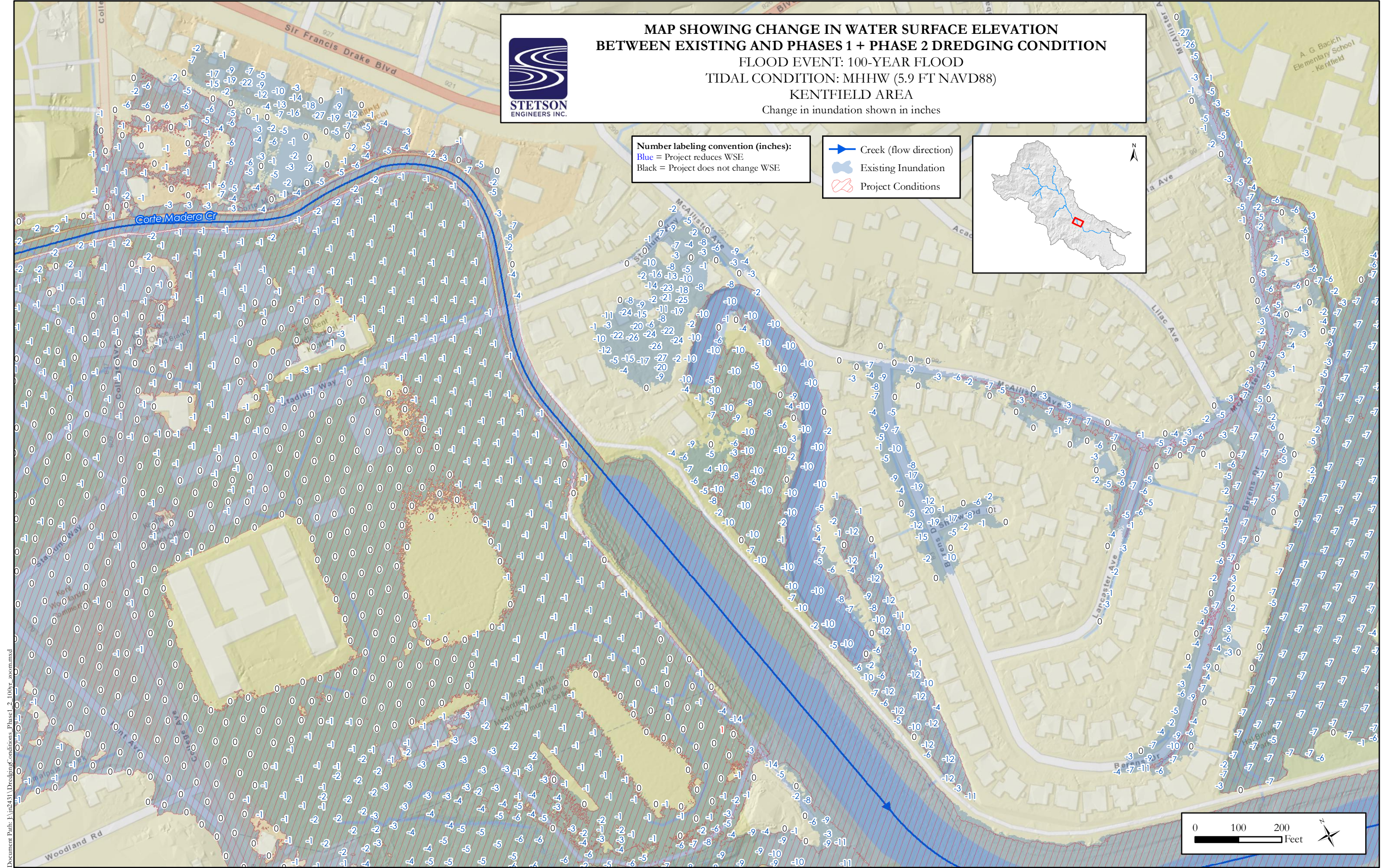


**MAP SHOWING CHANGE IN WATER SURFACE ELEVATION  
BETWEEN EXISTING AND PHASES 1 + PHASE 2 DREDGING CONDITION**  
 FLOOD EVENT: 100-YEAR FLOOD  
 TIDAL CONDITION: MHHW (5.9 FT NAVD88)  
 KENTFIELD AREA  
 Change in inundation shown in inches



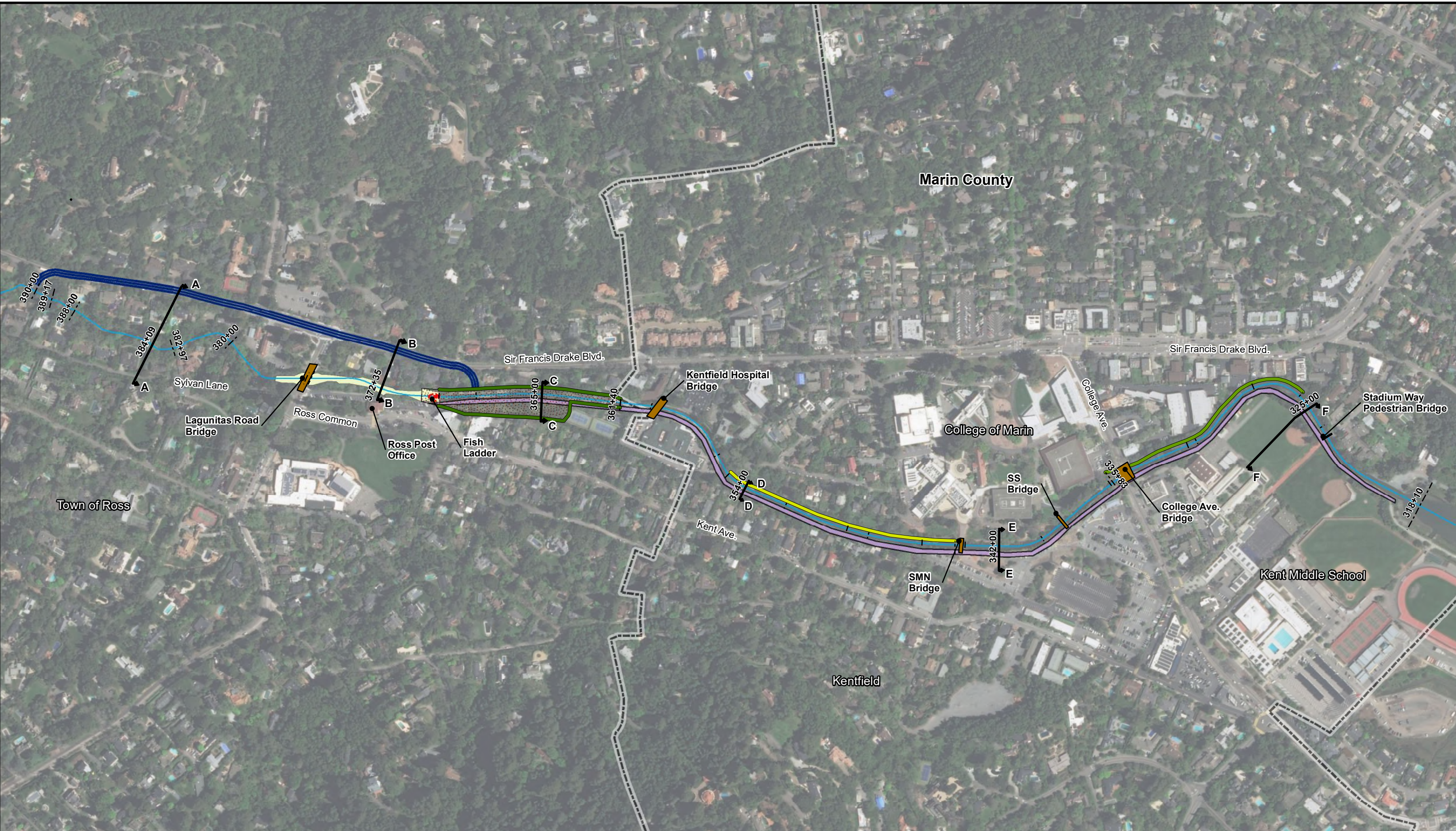
**Number labeling convention (inches):**  
 Blue = Project reduces WSE  
 Black = Project does not change WSE

— Creek (flow direction)  
 Existing Inundation  
 Project Conditions



Document Path: E:\12431 Dredging Conditions Phase 1\_2\_100yr\_zoom.mxd





**Alternative J**

- Underground Bypass
- - - Fish Ladder Removal
- Allen Park Riparian Corridor
- Fish Passage Transition Grading

**Maximum Top of Bank Floodwall Heights (feet)**

- 2
- 4
- 6

**Existing Features**

- Corte Madera Creek Centerline
- Bridges
- Existing Bike Lane
- Channel Stations
- Cross-Section Location

Date: 7/26/2018

Document Path: S:\Corte Madera EIS-EIR\GIS 2017 Files\Corte\_MaderaAlts\_11\_21\_17\CM\_Alternative F\_Current\_Overview.mxd

**Alternative J**  
**Figure 6 Overview**  
 Corte Madera Creek FRM EIS/EIR  
 Ross, Marin County

**Burleson Consulting, Inc.**

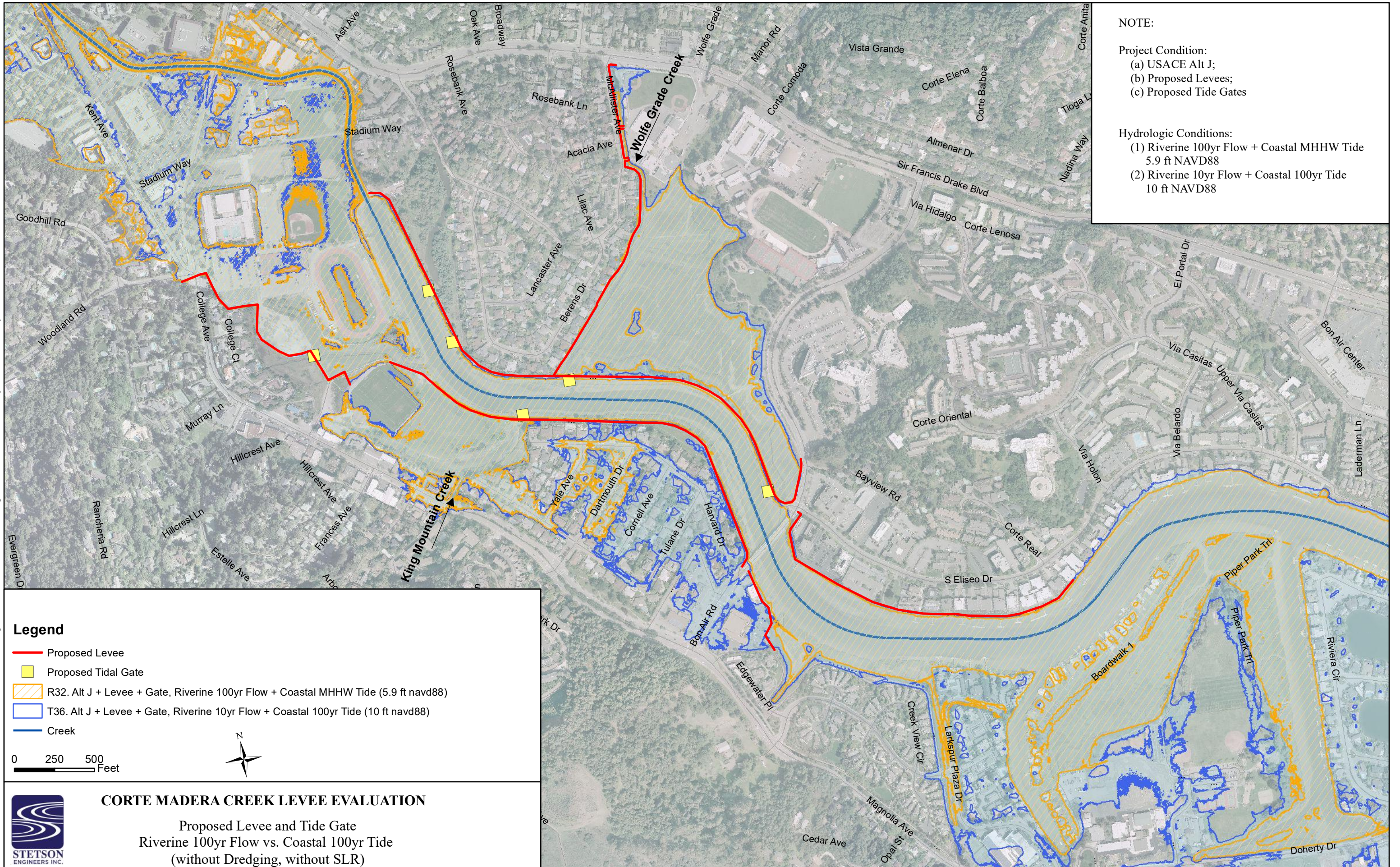
NOTE:

Project Condition:

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates

Hydrologic Conditions:

- (1) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88
- (2) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88



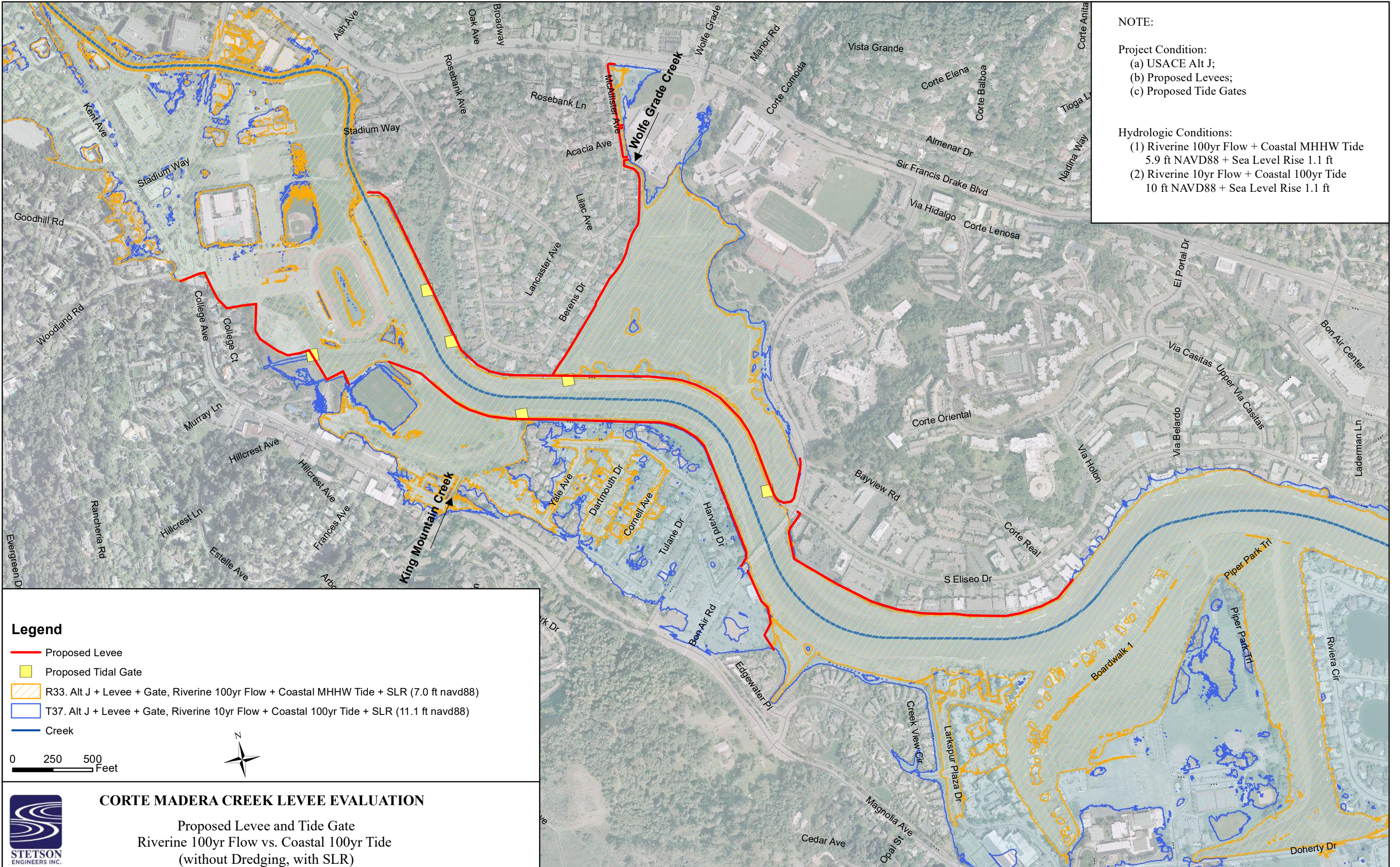
NOTE:

Project Condition:

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates

Hydrologic Conditions:

- (1) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88 + Sea Level Rise 1.1 ft
- (2) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88 + Sea Level Rise 1.1 ft



**Legend**

- Proposed Levee
- Proposed Tidal Gate
- R33. Alt J + Levee + Gate, Riverine 100yr Flow + Coastal MHHW Tide + SLR (7.0 ft navd88)
- T37. Alt J + Levee + Gate, Riverine 10yr Flow + Coastal 100yr Tide + SLR (11.1 ft navd88)
- Creek

0 250 500 Feet



**CORTE MADERA CREEK LEVEE EVALUATION**

Proposed Levee and Tide Gate  
Riverine 100yr Flow vs. Coastal 100yr Tide  
(without Dredging, with SLR)

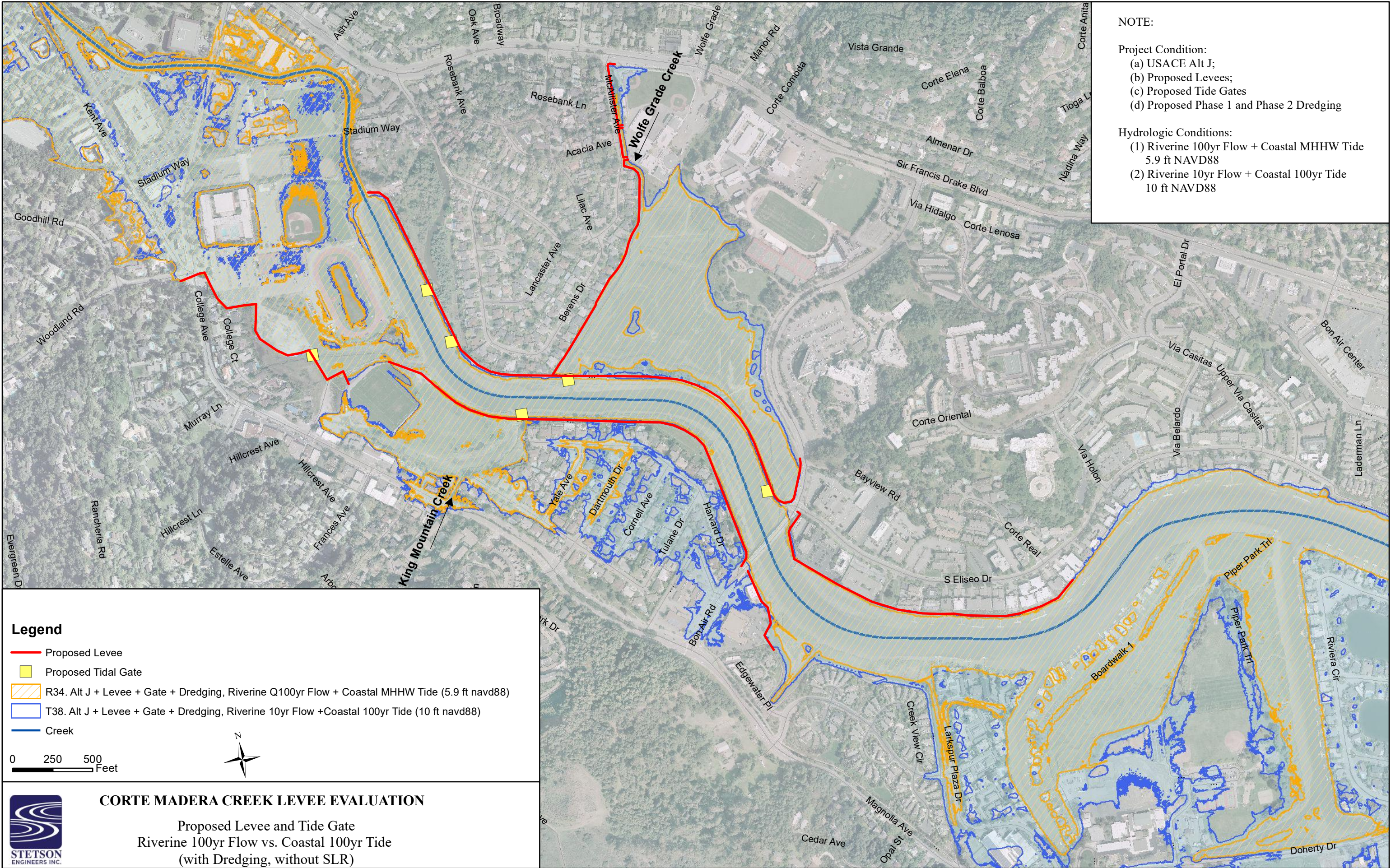
**NOTE:**

**Project Condition:**

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates
- (d) Proposed Phase 1 and Phase 2 Dredging

**Hydrologic Conditions:**

- (1) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88
- (2) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88



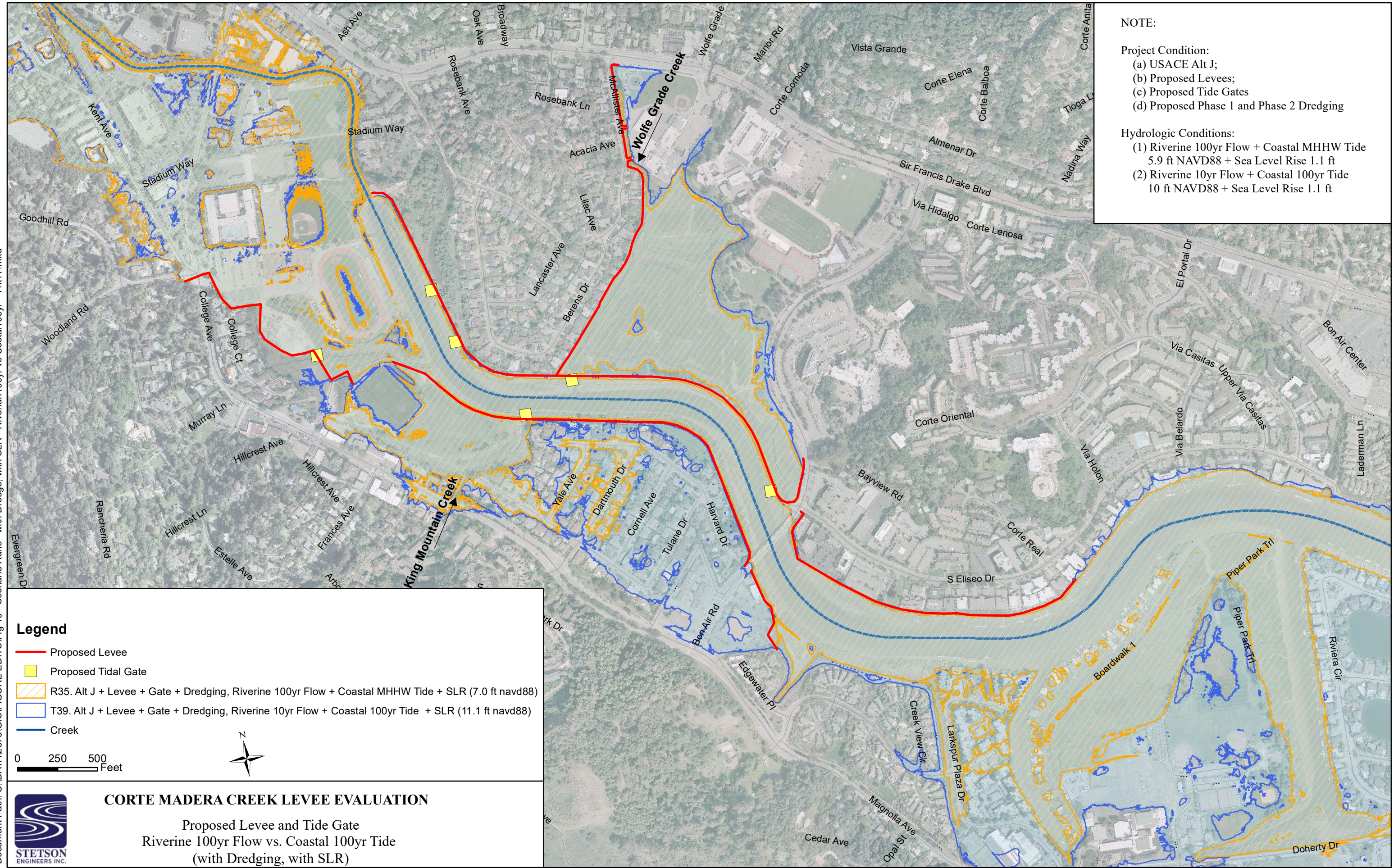
**NOTE:**

**Project Condition:**

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates
- (d) Proposed Phase 1 and Phase 2 Dredging

**Hydrologic Conditions:**

- (1) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88 + Sea Level Rise 1.1 ft
- (2) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88 + Sea Level Rise 1.1 ft



**Legend**

- Proposed Levee
- Proposed Tidal Gate
- R35. Alt J + Levee + Gate + Dredging, Riverine 100yr Flow + Coastal MHHW Tide + SLR (7.0 ft navd88)
- T39. Alt J + Levee + Gate + Dredging, Riverine 10yr Flow + Coastal 100yr Tide + SLR (11.1 ft navd88)
- Creek

0 250 500 Feet



**CORTE MADERA CREEK LEVEE EVALUATION**

Proposed Levee and Tide Gate  
Riverine 100yr Flow vs. Coastal 100yr Tide  
(with Dredging, with SLR)

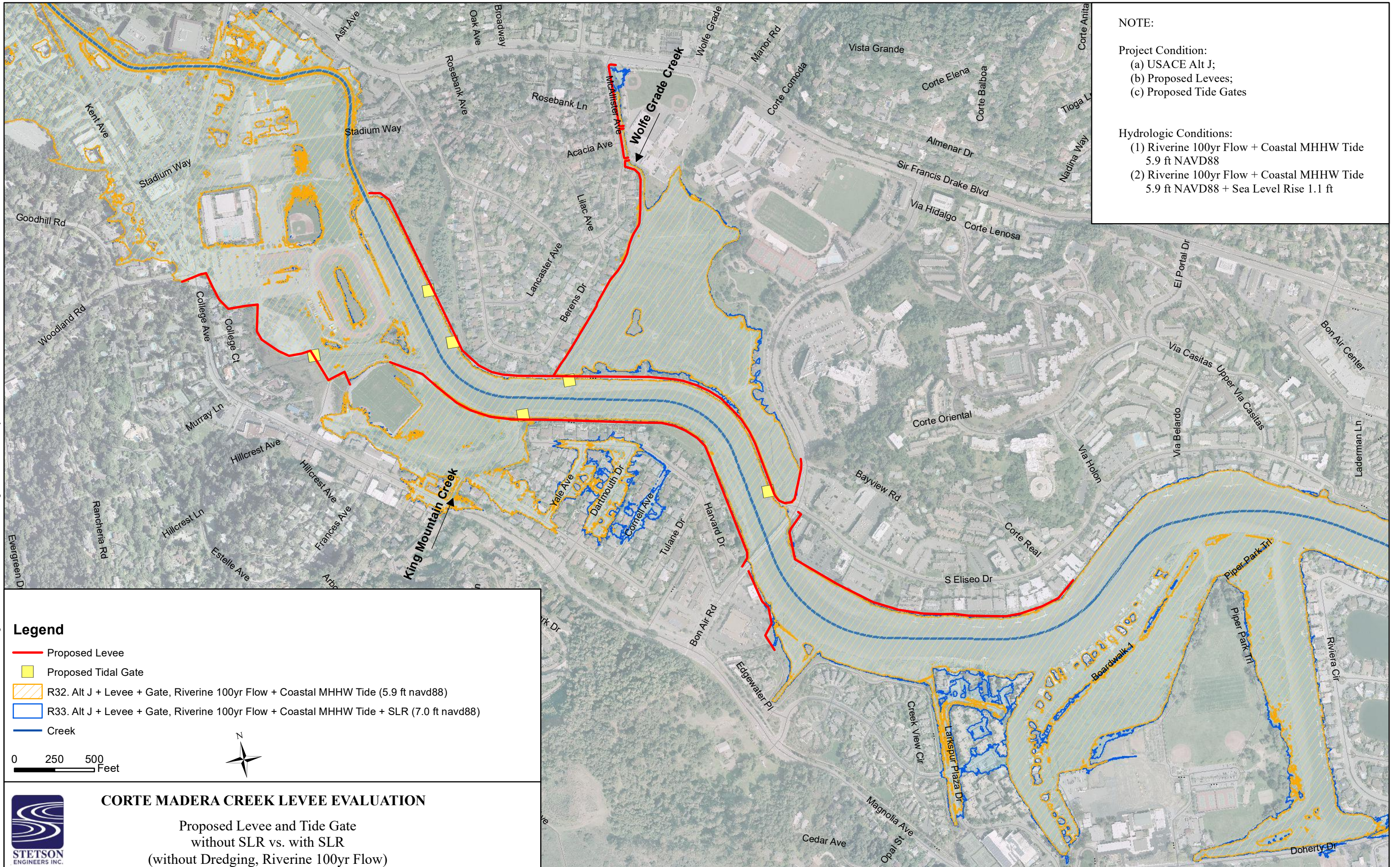
NOTE:

Project Condition:

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates

Hydrologic Conditions:

- (1) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88
- (2) Riverine 100yr Flow + Coastal MHHW Tide  
5.9 ft NAVD88 + Sea Level Rise 1.1 ft



Legend

- Proposed Levee
- Proposed Tidal Gate
- R32. Alt J + Levee + Gate, Riverine 100yr Flow + Coastal MHHW Tide (5.9 ft navd88)
- R33. Alt J + Levee + Gate, Riverine 100yr Flow + Coastal MHHW Tide + SLR (7.0 ft navd88)
- Creek

0 250 500 Feet



**CORTE MADERA CREEK LEVEE EVALUATION**

Proposed Levee and Tide Gate  
without SLR vs. with SLR  
(without Dredging, Riverine 100yr Flow)

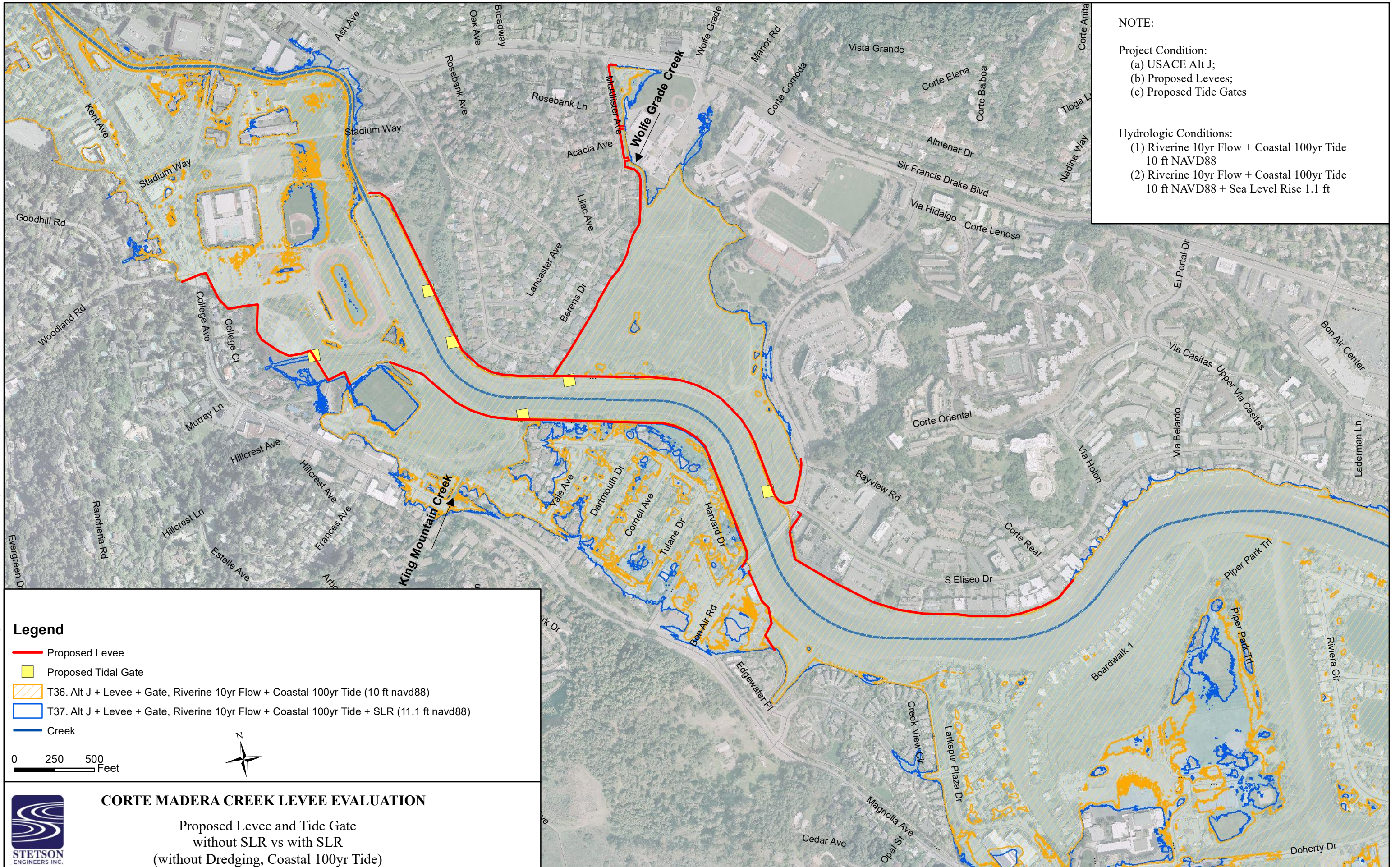
NOTE:

Project Condition:

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates

Hydrologic Conditions:

- (1) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88
- (2) Riverine 10yr Flow + Coastal 100yr Tide  
10 ft NAVD88 + Sea Level Rise 1.1 ft



**Legend**

- Proposed Levee
- Proposed Tidal Gate
- T36. Alt J + Levee + Gate, Riverine 10yr Flow + Coastal 100yr Tide (10 ft navd88)
- T37. Alt J + Levee + Gate, Riverine 10yr Flow + Coastal 100yr Tide + SLR (11.1 ft navd88)
- Creek

0 250 500 Feet



**CORTE MADERA CREEK LEVEE EVALUATION**

Proposed Levee and Tide Gate  
without SLR vs with SLR  
(without Dredging, Coastal 100yr Tide)

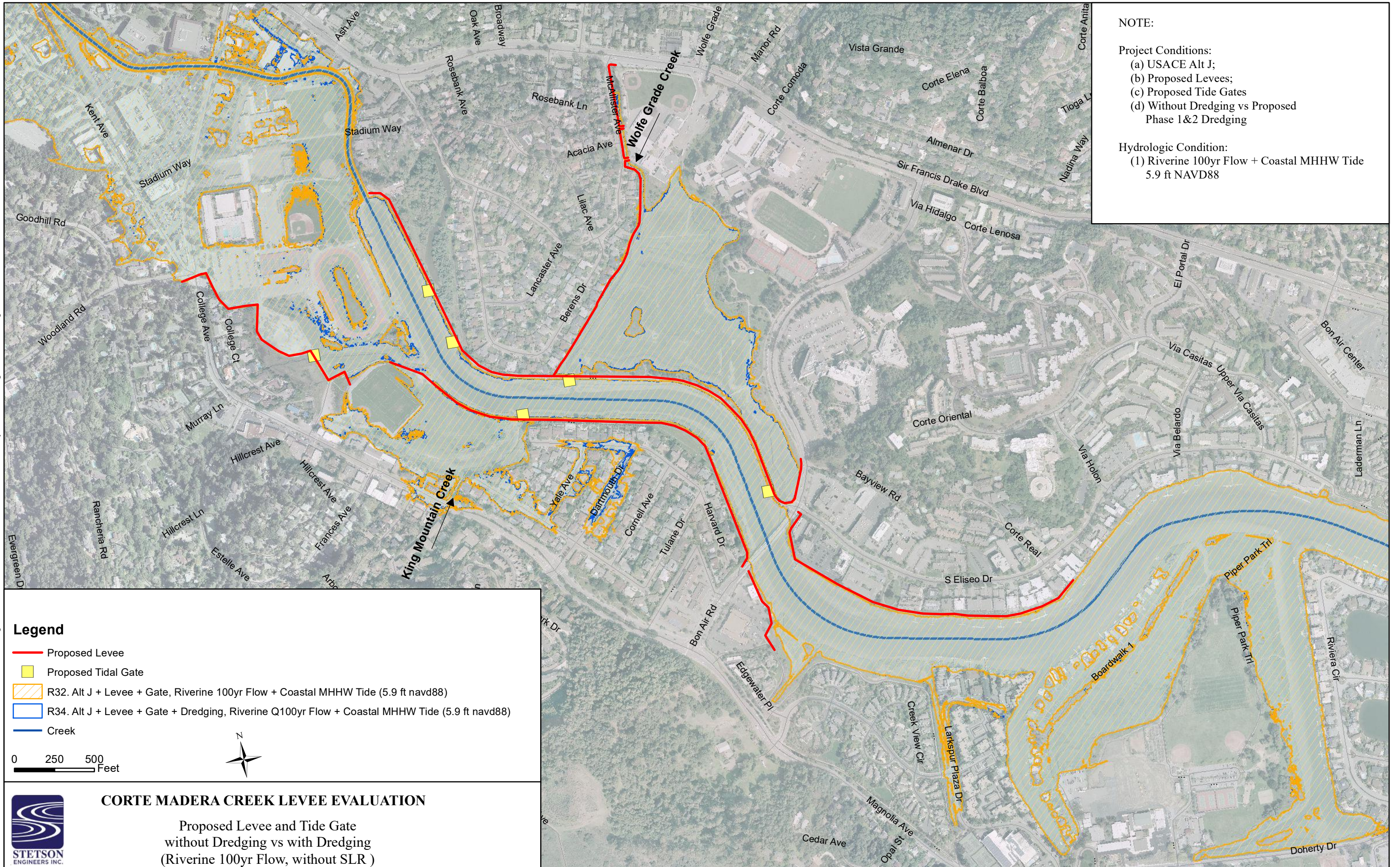
NOTE:

Project Conditions:

- (a) USACE Alt J;
- (b) Proposed Levees;
- (c) Proposed Tide Gates
- (d) Without Dredging vs Proposed Phase 1&2 Dredging

Hydrologic Condition:

- (1) Riverine 100yr Flow + Coastal MHHW Tide 5.9 ft NAVD88



**Legend**

- Proposed Levee
- Proposed Tidal Gate
- R32. Alt J + Levee + Gate, Riverine 100yr Flow + Coastal MHHW Tide (5.9 ft navd88)
- R34. Alt J + Levee + Gate + Dredging, Riverine Q100yr Flow + Coastal MHHW Tide (5.9 ft navd88)
- Creek

0 250 500 Feet



**CORTE MADERA CREEK LEVEE EVALUATION**

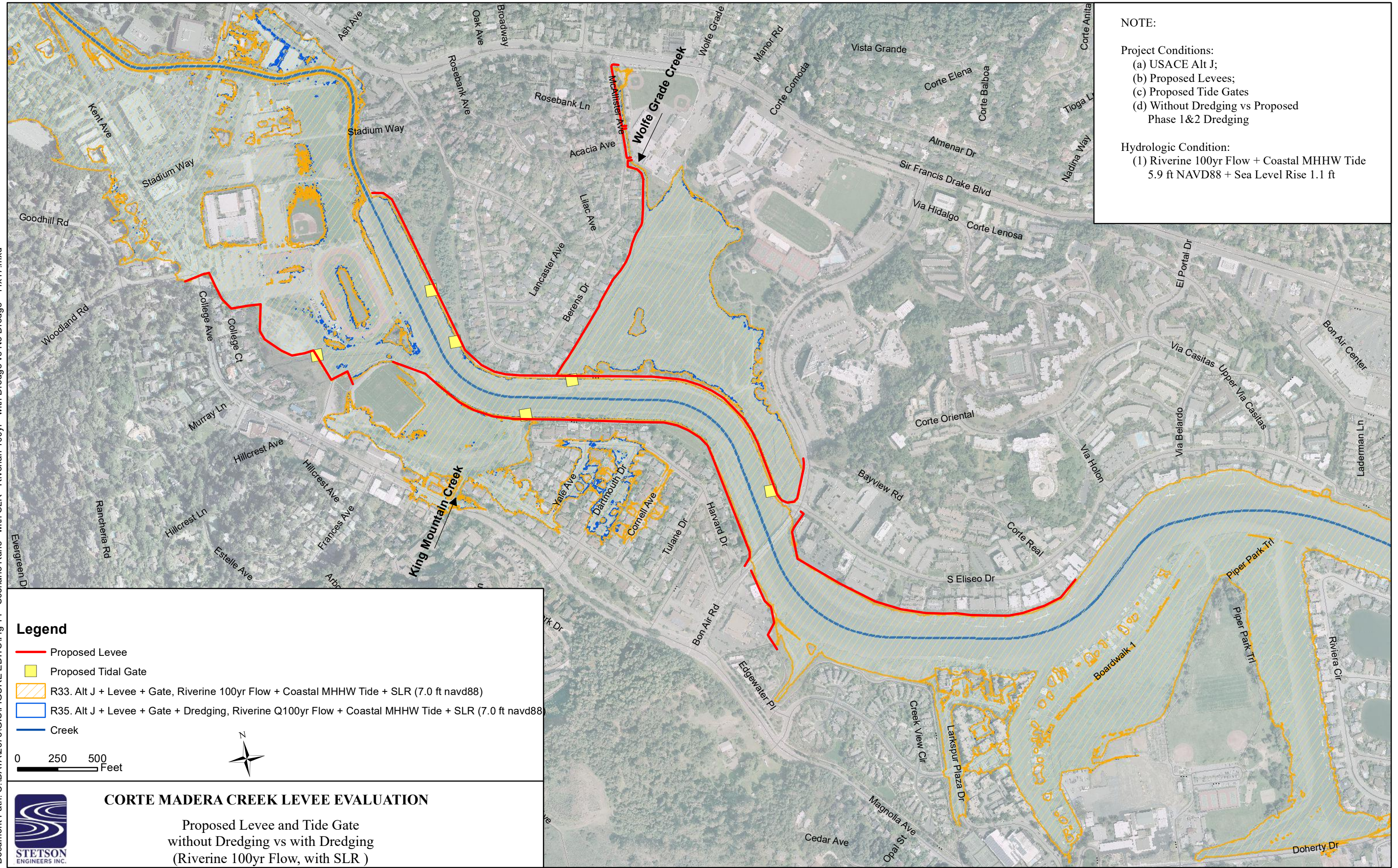
Proposed Levee and Tide Gate  
without Dredging vs with Dredging  
(Riverine 100yr Flow, without SLR)



NOTE:

Project Conditions:  
 (a) USACE Alt J;  
 (b) Proposed Levees;  
 (c) Proposed Tide Gates  
 (d) Without Dredging vs Proposed Phase 1&2 Dredging

Hydrologic Condition:  
 (1) Riverine 100yr Flow + Coastal MHHW Tide  
 5.9 ft NAVD88 + Sea Level Rise 1.1 ft



Document Path: C:\DATA\2673\GIS\FIGURE EDITS\Fig 14 - Scenario Runs - with SLR - Riverine 100yr - with Dredge vs No Dredge - 11x17.mxd

**Legend**

- Proposed Levee
- Proposed Tidal Gate
- R33. Alt J + Levee + Gate, Riverine 100yr Flow + Coastal MHHW Tide + SLR (7.0 ft navd88)
- R35. Alt J + Levee + Gate + Dredging, Riverine Q100yr Flow + Coastal MHHW Tide + SLR (7.0 ft navd88)
- Creek

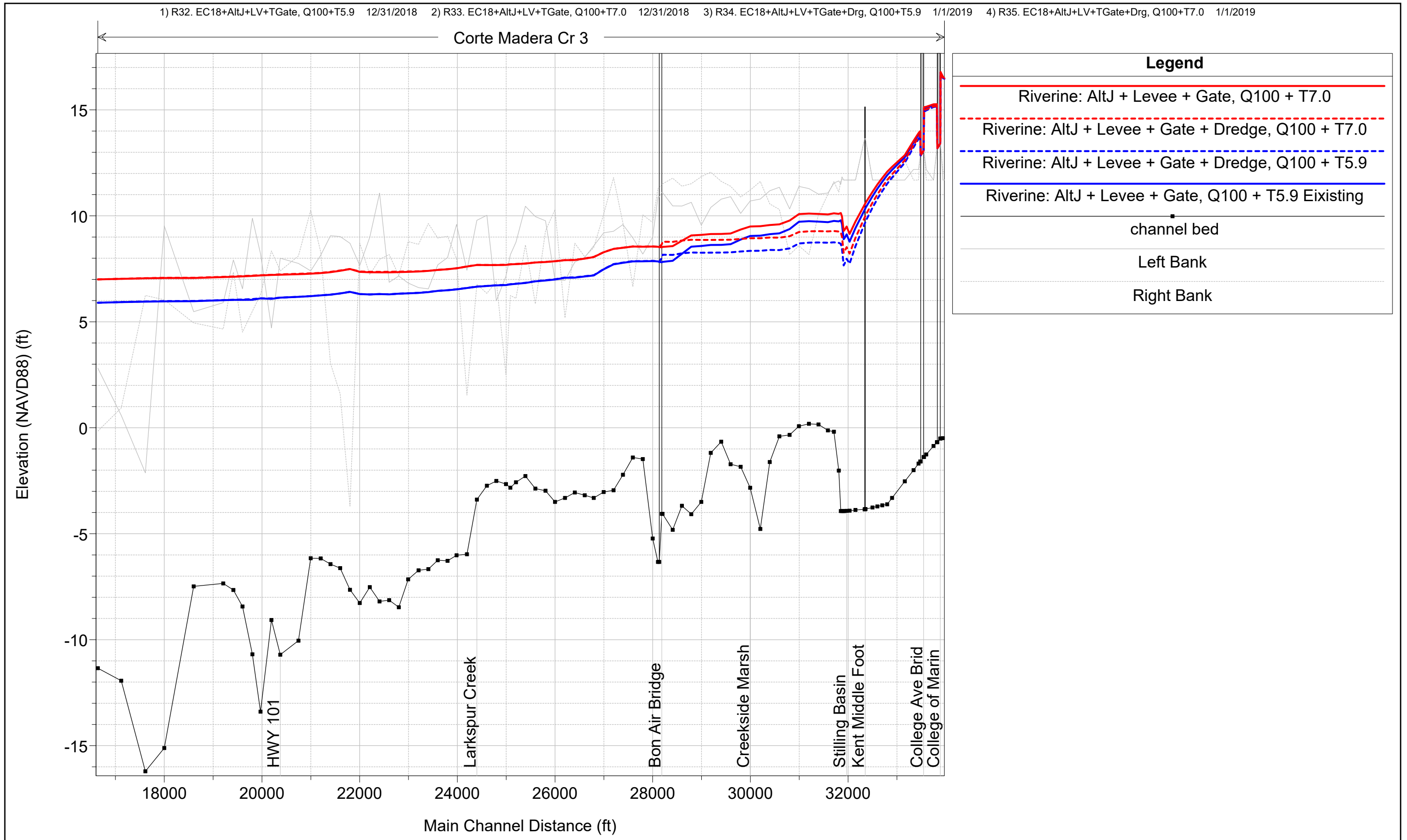
0 250 500 Feet

**STETSON ENGINEERS INC.**

**CORTE MADERA CREEK LEVEE EVALUATION**

Proposed Levee and Tide Gate  
 without Dredging vs with Dredging  
 (Riverine 100yr Flow, with SLR )

**Figure 15 Riverine 100-Year WSE Profiles; Scenarios #1, #2, #3, and #4 (solid blue, dashed blue, solid red, dashed red respectively)**



**Figure 16 Coastal 100-Year WSE Profiles; Scenarios #5, #6, #7, and #8 (solid blue, dashed blue, solid red, dashed red respectively)**

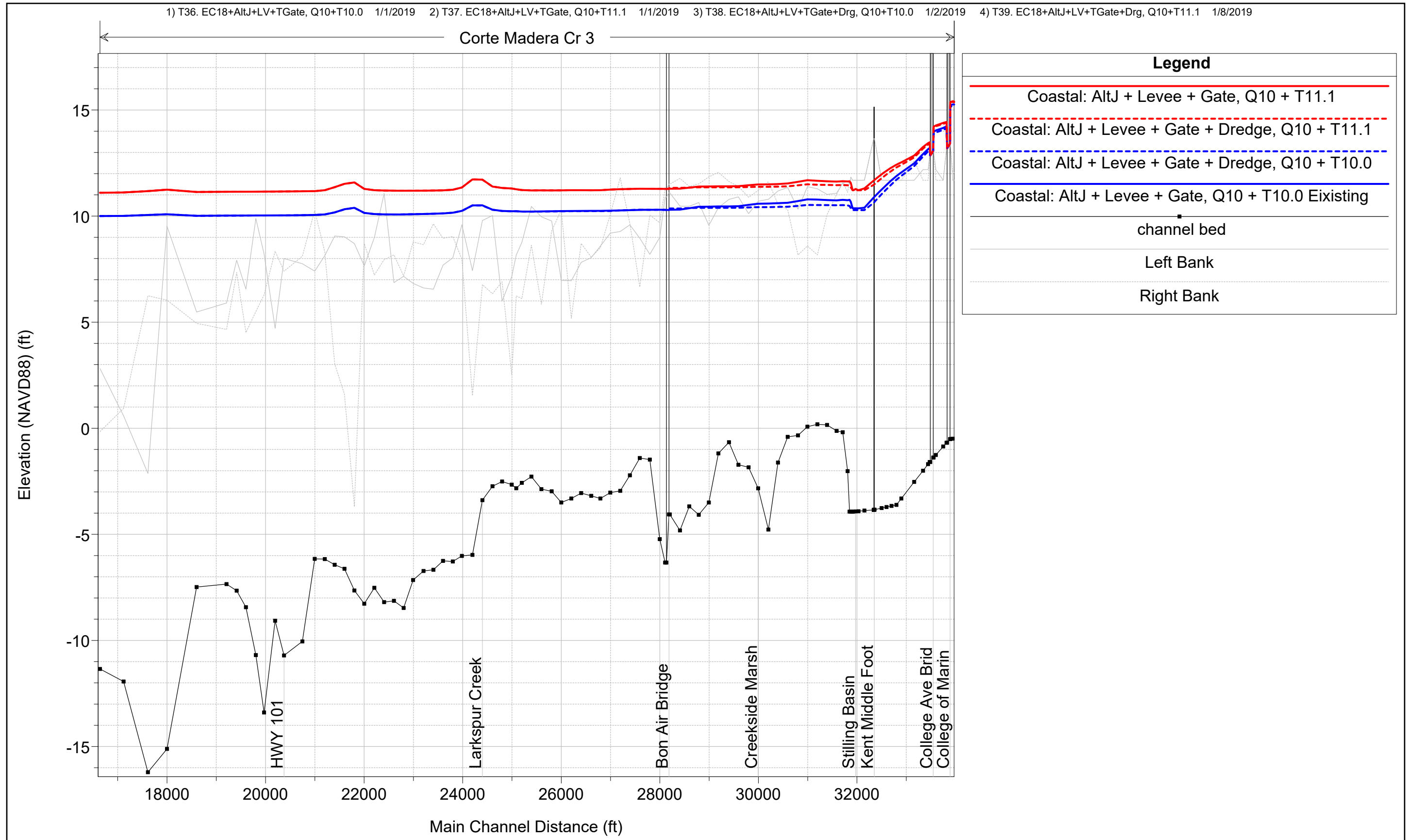


Figure 17 Riverine and Coastal 100-Year WSE Profiles; (no dredge, no SLR); Scenarios #1 and #5

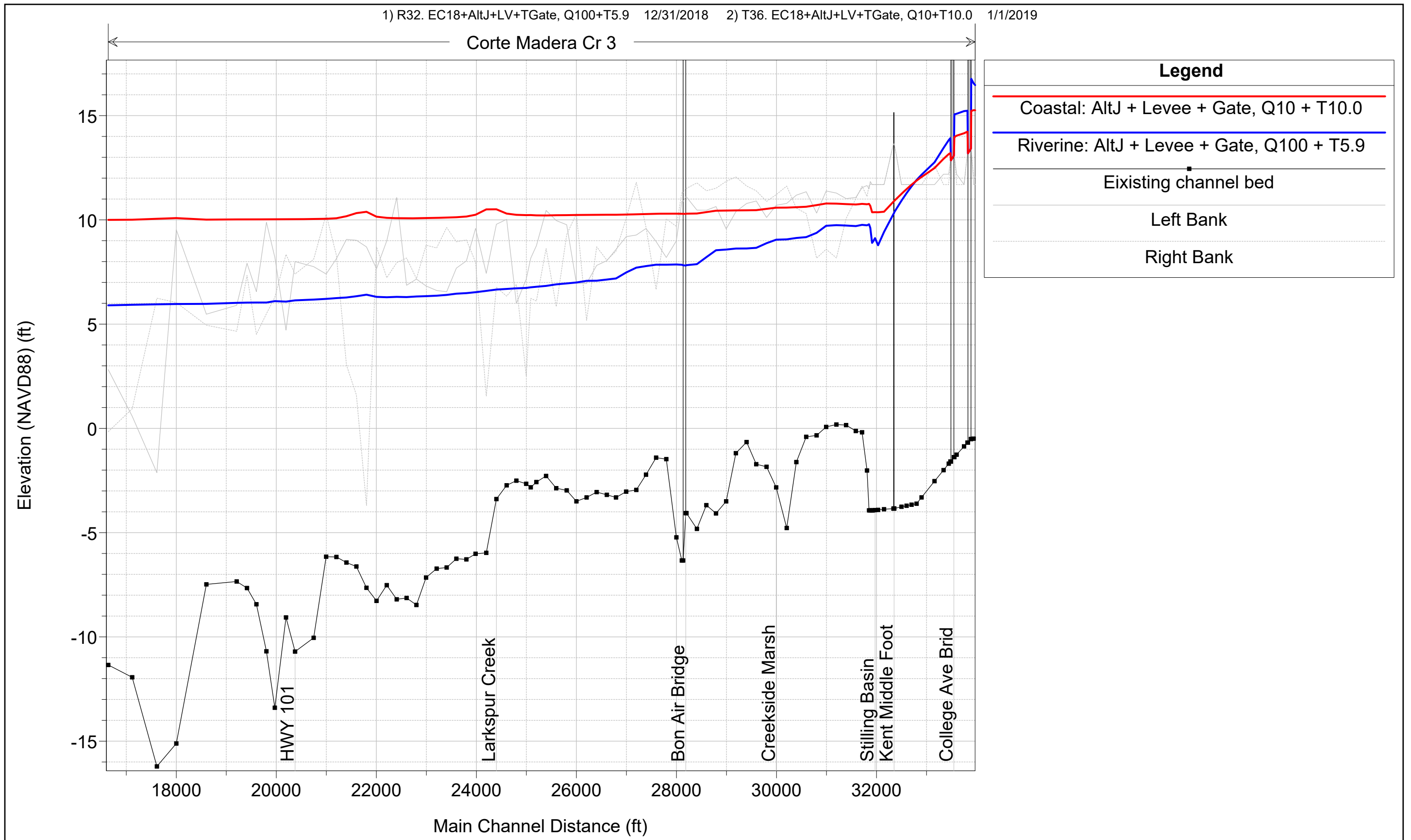


Figure 18 Riverine and Coastal 100-Year WSE Profiles; (no dredge, with SLR); Scenarios #2 and #6

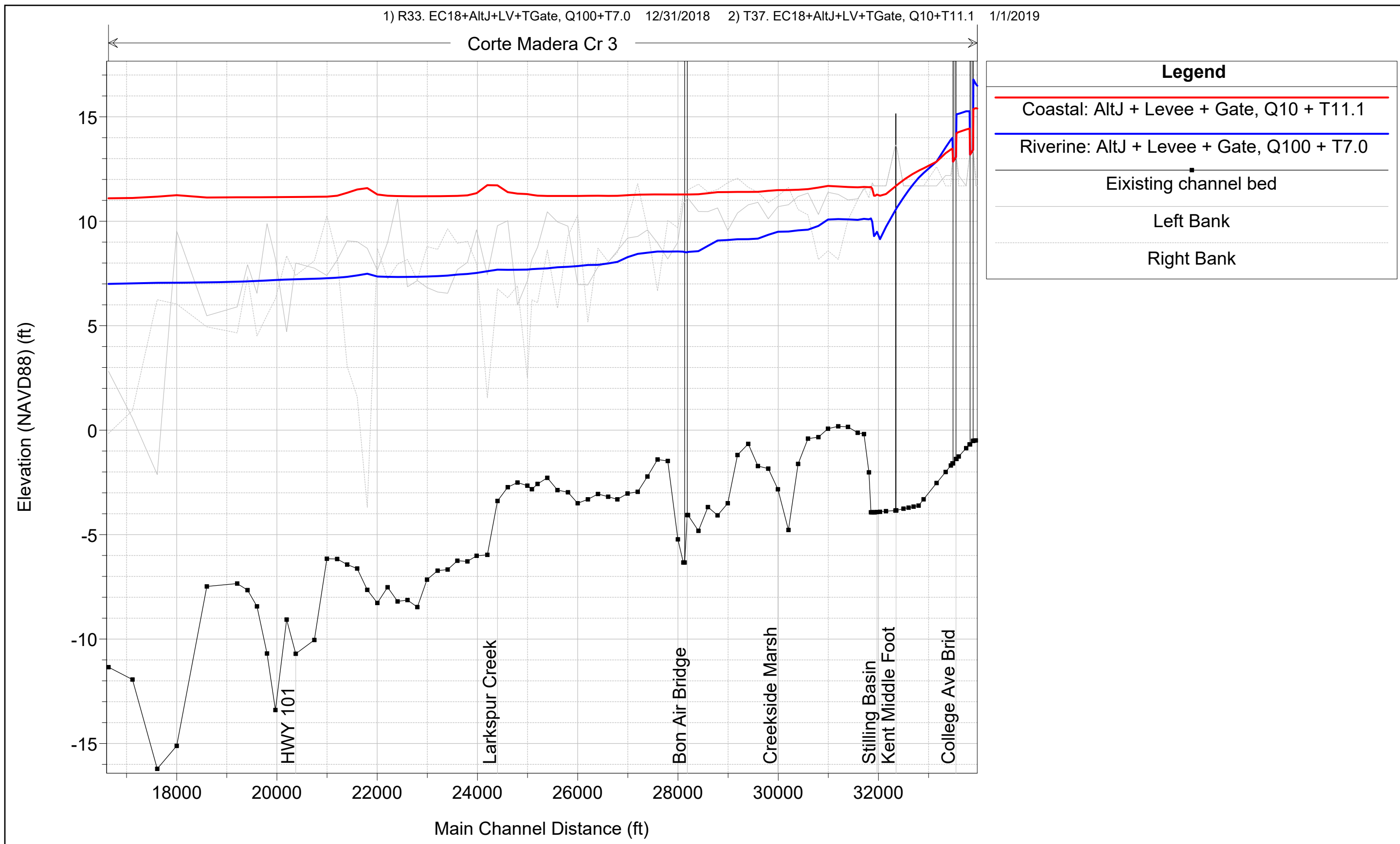


Figure 19 Riverine and Coastal 100-Year WSE Profiles; (with dredge, no SLR); Scenarios #3 and #7

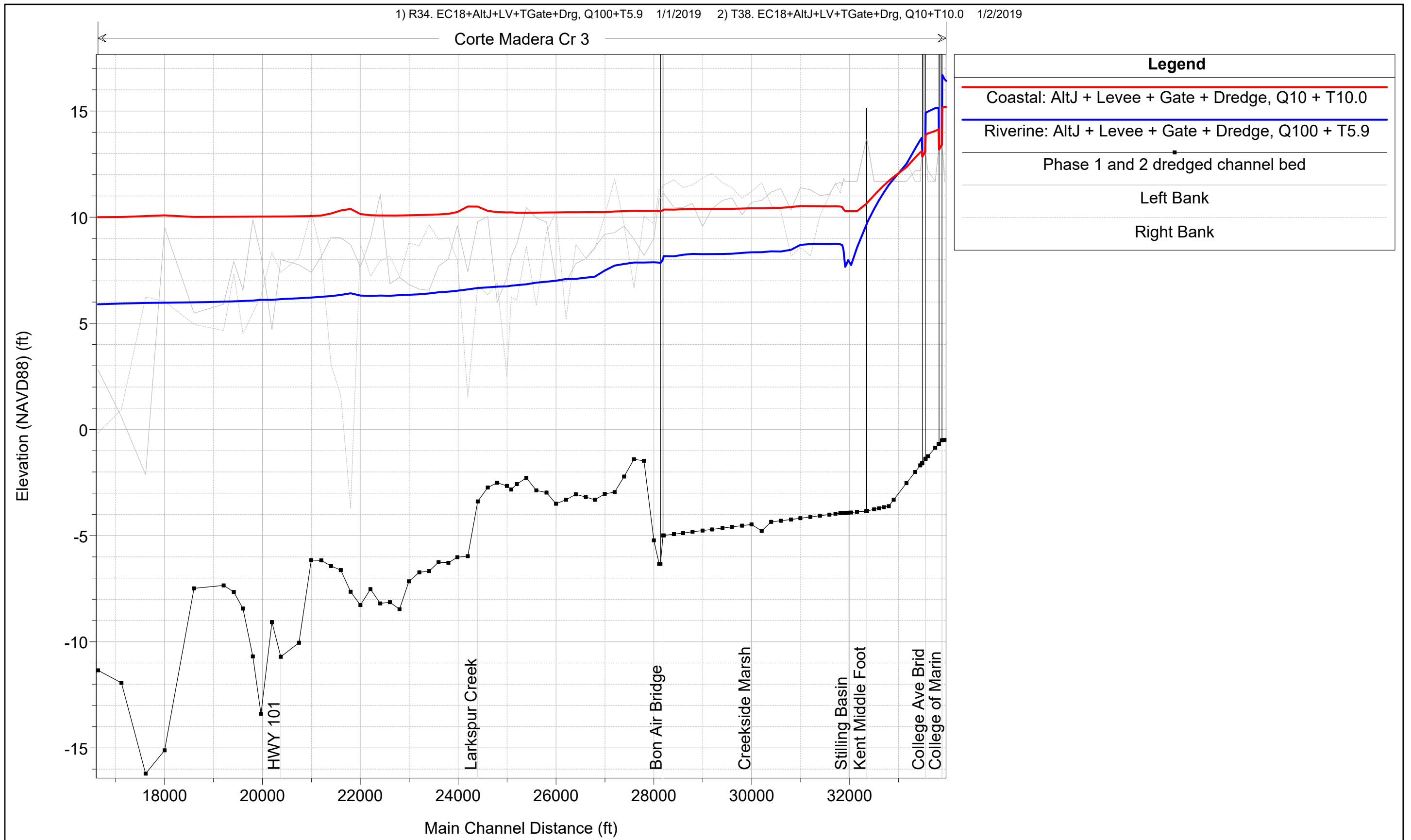


Figure 20 Riverine and Coastal 100-Year WSE Profiles; (with dredge, with SLR); Scenarios #4 and #8

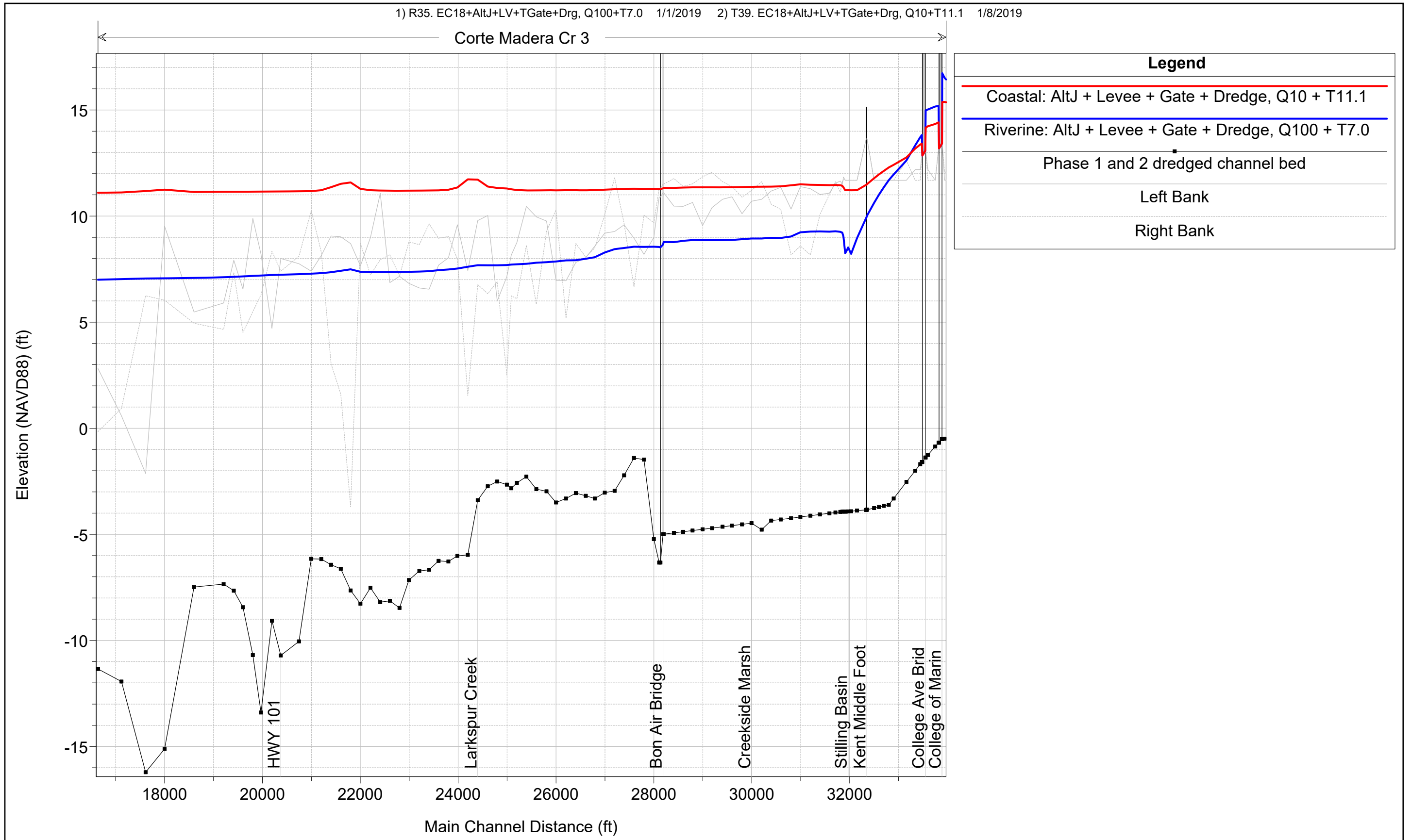


Figure 21a

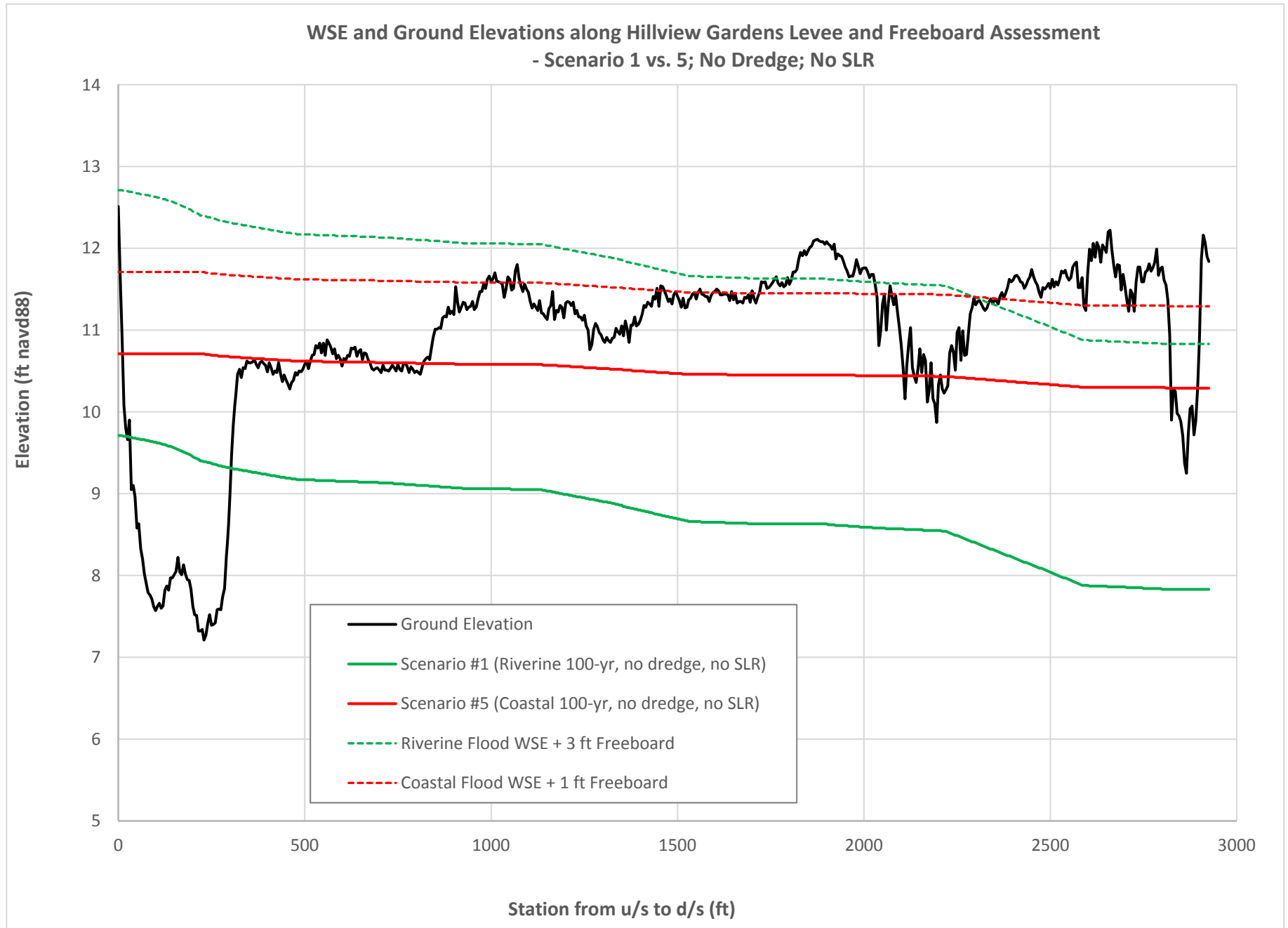




Figure 21b

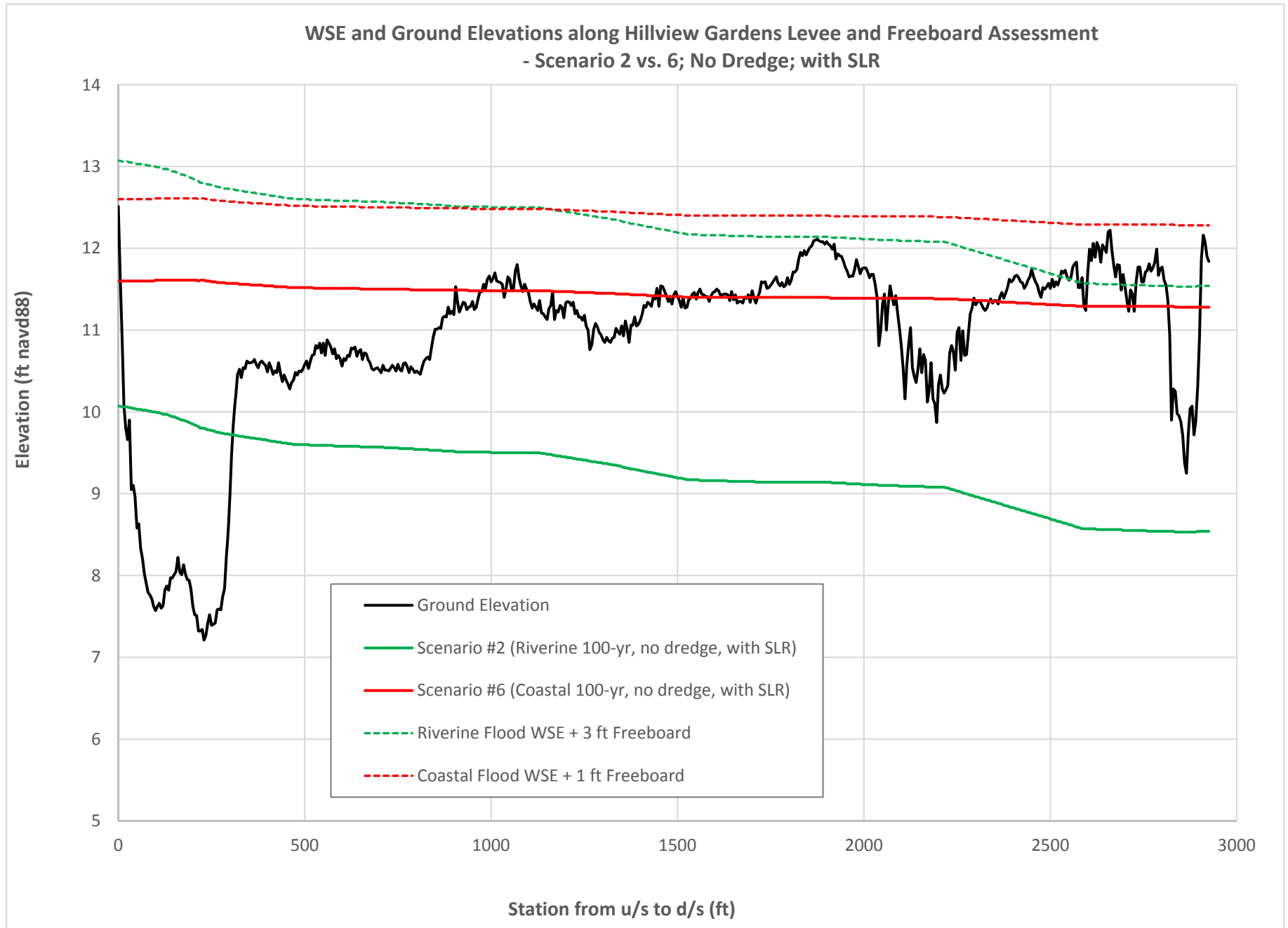


Figure 21c

WSE and Ground Elevations along Hillview Gardens Levee and Freeboard Assessment  
- Scenario 3 vs. 7; with Dredge; No SLR

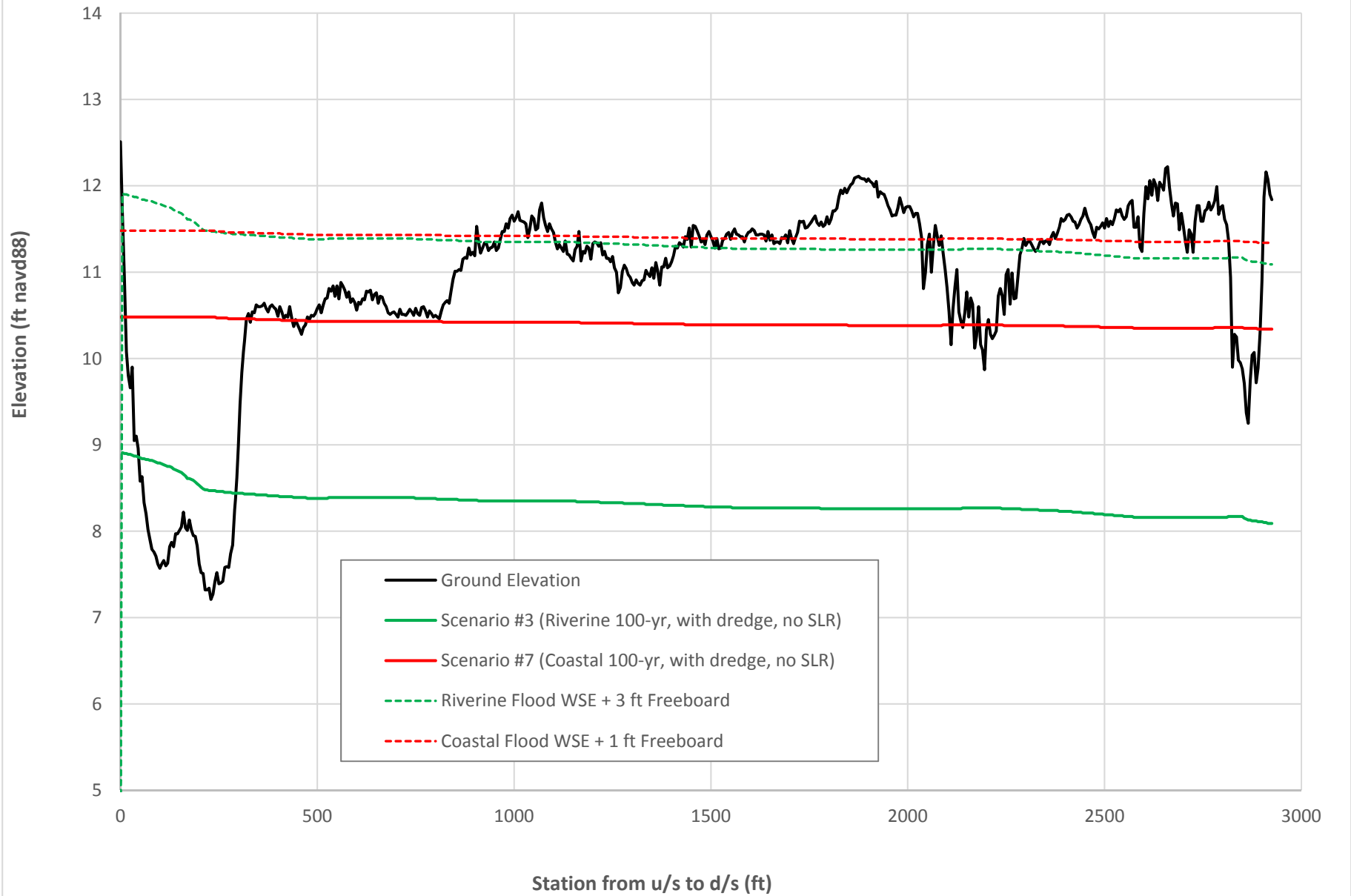


Figure 21d

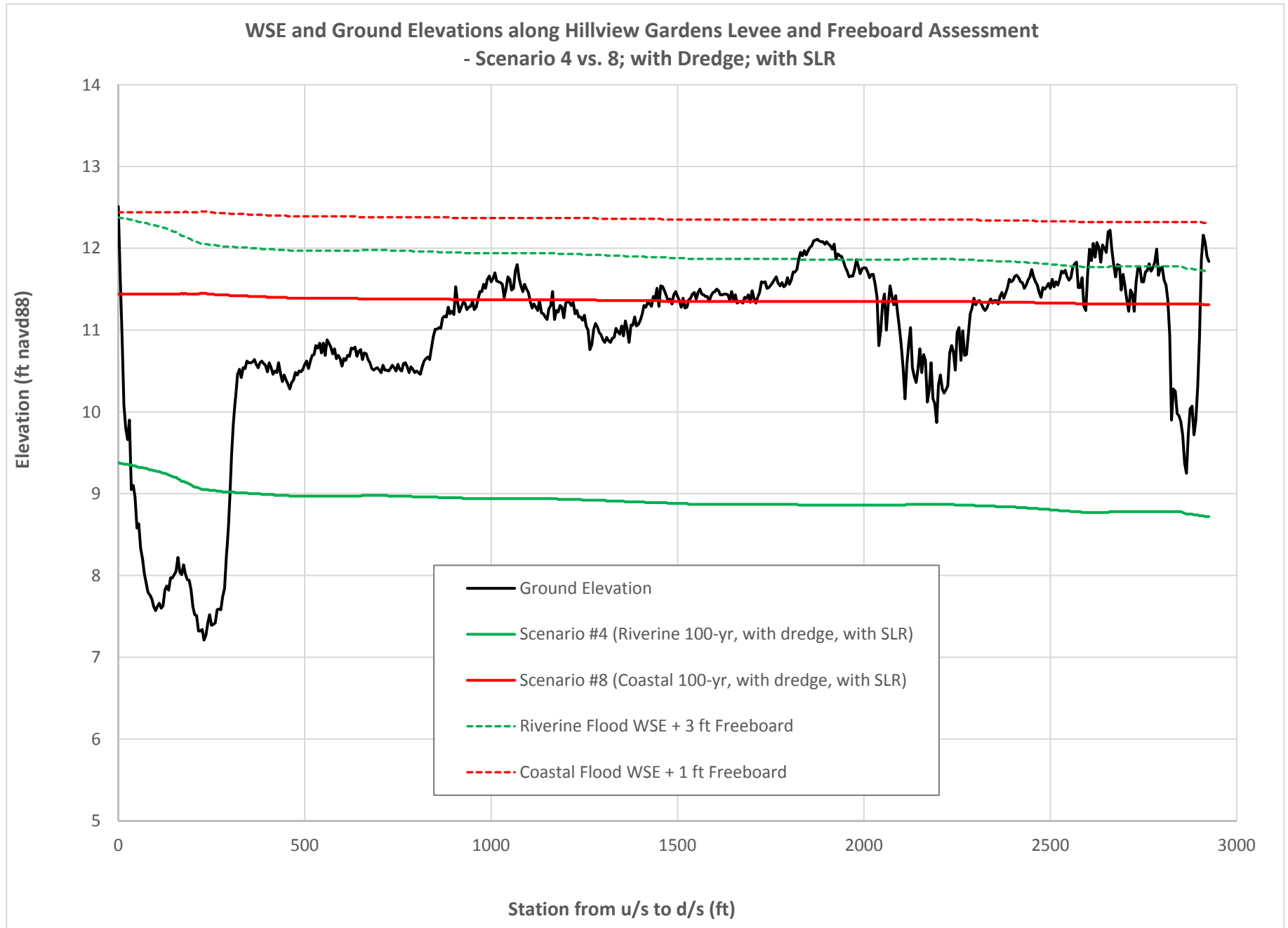


Figure 22a

WSE and Ground Elevations along Kentfield Gardens Levee and Freeboard Assessment  
- Scenario 1 vs. 5; No Dredge; No SLR

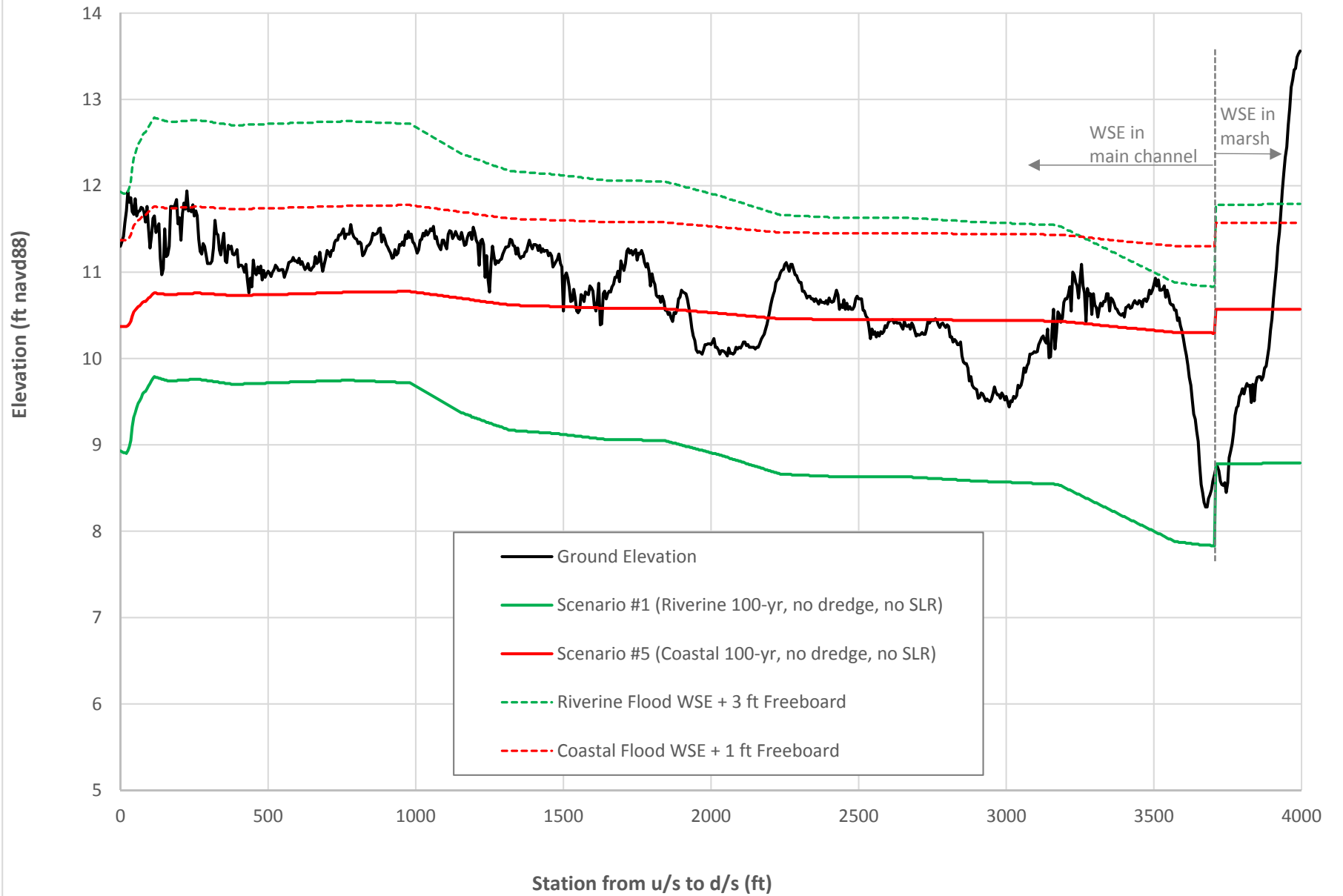


Figure 22b

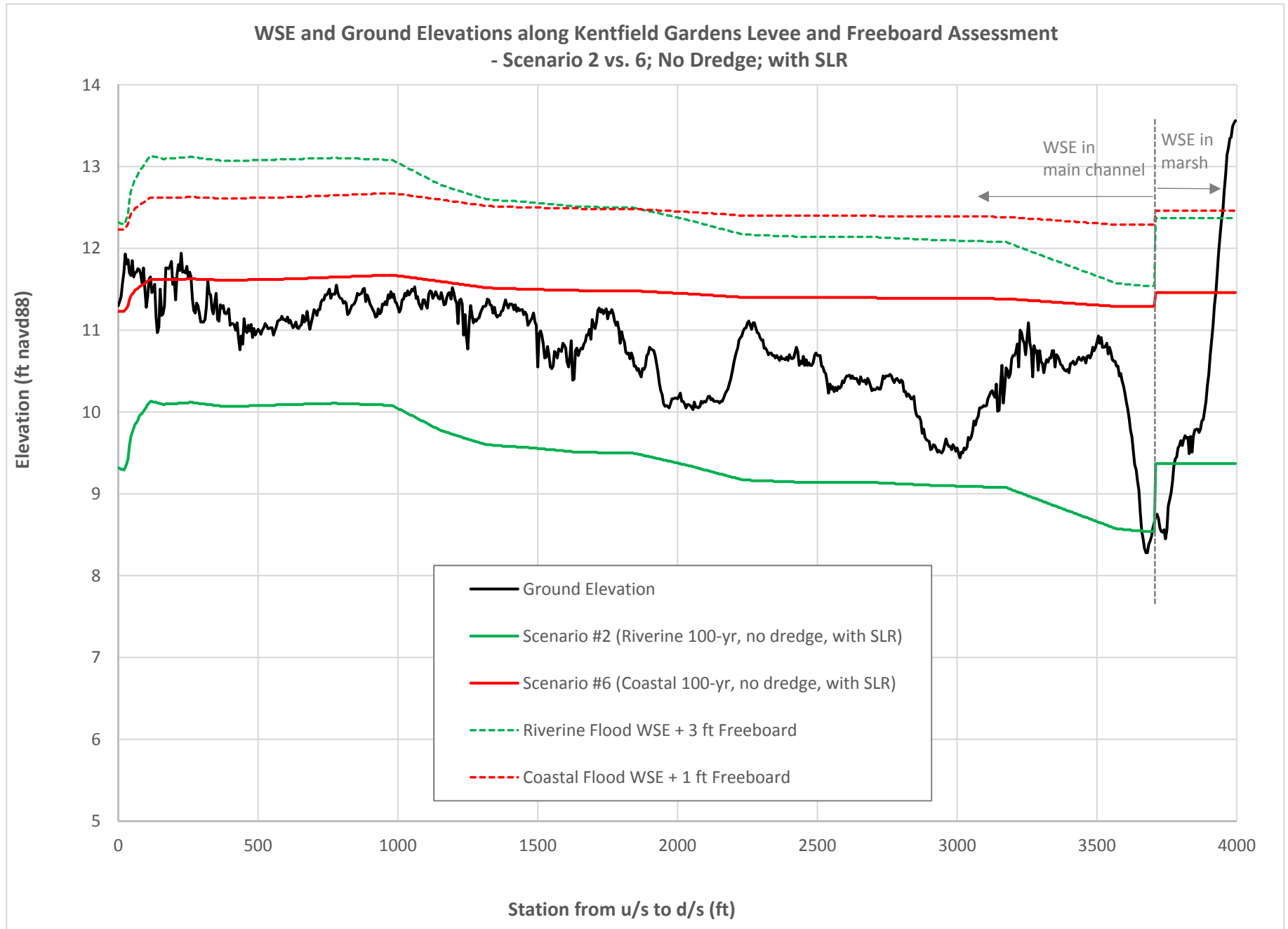


Figure 22c

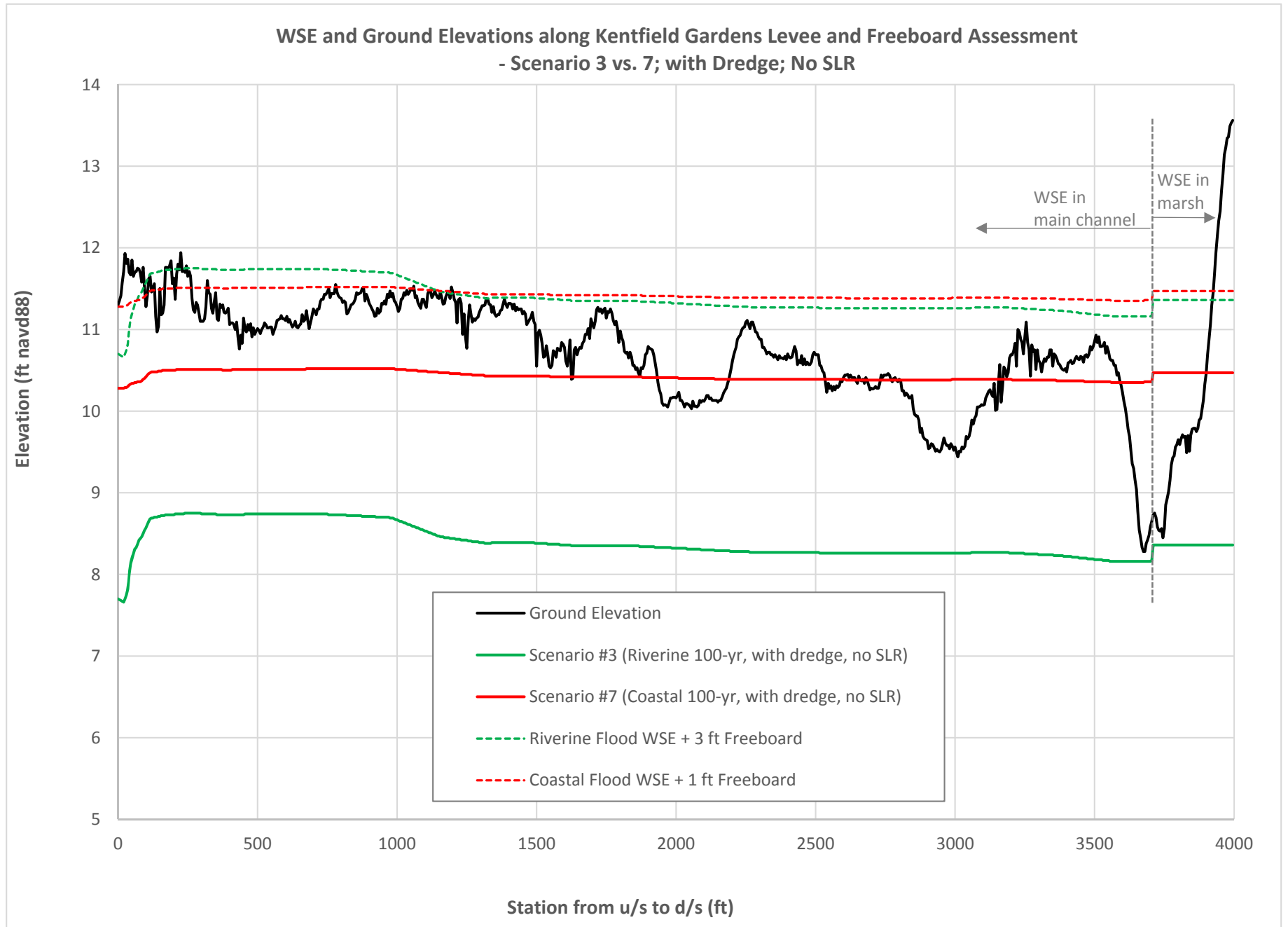


Figure 22d

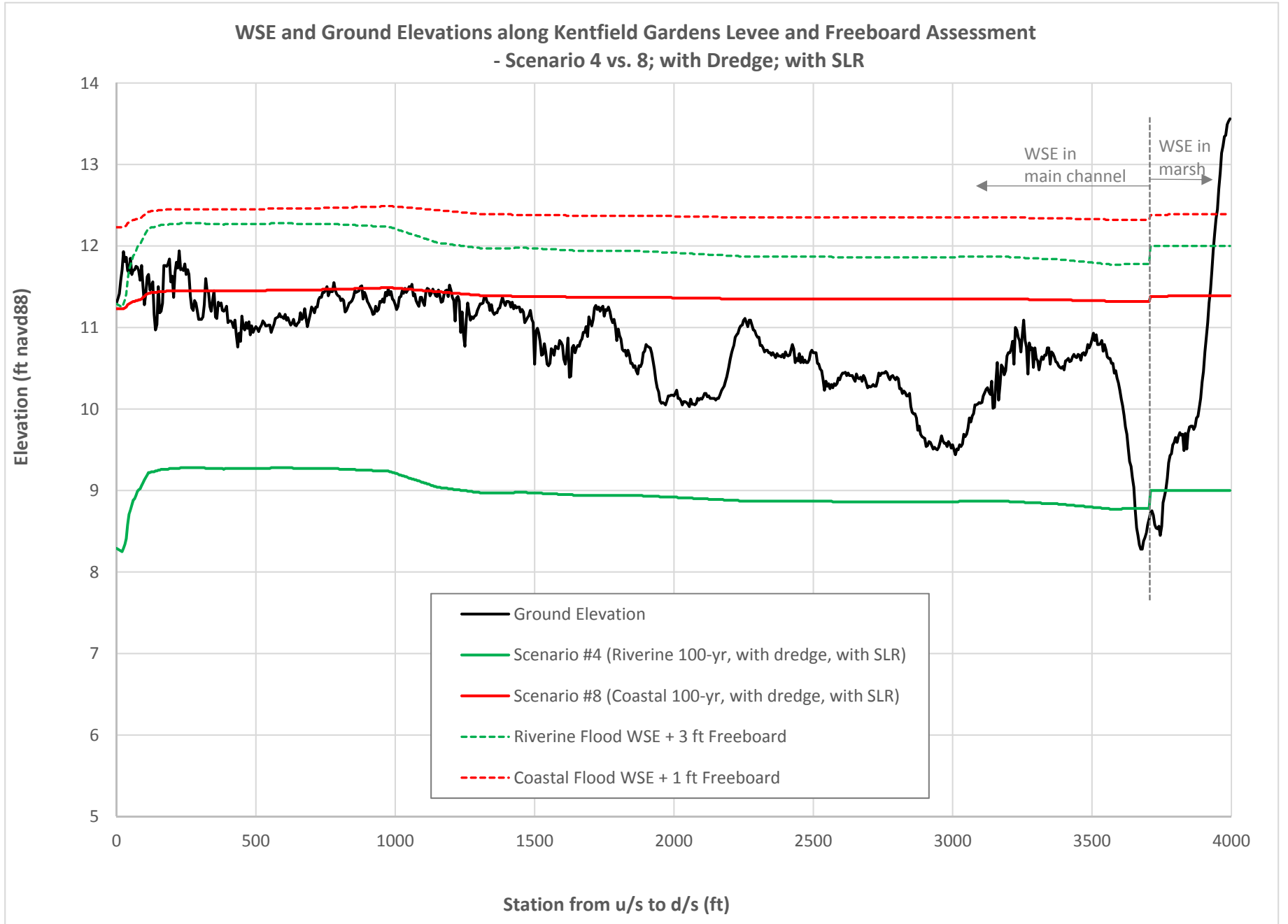


Figure 23a

WSE and Ground Elevations along College Ct Flood Barrier and Freeboard Assessment  
- Scenario 1 vs. 5; No Dredge; No SLR

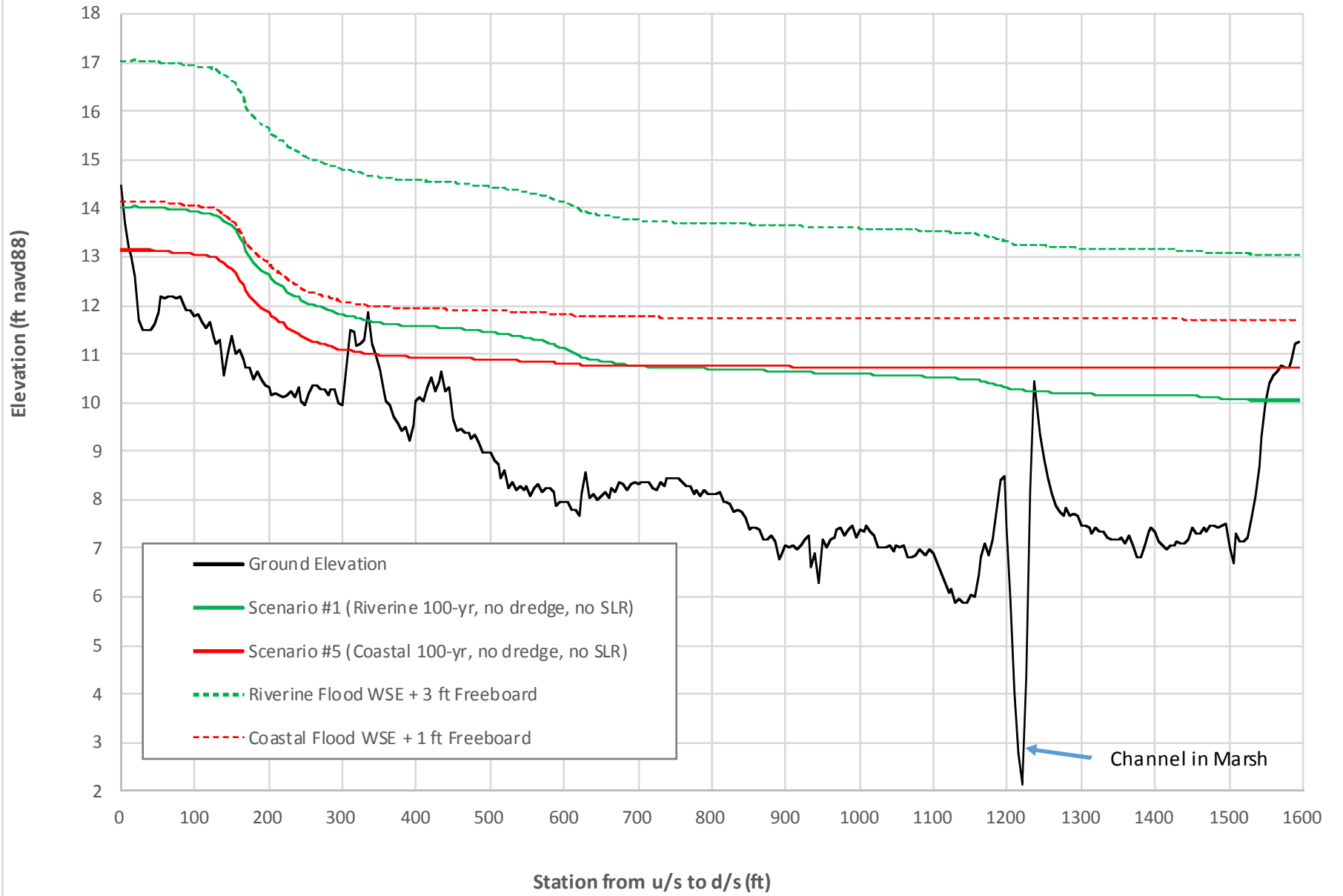




Figure 23b

WSE and Ground Elevations along College Ct Flood Barrier and Freeboard Assessment  
- Scenario 2 vs. 6; No Dredge; with SLR

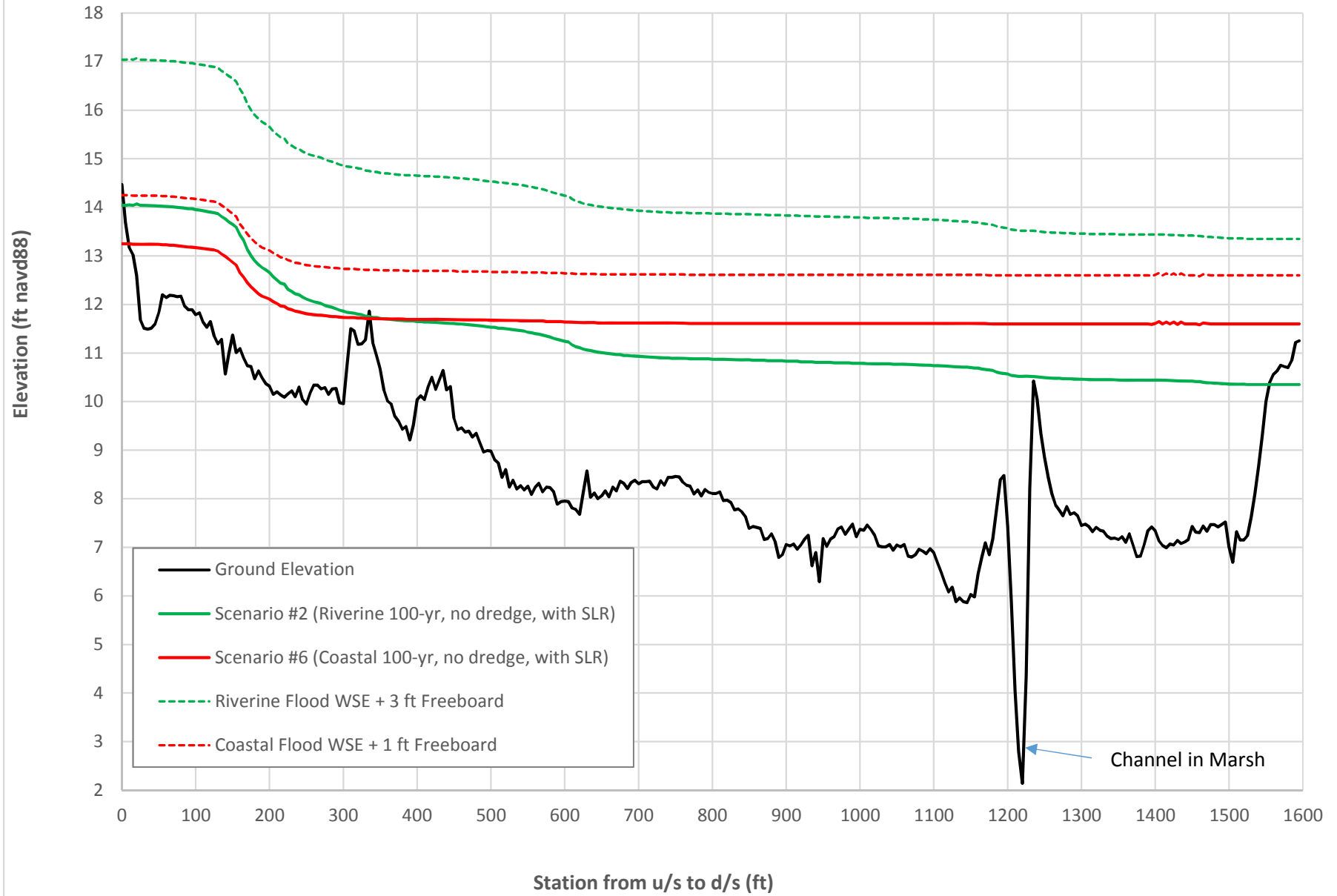


Figure 23c

WSE and Ground Elevations along College Ct Flood Barrier and Freeboard Assessment  
- Scenario 3 vs. 7; with Dredge; No SLR

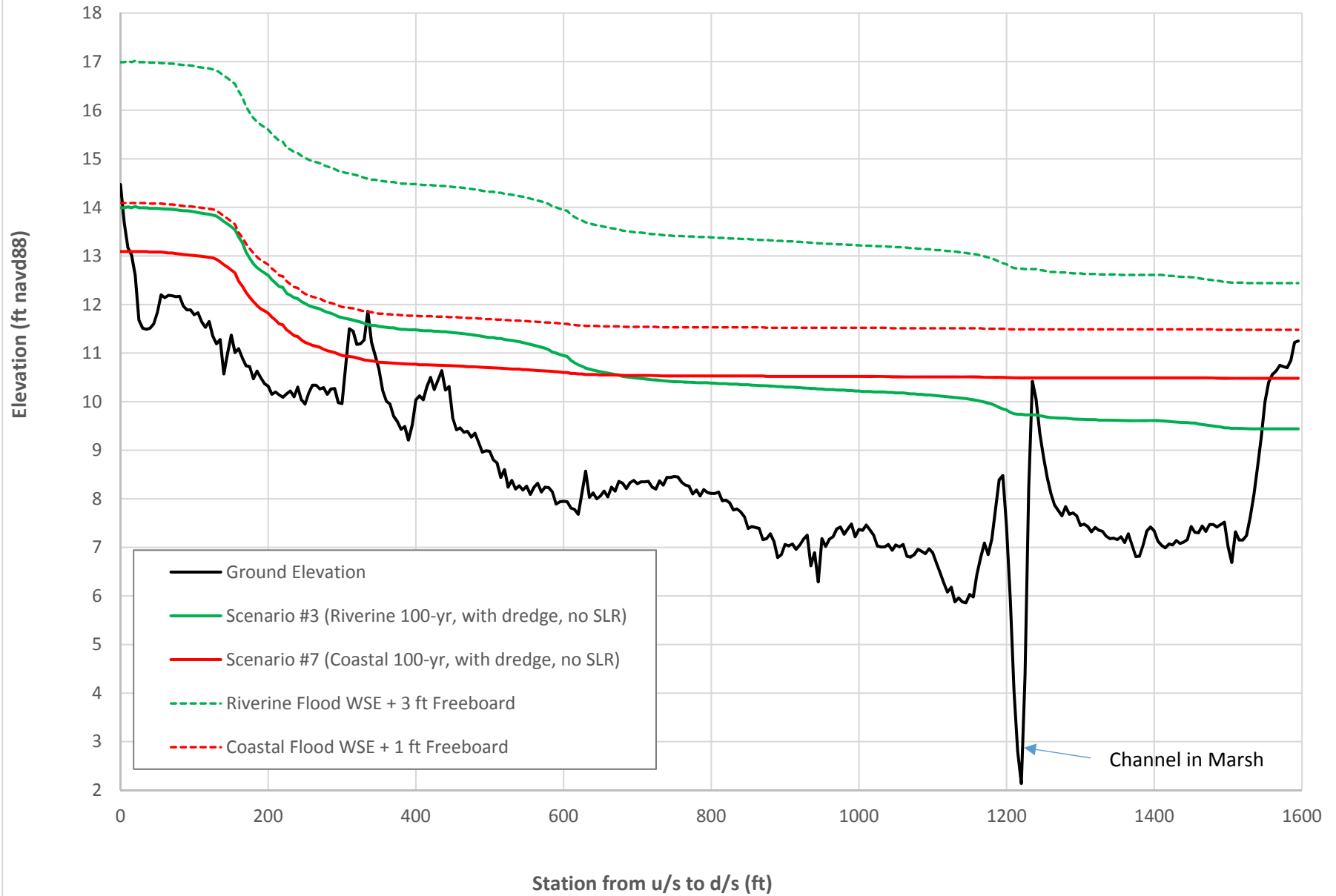


Figure 23d

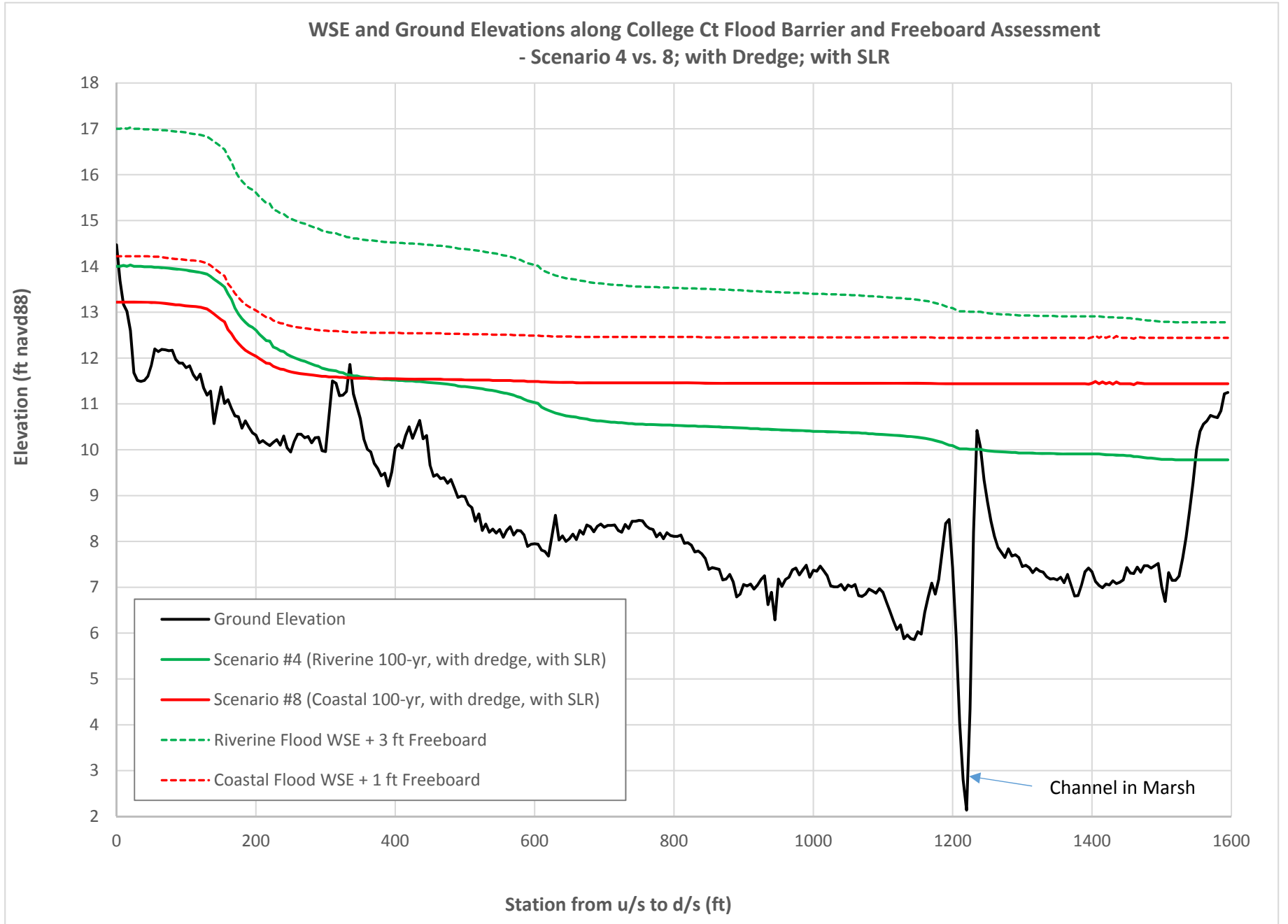


Figure 24a

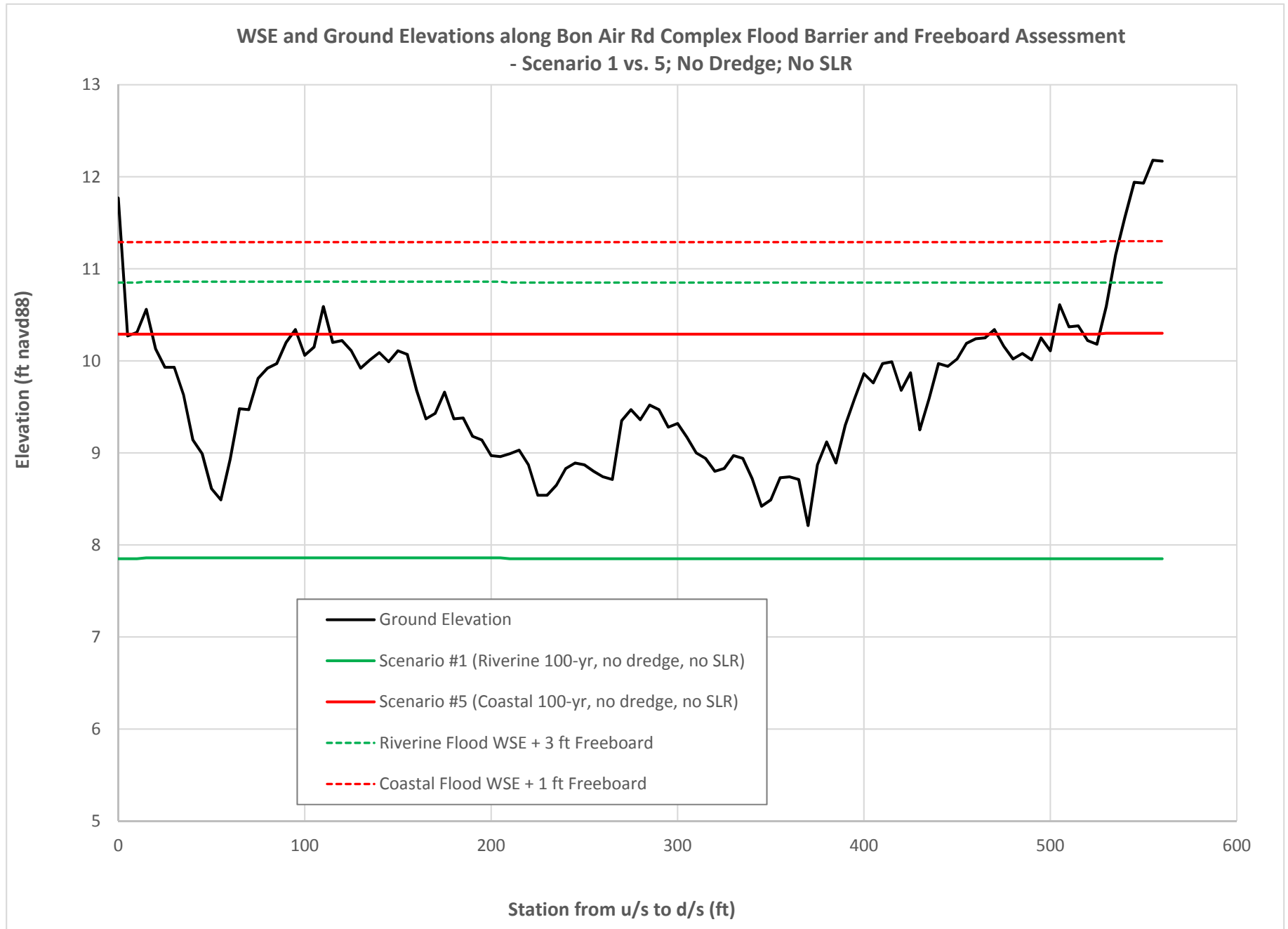


Figure 24b

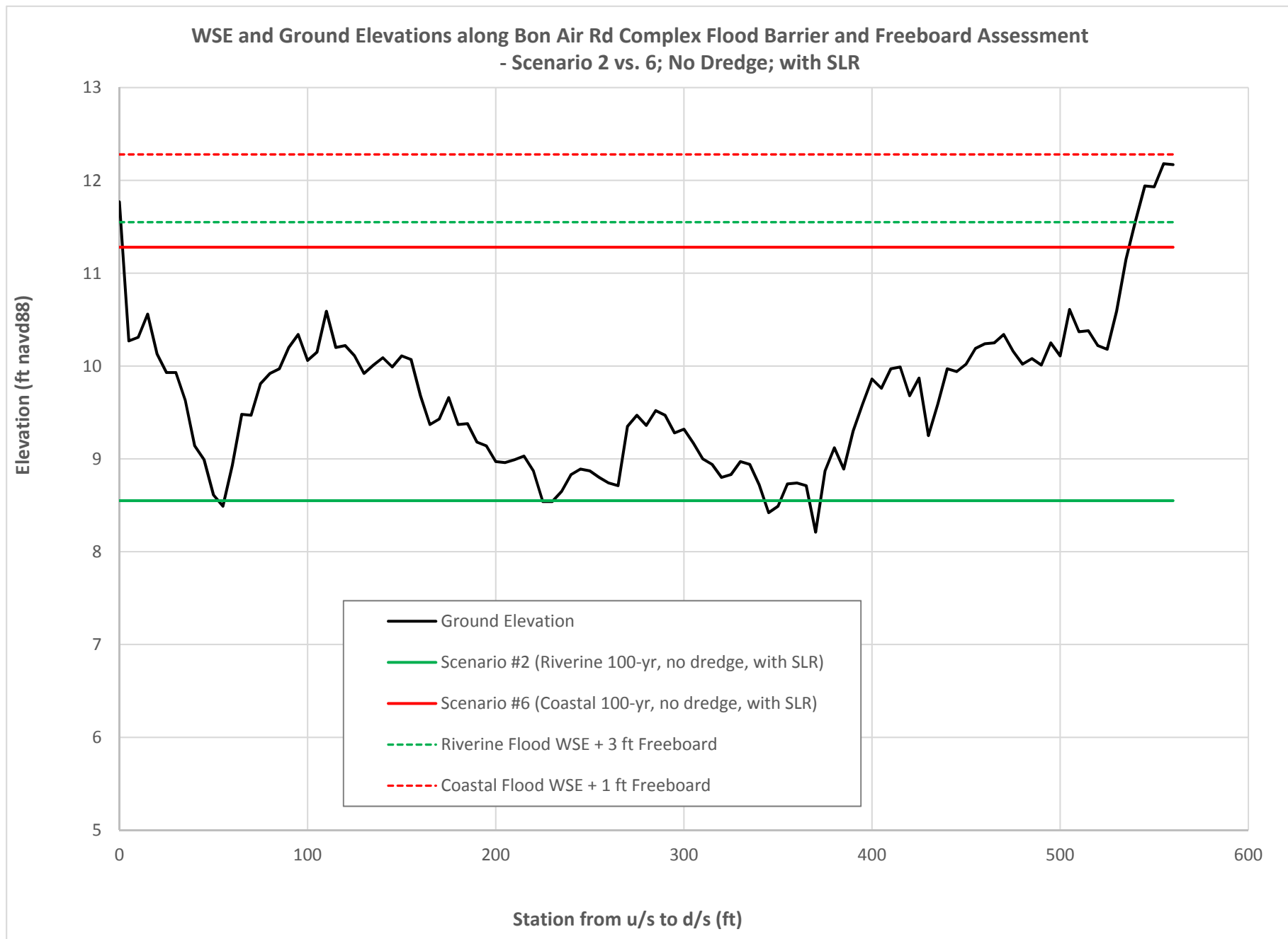


Figure 24c

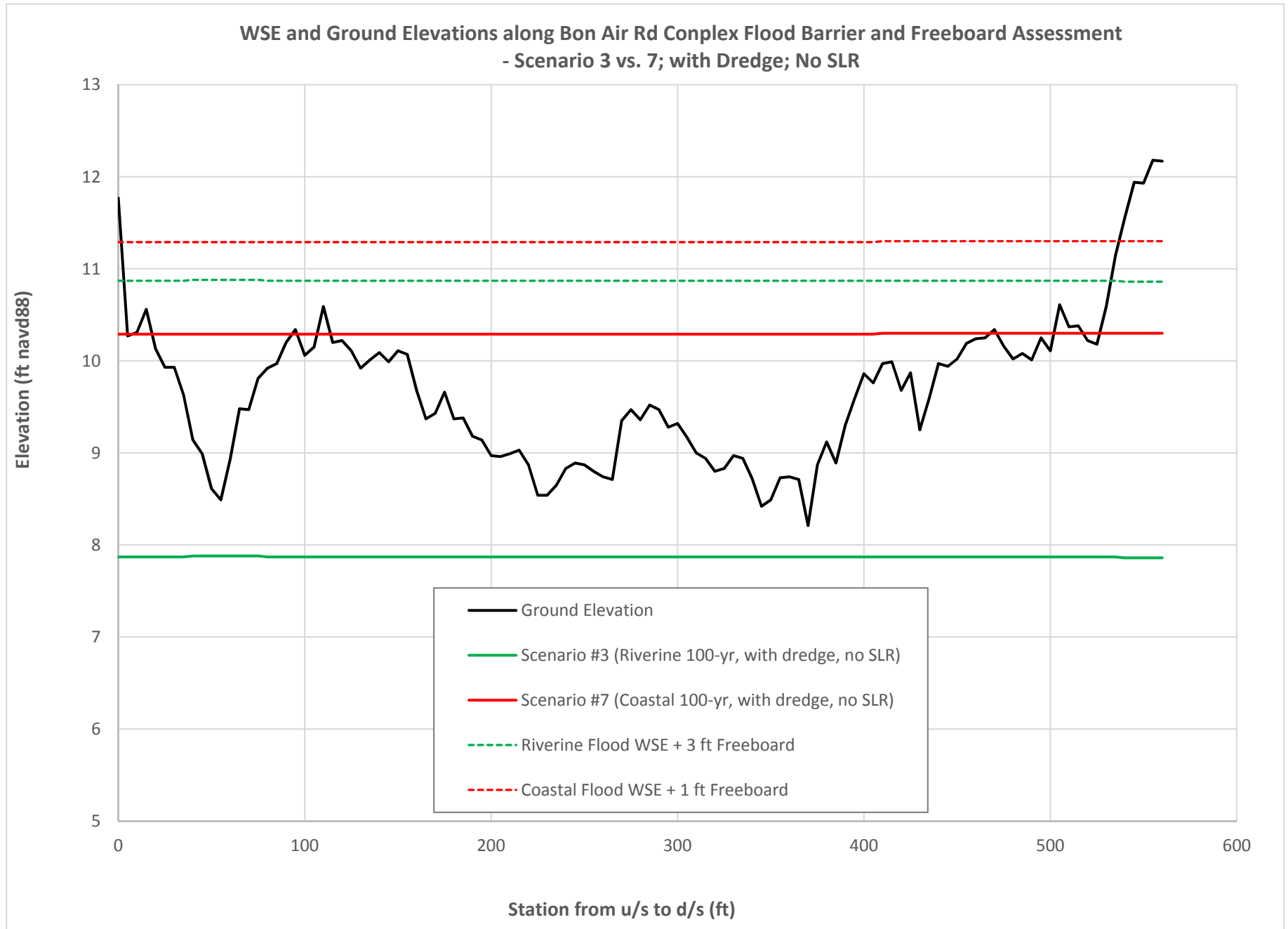


Figure 24d

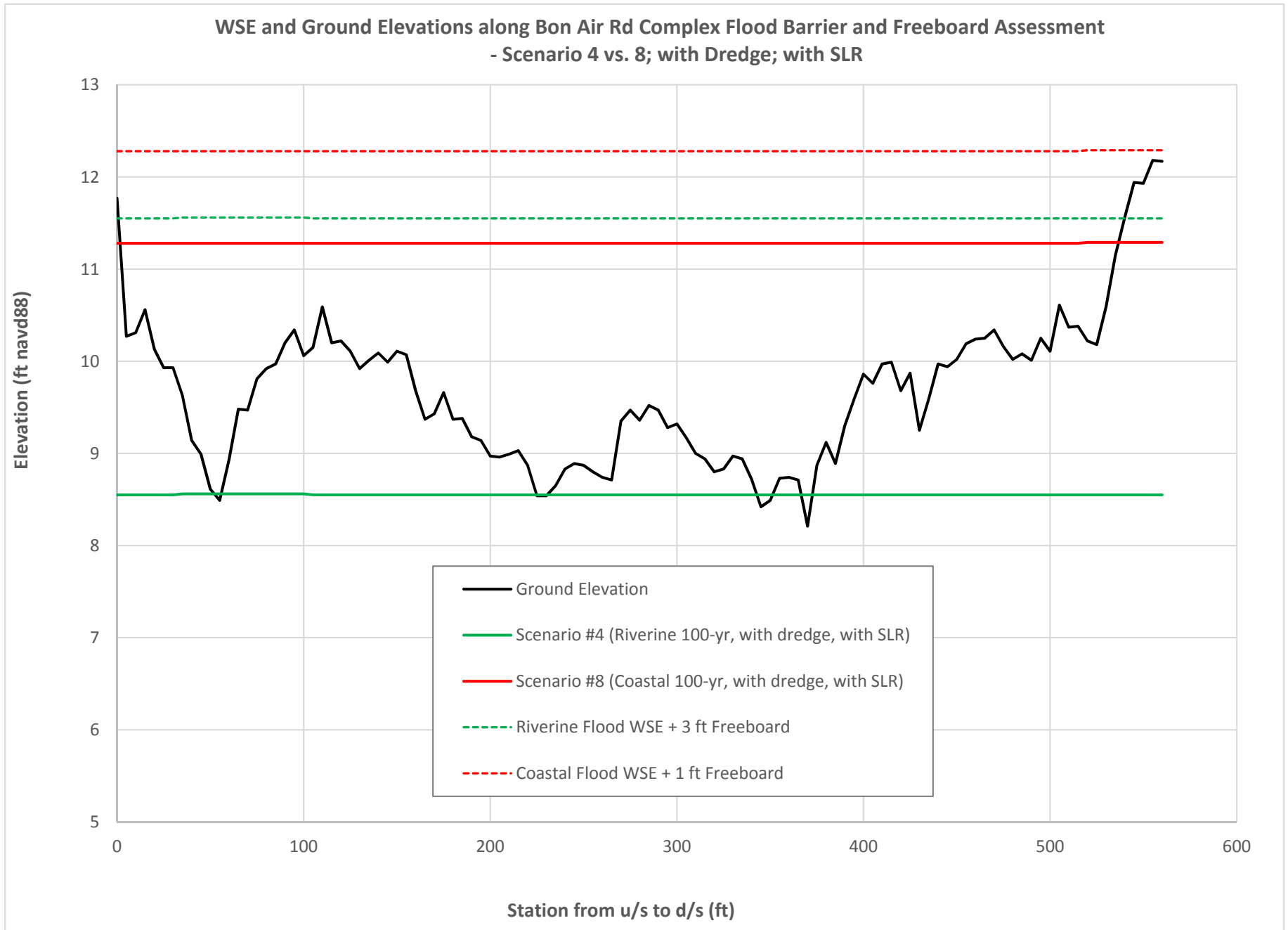


Figure 25a

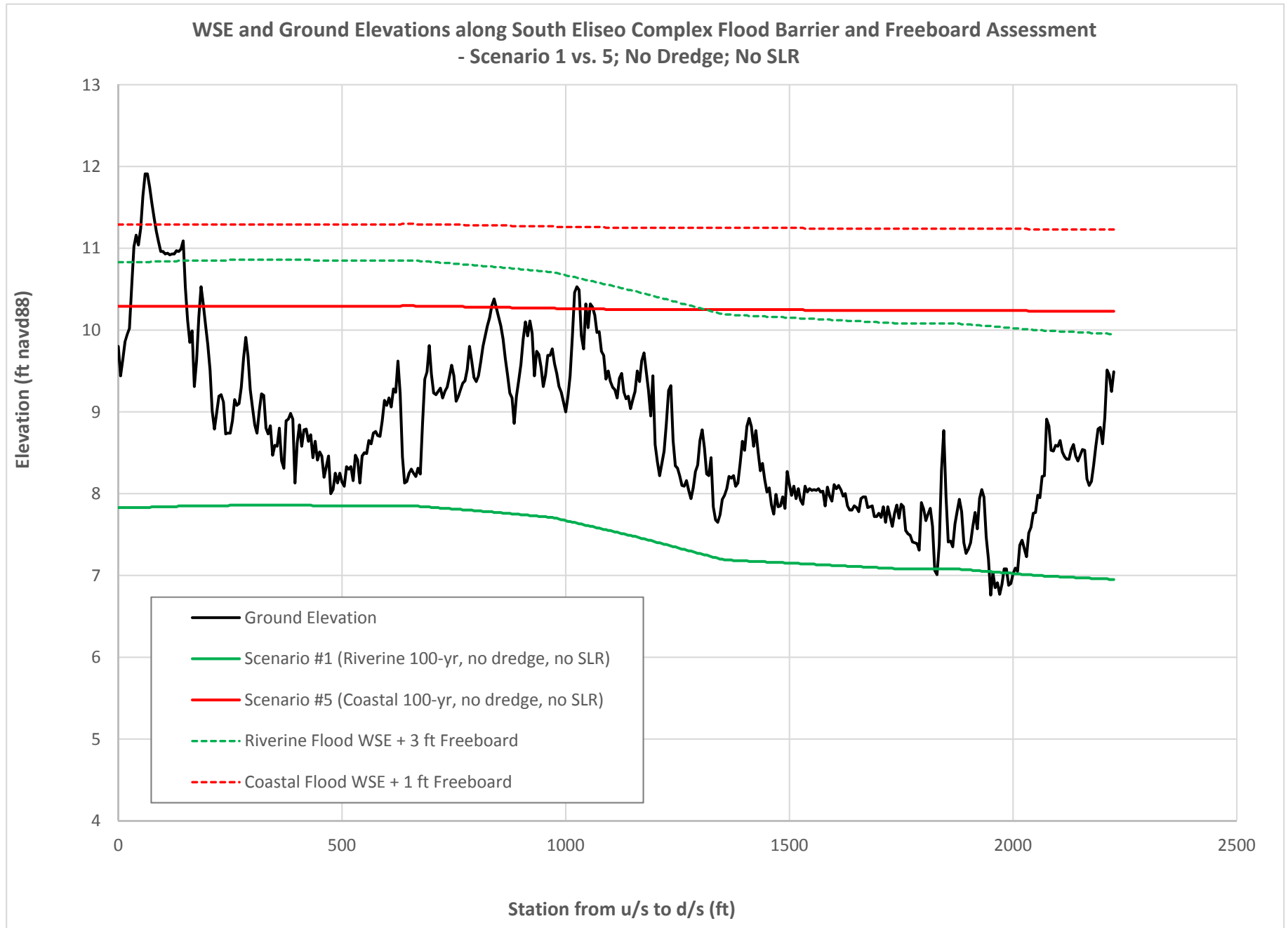




Figure 25b

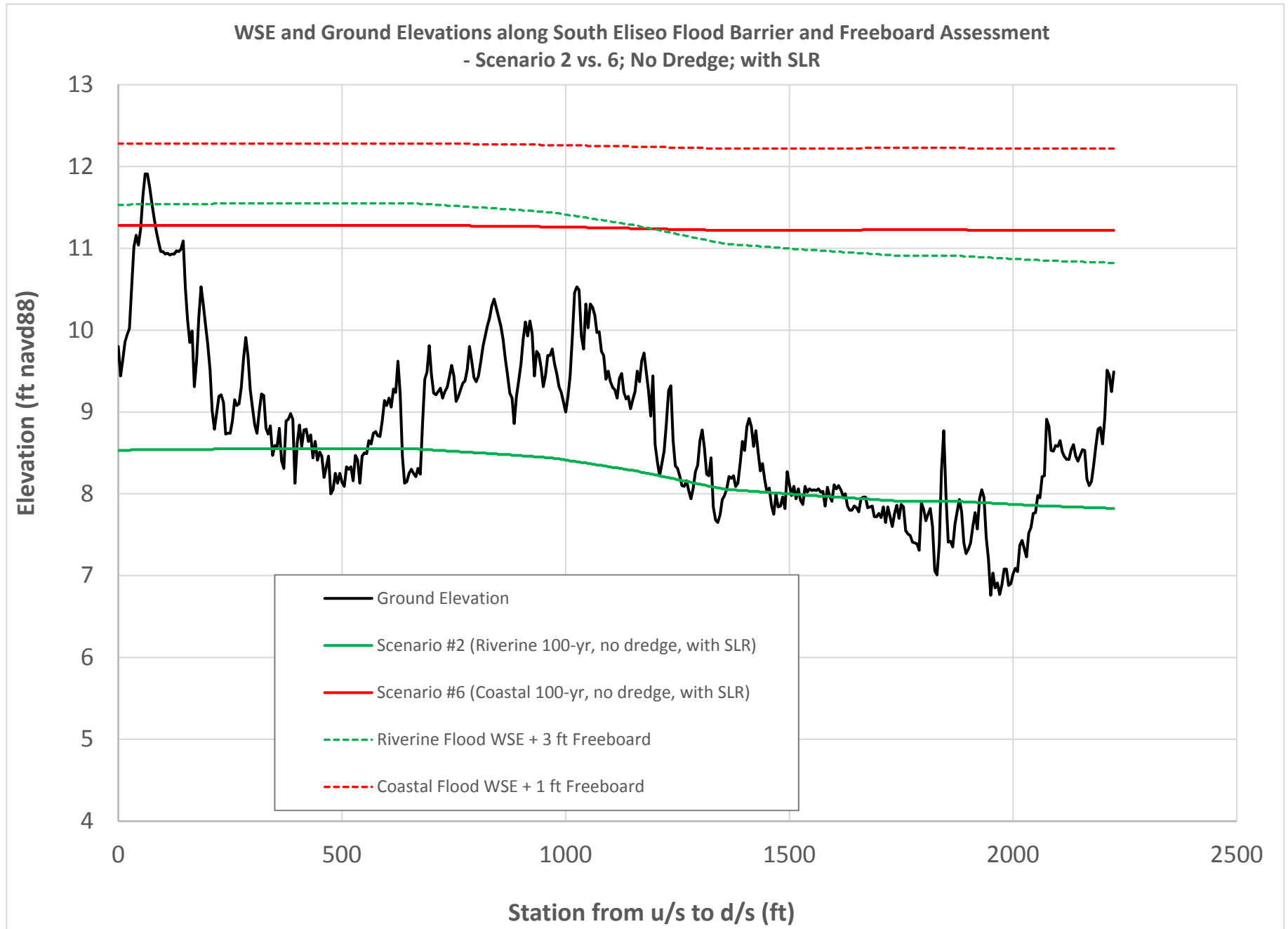


Figure 25c

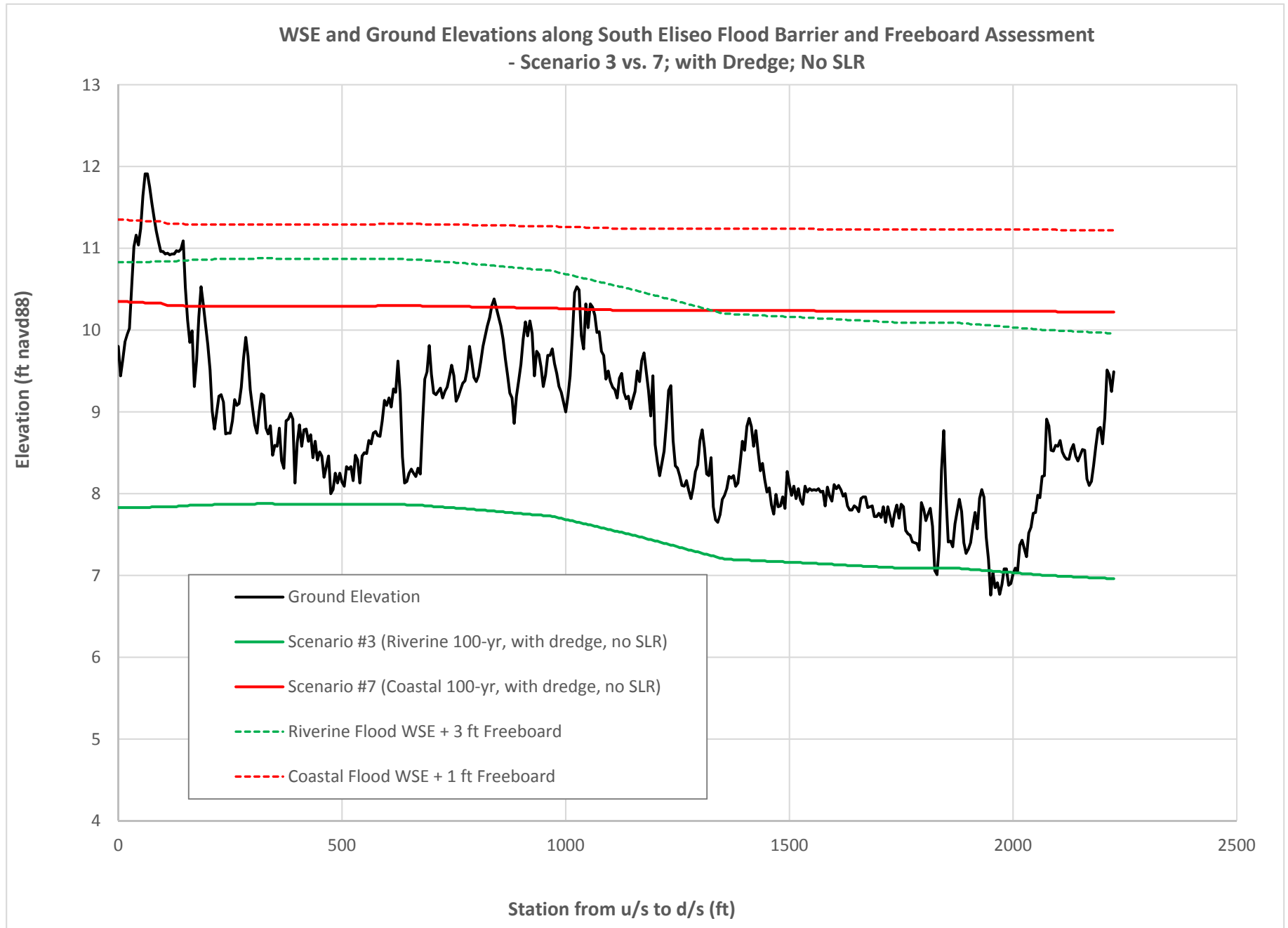


Figure 25d

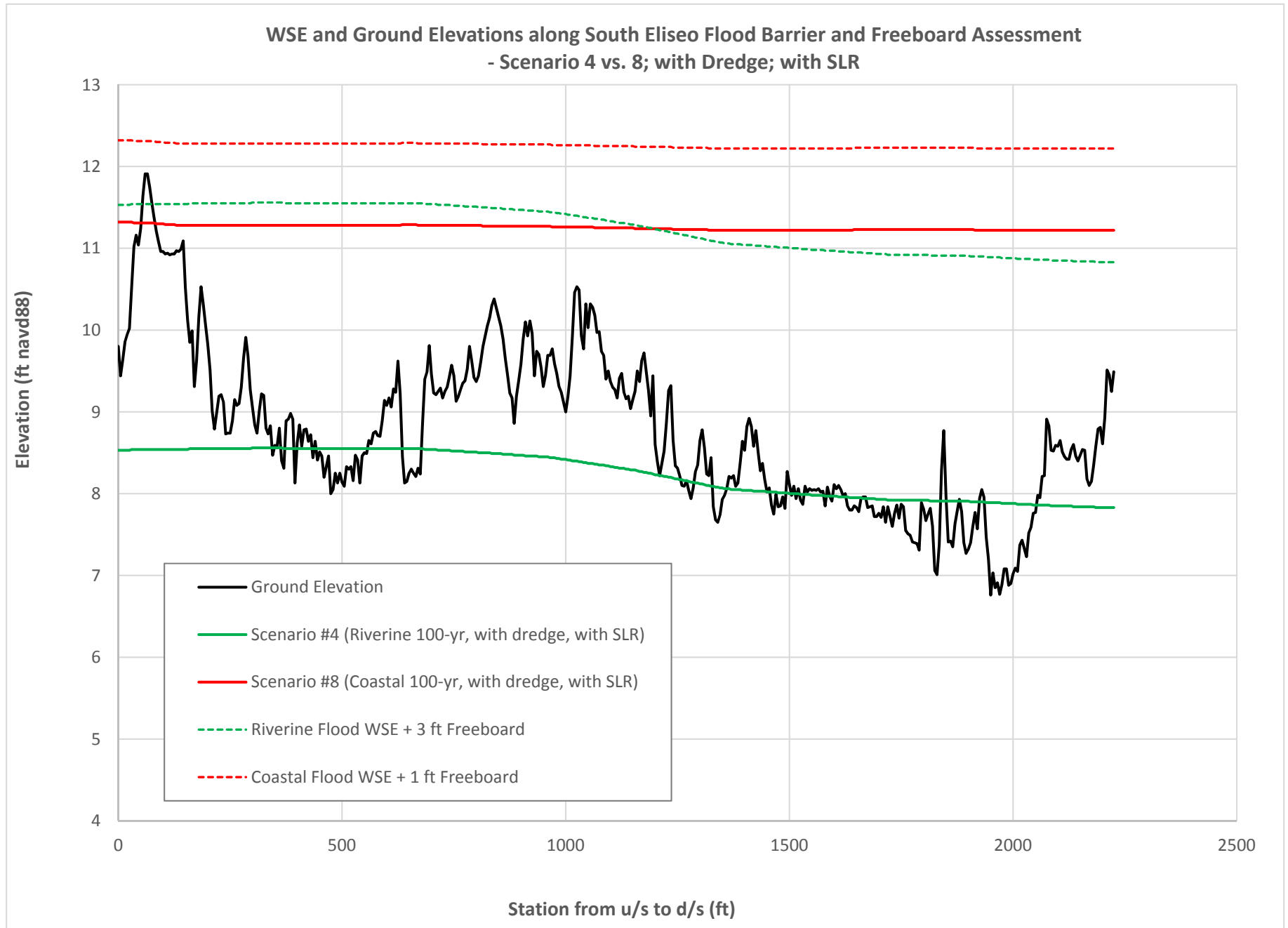


Figure 26a

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#1) and Freeboard Assessment  
- Scenario 1 vs. 5; No Dredge; No SLR

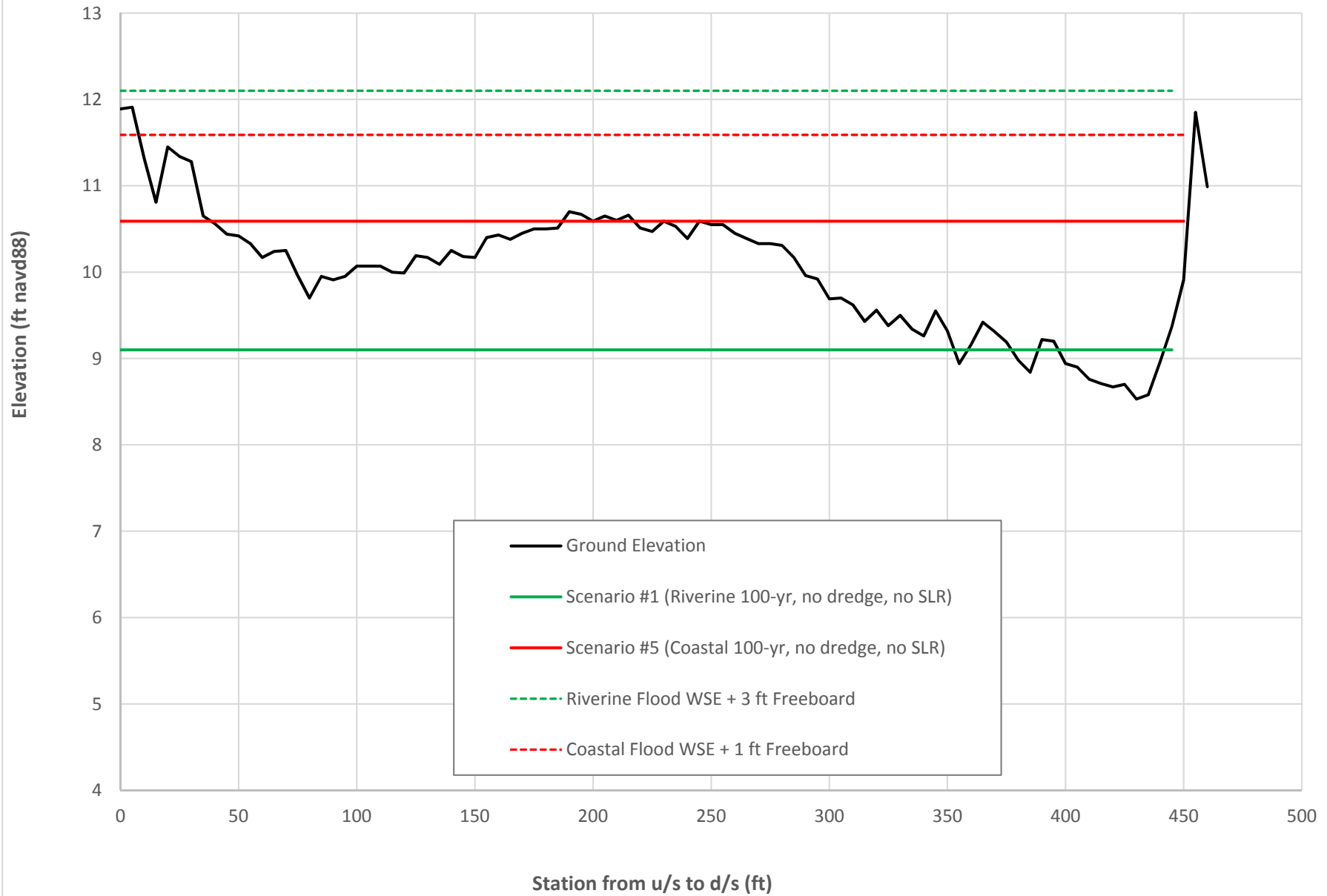


Figure 26b

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#1) and Freeboard Assessment  
- Scenario 2 vs. 6; No Dredge; with SLR

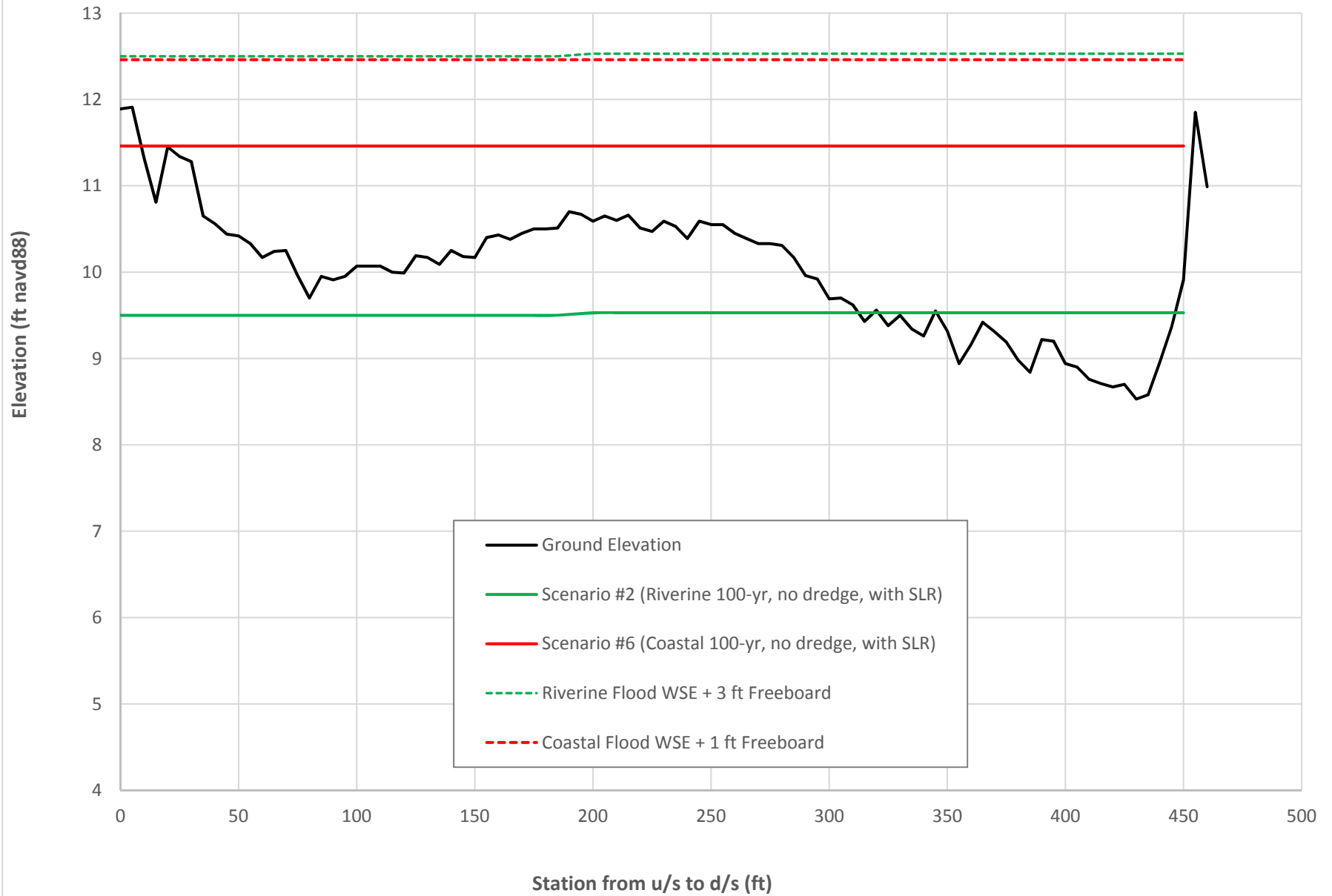


Figure 26c

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#1) and Freeboard Assessment  
- Scenario 3 vs. 7; with Dredge; No SLR

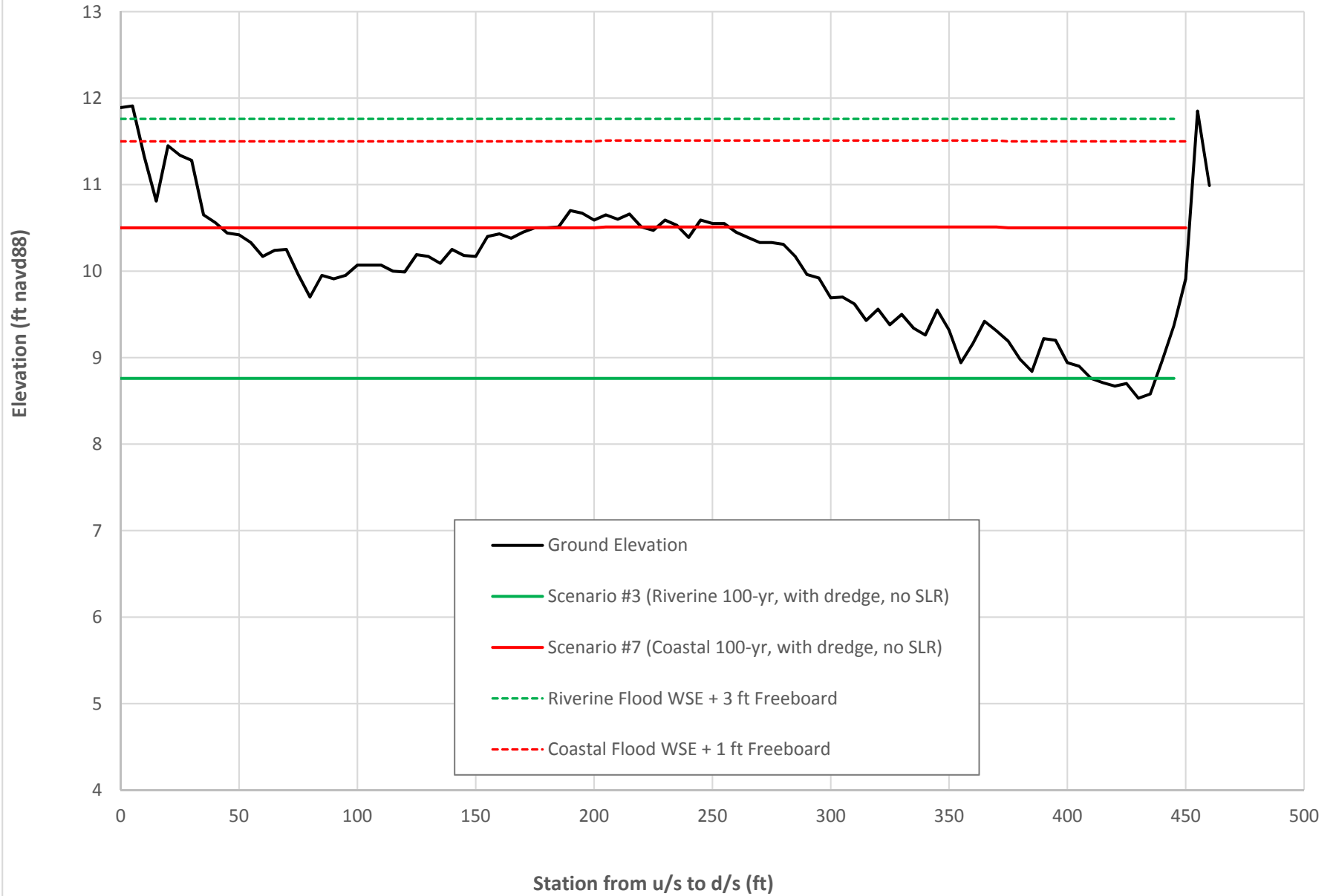


Figure 26d

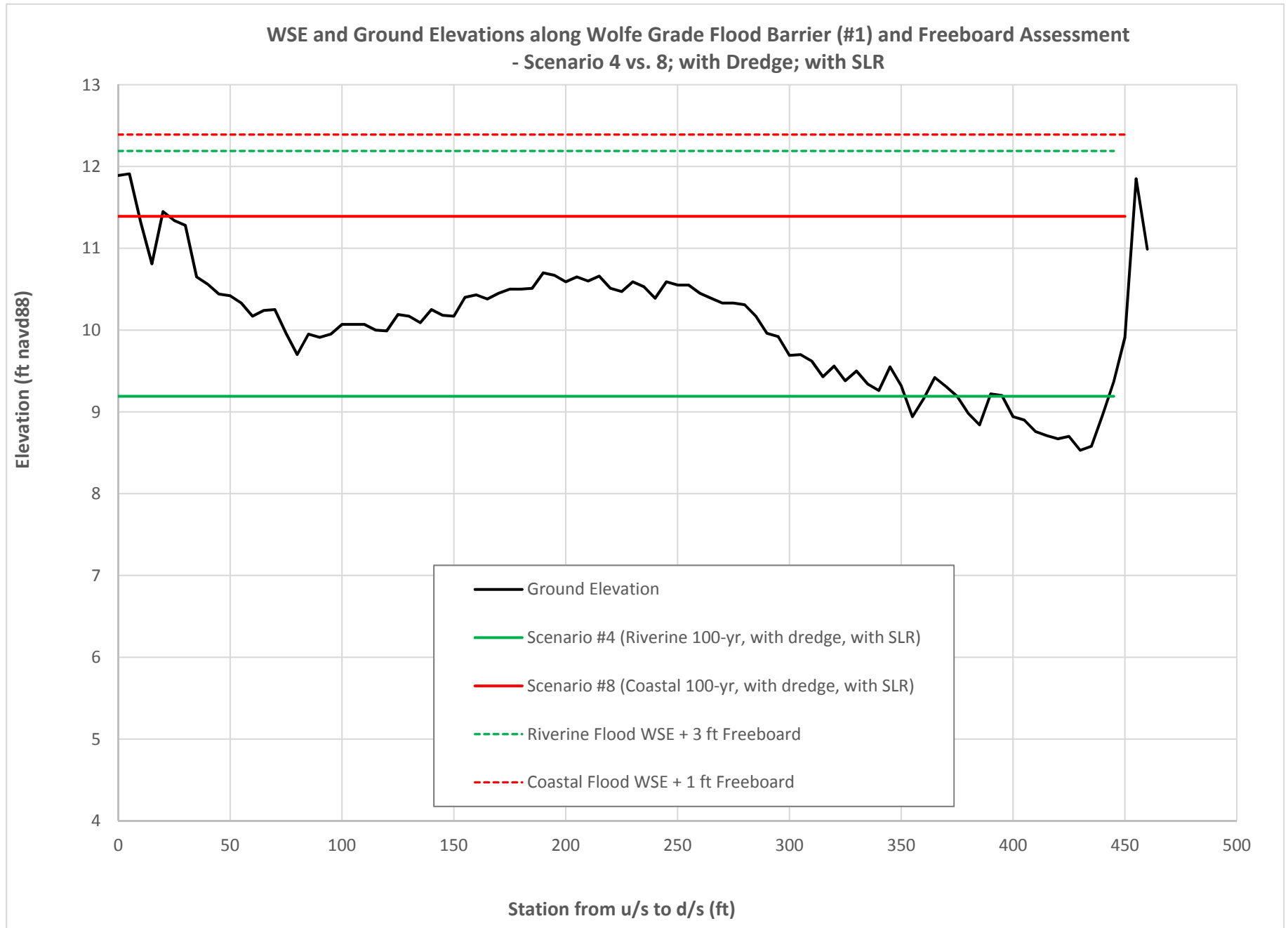


Figure 27a

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#2) and Freeboard Assessment  
- Scenario 1 vs. 5; No Dredge; No SLR

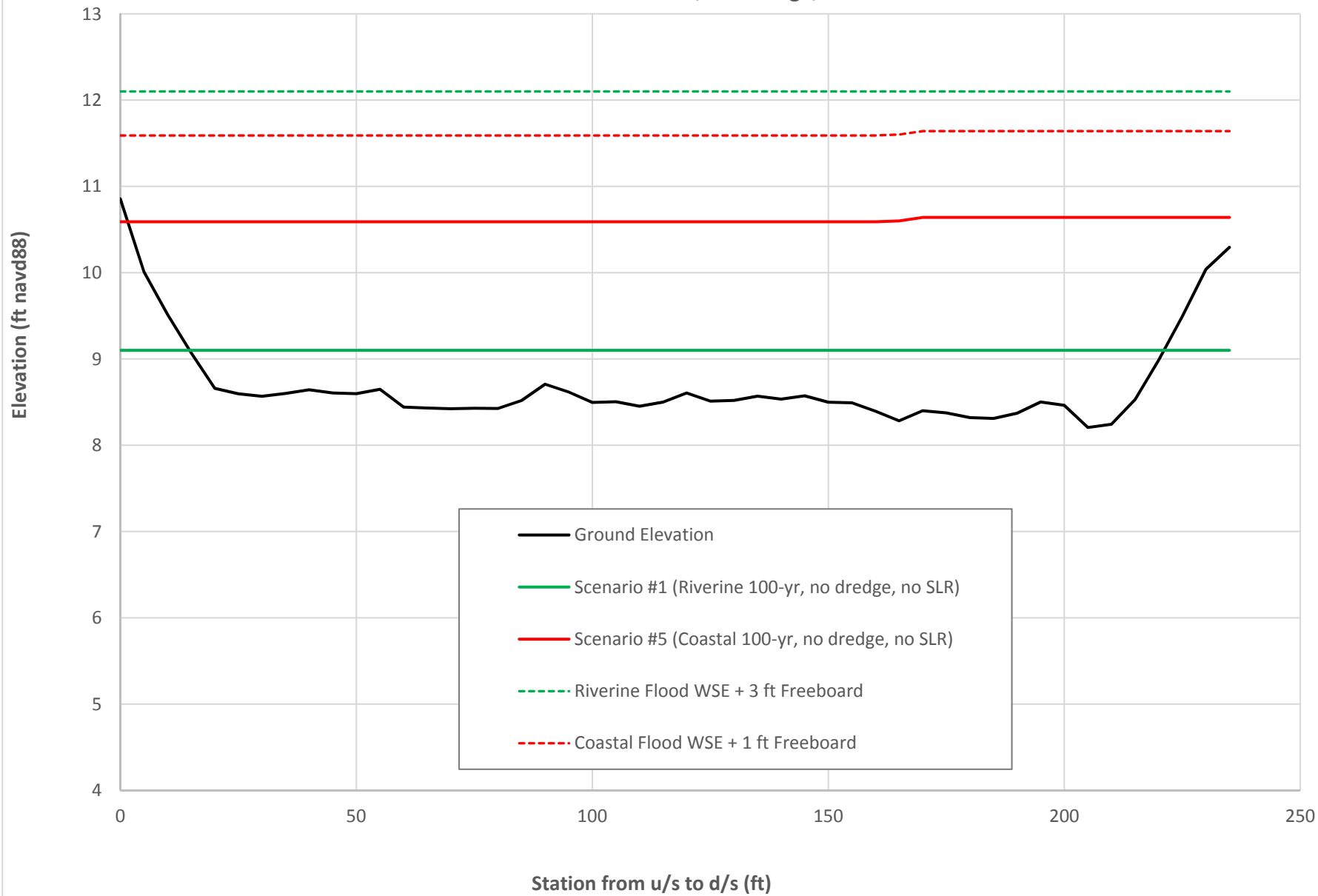




Figure 27b

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#2) and Freeboard Assessment  
- Scenario 2 vs. 6; No Dredge; with SLR

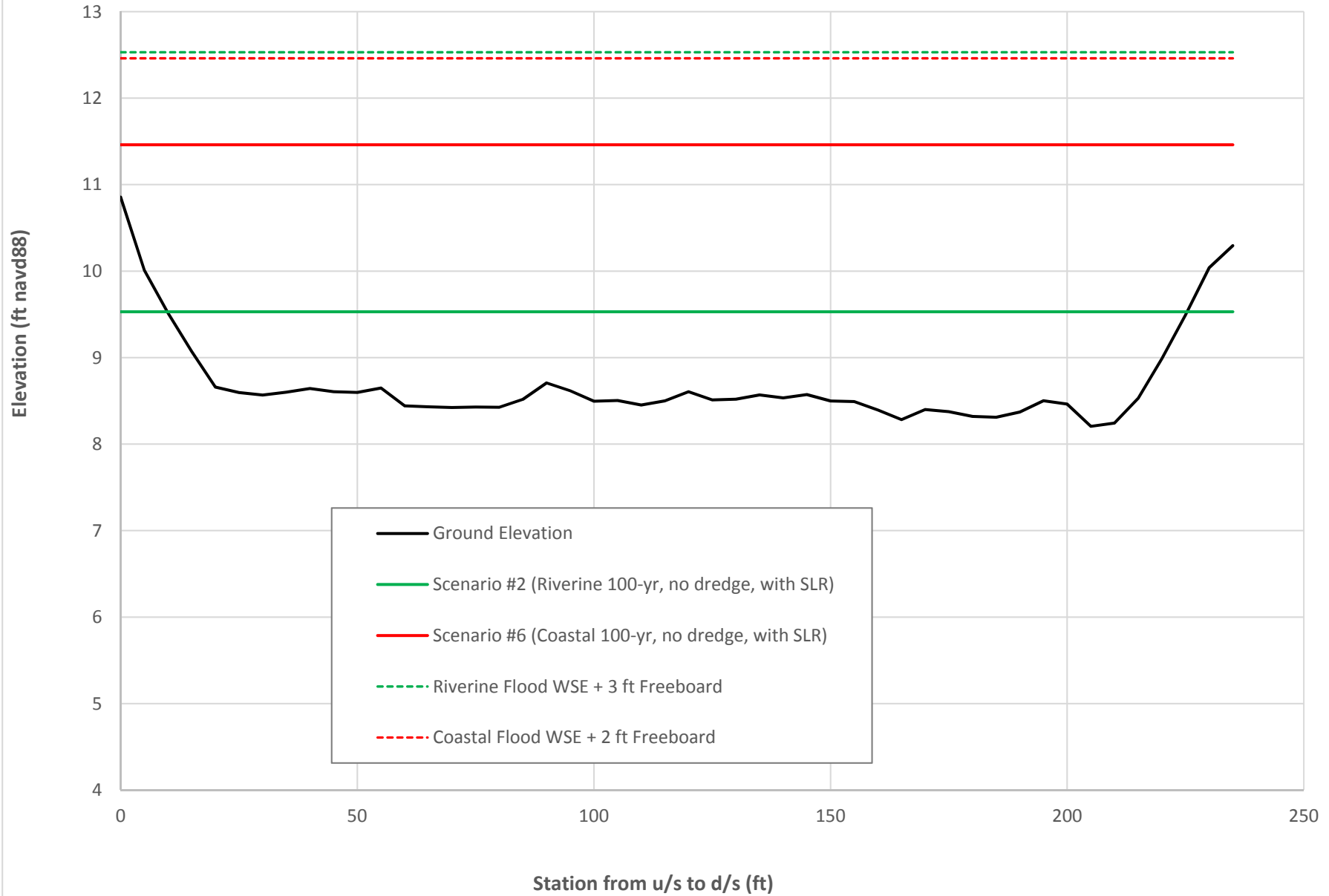


Figure 27c

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#2) and Freeboard Assessment  
- Scenario 3 vs. 7; with Dredge; No SLR

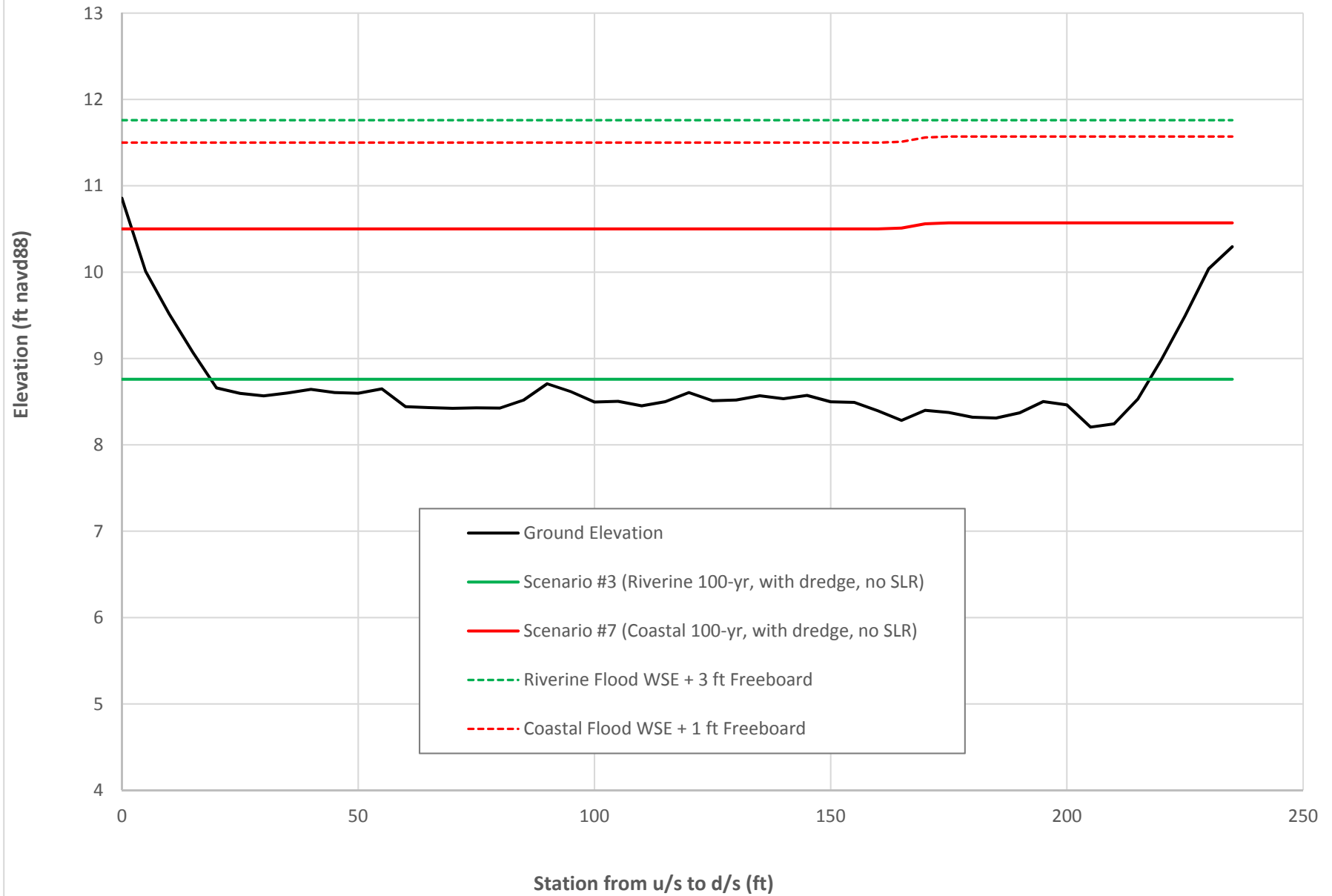


Figure 27d

WSE and Ground Elevations along Wolfe Grade Flood Barrier (#2) and Freeboard Assessment  
- Scenario 4 vs. 8; with Dredge; with SLR

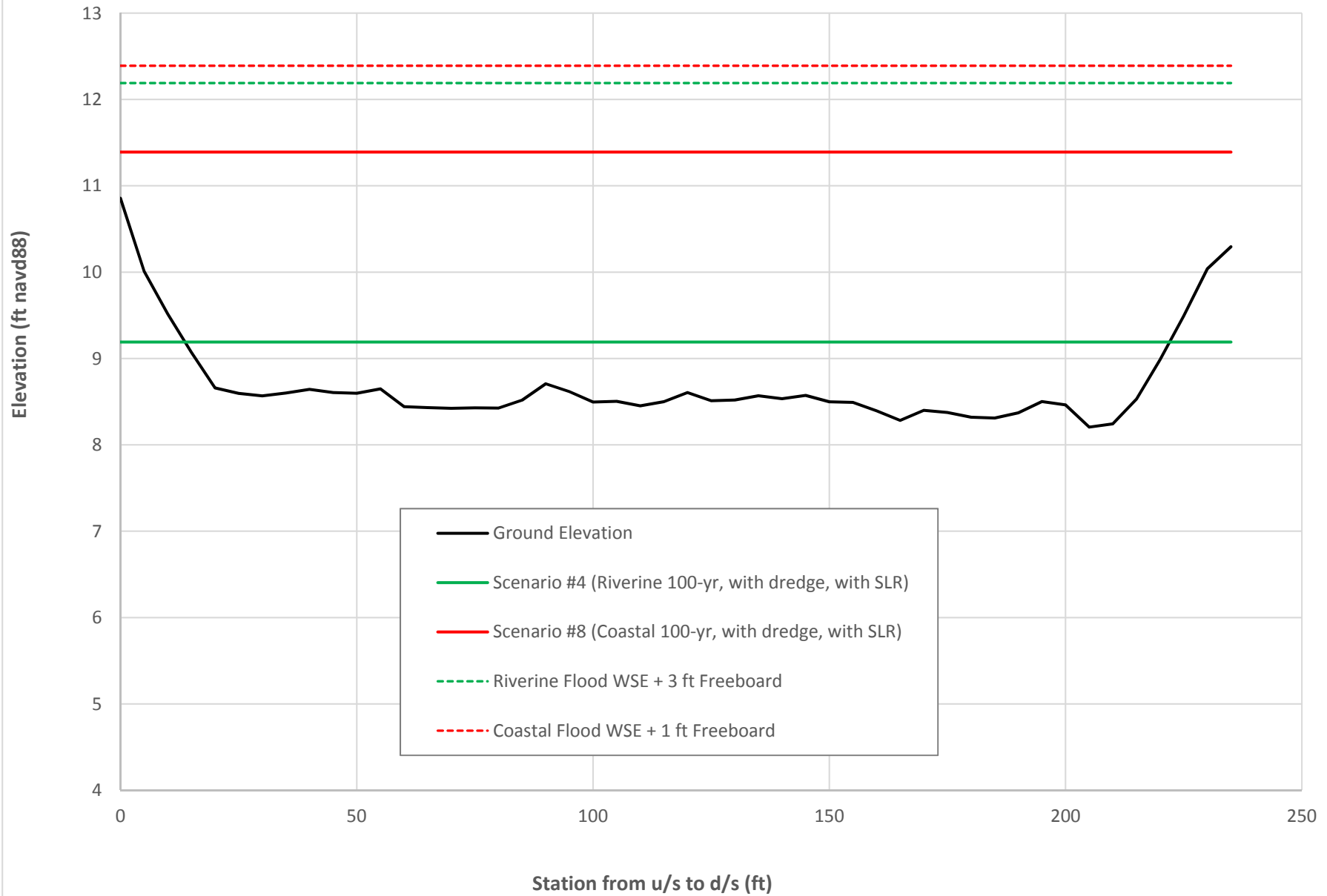


Figure 28a

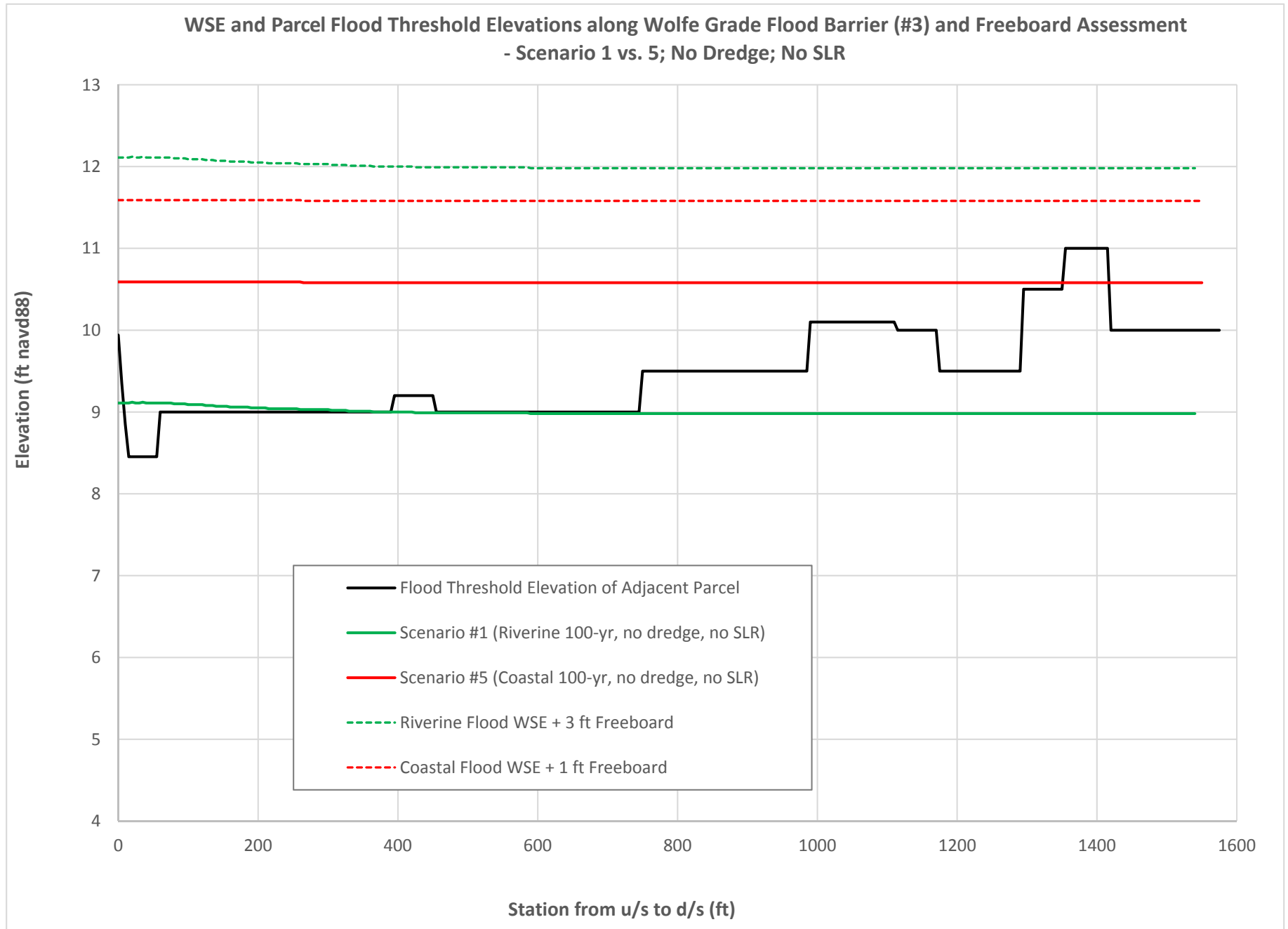


Figure 28b

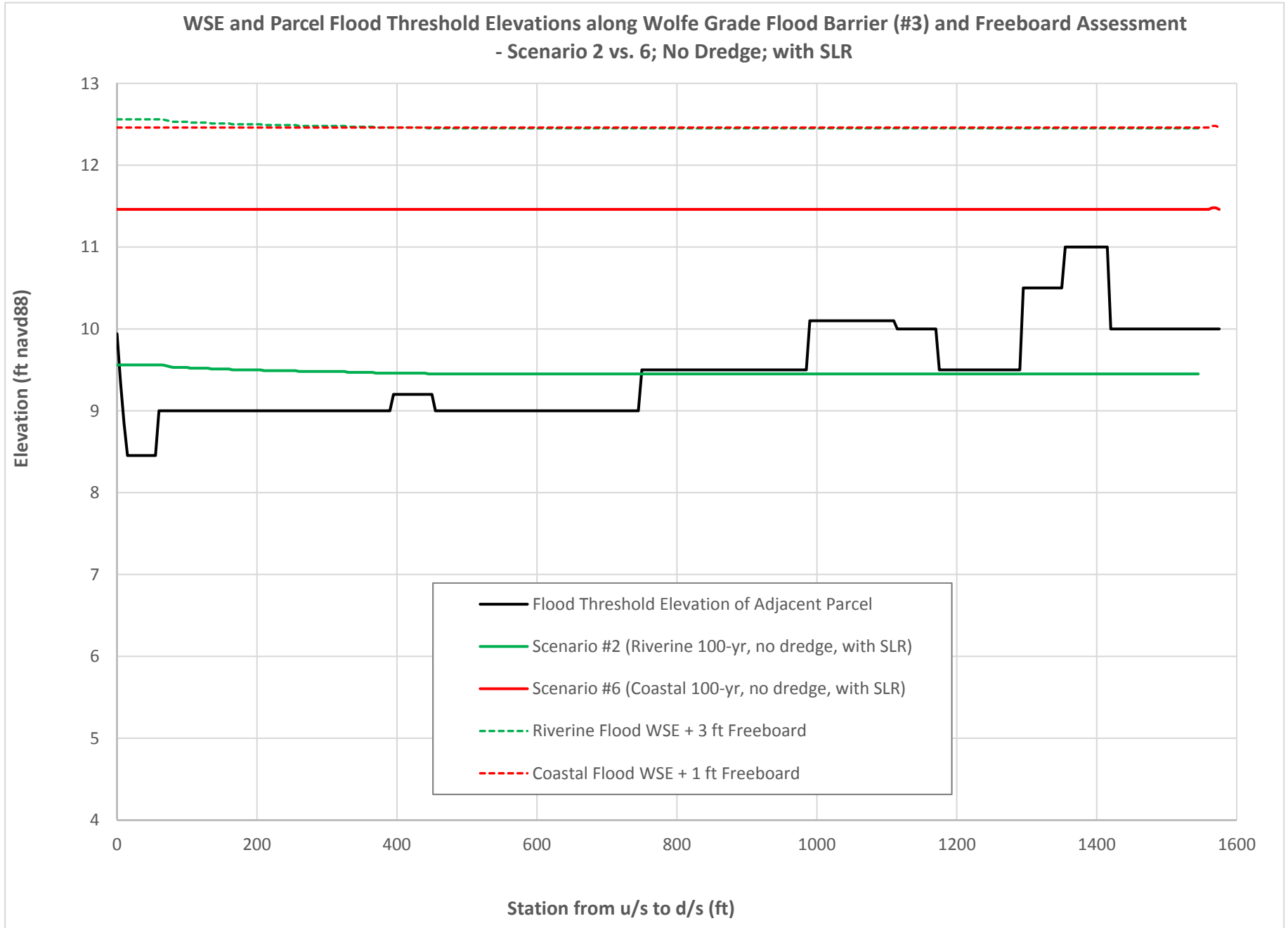


Figure 28c

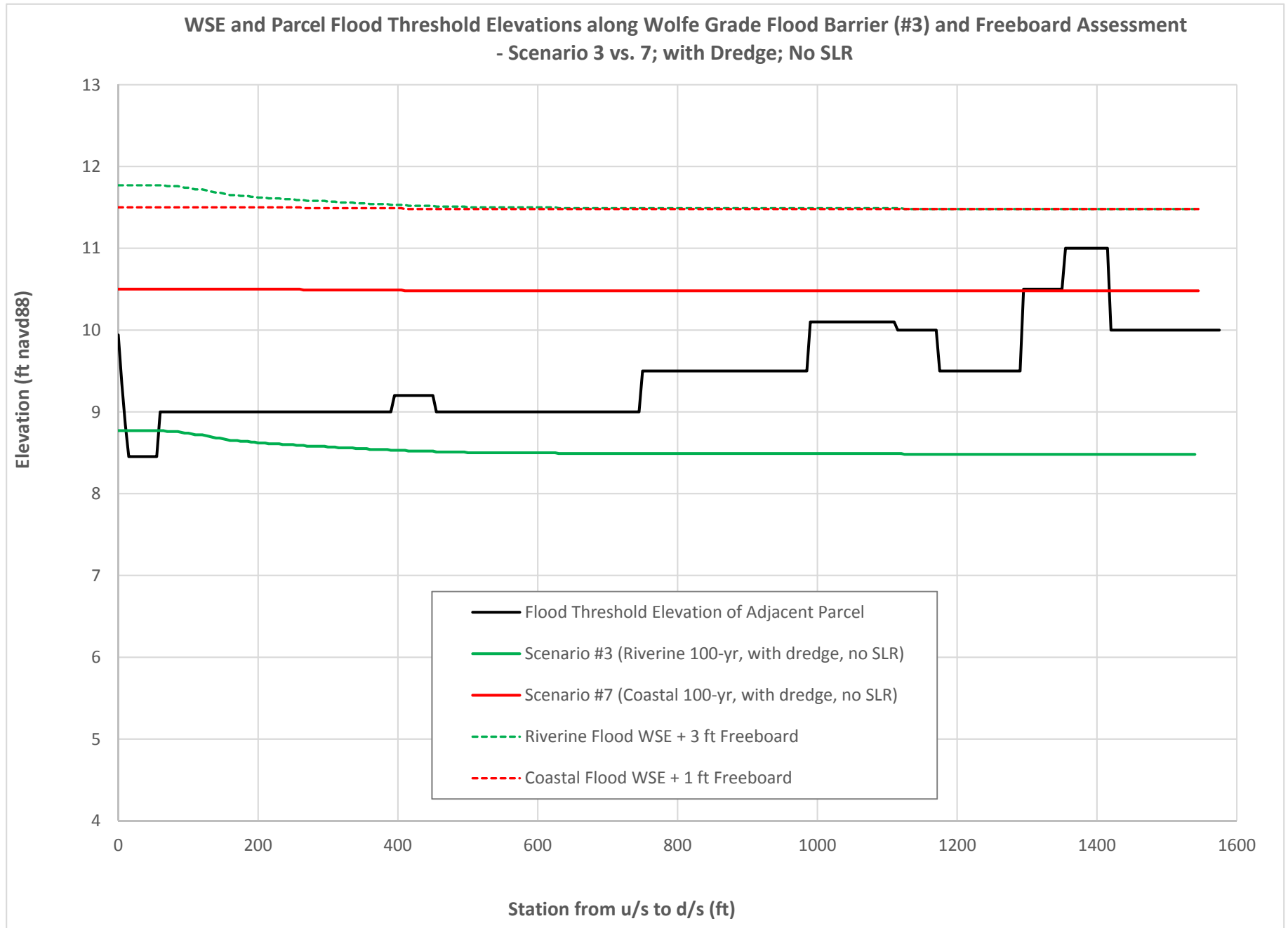
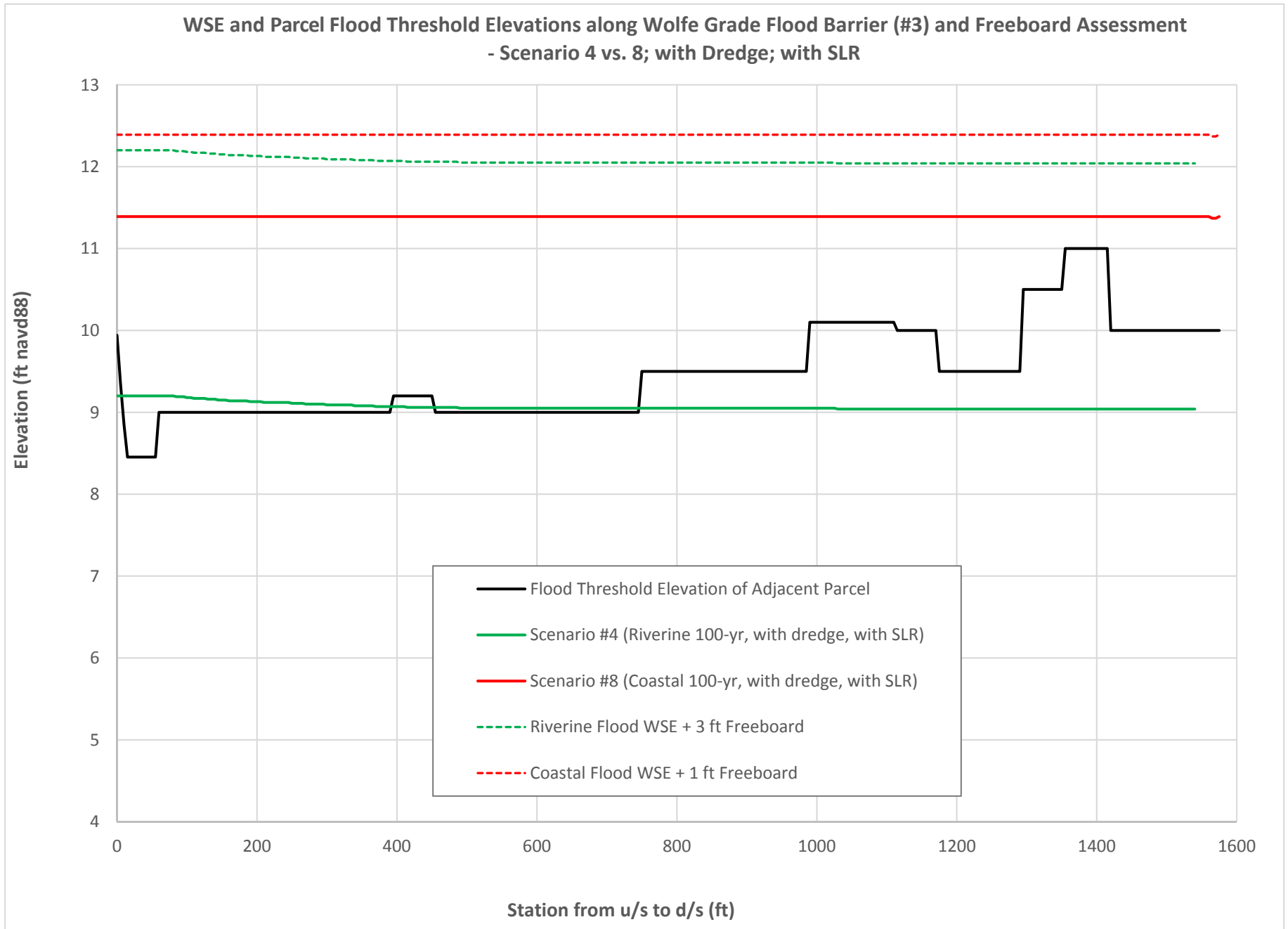
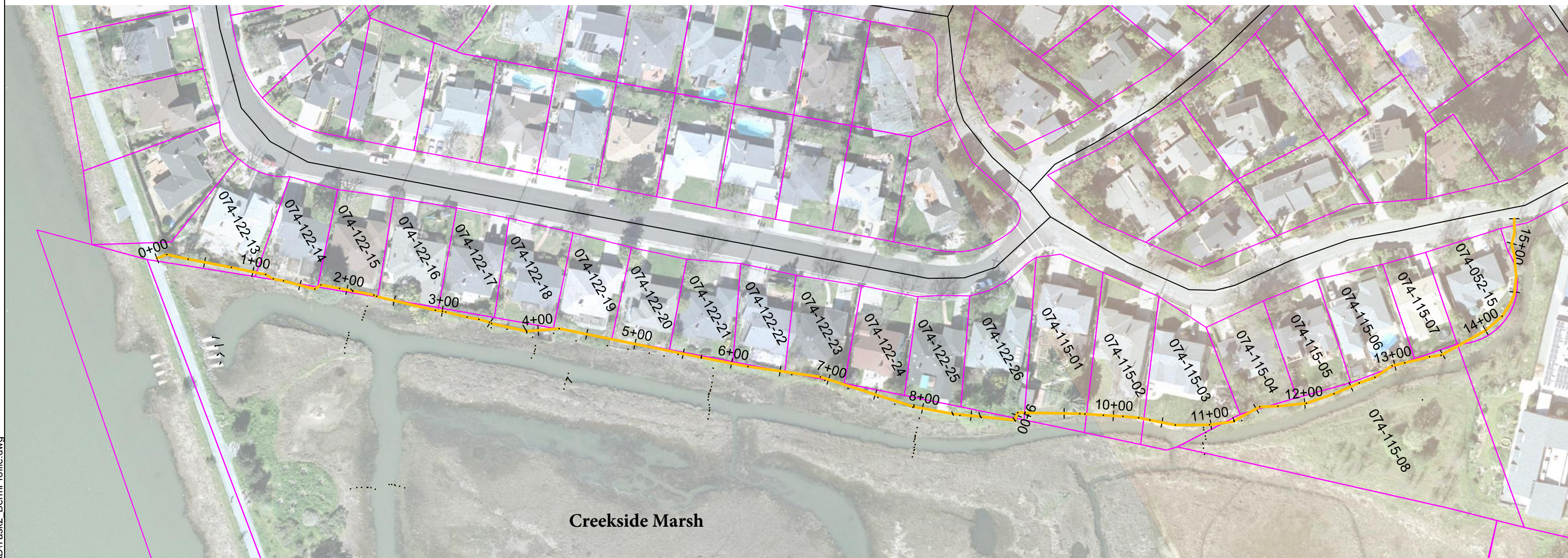
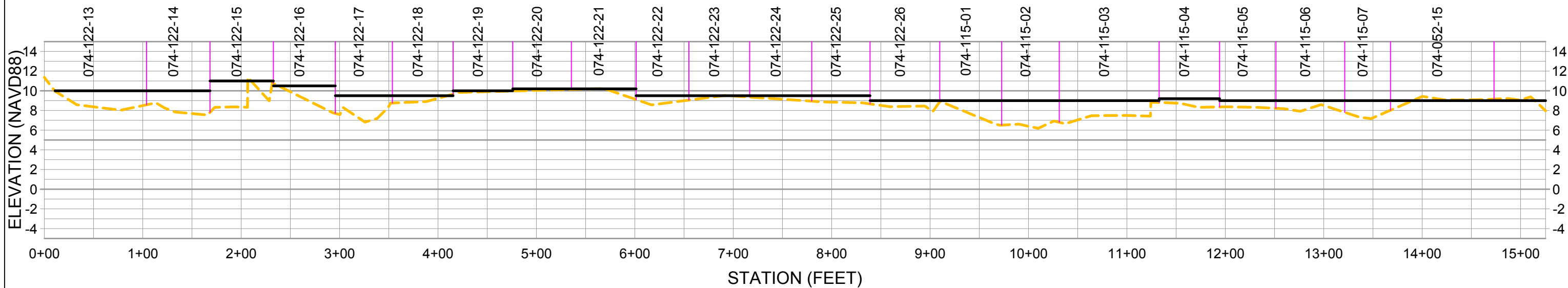


Figure 28d



### BERM PROFILE



**LEGEND**

- PARCEL EXTENT
- - - GROUND SURFACE ALONG PROFILE LINE
- PARCEL FLOOD THRESHOLD
- PROFILE ALIGNMENT LINE



DATE: October 22, 2012

JN: 2429

**CREEKSIDE MARSH NORTH-WEST BERM  
PLAN VIEW AND PROFILE**