

ATTACHMENT I
PRELIMINARY DRAINAGE AND HYDROLOGICAL STUDY

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**SOUTH COAST TECHNOLOGY
CENTER DEVELOPMENT
Preliminary Drainage and
Hydrological Study**

June 2024



06/06/2024

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INTRODUCTION

This hydrology study has been prepared for the South Coast Technology Center Development project in the City of Santa Ana, Orange County, CA. This project is partially in the developed land bounded by the USPS Distribution Center (south), Susan Street (west), Lake Center (north), and Greenville Banning Channel (GBC) (east); and within the undeveloped, dirt lot bounded by a private road adjacent to commercial businesses (south), railroad tracks (west), Lake Center (north), and Susan Street (east). The purpose of this report is to determine the pre-and post-project peak runoffs during the 10-year storm event for use in the analysis and design of the on-site grading and storm drainage systems. This is a preliminary hydrology and hydraulics (HnH) report and is intended to show the project's intent and general compliance with quality engineering practices. As the design of the project progresses, this report will be updated to eventually be a final HnH report.

PROJECT DESCRIPTION

This project proposes 15.8 acres of developed commercial land and undeveloped open land to be improved into industrial buildings, with (3) warehouses, landscaping, surface parking, and subsurface improvements (see Appendix A).

Runoff from the existing site flows either off-site into the nearest roadway, where the water will sheet flow into the nearest catch basin (given the environment's grading), or into the existing, on-site storm water drainage system. All runoff eventually discharges into the GBC, which outfalls into the Santa Ana River.

Runoff from the proposed project will generally follow the same flow path; however, less water will surface flow off-site, compared to the existing conditions. A new on-site storm drain system will collect surface water from the on-site BMP catch basins. The piped system will be connected to the existing underground system near the perimeter of the project, which flows to the off-site GBC or south to the mainline under Sunflower Avenue (discharging to the GBC/Santa Ana River, as previously stated).

The project's site will discharge stormwater at a decreased peak rate compared the existing condition on the development east of Susan St, at an increased peak rate compared to the existing conditions on the development west of Susan St; however, at a decreased peak rate overall. Due to the site being in a "non-susceptible to hydromodification region", no additional hydromodification measures will need to be conducted (see the preliminary WQMP developed for the project.)

METHODOLOGY

HYDROLOGIC CALCULATIONS

The hydrology calculations were completed following the methodology prescribed by the Rational Method of the Orange County Hydrology Manual - 1986 edition, and the Orange County Hydrology Manual Addendum No. I, dated 1996. To complete the standard procedures within the manual, the project location was surveyed to determine the different drainage subareas of the site and their characteristics (for the proposed conditions, all statistics were based on a designed site plan) – see Appendix B for the maps of the existing and proposed drainage mapping conditions. From the site visit and a web soil survey report from the United States Department of Agriculture (updated September 30, 2022 and see Appendix D) the existing site's Hydrologic Soils Group, Curve Number, Cover Type, and Quality of Cover were estimated. As permissible by the hydrology manual, the hydrologic calculator program RATSCX from the Advanced Engineering Software (AES) was used to conduct the calculations. The results produced from the methodology described above are tabulated below, in Hydrology Calculation Results. See Appendix F for the software results of the hydrology calculations described above.

Additional hydrologic comparisons were conducted to determine the impact the site's design has on the offsite points of connection (see Table 3 from the Results section).

HYDRAULIC CALCULATIONS

The preliminary design of the on-site storm drainage system was based on the following criteria: ensure the adequate size and performance of the network's pipes for the proposed hydrologic conditions.

To calculate adequate pipe sizing of the system's mainlines, the site was divided per tributary area of each mainline. Using Manning's Equation, the maximum flow of the mainlines were used to determine the adequate size of pipes under the 10-year storm. The onsite storm drain network (see the Proposed Hydrology Map under Appendix B) was modeled using the following assumptions:

- Proposed Pipes are HDPE ($n = 0.011$).
- Existing RCP Pipe $n = 0.013$.
- Minimum allowed pipe slope = 0.25%.
- Desired for discharge to remain in pipe (<100% full)

See the Hydraulic Results section for the results and see Appendix G for the supporting figures based on the hydraulics methodology described above.

HYDROLOGY CALCULATION RESULTS

The following are the summarized results of the Orange County Rational Method calculations for the 10-year storm event:

Table 1: Existing Hydrology Results

Subarea	Area (ac)	Impervious Percentage	10-Year Peak Runoff (cfs)
E1	3.32	60%	7.28
E2	0.07	25%	0.25
E3	0.28	0%	1.02
E4	0.24	0%	0.87
E5	0.36	0%	1.31
E6	0.83	0%	3.01
E7	0.12	100%	0.41
E8	0.09	100%	0.31
E9	0.82	19%	2.96
E10	0.47	26%	1.69
E11	0.11	41%	0.39
E12	0.36	53%	1.03
E13	1.07	31%	3.08
E14	0.47	78%	1.59
E15	0.47	0%	1.71
E16	0.44	0%	1.60
E17	0.47	0%	1.71
E18	0.42	27%	1.22
E19	0.04	41%	0.14
E20	0.45	100%	1.51
W1	1.79	100%	3.10
W2	1.03	100%	2.50
W3	2.99	100%	6.64

Table 2: Proposed Hydrology Results

Subarea	Area (ac)	Impervious Percentage	10-Year Peak Runoff (cfs)
W1	0.50	72%	1.47
W2	0.27	66%	0.83
W3	0.67	68%	2.06
W4	0.59	100%	1.39
W5	0.81	69%	2.5
W6	0.59	100%	1.39
W7	0.64	100%	1.45
W8	0.42	71%	1.23
W9	0.43	87%	1.24
W10	0.26	100%	0.8

W11	0.59	100%	1.35
SE1	0.68	68%	1.62
SE2	0.97	77%	2.29
SE3	0.35	79%	0.81
SE4	0.90	84%	2.07
SE5	0.76	72%	1.77
SE6	0.63	100%	1.44
SE7	0.65	100%	1.52
SE8	0.65	100%	1.49
SE9	0.63	100%	1.44
SE10	0.22	100%	0.72
E1	0.63	60%	1.61
E2	1.39	80%	3.45
E3	0.50	87%	1.23
E4	0.20	100%	0.65
E5	0.46	100%	1.13
E6	0.45	100%	1.12
E7	0.38	100%	0.94
E8	0.46	100%	1.07

Table 3: Project Hydrology Runoff to Watershed Outfalls Summary

Return Period 10-YR	Existing Site Runoff (cfs)	Proposed Site Runoff (cfs)
Susan Street (E1 and W3)	13.92	7.45
Southeast GBC Pipe	6.87	0
East GBC Pipe	18.09	25.99
Railroad Space (W2)	2.50	0
Private Street CB (W1)	3.10	7.40
TOTAL	44.48	40.84

Note: The values in Table 3 are only concerning the subareas/land that have changed from the proposed development (any conditions that will not change, are not considered for analysis). See the Watershed Maps in Appendix B for the location of the outfalls.

The calculations and results produced by the AES software are included in Appendix F.

HYDRAULIC CALCULATION RESULTS

The following are the summarized results of the storm drain analysis.

Table 4: Storm Drain Mainline Capacity Summary

Mainline Location	Mainline Pipe Size	Pipe Fullness (%)	Discharge Capacity (cfs)*
W1 Watershed	18" DIA (RCP)	95	7.43
E Watershed	24" DIA	95	14.64
	27" DIA	95	20.05
	39" DIA (RCP)	95	36.92

Note: Table 4 depicts the worst-case scenario that would occur.

* Discharge Capacities expected to increase when evaluated during final design and accounting for HGL above pipe soffit.

Watershed W1's storm drain system is connecting to an existing 18" RCP line, that of which is to be protected in place. The proposed flow beyond determined capacity of the pipe (to be confirmed during final design), will discharge to under sidewalk culverts placed along Lake Center Ave. and Susan St. to discharge excess water into the existing curb and gutters. The existing site is currently discharging to the adjacent streets, such that the proposed design will utilize the adjacent infrastructure as necessary. (The underside walk culverts will be placed east of Susan as well)

The mainlines under watershed E will be conveying the watershed's runoff and upstream discharge to the GBC. Upstream of watershed E are (2) existing 24" lines that will be used for the development of the site (east and west mainlines). The westerly line will be rerouted through the proposed development. The design of it will match the existing size until stepping up to a 27" line to convey the discharge to the existing manhole, combining flows from the westerly and easterly halves of the watershed. The easterly existing 24" line will be protected in place and utilized accordingly. The existing 39" line, downstream of the intersection manhole, discharges into the GBC. The existing southeast storm drain connection to the GBC will also be protected in place and available for discharge during final design as needed.

The calculations are included in Appendix G.

CONCLUSION

The project will increase imperviousness; however, due to modifications in the stormwater flow paths, resulting in an increase in time of concentration, the peak runoff produced from the project is expected to decrease or have no change.

The development has been designed to effectively capture and convey the project's storm water to the existing/public systems during a 10-year storm. This system continues the flow patterns of the existing conditions, utilizing the street's infrastructure and an onsite storm drain system.

See the following appendices for supporting calculations and graphics.

REFERENCES

City of Santa Ana Zoning Map, February 2023.

Orange County Hydrology Manual, 1986.

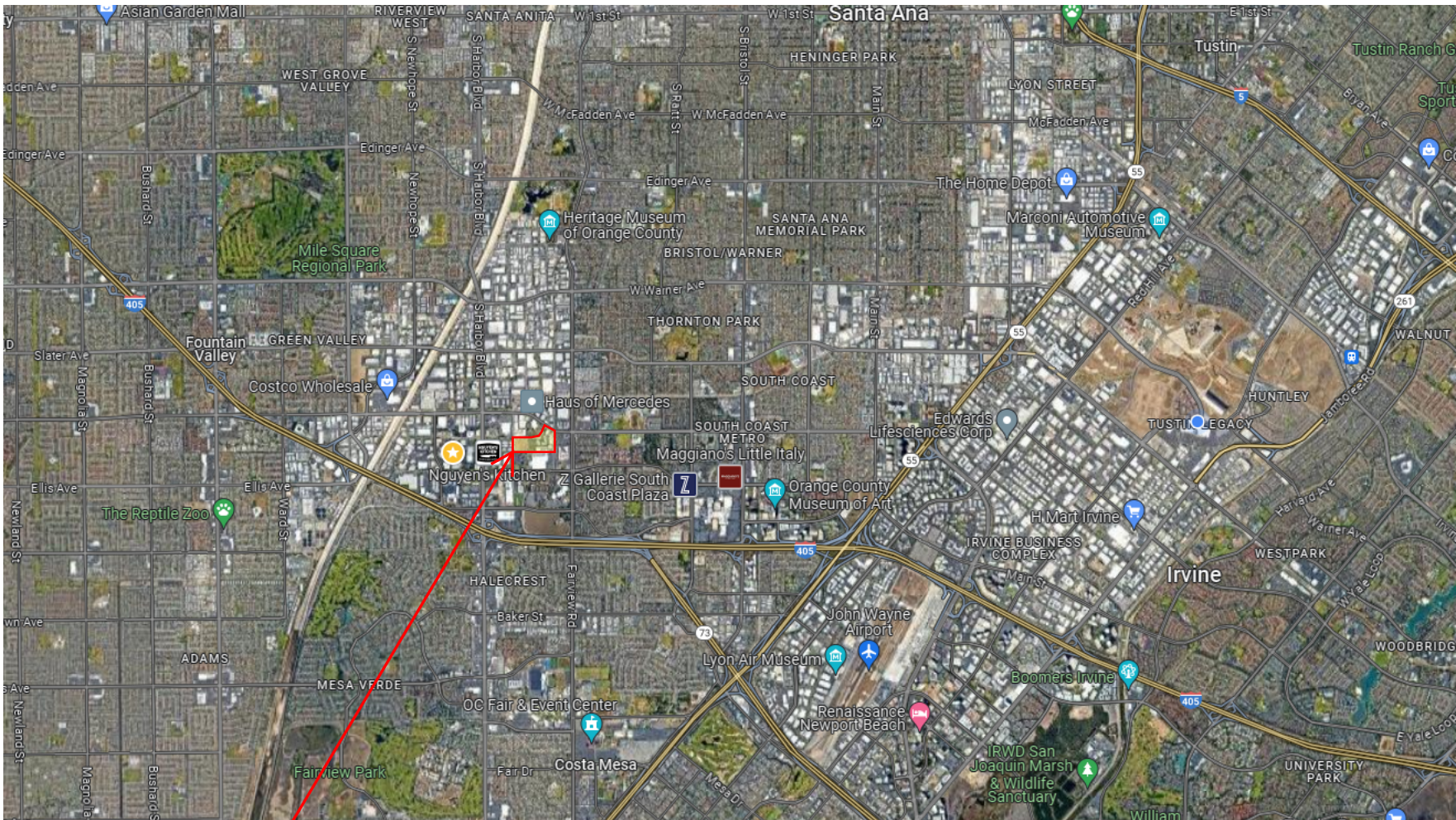
Orange County Hydrology Manual Addendum No. I, 1996.

National Oceanic and Atmospheric Administration - Atlas 14 Point Precipitation Frequency Estimates: CA.

Orange County Local Drainage Manual, May 2021.

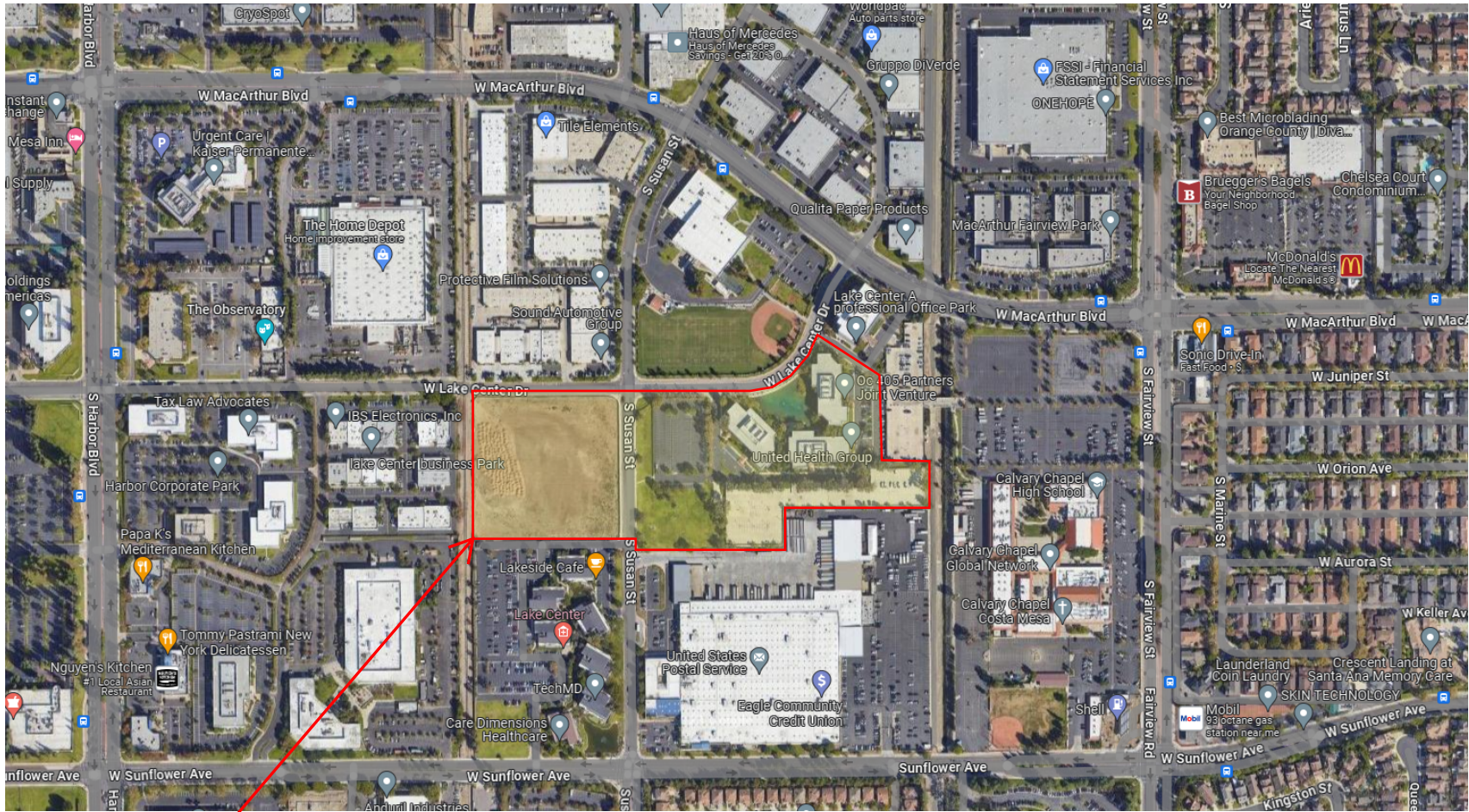
United States Department of Agriculture – National Resources Conservation Services Web Soil Survey, September 2022.

APPENDIX A- PROJECT LOCATION MAPS



**PROJECT
LOCATION**

**LOCATION MAP
SOUTH COAST TECHNOLOGY CENTER DEVELOPMENT**



PROJECT SITE

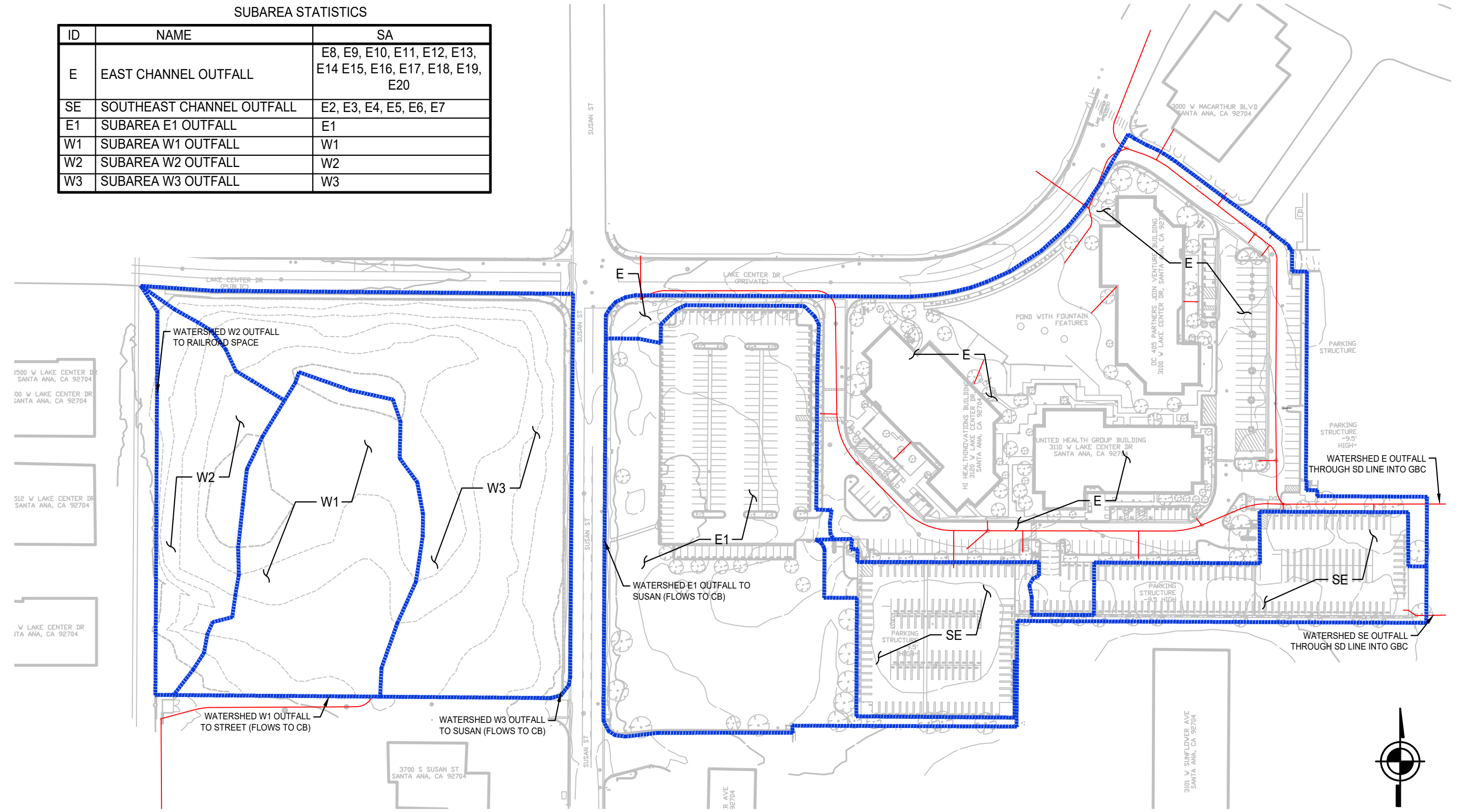
VICINITY MAP
SOUTH COAST TECHNOLOGY CENTER DEVELOPMENT

APPENDIX B- PROJECT DRAINAGE MAPS

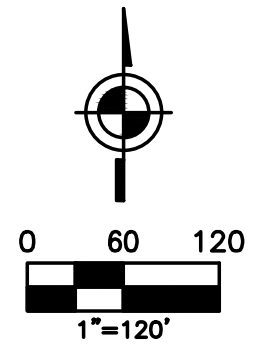
SUBAREA STATISTICS

ID	NAME	SA
E	EAST CHANNEL OUTFALL	E8, E9, E10, E11, E12, E13, E14, E15, E16, E17, E18, E19, E20
SE	SOUTHEAST CHANNEL OUTFALL	E2, E3, E4, E5, E6, E7
E1	SUBAREA E1 OUTFALL	E1
W1	SUBAREA W1 OUTFALL	W1
W2	SUBAREA W2 OUTFALL	W2
W3	SUBAREA W3 OUTFALL	W3

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1/15/2024



- LEGEND:
- - - - - WATERSHED BOUNDARY
 - STORM DRAIN CONVEYANCE
 - / / / / / ABANDONED OR DEMOLISHED STORM DRAIN

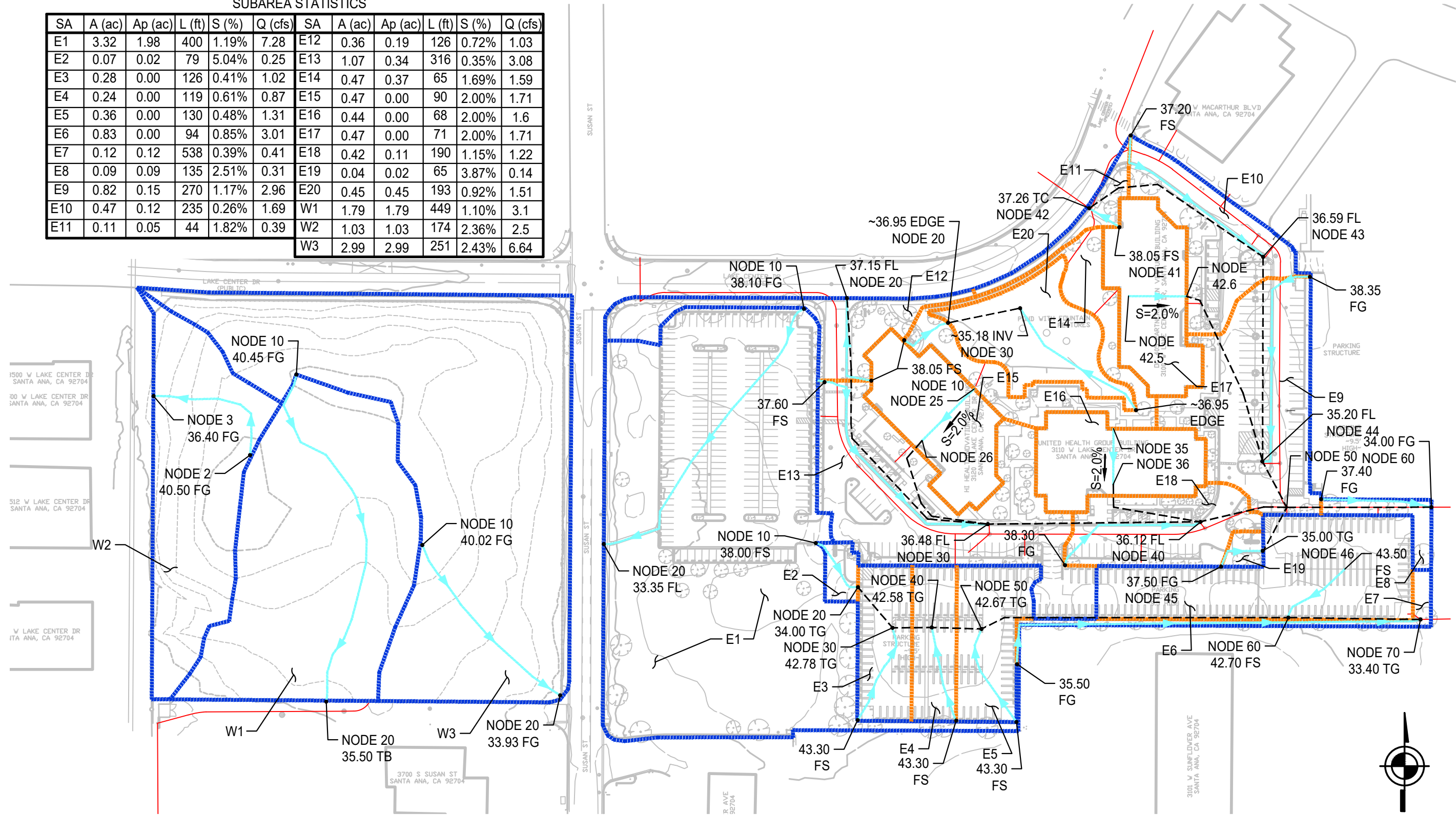


EXISTING WATERSHED MAP
SOUTH COAST TECHNOLOGY CENTER
DECEMBER 2023

SUBAREA STATISTICS

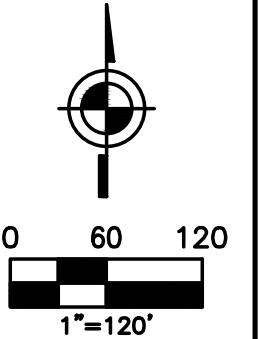
SA	A (ac)	Ap (ac)	L (ft)	S (%)	Q (cfs)	SA	A (ac)	Ap (ac)	L (ft)	S (%)	Q (cfs)
E1	3.32	1.98	400	1.19%	7.28	E12	0.36	0.19	126	0.72%	1.03
E2	0.07	0.02	79	5.04%	0.25	E13	1.07	0.34	316	0.35%	3.08
E3	0.28	0.00	126	0.41%	1.02	E14	0.47	0.37	65	1.69%	1.59
E4	0.24	0.00	119	0.61%	0.87	E15	0.47	0.00	90	2.00%	1.71
E5	0.36	0.00	130	0.48%	1.31	E16	0.44	0.00	68	2.00%	1.6
E6	0.83	0.00	94	0.85%	3.01	E17	0.47	0.00	71	2.00%	1.71
E7	0.12	0.12	538	0.39%	0.41	E18	0.42	0.11	190	1.15%	1.22
E8	0.09	0.09	135	2.51%	0.31	E19	0.04	0.02	65	3.87%	0.14
E9	0.82	0.15	270	1.17%	2.96	E20	0.45	0.45	193	0.92%	1.51
E10	0.47	0.12	235	0.26%	1.69	W1	1.79	1.79	449	1.10%	3.1
E11	0.11	0.05	44	1.82%	0.39	W2	1.03	1.03	174	2.36%	2.5
						W3	2.99	2.99	251	2.43%	6.64

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LEGEND:

- SUBAREA BOUNDARY
- WATERSHED BOUNDARY
- FLOW LINE
- STORM DRAIN CONVEYANCE
- HYDROLOGIC ROUTING PATH (SUBSEQUENT TO FLOW LINE)
- ABANDONED OR DEMOLISHED STORM DRAIN



NOTE:
ALL MAPPING AND STATISTICS SHOWN HEREON ARE IDENTICAL FOR ALL STORM EVENTS. 10-YEAR Q VALUES SHOWN FOR REFERENCE.

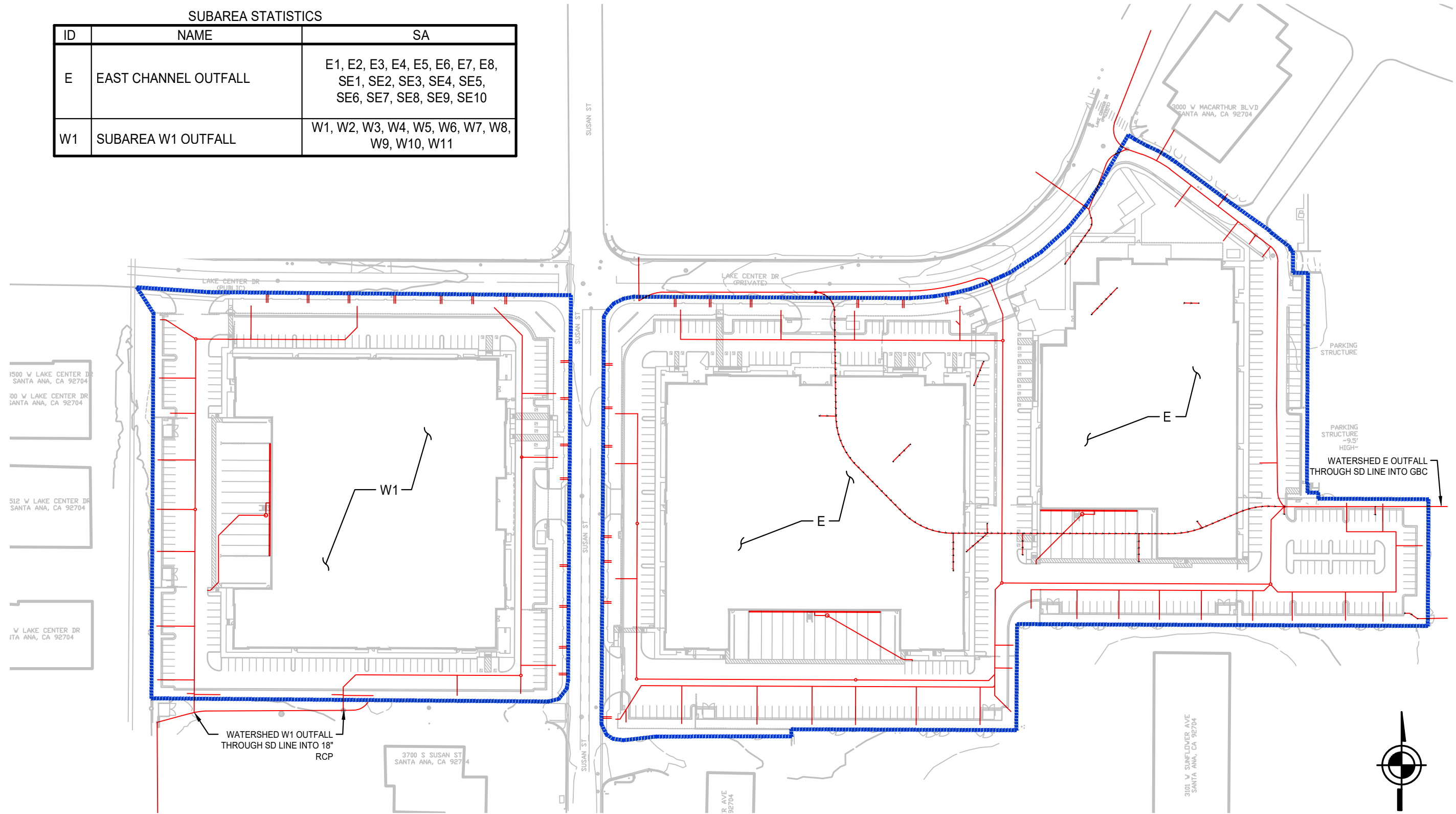
NOTE:
ROUTING NODES SHOWN HEREON MATCH NODES WITHIN CALCULATIONS. LARGER NODES ARE DOWNSTREAM OF SMALLER NODES.

EXISTING HYDROLOGY PLAN
SOUTH COAST TECHNOLOGY CENTER
DECEMBER 2023

SUBAREA STATISTICS

ID	NAME	SA
E	EAST CHANNEL OUTFALL	E1, E2, E3, E4, E5, E6, E7, E8, SE1, SE2, SE3, SE4, SE5, SE6, SE7, SE8, SE9, SE10
W1	SUBAREA W1 OUTFALL	W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11

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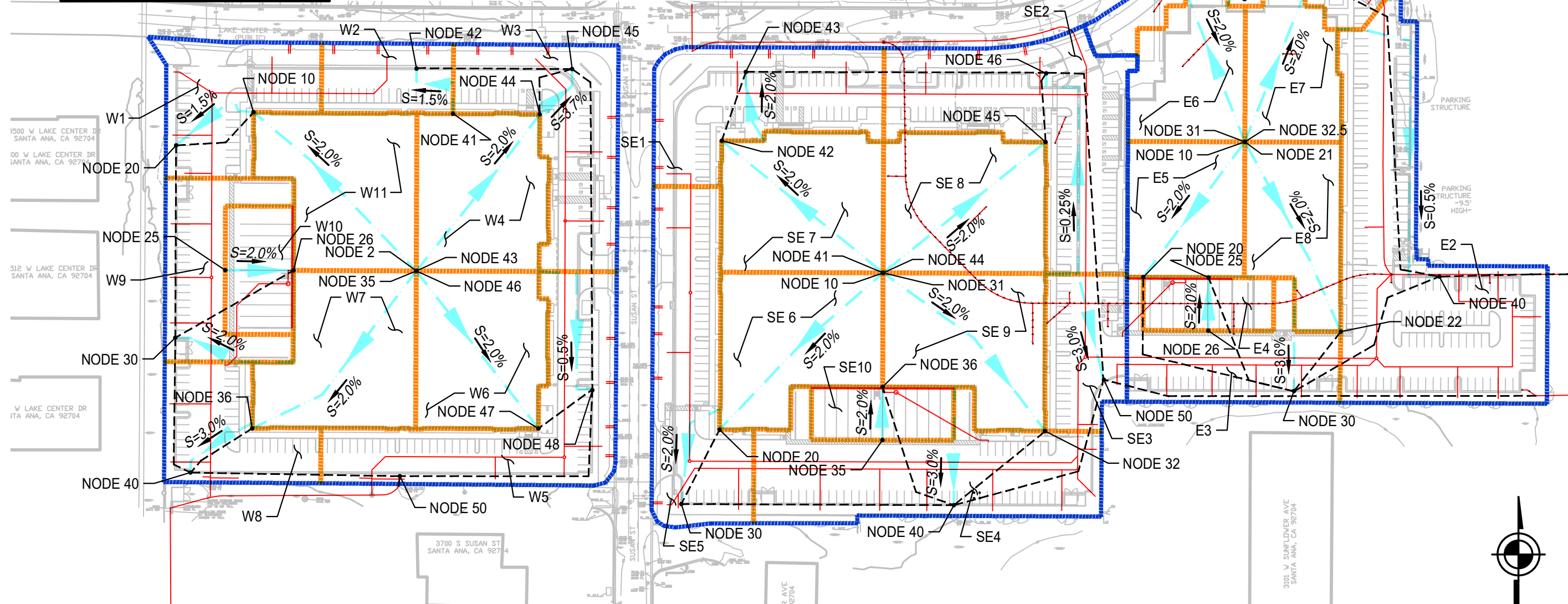


LEGEND:

- WATERSHED BOUNDARY
- STORM DRAIN CONVEYANCE
- ABANDONED OR DEMOLISHED STORM DRAIN

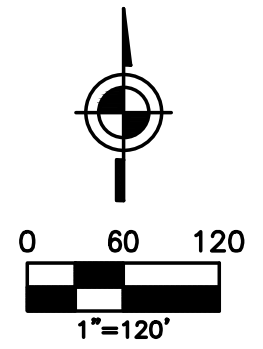
SUBAREA STATISTICS

SA	A (ac)	Ap (ac)	L (ft)	S (%)	Q (cfs)	SA	A (ac)	Ap (ac)	L (ft)	S (%)	Q (cfs)	SA	A (ac)	Ap (ac)	L (ft)	S (%)	Q (cfs)
W1	0.50	0.14	111.1	1.5%	1.47	SE1	0.68	0.22	82	2.0%	1.62	E1	0.63	0.26	76.3	1.5%	1.61
W2	0.27	0.09	91	1.5%	0.83	SE2	0.97	0.22	299	0.3%	2.29	E2	1.39	0.28	384	0.5%	3.45
W3	0.67	0.21	68	3.7%	2.06	SE3	0.35	0.07	160	3.0%	0.81	E3	0.50	0.07	128	3.6%	1.23
W4	0.59	0.00	225	2.0%	1.39	SE4	0.90	0.15	133	3.0%	2.07	E4	0.20	0.00	60	2.0%	0.65
W5	0.81	0.25	190	0.5%	2.5	SE5	0.76	0.21	128.5	2.0%	1.77	E5	0.46	0.00	191	2.0%	1.13
W6	0.59	0.00	225	2.0%	1.39	SE6	0.63	0.00	256	2.0%	1.44	E6	0.45	0.00	182.5	2.0%	1.12
W7	0.64	0.00	263	2.0%	1.45	SE7	0.65	0.00	235	2.0%	1.52	E7	0.38	0.00	182.5	2.0%	0.94
W8	0.42	0.12	95	3.0%	1.23	SE8	0.65	0.00	253	2.0%	1.49	E8	0.46	0.00	240	2.0%	1.07
W9	0.43	0.06	139	2.0%	1.24	SE9	0.63	0.00	258	2.0%	1.44						
W10	0.26	0.00	77	2.0%	0.8	SE10	0.22	0.00	60	2.0%	0.72						
W11	0.59	0.00	257	2.0%	1.35												



LEGEND:

- SUBAREA BOUNDARY
- WATERSHED BOUNDARY
- FLOW LINE
- STORM DRAIN CONVEYANCE
- HYDROLOGIC ROUTING PATH (SUBSEQUENT TO FLOW LINE)
- ABANDONED OR DEMOLISHED STORM DRAIN



NOTE:
ALL MAPPING AND STATISTICS SHOWN HEREON ARE IDENTICAL FOR ALL STORM EVENTS. 10-YEAR Q VALUES SHOWN FOR REFERENCE.

NOTE:
ROUTING NODES SHOWN HEREON MATCH NODES WITHIN CALCULATIONS. LARGER NODES ARE DOWNSTREAM OF SMALLER NODES.

PROPOSED HYDROLOGY PLAN
SOUTH COAST TECHNOLOGY CENTER
APRIL 2024

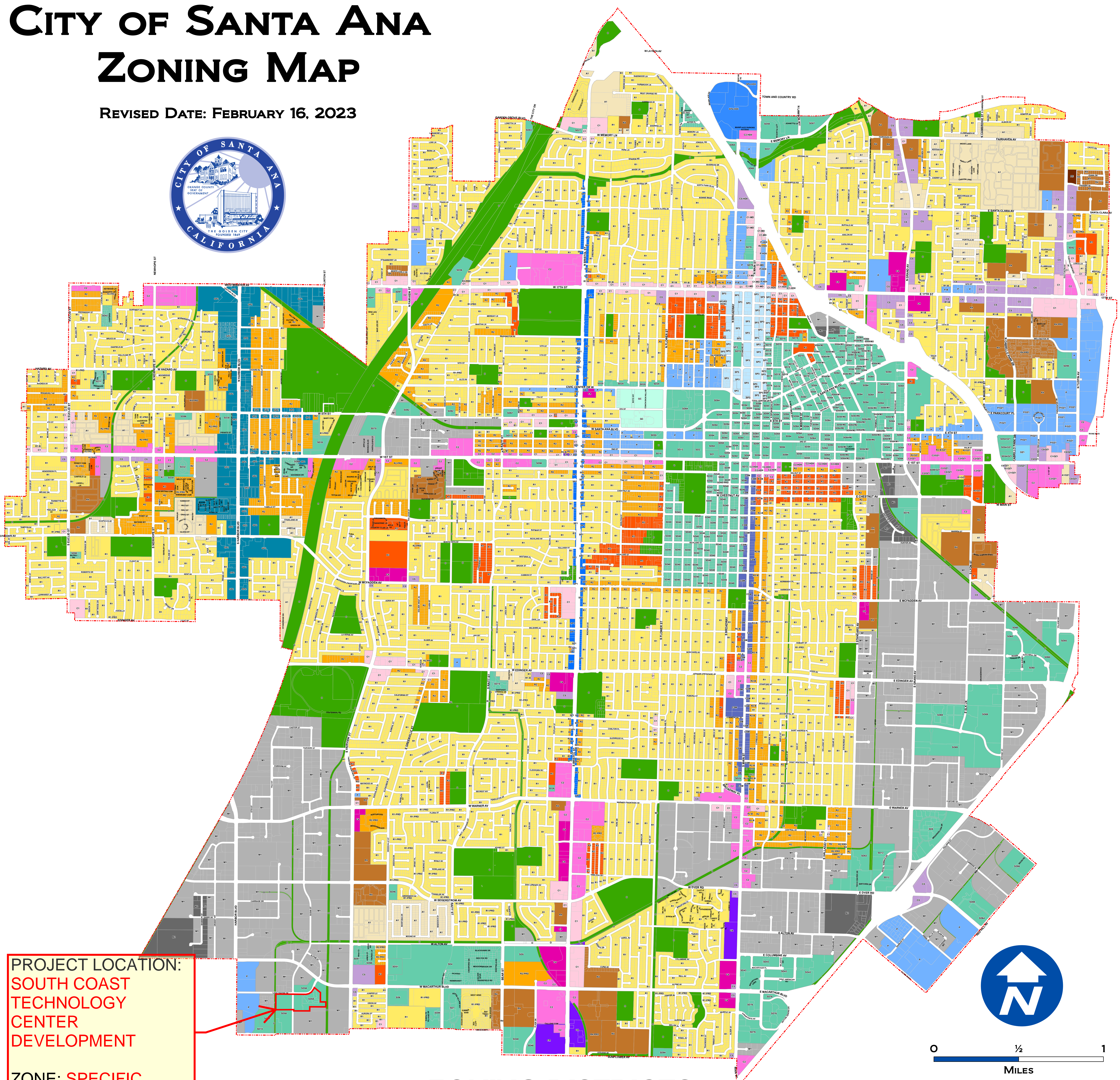
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APPENDIX C- LAND USE MAPS

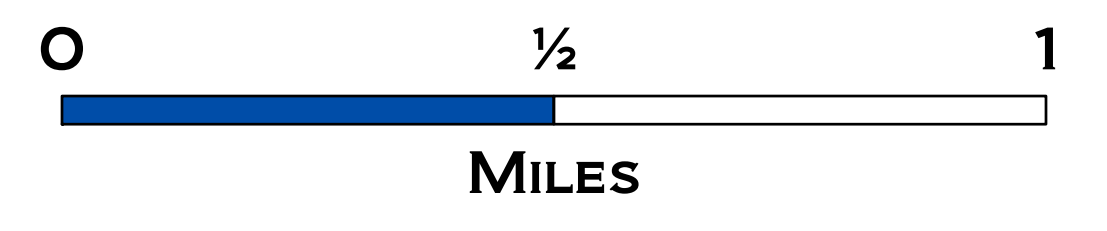
CITY OF SANTA ANA ZONING MAP

REVISED DATE: FEBRUARY 16, 2023



PROJECT LOCATION:
SOUTH COAST
TECHNOLOGY
CENTER
DEVELOPMENT

**ZONE: SPECIFIC
DEVELOPMENT (SD)**

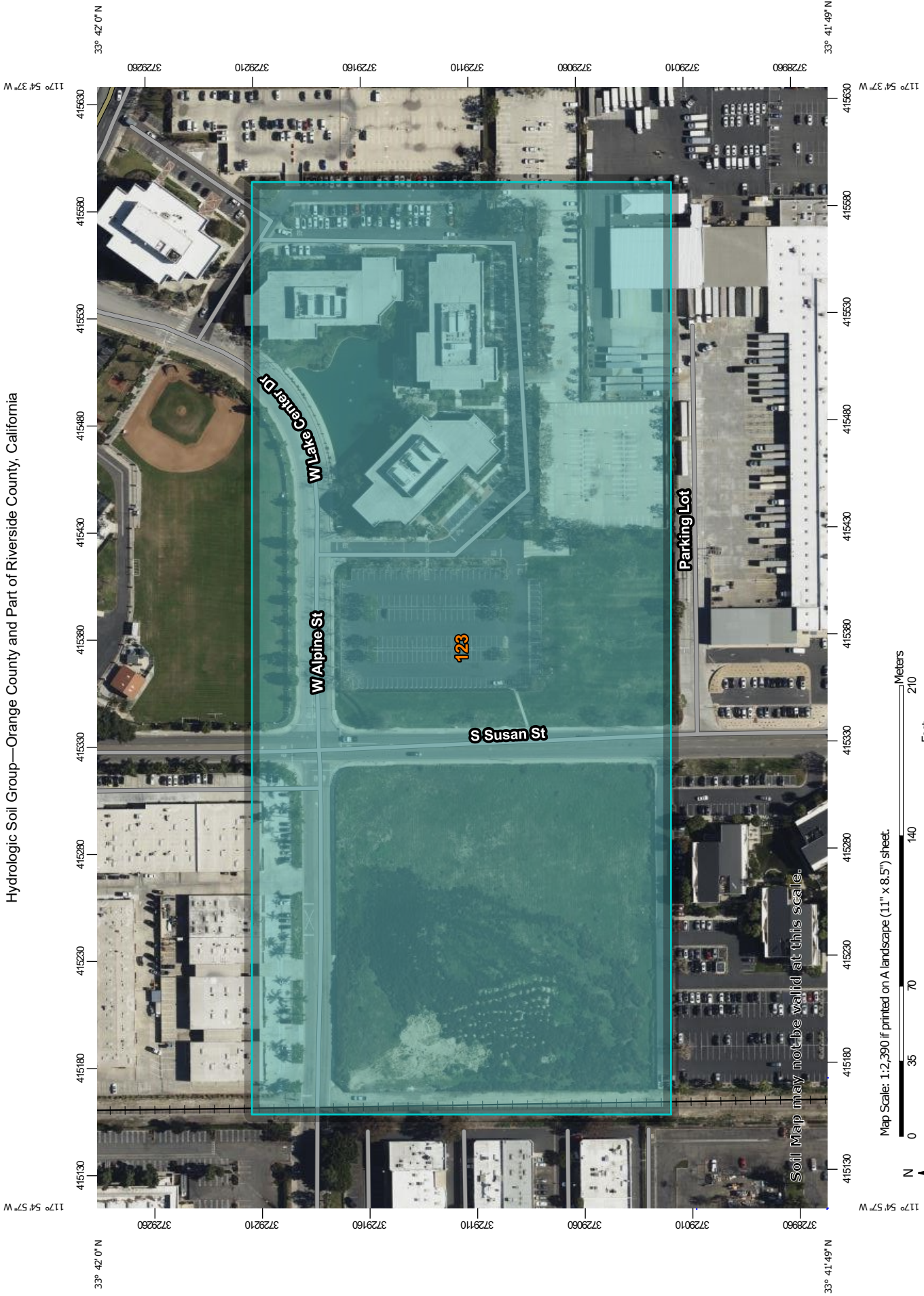


ZONING DISTRICTS

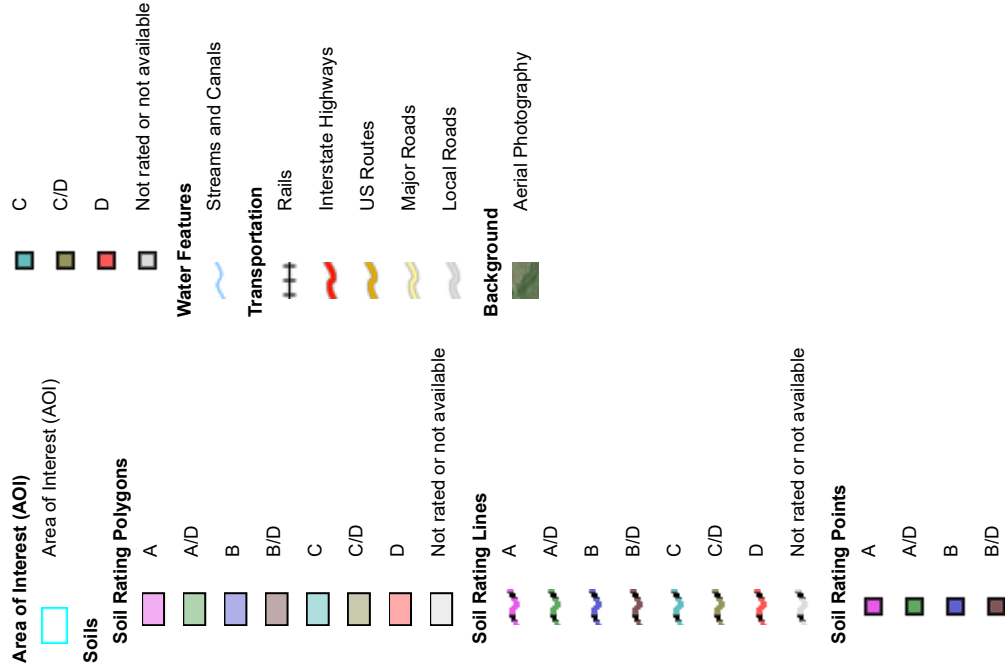
	A1	GENERAL AGRICULTURAL
	R1	SINGLE-FAMILY RESIDENCE
	R2	TWO-FAMILY RESIDENCE
	R3	MULTIPLE-FAMILY RESIDENCE
	R4	SUBURBAN APARTMENT
	RE	RESIDENTIAL ESTATE
	C1	COMMUNITY COMMERCIAL
	C2	GENERAL COMMERCIAL
	C4	PLANNED SHOPPING CENTER
	C5	ARTERIAL COMMERCIAL
	CR	COMMERCIAL RESIDENTIAL
	CSM	SOUTH MAIN STREET COMMERCIAL DISTRICT
	P	PROFESSIONAL
	GC	GOVERNMENT CENTER
	SD	SPECIFIC DEVELOPMENT

	M1	LIGHT INDUSTRIAL
	M2	HEAVY INDUSTRIAL
	O	OPEN SPACE LAND
	OZ1	METRO EAST MIXED USE OVERLAY ZONE
SPECIFIC PLANS		
	SP1	BRISTOL STREET CORRIDOR SPECIFIC PLAN
	SP2	HARBOR MIXED USE TRANSIT CORRIDOR SPECIFIC PLAN
	SP3	MIDTOWN SPECIFIC PLAN
	SP4	MAINPLACE SPECIFIC PLAN
SUFFIXES		
-B		PARKING MODIFICATION
-OZ1		METRO EAST MIXED USE OVERLAY ZONE
-OZ-M1/M2		TRANSIT ZONING CODE M1/M2 INDUSTRIAL OVERLAY ZONE
-PRD		PLANNED RESIDENTIAL DEVELOPMENT
-HDII		HEIGHT DISTRICT II
-MD		MUSEUM DISTRICT

APPENDIX D- SOILS GROUP REPORT



MAP LEGEND



MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Orange County and Part of Riverside County, California
 Survey Area Data: Version 17, Aug 30, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jan 17, 2023—Feb 8, 2023

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
123	Bolsa silt loam, drained	C	21.0	100.0%
Totals for Area of Interest			21.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Tie-break Rule: Higher

Component Percent Cutoff: None Specified

APPENDIX E- RAINFALL INTENSITY REPORT

Note:
 -Rainfall Intensity values were calculated from the AES values corresponding to the relative Time of Concentration.

-Time of Concentrations for durations less than 5-min are assumed to be 5-min.



NOAA Atlas 14, Volume 6, Version 2
Location name: Santa Ana, California, USA*
Latitude: 33.6987°, Longitude: -117.9118°
Elevation: 40 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

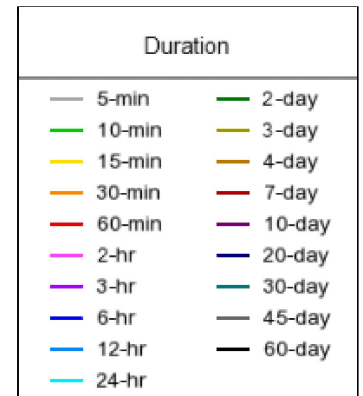
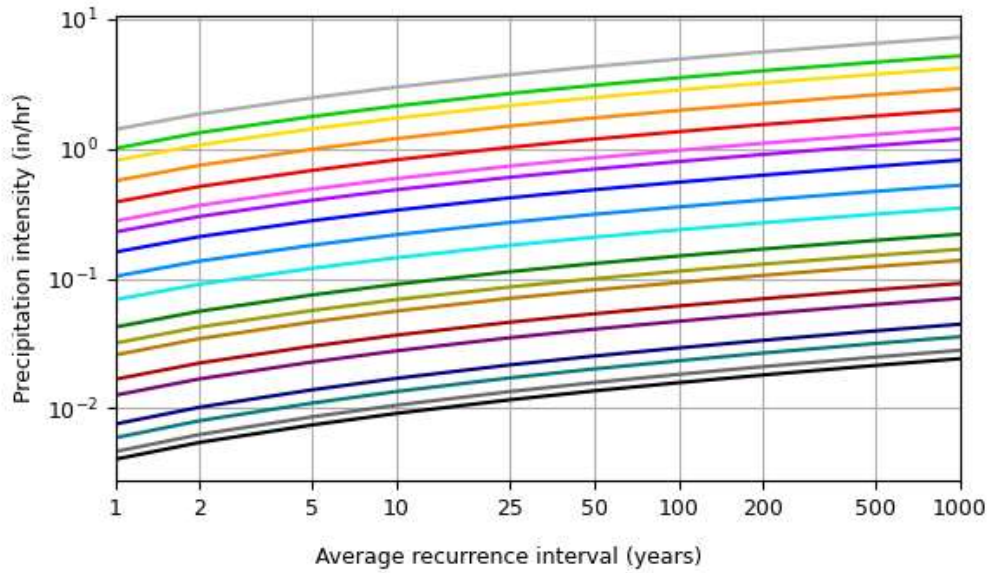
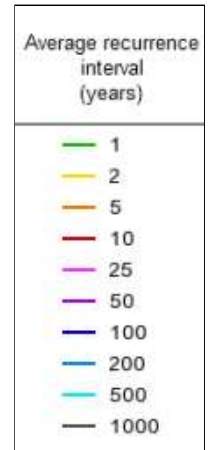
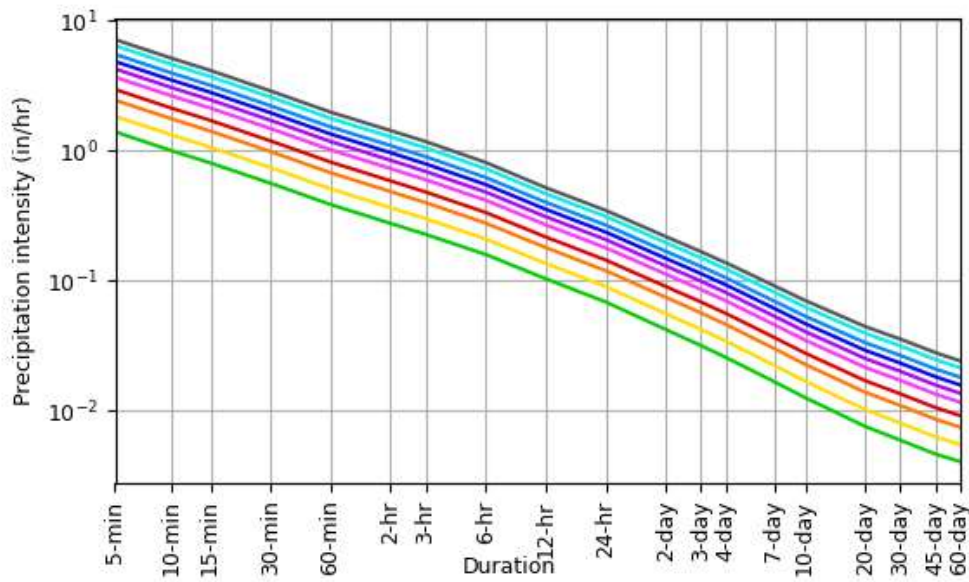
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	1.40 (1.18-1.68)	1.85 (1.55-2.23)	2.46 (2.05-2.98)	2.98 (2.46-3.62)	3.70 (2.95-4.68)	4.28 (3.35-5.53)	4.88 (3.72-6.49)	5.54 (4.09-7.57)	6.46 (4.56-9.23)	7.20 (4.91-10.7)
10-min	1.00 (0.840-1.21)	1.33 (1.11-1.60)	1.76 (1.47-2.13)	2.13 (1.76-2.60)	2.65 (2.12-3.35)	3.07 (2.39-3.97)	3.50 (2.67-4.65)	3.97 (2.93-5.43)	4.63 (3.27-6.62)	5.16 (3.52-7.66)
15-min	0.808 (0.680-0.976)	1.07 (0.896-1.29)	1.42 (1.19-1.72)	1.72 (1.42-2.10)	2.14 (1.71-2.70)	2.47 (1.93-3.20)	2.82 (2.15-3.75)	3.20 (2.36-4.38)	3.73 (2.64-5.34)	4.16 (2.84-6.17)
30-min	0.564 (0.472-0.678)	0.744 (0.624-0.896)	0.988 (0.826-1.19)	1.19 (0.990-1.46)	1.49 (1.19-1.88)	1.72 (1.34-2.22)	1.97 (1.50-2.61)	2.23 (1.64-3.04)	2.60 (1.83-3.71)	2.89 (1.97-4.29)
60-min	0.387 (0.325-0.466)	0.511 (0.428-0.615)	0.679 (0.568-0.821)	0.821 (0.680-1.00)	1.02 (0.816-1.29)	1.18 (0.923-1.53)	1.35 (1.03-1.79)	1.53 (1.13-2.09)	1.78 (1.26-2.55)	1.99 (1.36-2.95)
2-hr	0.277 (0.232-0.333)	0.366 (0.307-0.441)	0.487 (0.407-0.588)	0.588 (0.487-0.717)	0.732 (0.585-0.925)	0.847 (0.662-1.10)	0.969 (0.737-1.28)	1.10 (0.812-1.50)	1.28 (0.907-1.84)	1.44 (0.977-2.13)
3-hr	0.227 (0.191-0.274)	0.300 (0.252-0.362)	0.399 (0.333-0.482)	0.482 (0.399-0.588)	0.600 (0.479-0.758)	0.694 (0.543-0.898)	0.794 (0.605-1.05)	0.902 (0.666-1.23)	1.06 (0.745-1.51)	1.18 (0.804-1.75)
6-hr	0.159 (0.133-0.192)	0.210 (0.176-0.253)	0.278 (0.232-0.336)	0.336 (0.278-0.409)	0.417 (0.333-0.527)	0.482 (0.376-0.623)	0.551 (0.419-0.731)	0.624 (0.461-0.854)	0.729 (0.515-1.04)	0.815 (0.555-1.21)
12-hr	0.103 (0.087-0.124)	0.136 (0.114-0.164)	0.180 (0.150-0.218)	0.217 (0.180-0.265)	0.269 (0.215-0.341)	0.311 (0.243-0.402)	0.355 (0.270-0.472)	0.402 (0.297-0.550)	0.468 (0.331-0.669)	0.521 (0.355-0.774)
24-hr	0.068 (0.060-0.079)	0.090 (0.079-0.104)	0.119 (0.105-0.138)	0.144 (0.126-0.168)	0.179 (0.152-0.216)	0.207 (0.172-0.255)	0.237 (0.192-0.299)	0.268 (0.211-0.347)	0.312 (0.236-0.421)	0.348 (0.255-0.486)
2-day	0.042 (0.037-0.048)	0.056 (0.049-0.064)	0.074 (0.065-0.086)	0.090 (0.079-0.105)	0.112 (0.095-0.135)	0.130 (0.108-0.160)	0.148 (0.120-0.187)	0.168 (0.132-0.218)	0.196 (0.148-0.265)	0.219 (0.160-0.305)
3-day	0.031 (0.028-0.036)	0.042 (0.037-0.048)	0.056 (0.049-0.065)	0.068 (0.060-0.080)	0.085 (0.072-0.103)	0.099 (0.082-0.122)	0.113 (0.092-0.143)	0.129 (0.101-0.167)	0.150 (0.113-0.202)	0.167 (0.122-0.234)
4-day	0.025 (0.022-0.029)	0.034 (0.030-0.039)	0.046 (0.040-0.053)	0.056 (0.049-0.065)	0.070 (0.059-0.084)	0.081 (0.067-0.100)	0.093 (0.075-0.117)	0.105 (0.083-0.137)	0.123 (0.093-0.166)	0.137 (0.100-0.192)
7-day	0.016 (0.014-0.019)	0.022 (0.019-0.025)	0.030 (0.026-0.034)	0.036 (0.032-0.042)	0.046 (0.038-0.055)	0.053 (0.044-0.065)	0.061 (0.049-0.077)	0.069 (0.055-0.090)	0.081 (0.062-0.110)	0.091 (0.067-0.127)
10-day	0.012 (0.011-0.014)	0.016 (0.014-0.019)	0.022 (0.020-0.026)	0.027 (0.024-0.032)	0.034 (0.029-0.042)	0.040 (0.033-0.050)	0.046 (0.037-0.059)	0.053 (0.042-0.069)	0.062 (0.047-0.084)	0.070 (0.051-0.098)
20-day	0.007 (0.006-0.008)	0.010 (0.009-0.011)	0.013 (0.012-0.016)	0.017 (0.014-0.019)	0.021 (0.018-0.026)	0.025 (0.020-0.031)	0.029 (0.023-0.036)	0.033 (0.026-0.043)	0.039 (0.029-0.053)	0.044 (0.032-0.062)
30-day	0.005 (0.005-0.006)	0.008 (0.007-0.009)	0.010 (0.009-0.012)	0.013 (0.011-0.015)	0.017 (0.014-0.020)	0.020 (0.016-0.024)	0.023 (0.018-0.029)	0.026 (0.021-0.034)	0.031 (0.023-0.042)	0.035 (0.026-0.049)
45-day	0.004 (0.004-0.005)	0.006 (0.005-0.007)	0.008 (0.007-0.009)	0.010 (0.009-0.012)	0.013 (0.011-0.016)	0.015 (0.013-0.019)	0.018 (0.014-0.023)	0.020 (0.016-0.027)	0.024 (0.018-0.033)	0.028 (0.020-0.039)
60-day	0.004 (0.003-0.004)	0.005 (0.004-0.006)	0.007 (0.006-0.008)	0.009 (0.008-0.010)	0.011 (0.009-0.014)	0.013 (0.011-0.016)	0.015 (0.012-0.019)	0.018 (0.014-0.023)	0.021 (0.016-0.028)	0.024 (0.017-0.033)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
 Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
 Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based intensity-duration-frequency (IDF) curves
 Latitude: 33.6987°, Longitude: -117.9118°



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Maps & aerials

Small scale terrain



Large scale terrain



Large scale map



Large scale aerial



[Back to Top](#)

[US Department of Commerce](#)
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[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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APPENDIX F- HYDROLOGY CALCULATIONS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
(c) Copyright 1983-2014 Advanced Engineering Software (aes)
Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* SUBAREA W1 *

FILE NAME: E10W1.DAT
TIME/DATE OF STUDY: 15:55 12/29/2023

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 449.00

ELEVATION DATA: UPSTREAM (FEET) = 40.45 DOWNSTREAM (FEET) = 35.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 14.879

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.173

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
NATURAL POOR COVER "BARREN"	C	1.79	0.25	1.000	91	14.88

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 1.000

SUBAREA RUNOFF (CFS) = 3.10

TOTAL AREA (ACRES) = 1.79 PEAK FLOW RATE (CFS) = 3.10

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 1.8 T_c (MIN.) = 14.88

EFFECTIVE AREA (ACRES) = 1.79 AREA-AVERAGED F_m (INCH/HR) = 0.25

AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 1.000

PEAK FLOW RATE (CFS) = 3.10

=====

END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* SUB AREA W2 *

FILE NAME: E10W2.DAT
TIME/DATE OF STUDY: 16:06 12/29/2023

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 174.00

ELEVATION DATA: UPSTREAM (FEET) = 40.50 DOWNSTREAM (FEET) = 36.40

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**0.20}$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.748

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.946

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
NATURAL POOR COVER "BARREN"	C	1.03	0.25	1.000	91	8.75

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 1.000

SUBAREA RUNOFF (CFS) = 2.50

TOTAL AREA (ACRES) = 1.03 PEAK FLOW RATE (CFS) = 2.50

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 1.0 T_c (MIN.) = 8.75

EFFECTIVE AREA (ACRES) = 1.03 AREA-AVERAGED F_m (INCH/HR) = 0.25

AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 1.000

PEAK FLOW RATE (CFS) = 2.50

=====

END OF RATIONAL METHOD ANALYSIS



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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* SUB AREA W3 *

FILE NAME: E10W3.DAT
TIME/DATE OF STUDY: 16:09 12/29/2023

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00
ELEVATION DATA: UPSTREAM (FEET) = 40.00 DOWNSTREAM (FEET) = 33.93

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.077

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.717

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
NATURAL POOR COVER "BARREN"	C	2.99	0.25	1.000	91	10.08

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 1.000

SUBAREA RUNOFF (CFS) = 6.64

TOTAL AREA (ACRES) = 2.99 PEAK FLOW RATE (CFS) = 6.64

=====

END OF STUDY SUMMARY:
TOTAL AREA (ACRES) = 3.0 T_c (MIN.) = 10.08

EFFECTIVE AREA (ACRES) = 2.99 AREA-AVERAGED F_m (INCH/HR) = 0.25

AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 1.000

PEAK FLOW RATE (CFS) = 6.64

=====

END OF RATIONAL METHOD ANALYSIS



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Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* SUB AREA E1 *

FILE NAME: E10E1.DAT
TIME/DATE OF STUDY: 16:15 12/29/2023

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:		CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN-	OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE /	SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020		0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 400.00

ELEVATION DATA: UPSTREAM (FEET) = 38.10 DOWNSTREAM (FEET) = 33.35

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.985

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.586

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
SCHOOL	C	3.32	0.25	0.600	69	10.98

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600

SUBAREA RUNOFF (CFS) = 7.28

TOTAL AREA (ACRES) = 3.32 PEAK FLOW RATE (CFS) = 7.28

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 3.3 TC (MIN.) = 10.98

EFFECTIVE AREA (ACRES) = 3.32 AREA-AVERAGED F_m (INCH/HR) = 0.15

AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 0.600

PEAK FLOW RATE (CFS) = 7.28

END OF RATIONAL METHOD ANALYSIS



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Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXSITING 10-YEAR HYDRO STUDY *
* SOUTHEAST OUTFALL *

FILE NAME: E10.DAT
TIME/DATE OF STUDY: 15:43 12/29/2023

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

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GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- 1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
- 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH (FEET) = 79.30
ELEVATION DATA: UPSTREAM (FEET) = 38.00 DOWNSTREAM (FEET) = 34.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 5.000

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
MOBILE HOME PARK	C	0.07	0.25	0.250	69	5.00

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.250

SUBAREA RUNOFF (CFS) = 0.25

TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.25

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE T_c (MIN.) = 5.00

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/	SCS SOIL	AREA	F_p	A_p	SCS
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      LAND USE          GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
COMMERCIAL            C         0.28     0.25     0.100     69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 0.28      SUBAREA RUNOFF(CFS) = 1.02
EFFECTIVE AREA(ACRES) = 0.35    AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.25  AREA-AVERAGED Ap = 0.13
TOTAL AREA(ACRES) = 0.3        PEAK FLOW RATE(CFS) = 1.27

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FLOW PROCESS FROM NODE 30.00 TO NODE 40.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 5.00
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 4.060
SUBAREA LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap          SCS
    LAND USE            GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
COMMERCIAL              C         0.24     0.25     0.100     69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 0.24      SUBAREA RUNOFF(CFS) = 0.87
EFFECTIVE AREA(ACRES) = 0.59    AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.25  AREA-AVERAGED Ap = 0.12
TOTAL AREA(ACRES) = 0.6        PEAK FLOW RATE(CFS) = 2.14

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FLOW PROCESS FROM NODE 40.00 TO NODE 50.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 5.00
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 4.060
SUBAREA LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap          SCS
    LAND USE            GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
COMMERCIAL              C         0.36     0.25     0.100     69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25

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SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA AREA (ACRES) = 0.36 SUBAREA RUNOFF (CFS) = 1.31
 EFFECTIVE AREA (ACRES) = 0.95 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED $A_p = 0.11$
 TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 3.45

 FLOW PROCESS FROM NODE 50.00 TO NODE 60.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE T_c (MIN.) = 5.00
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
COMMERCIAL	C	0.83	0.25	0.100	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA AREA (ACRES) = 0.83 SUBAREA RUNOFF (CFS) = 3.01
 EFFECTIVE AREA (ACRES) = 1.78 AREA-AVERAGED F_m (INCH/HR) = 0.03
 AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED $A_p = 0.11$
 TOTAL AREA (ACRES) = 1.8 PEAK FLOW RATE (CFS) = 6.46

 FLOW PROCESS FROM NODE 60.00 TO NODE 70.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE T_c (MIN.) = 5.00
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN
NATURAL GOOD COVER "WOODLAND"	C	0.12	0.25	1.000	70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 1.000$
 SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.41

EFFECTIVE AREA (ACRES) = 1.90 AREA-AVERAGED Fm (INCH/HR) = 0.04
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.16
TOTAL AREA (ACRES) = 1.9 PEAK FLOW RATE (CFS) = 6.87

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 1.9 TC (MIN.) = 5.00
EFFECTIVE AREA (ACRES) = 1.90 AREA-AVERAGED Fm (INCH/HR) = 0.04
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.162
PEAK FLOW RATE (CFS) = 6.87

END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* EAST OUTFALL *

FILE NAME: E10E.DAT
TIME/DATE OF STUDY: 11:07 01/03/2024

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

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GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH (FEET) = 125.50
ELEVATION DATA: UPSTREAM (FEET) = 38.05 DOWNSTREAM (FEET) = 37.15

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 7.216

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.290

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	0.36	0.25	0.500	69	7.22
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SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.500

SUBAREA RUNOFF (CFS) = 1.03

TOTAL AREA (ACRES) = 0.36 PEAK FLOW RATE (CFS) = 1.03

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE T_c (MIN.) = 7.22

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.290

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
CONDOMINIUMS	C	1.07	0.25	0.350	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.350					
SUBAREA AREA(ACRES) =		1.07	SUBAREA RUNOFF(CFS) =		3.08
EFFECTIVE AREA(ACRES) =		1.43	AREA-AVERAGED Fm(INCH/HR) =		0.10
AREA-AVERAGED Fp(INCH/HR) =		0.25	AREA-AVERAGED Ap =		0.39
TOTAL AREA(ACRES) =		1.4	PEAK FLOW RATE(CFS) =		4.11

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.50
 ELEVATION DATA: UPSTREAM(FEET) = 500.00 DOWNSTREAM(FEET) = 498.21

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 5.000

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 4.060

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	C	0.47	0.25	0.100	69	5.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100						
SUBAREA RUNOFF(CFS) =		1.71				
TOTAL AREA(ACRES) =		0.47	PEAK FLOW RATE(CFS) =		1.71	

FLOW PROCESS FROM NODE 26.00 TO NODE 30.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
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** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.71	5.00	4.060	0.25(0.03)	0.10	0.5	25.00
LONGEST FLOWPATH FROM NODE					25.00 TO NODE	30.00 =	89.50 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.11	7.22	3.290	0.25(0.10)	0.39	1.4	10.00
LONGEST FLOWPATH FROM NODE					10.00 TO NODE	30.00 =	125.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.24	5.00	4.060	0.25(0.07)	0.30	1.5	25.00
2	5.49	7.22	3.290	0.25(0.08)	0.32	1.9	10.00
TOTAL AREA (ACRES) =			1.9				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.49 Tc (MIN.) = 7.216
 EFFECTIVE AREA (ACRES) = 1.90 AREA-AVERAGED Fm (INCH/HR) = 0.08
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.32
 TOTAL AREA (ACRES) = 1.9
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 30.00 = 125.50 FEET.

 FLOW PROCESS FROM NODE 30.00 TO NODE 40.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
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MAINLINE Tc (MIN.) = 7.22
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.290
 SUBAREA LOSS RATE DATA (AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 MOBILE HOME PARK C 0.42 0.25 0.250 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.250
 SUBAREA AREA (ACRES) = 0.42 SUBAREA RUNOFF (CFS) = 1.22
 EFFECTIVE AREA (ACRES) = 2.32 AREA-AVERAGED F_m (INCH/HR) = 0.08
 AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 0.30
 TOTAL AREA (ACRES) = 2.3 PEAK FLOW RATE (CFS) = 6.71

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.75	5.00	4.060	0.25 (0.07)	0.29	1.9	25.00
2	6.71	7.22	3.290	0.25 (0.08)	0.30	2.3	10.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 6.75 Tc (MIN.) = 5.00
 AREA-AVERAGED F_m (INCH/HR) = 0.07 AREA-AVERAGED F_p (INCH/HR) = 0.25
 AREA-AVERAGED A_p = 0.29 EFFECTIVE AREA (ACRES) = 1.88

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 67.80
 ELEVATION DATA: UPSTREAM (FEET) = 501.00 DOWNSTREAM (FEET) = 499.64

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 5.000

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS	Tc
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LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
 COMMERCIAL C 0.44 0.25 0.100 69 5.00
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100
 SUBAREA RUNOFF (CFS) = 1.60
 TOTAL AREA (ACRES) = 0.44 PEAK FLOW RATE (CFS) = 1.60

 FLOW PROCESS FROM NODE 36.00 TO NODE 40.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<
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** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.60	5.00	4.060	0.25 (0.03)	0.10	0.4	35.00
LONGEST FLOWPATH FROM NODE			35.00 TO NODE		40.00 =		67.80 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.75	5.00	4.060	0.25 (0.07)	0.29	1.9	25.00
2	6.71	7.22	3.290	0.25 (0.08)	0.30	2.3	10.00
LONGEST FLOWPATH FROM NODE			10.00 TO NODE		40.00 =		125.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.35	5.00	4.060	0.25 (0.06)	0.25	2.3	35.00
2	8.35	5.00	4.060	0.25 (0.06)	0.25	2.3	25.00
3	8.00	7.22	3.290	0.25 (0.07)	0.27	2.8	10.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 8.35 Tc (MIN.) = 5.000
 EFFECTIVE AREA (ACRES) = 2.32 AREA-AVERAGED F_m (INCH/HR) = 0.06
 AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 0.25
 TOTAL AREA (ACRES) = 2.8

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 40.00 = 125.50 FEET.

FLOW PROCESS FROM NODE 40.00 TO NODE 50.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 30.00 DOWNSTREAM(FEET) = 29.58
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.46
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 8.35
PIPE TRAVEL TIME(MIN.) = 0.39 Tc(MIN.) = 5.39
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 50.00 = 230.50 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 43.50
ELEVATION DATA: UPSTREAM(FEET) = 38.05 DOWNSTREAM(FEET) = 37.26

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 5.000

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 4.060

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						

"8-10 DWELLINGS/ACRE" C 0.11 0.25 0.400 69 5.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400
SUBAREA RUNOFF(CFS) = 0.39
TOTAL AREA(ACRES) = 0.11 PEAK FLOW RATE(CFS) = 0.39

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE T_c (MIN.) =	5.00				
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =	4.060				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	F_p	A_p	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
MOBILE HOME PARK	C	0.47	0.25	0.250	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) =		0.25			
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p =		0.250			
SUBAREA AREA(ACRES) =	0.47	SUBAREA RUNOFF(CFS) =		1.69	
EFFECTIVE AREA(ACRES) =	0.58	AREA-AVERAGED F_m (INCH/HR) =		0.07	
AREA-AVERAGED F_p (INCH/HR) =	0.25	AREA-AVERAGED A_p =		0.28	
TOTAL AREA(ACRES) =	0.6	PEAK FLOW RATE(CFS) =		2.08	

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 43.00 TO NODE 43.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 42.50 TO NODE 42.60 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 70.70
 ELEVATION DATA: UPSTREAM (FEET) = 502.00 DOWNSTREAM (FEET) = 500.59

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 5.000
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	C	0.47	0.25	0.100	69	5.00

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.100

SUBAREA RUNOFF (CFS) = 1.71

TOTAL AREA (ACRES) = 0.47 PEAK FLOW RATE (CFS) = 1.71

FLOW PROCESS FROM NODE 42.60 TO NODE 43.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.71	5.00	4.060	0.25 (0.03)	0.10	0.5	42.50

LONGEST FLOWPATH FROM NODE 42.50 TO NODE 43.00 = 70.70 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.08	5.00	4.060	0.25 (0.07)	0.28	0.6	41.00

LONGEST FLOWPATH FROM NODE 41.00 TO NODE 43.00 = 43.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (F_m) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
------------------	--------------	-----------------	------------------------	------------------------------	-------	------------------	-------------------

1	3.79	5.00	4.060	0.25 (0.05)	0.20	1.0	42.50
2	3.79	5.00	4.060	0.25 (0.05)	0.20	1.0	41.00
TOTAL AREA (ACRES) =			1.0				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.79 Tc (MIN.) = 5.000
EFFECTIVE AREA (ACRES) = 1.05 AREA-AVERAGED Fm (INCH/HR) = 0.05
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.20
TOTAL AREA (ACRES) = 1.0
LONGEST FLOWPATH FROM NODE 42.50 TO NODE 43.00 = 70.70 FEET.

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 5.00
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
APARTMENTS	C	0.82	0.25	0.200	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA (ACRES) = 0.82 SUBAREA RUNOFF (CFS) = 2.96
EFFECTIVE AREA (ACRES) = 1.87 AREA-AVERAGED Fm (INCH/HR) = 0.05
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.20
TOTAL AREA (ACRES) = 1.9 PEAK FLOW RATE (CFS) = 6.75

FLOW PROCESS FROM NODE 44.00 TO NODE 44.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 44.00 TO NODE 50.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

=====
 ** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.75	5.00	4.060	0.25 (0.05)	0.20	1.9	42.50
2	6.75	5.00	4.060	0.25 (0.05)	0.20	1.9	41.00
LONGEST FLOWPATH FROM NODE			42.50	TO NODE	50.00 =		70.70 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.35	5.39	3.888	0.25 (0.06)	0.25	2.3	35.00
2	8.35	5.39	3.888	0.25 (0.06)	0.25	2.3	25.00
3	8.00	7.61	3.191	0.25 (0.07)	0.27	2.8	10.00
LONGEST FLOWPATH FROM NODE			10.00	TO NODE	50.00 =		230.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.84	5.00	4.060	0.25 (0.06)	0.23	4.0	42.50
2	14.84	5.00	4.060	0.25 (0.06)	0.23	4.0	41.00
3	14.81	5.39	3.888	0.25 (0.06)	0.23	4.2	35.00
4	14.81	5.39	3.888	0.25 (0.06)	0.23	4.2	25.00
5	13.29	7.61	3.191	0.25 (0.06)	0.24	4.6	10.00
TOTAL AREA (ACRES) =			4.6				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 14.84 Tc (MIN.) = 5.000
 EFFECTIVE AREA (ACRES) = 4.02 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.23
 TOTAL AREA (ACRES) = 4.6
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 50.00 = 230.50 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 3 <<<<<<

=====

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
=====

FLOW PROCESS FROM NODE 45.00 TO NODE 46.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 64.60
ELEVATION DATA: UPSTREAM (FEET) = 37.50 DOWNSTREAM (FEET) = 35.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 5.000

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

"8-10 DWELLINGS/ACRE" C 0.04 0.25 0.400 69 5.00

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400

SUBAREA RUNOFF (CFS) = 0.14

TOTAL AREA (ACRES) = 0.04 PEAK FLOW RATE (CFS) = 0.14

FLOW PROCESS FROM NODE 46.00 TO NODE 50.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	0.14	5.00	4.060	0.25 (0.10)	0.40	0.0	45.00

LONGEST FLOWPATH FROM NODE 45.00 TO NODE 50.00 = 64.60 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.84	5.00	4.060	0.25 (0.06)	0.23	4.0	42.50
2	14.84	5.00	4.060	0.25 (0.06)	0.23	4.0	41.00
3	14.81	5.39	3.888	0.25 (0.06)	0.23	4.2	35.00
4	14.81	5.39	3.888	0.25 (0.06)	0.23	4.2	25.00
5	13.29	7.61	3.191	0.25 (0.06)	0.24	4.6	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 50.00 = 230.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.98	5.00	4.060	0.25 (0.06)	0.23	4.1	45.00
2	14.98	5.00	4.060	0.25 (0.06)	0.23	4.1	42.50
3	14.98	5.00	4.060	0.25 (0.06)	0.23	4.1	41.00
4	14.94	5.39	3.888	0.25 (0.06)	0.23	4.2	35.00
5	14.94	5.39	3.888	0.25 (0.06)	0.23	4.2	25.00
6	13.40	7.61	3.191	0.25 (0.06)	0.24	4.7	10.00

TOTAL AREA (ACRES) = 4.7

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 14.98 Tc (MIN.) = 5.000
 EFFECTIVE AREA (ACRES) = 4.06 AREA-AVERAGED Fm (INCH/HR) = 0.06
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.23
 TOTAL AREA (ACRES) = 4.7
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 50.00 = 230.50 FEET.

 FLOW PROCESS FROM NODE 50.00 TO NODE 60.00 IS CODE = 81

>>>> ADDITION OF SUBAREA TO MAINLINE PEAK FLOW <<<<<

=====

MAINLINE Tc (MIN.) = 5.00
 * 10 YEAR RAINFALL INTENSITY (INCH/HR) = 4.060
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
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LAND USE          GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
NATURAL GOOD COVER
"WOODLAND"       C         0.09      0.25      1.000      70
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA AREA(ACRES) = 0.09      SUBAREA RUNOFF(CFS) = 0.31
EFFECTIVE AREA(ACRES) = 4.15      AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.25  AREA-AVERAGED Ap = 0.24
TOTAL AREA(ACRES) = 4.8          PEAK FLOW RATE(CFS) = 14.98
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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END OF STUDY SUMMARY:

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TOTAL AREA(ACRES) = 4.8      TC(MIN.) = 5.00
EFFECTIVE AREA(ACRES) = 4.15  AREA-AVERAGED Fm(INCH/HR) = 0.06
AREA-AVERAGED Fp(INCH/HR) = 0.25  AREA-AVERAGED Ap = 0.245
PEAK FLOW RATE(CFS) = 14.98

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** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.98	5.00	4.060	0.25(0.06)	0.24	4.2	45.00
2	14.98	5.00	4.060	0.25(0.06)	0.24	4.2	42.50
3	14.98	5.00	4.060	0.25(0.06)	0.24	4.2	41.00
4	14.94	5.39	3.888	0.25(0.06)	0.25	4.3	35.00
5	14.94	5.39	3.888	0.25(0.06)	0.25	4.3	25.00
6	13.40	7.61	3.191	0.25(0.06)	0.26	4.8	10.00

=====
END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* EXISTING 10-YEAR HYDRO STUDY *
* LAKE OUTFALL *

FILE NAME: E10L.DAT
TIME/DATE OF STUDY: 10:05 01/03/2024

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:			CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN-	/	OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE	/	SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018	/	0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 65.00
ELEVATION DATA: UPSTREAM (FEET) = 38.05 DOWNSTREAM (FEET) = 36.95

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 5.259

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.944

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.47	0.25	0.700	69	5.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.700

SUBAREA RUNOFF (CFS) = 1.59

TOTAL AREA (ACRES) = 0.47 PEAK FLOW RATE (CFS) = 1.59

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE T_c (MIN.) = 5.26

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.944

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	C	0.45	0.25	0.850	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850					
SUBAREA AREA(ACRES) =		0.45	SUBAREA RUNOFF(CFS) =		1.51
EFFECTIVE AREA(ACRES) =		0.92	AREA-AVERAGED Fm(INCH/HR) =		0.19
AREA-AVERAGED Fp(INCH/HR) =		0.25	AREA-AVERAGED Ap =		0.77
TOTAL AREA(ACRES) =		0.9	PEAK FLOW RATE(CFS) =		3.11

=====
 END OF STUDY SUMMARY:

TOTAL AREA(ACRES) =	0.9	TC(MIN.) =	5.26
EFFECTIVE AREA(ACRES) =	0.92	AREA-AVERAGED Fm(INCH/HR) =	0.19
AREA-AVERAGED Fp(INCH/HR) =	0.25	AREA-AVERAGED Ap =	0.773
PEAK FLOW RATE(CFS) =	3.11		

=====
 END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* PROPOSED 10-YAR HYDRO STUDY *
* WATERSHED W1 *

FILE NAME: P10W.DAT
TIME/DATE OF STUDY: 17:51 01/08/2024

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- 1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
- 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 111.10
ELEVATION DATA: UPSTREAM (FEET) = 37.25 DOWNSTREAM (FEET) = 35.58

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 6.674

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.441

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.50	0.25	0.700	69	6.67

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.700

SUBAREA RUNOFF (CFS) = 1.47

TOTAL AREA (ACRES) = 0.50 PEAK FLOW RATE (CFS) = 1.47

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE T_c (MIN.) = 6.67

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.441

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.43	0.25	0.900	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900					
SUBAREA AREA(ACRES) = 0.43 SUBAREA RUNOFF(CFS) = 1.24					
EFFECTIVE AREA(ACRES) = 0.93 AREA-AVERAGED Fm(INCH/HR) = 0.20					
AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.79					
TOTAL AREA(ACRES) = 0.9 PEAK FLOW RATE(CFS) = 2.71					

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 77.00
 ELEVATION DATA: UPSTREAM(FEET) = 37.25 DOWNSTREAM(FEET) = 35.71

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.052

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.639

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.26	0.25	0.900	69	6.05
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900						
SUBAREA RUNOFF(CFS) = 0.80						
TOTAL AREA(ACRES) = 0.26 PEAK FLOW RATE(CFS) = 0.80						

FLOW PROCESS FROM NODE 26.00 TO NODE 30.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.80	6.05	3.639	0.25 (0.22)	0.90	0.3	25.00
LONGEST FLOWPATH FROM NODE				25.00 TO NODE	30.00 =		77.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.71	6.67	3.441	0.25 (0.20)	0.79	0.9	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	30.00 =		111.10 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.41	6.05	3.639	0.25 (0.20)	0.82	1.1	25.00
2	3.47	6.67	3.441	0.25 (0.20)	0.82	1.2	10.00
TOTAL AREA (ACRES) =			1.2				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.47 Tc (MIN.) = 6.674
EFFECTIVE AREA (ACRES) = 1.19 AREA-AVERAGED Fm (INCH/HR) = 0.20
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.82
TOTAL AREA (ACRES) = 1.2
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 30.00 = 111.10 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 13

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

FLOW PROCESS FROM NODE 2.00 TO NODE 10.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 257.00
ELEVATION DATA: UPSTREAM (FEET) = 37.25 DOWNSTREAM (FEET) = 32.11

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.802

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.760

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

".4 DWELLING/ACRE" C 0.59 0.25 0.900 69 9.80

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.35

TOTAL AREA (ACRES) = 0.59 PEAK FLOW RATE (CFS) = 1.35

FLOW PROCESS FROM NODE 10.00 TO NODE 10.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.35	9.80	2.760	0.25 (0.22)	0.90	0.6	2.00

LONGEST FLOWPATH FROM NODE 2.00 TO NODE 10.00 = 257.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.41	6.05	3.639	0.25 (0.20)	0.82	1.1	25.00
2	3.47	6.67	3.441	0.25 (0.20)	0.82	1.2	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 10.00 = 111.10 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.53	6.05	3.639	0.25 (0.21)	0.84	1.5	25.00
2	4.63	6.67	3.441	0.25 (0.21)	0.84	1.6	10.00
3	4.08	9.80	2.760	0.25 (0.21)	0.84	1.8	2.00

TOTAL AREA (ACRES) = 1.8

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.63 Tc (MIN.) = 6.674
 EFFECTIVE AREA (ACRES) = 1.59 AREA-AVERAGED Fm (INCH/HR) = 0.21
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.84
 TOTAL AREA (ACRES) = 1.8
 LONGEST FLOWPATH FROM NODE 2.00 TO NODE 10.00 = 257.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 40.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 6.67

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.441

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					

"2 DWELLINGS/ACRE"	C	0.42	0.25	0.700	69
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SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700

SUBAREA AREA (ACRES) = 0.42 SUBAREA RUNOFF (CFS) = 1.23

EFFECTIVE AREA (ACRES) = 2.01 AREA-AVERAGED Fm (INCH/HR) = 0.20

AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.81

TOTAL AREA (ACRES) = 2.2 PEAK FLOW RATE (CFS) = 5.86

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 263.00

ELEVATION DATA: UPSTREAM (FEET) = 37.25 DOWNSTREAM (FEET) = 31.99

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.892

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.746

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

".4 DWELLING/ACRE"	C	0.64	0.25	0.900	69	9.89
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SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.45
 TOTAL AREA (ACRES) = 0.64 PEAK FLOW RATE (CFS) = 1.45

FLOW PROCESS FROM NODE 36.00 TO NODE 40.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
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** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.45	9.89	2.746	0.25 (0.22)	0.90	0.6	35.00
LONGEST FLOWPATH FROM NODE 35.00 TO NODE					40.00 =	263.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.84	6.05	3.639	0.25 (0.20)	0.81	1.9	25.00
2	5.86	6.67	3.441	0.25 (0.20)	0.81	2.0	10.00
3	5.06	9.80	2.760	0.25 (0.20)	0.82	2.2	2.00
LONGEST FLOWPATH FROM NODE 2.00 TO NODE					40.00 =	257.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.04	6.05	3.639	0.25 (0.21)	0.82	2.3	25.00
2	7.11	6.67	3.441	0.25 (0.21)	0.82	2.4	10.00
3	6.51	9.80	2.760	0.25 (0.21)	0.84	2.8	2.00
4	6.48	9.89	2.746	0.25 (0.21)	0.84	2.8	35.00
TOTAL AREA (ACRES) =			2.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.11 Tc (MIN.) = 6.674
 EFFECTIVE AREA (ACRES) = 2.44 AREA-AVERAGED Fm (INCH/HR) = 0.21
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.84
 TOTAL AREA (ACRES) = 2.8
 LONGEST FLOWPATH FROM NODE 35.00 TO NODE 40.00 = 263.00 FEET.

FLOW PROCESS FROM NODE 40.00 TO NODE 50.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH(FEET) = 230.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.91
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.11
PIPE TRAVEL TIME(MIN.) = 0.78 Tc(MIN.) = 7.45
LONGEST FLOWPATH FROM NODE 35.00 TO NODE 50.00 = 493.00 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<
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FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 91.00
ELEVATION DATA: UPSTREAM(FEET) = 37.25 DOWNSTREAM(FEET) = 35.89

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.169
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.599

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.27	0.25	0.700	69	6.17

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700
SUBAREA RUNOFF (CFS) = 0.83
TOTAL AREA (ACRES) = 0.27 PEAK FLOW RATE (CFS) = 0.83

FLOW PROCESS FROM NODE 42.00 TO NODE 45.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 6.17
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.599

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.67	0.25	0.700	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700
SUBAREA AREA (ACRES) = 0.67 SUBAREA RUNOFF (CFS) = 2.06
EFFECTIVE AREA (ACRES) = 0.94 AREA-AVERAGED Fm (INCH/HR) = 0.17
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.70
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 2.90

FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00
ELEVATION DATA: UPSTREAM (FEET) = 37.25 DOWNSTREAM (FEET) = 32.75

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.294

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.846

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

".4 DWELLING/ACRE" C 0.59 0.25 0.900 69 9.29

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.39

TOTAL AREA (ACRES) = 0.59 PEAK FLOW RATE (CFS) = 1.39

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

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** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.39	9.29	2.846	0.25 (0.22)	0.90	0.6	43.00

LONGEST FLOWPATH FROM NODE 43.00 TO NODE 45.00 = 225.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	2.90	6.17	3.599	0.25 (0.17)	0.70	0.9	41.00

LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 91.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
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1	4.09	6.17	3.599	0.25 (0.19)	0.76	1.3	41.00
2	3.65	9.29	2.846	0.25 (0.19)	0.78	1.5	43.00
TOTAL AREA (ACRES) =			1.5				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.09 Tc (MIN.) = 6.169
EFFECTIVE AREA (ACRES) = 1.33 AREA-AVERAGED Fm (INCH/HR) = 0.19
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.78
TOTAL AREA (ACRES) = 1.5
LONGEST FLOWPATH FROM NODE 43.00 TO NODE 45.00 = 225.00 FEET.

FLOW PROCESS FROM NODE 45.00 TO NODE 48.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 6.17
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.599
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.81	0.25	0.700	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700
SUBAREA AREA (ACRES) = 0.81 SUBAREA RUNOFF (CFS) = 2.50
EFFECTIVE AREA (ACRES) = 2.14 AREA-AVERAGED Fm (INCH/HR) = 0.18
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.74
TOTAL AREA (ACRES) = 2.3 PEAK FLOW RATE (CFS) = 6.58

FLOW PROCESS FROM NODE 48.00 TO NODE 48.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 48.00 TO NODE 48.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 46.00 TO NODE 47.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 225.00

ELEVATION DATA: UPSTREAM (FEET) = 37.25 DOWNSTREAM (FEET) = 32.75

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.294

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.846

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

" .4 DWELLING/ACRE "	C	0.59	0.25	0.900	69	9.29
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SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.39

TOTAL AREA (ACRES) = 0.59 PEAK FLOW RATE (CFS) = 1.39

FLOW PROCESS FROM NODE 47.00 TO NODE 48.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.39	9.29	2.846	0.25 (0.22)	0.90	0.6	46.00

LONGEST FLOWPATH FROM NODE 46.00 TO NODE 48.00 = 225.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM	Q	T_c	Intensity	F_p (Fm)	A_p	A_e	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	6.58	6.17	3.599	0.25 (0.18)	0.74	2.1	41.00
2	5.60	9.29	2.846	0.25 (0.19)	0.75	2.3	43.00

LONGEST FLOWPATH FROM NODE 43.00 TO NODE 48.00 = 225.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.77	6.17	3.599	0.25 (0.19)	0.76	2.5	41.00
2	6.99	9.29	2.846	0.25 (0.20)	0.78	2.9	46.00
3	6.99	9.29	2.846	0.25 (0.20)	0.78	2.9	43.00

TOTAL AREA (ACRES) = 2.9

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.77 Tc (MIN.) = 6.169
 EFFECTIVE AREA (ACRES) = 2.53 AREA-AVERAGED Fm (INCH/HR) = 0.19
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.78
 TOTAL AREA (ACRES) = 2.9
 LONGEST FLOWPATH FROM NODE 46.00 TO NODE 48.00 = 225.00 FEET.

FLOW PROCESS FROM NODE 48.00 TO NODE 50.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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REPRESENTATIVE SLOPE = 0.0050
 FLOW LENGTH (FEET) = 310.74 MANNING'S N = 0.012
 DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.6 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.14
 ESTIMATED PIPE DIAMETER (INCH) = 21.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 7.77
 PIPE TRAVEL TIME (MIN.) = 1.01 Tc (MIN.) = 7.18
 LONGEST FLOWPATH FROM NODE 46.00 TO NODE 50.00 = 535.74 FEET.

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

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 ** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.77	7.18	3.300	0.25 (0.19)	0.76	2.5	41.00
2	6.99	10.35	2.675	0.25 (0.20)	0.78	2.9	46.00
3	6.99	10.35	2.675	0.25 (0.20)	0.78	2.9	43.00
LONGEST FLOWPATH FROM NODE			46.00	TO NODE	50.00 =	535.74	FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.04	6.83	3.394	0.25 (0.21)	0.82	2.3	25.00
2	7.11	7.45	3.229	0.25 (0.21)	0.82	2.4	10.00
3	6.51	10.59	2.640	0.25 (0.21)	0.84	2.8	2.00
4	6.48	10.68	2.628	0.25 (0.21)	0.84	2.8	35.00
LONGEST FLOWPATH FROM NODE			35.00	TO NODE	50.00 =	493.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.67	6.83	3.394	0.25 (0.20)	0.79	4.7	25.00
2	14.85	7.18	3.300	0.25 (0.20)	0.79	4.9	41.00
3	14.82	7.45	3.229	0.25 (0.20)	0.79	5.0	10.00
4	13.54	10.35	2.675	0.25 (0.20)	0.81	5.7	46.00
5	13.54	10.35	2.675	0.25 (0.20)	0.81	5.7	43.00
6	13.40	10.59	2.640	0.25 (0.20)	0.81	5.8	2.00
7	13.34	10.68	2.628	0.25 (0.20)	0.81	5.8	35.00
TOTAL AREA (ACRES) =			5.8				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 14.85 Tc (MIN.) = 7.176
 EFFECTIVE AREA (ACRES) = 4.90 AREA-AVERAGED Fm (INCH/HR) = 0.20
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.79
 TOTAL AREA (ACRES) = 5.8
 LONGEST FLOWPATH FROM NODE 46.00 TO NODE 50.00 = 535.74 FEET.

=====
 END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 5.8 TC (MIN.) = 7.18
 EFFECTIVE AREA (ACRES) = 4.90 AREA-AVERAGED Fm (INCH/HR) = 0.20
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.792
 PEAK FLOW RATE (CFS) = 14.85

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.67	6.83	3.394	0.25 (0.20)	0.79	4.7	25.00
2	14.85	7.18	3.300	0.25 (0.20)	0.79	4.9	41.00
3	14.82	7.45	3.229	0.25 (0.20)	0.79	5.0	10.00
4	13.54	10.35	2.675	0.25 (0.20)	0.81	5.7	46.00
5	13.54	10.35	2.675	0.25 (0.20)	0.81	5.7	43.00
6	13.40	10.59	2.640	0.25 (0.20)	0.81	5.8	2.00
7	13.34	10.68	2.628	0.25 (0.20)	0.81	5.8	35.00

=====
 END OF RATIONAL METHOD ANALYSIS
 =====



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* PROPOSED 10-YEAR HYDRO STUDY *
* WATERSHED SE (TO GBC) *

FILE NAME: P10SE.DAT
TIME/DATE OF STUDY: 09:59 01/09/2024

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

---*TIME-OF-CONCENTRATION MODEL*---

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:			CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN-	/	OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE	/	SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018	/	0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- 1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
- 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 256.00
ELEVATION DATA: UPSTREAM (FEET) = 38.75 DOWNSTREAM (FEET) = 33.63

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.786

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.763

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.63	0.25	0.900	69	9.79

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.44

TOTAL AREA (ACRES) = 0.63 PEAK FLOW RATE (CFS) = 1.44

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE T_c (MIN.) = 9.79

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.763

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.76	0.25	0.700	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700					
SUBAREA AREA(ACRES) = 0.76 SUBAREA RUNOFF(CFS) = 1.77					
EFFECTIVE AREA(ACRES) = 1.39 AREA-AVERAGED Fm(INCH/HR) = 0.20					
AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.79					
TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 3.21					

FLOW PROCESS FROM NODE 30.00 TO NODE 40.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 9.79
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.763
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	C	0.90	0.25	0.850	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850					
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.07					
EFFECTIVE AREA(ACRES) = 2.29 AREA-AVERAGED Fm(INCH/HR) = 0.20					
AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.81					
TOTAL AREA(ACRES) = 2.3 PEAK FLOW RATE(CFS) = 5.27					

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 258.00
ELEVATION DATA: UPSTREAM (FEET) = 38.75 DOWNSTREAM (FEET) = 33.59

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.817

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.758

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

".4 DWELLING/ACRE" C 0.63 0.25 0.900 69 9.82

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.44

TOTAL AREA (ACRES) = 0.63 PEAK FLOW RATE (CFS) = 1.44

FLOW PROCESS FROM NODE 32.00 TO NODE 40.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	1.44	9.82	2.758	0.25 (0.22)	0.90	0.6	31.00

LONGEST FLOWPATH FROM NODE 31.00 TO NODE 40.00 = 258.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	5.27	9.79	2.763	0.25 (0.20)	0.81	2.3	10.00

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 40.00 = 256.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
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1	6.71	9.79	2.763	0.25 (0.21)	0.83	2.9	10.00
2	6.70	9.82	2.758	0.25 (0.21)	0.83	2.9	31.00
TOTAL AREA (ACRES) =			2.9				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.71 Tc (MIN.) = 9.786
EFFECTIVE AREA (ACRES) = 2.92 AREA-AVERAGED Fm (INCH/HR) = 0.21
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.83
TOTAL AREA (ACRES) = 2.9
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 40.00 = 258.00 FEET.

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
ELEVATION DATA: UPSTREAM (FEET) = 38.75 DOWNSTREAM (FEET) = 37.55

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] **0.20
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 5.478
* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.853

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						

" .4 DWELLING/ACRE" C 0.22 0.25 0.900 69 5.48
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900
 SUBAREA RUNOFF(CFS) = 0.72
 TOTAL AREA(ACRES) = 0.22 PEAK FLOW RATE(CFS) = 0.72

 FLOW PROCESS FROM NODE 36.00 TO NODE 40.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.72	5.48	3.853	0.25(0.22)	0.90	0.2	35.00
LONGEST FLOWPATH FROM NODE				35.00 TO NODE	40.00 =		60.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.71	9.79	2.763	0.25(0.21)	0.83	2.9	10.00
2	6.70	9.82	2.758	0.25(0.21)	0.83	2.9	31.00
LONGEST FLOWPATH FROM NODE				31.00 TO NODE	40.00 =		258.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.08	5.48	3.853	0.25(0.21)	0.84	1.9	35.00
2	7.21	9.79	2.763	0.25(0.21)	0.84	3.1	10.00
3	7.20	9.82	2.758	0.25(0.21)	0.84	3.1	31.00
TOTAL AREA(ACRES) =			3.1				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 7.21 Tc(MIN.) = 9.786
 EFFECTIVE AREA(ACRES) = 3.14 AREA-AVERAGED F_m (INCH/HR) = 0.21
 AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 0.84
 TOTAL AREA(ACRES) = 3.1
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 40.00 = 258.00 FEET.

FLOW PROCESS FROM NODE 40.00 TO NODE 50.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 9.79

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.763

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
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RESIDENTIAL

"1 DWELLING/ACRE" C 0.35 0.25 0.800 69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800

SUBAREA AREA (ACRES) = 0.35 SUBAREA RUNOFF (CFS) = 0.81

EFFECTIVE AREA (ACRES) = 3.49 AREA-AVERAGED Fm (INCH/HR) = 0.21

AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.83

TOTAL AREA (ACRES) = 3.5 PEAK FLOW RATE (CFS) = 8.02

FLOW PROCESS FROM NODE 50.00 TO NODE 50.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 235.00

ELEVATION DATA: UPSTREAM (FEET) = 38.75 DOWNSTREAM (FEET) = 34.05

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)] ** 0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 9.457

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.818

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.65	0.25	0.900	69	9.46
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900						
SUBAREA RUNOFF(CFS) = 1.52						
TOTAL AREA(ACRES) = 0.65 PEAK FLOW RATE(CFS) = 1.52						

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE Tc(MIN.) = 9.46
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.818
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	C	0.68	0.25	0.700	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700					
SUBAREA AREA(ACRES) = 0.68 SUBAREA RUNOFF(CFS) = 1.62					
EFFECTIVE AREA(ACRES) = 1.33 AREA-AVERAGED Fm(INCH/HR) = 0.20					
AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.80					
TOTAL AREA(ACRES) = 1.3 PEAK FLOW RATE(CFS) = 3.13					

FLOW PROCESS FROM NODE 43.00 TO NODE 46.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 9.46
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.818
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					

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"1 DWELLING/ACRE"          C          0.97      0.25      0.800      69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800
SUBAREA AREA(ACRES) =      0.97      SUBAREA RUNOFF(CFS) =      2.29
EFFECTIVE AREA(ACRES) =      2.30      AREA-AVERAGED Fm(INCH/HR) = 0.20
AREA-AVERAGED Fp(INCH/HR) = 0.25      AREA-AVERAGED Ap = 0.80
TOTAL AREA(ACRES) =      2.3          PEAK FLOW RATE(CFS) =      5.42

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FLOW PROCESS FROM NODE      46.00 TO NODE      46.00 IS CODE = 10
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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<
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FLOW PROCESS FROM NODE      44.00 TO NODE      45.00 IS CODE = 21
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
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INITIAL SUBAREA FLOW-LENGTH(FEET) =      253.00
ELEVATION DATA: UPSTREAM(FEET) =      38.75  DOWNSTREAM(FEET) =      33.69

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =      9.740
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.770

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SUBAREA Tc AND LOSS RATE DATA(AMC II):

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DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						
"1.4 DWELLING/ACRE"	C	0.65	0.25	0.900	69	9.74
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900						
SUBAREA RUNOFF(CFS) = 1.49						
TOTAL AREA(ACRES) = 0.65 PEAK FLOW RATE(CFS) = 1.49						

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FLOW PROCESS FROM NODE      45.00 TO NODE      46.00 IS CODE = 11
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>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.49	9.74	2.770	0.25(0.22)	0.90	0.6	44.00
LONGEST FLOWPATH FROM NODE				44.00 TO NODE	46.00 =	253.00 FEET.	

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.42	9.46	2.818	0.25(0.20)	0.80	2.3	41.00
LONGEST FLOWPATH FROM NODE				41.00 TO NODE	46.00 =	235.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.89	9.46	2.818	0.25(0.21)	0.82	2.9	41.00
2	6.81	9.74	2.770	0.25(0.21)	0.82	2.9	44.00
TOTAL AREA (ACRES) =				2.9			

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.89 Tc (MIN.) = 9.457
EFFECTIVE AREA (ACRES) = 2.93 AREA-AVERAGED Fm (INCH/HR) = 0.21
AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.82
TOTAL AREA (ACRES) = 2.9
LONGEST FLOWPATH FROM NODE 44.00 TO NODE 46.00 = 253.00 FEET.

FLOW PROCESS FROM NODE 46.00 TO NODE 50.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.89	9.46	2.818	0.25(0.21)	0.82	2.9	41.00

2 6.81 9.74 2.770 0.25(0.21) 0.82 2.9 44.00
 LONGEST FLOWPATH FROM NODE 44.00 TO NODE 50.00 = 253.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.23	5.48	3.853	0.25(0.21)	0.83	2.2	35.00
2	8.02	9.79	2.763	0.25(0.21)	0.83	3.5	10.00
3	8.01	9.82	2.758	0.25(0.21)	0.83	3.5	31.00
LONGEST FLOWPATH FROM NODE			31.00	TO NODE	50.00 =	258.00	FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.80	5.48	3.853	0.25(0.21)	0.83	3.9	35.00
2	14.85	9.46	2.818	0.25(0.21)	0.83	6.3	41.00
3	14.82	9.74	2.770	0.25(0.21)	0.83	6.4	44.00
4	14.81	9.79	2.763	0.25(0.21)	0.83	6.4	10.00
5	14.79	9.82	2.758	0.25(0.21)	0.83	6.4	31.00
TOTAL AREA (ACRES) =			6.4				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 14.85 Tc (MIN.) = 9.457
 EFFECTIVE AREA (ACRES) = 6.32 AREA-AVERAGED Fm (INCH/HR) = 0.21
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.83
 TOTAL AREA (ACRES) = 6.4
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 50.00 = 258.00 FEET.

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END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 6.4 TC (MIN.) = 9.46
 EFFECTIVE AREA (ACRES) = 6.32 AREA-AVERAGED Fm (INCH/HR) = 0.21
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.827
 PEAK FLOW RATE (CFS) = 14.85

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.80	5.48	3.853	0.25(0.21)	0.83	3.9	35.00
2	14.85	9.46	2.818	0.25(0.21)	0.83	6.3	41.00

3	14.82	9.74	2.770	0.25 (0.21)	0.83	6.4	44.00
4	14.81	9.79	2.763	0.25 (0.21)	0.83	6.4	10.00
5	14.79	9.82	2.758	0.25 (0.21)	0.83	6.4	31.00

=====
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END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Ver. 21.0 Release Date: 06/01/2014 License ID 1624

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 503.027 LAKE CENTER INDUSTRIAL COMPLEX *
* PROPOSED 10-YEAR HYDRO STUDY *
* WATERSHED E (TO GBC) *

FILE NAME: P10E.DAT
TIME/DATE OF STUDY: 10:26 01/09/2024

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-	CROWN TO	STREET-CROSSFALL:	CURB	GUTTER-GEOMETRIES:			MANNING
	WIDTH	CROSSFALL	IN- / OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE	FACTOR
	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- 1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
- 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 10.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 191.00
ELEVATION DATA: UPSTREAM (FEET) = 39.00 DOWNSTREAM (FEET) = 35.18

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 8.704

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.955

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.46	0.25	0.900	69	8.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 1.13

TOTAL AREA (ACRES) = 0.46 PEAK FLOW RATE (CFS) = 1.13

FLOW PROCESS FROM NODE 20.00 TO NODE 30.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE T_c (MIN.) = 8.70

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.955

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.50	0.25	0.900	69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900					
SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 1.23					
EFFECTIVE AREA(ACRES) = 0.96 AREA-AVERAGED Fm(INCH/HR) = 0.22					
AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.90					
TOTAL AREA(ACRES) = 1.0 PEAK FLOW RATE(CFS) = 2.36					

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 240.00
 ELEVATION DATA: UPSTREAM(FEET) = 39.00 DOWNSTREAM(FEET) = 34.20

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.537

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.804

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.46	0.25	0.900	69	9.54
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900						
SUBAREA RUNOFF(CFS) = 1.07						
TOTAL AREA(ACRES) = 0.46 PEAK FLOW RATE(CFS) = 1.07						

FLOW PROCESS FROM NODE 22.00 TO NODE 30.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.07	9.54	2.804	0.25 (0.22)	0.90	0.5	21.00
LONGEST FLOWPATH FROM NODE				21.00 TO NODE	30.00 =	240.00 FEET.	

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.36	8.70	2.955	0.25 (0.22)	0.90	1.0	10.00
LONGEST FLOWPATH FROM NODE				10.00 TO NODE	30.00 =	191.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.39	8.70	2.955	0.25 (0.22)	0.90	1.4	10.00
2	3.30	9.54	2.804	0.25 (0.22)	0.90	1.4	21.00
TOTAL AREA (ACRES) =			1.4				

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.39 Tc (MIN.) = 8.704
 EFFECTIVE AREA (ACRES) = 1.38 AREA-AVERAGED Fm (INCH/HR) = 0.22
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.90
 TOTAL AREA (ACRES) = 1.4
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 30.00 = 240.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
ELEVATION DATA: UPSTREAM (FEET) = 39.00 DOWNSTREAM (FEET) = 37.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 5.478

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.853

SUBAREA T_c AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

".4 DWELLING/ACRE" C 0.20 0.25 0.900 69 5.48

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.900

SUBAREA RUNOFF (CFS) = 0.65

TOTAL AREA (ACRES) = 0.20 PEAK FLOW RATE (CFS) = 0.65

FLOW PROCESS FROM NODE 26.00 TO NODE 30.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	F_p (Fm) (INCH/HR)	A_p	A_e (ACRES)	HEADWATER NODE
1	0.65	5.48	3.853	0.25 (0.22)	0.90	0.2	25.00

LONGEST FLOWPATH FROM NODE 25.00 TO NODE 30.00 = 60.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.39	8.70	2.955	0.25 (0.22)	0.90	1.4	10.00
2	3.30	9.54	2.804	0.25 (0.22)	0.90	1.4	21.00

LONGEST FLOWPATH FROM NODE 21.00 TO NODE 30.00 = 240.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.49	5.48	3.853	0.25 (0.22)	0.90	1.1	25.00
2	3.88	8.70	2.955	0.25 (0.22)	0.90	1.6	10.00
3	3.76	9.54	2.804	0.25 (0.22)	0.90	1.6	21.00

TOTAL AREA (ACRES) = 1.6

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.88 Tc (MIN.) = 8.704
 EFFECTIVE AREA (ACRES) = 1.58 AREA-AVERAGED Fm (INCH/HR) = 0.22
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.90
 TOTAL AREA (ACRES) = 1.6
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 30.00 = 240.00 FEET.

FLOW PROCESS FROM NODE 30.00 TO NODE 40.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 8.70

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.955

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					

"1 DWELLING/ACRE"	C	1.39	0.25	0.800	69
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SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800

SUBAREA AREA (ACRES) = 1.39 SUBAREA RUNOFF (CFS) = 3.45

EFFECTIVE AREA (ACRES) = 2.97 AREA-AVERAGED Fm (INCH/HR) = 0.21

AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.85

TOTAL AREA (ACRES) = 3.0 PEAK FLOW RATE (CFS) = 7.33

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.06	5.48	3.853	0.25 (0.21)	0.84	2.5	25.00
2	7.33	8.70	2.955	0.25 (0.21)	0.85	3.0	10.00
3	7.02	9.54	2.804	0.25 (0.21)	0.85	3.0	21.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 8.06 Tc (MIN.) = 5.48
AREA-AVERAGED Fm (INCH/HR) = 0.21 AREA-AVERAGED Fp (INCH/HR) = 0.25
AREA-AVERAGED Ap = 0.84 EFFECTIVE AREA (ACRES) = 2.46

FLOW PROCESS FROM NODE 40.00 TO NODE 40.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 182.50
ELEVATION DATA: UPSTREAM (FEET) = 39.00 DOWNSTREAM (FEET) = 35.35

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.547

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.986

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.45	0.25	0.900	69	8.55

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900

SUBAREA RUNOFF (CFS) = 1.12

TOTAL AREA (ACRES) = 0.45 PEAK FLOW RATE (CFS) = 1.12

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 8.55

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.986

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------

RESIDENTIAL

"3-4 DWELLINGS/ACRE" C 0.63 0.25 0.600 69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.25

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600

SUBAREA AREA (ACRES) = 0.63 SUBAREA RUNOFF (CFS) = 1.61

EFFECTIVE AREA (ACRES) = 1.08 AREA-AVERAGED Fm (INCH/HR) = 0.18

AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.73

TOTAL AREA (ACRES) = 1.1 PEAK FLOW RATE (CFS) = 2.73

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

FLOW PROCESS FROM NODE 32.50 TO NODE 32.60 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 182.50

ELEVATION DATA: UPSTREAM (FEET) = 39.00 DOWNSTREAM (FEET) = 35.35

Tc = K * [(LENGTH** 3.00) / (ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 8.547

* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.986

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL ".4 DWELLING/ACRE"	C	0.38	0.25	0.900	69	8.55
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.25						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900						
SUBAREA RUNOFF(CFS) = 0.94						
TOTAL AREA(ACRES) = 0.38 PEAK FLOW RATE(CFS) = 0.94						

FLOW PROCESS FROM NODE 32.60 TO NODE 33.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	0.94	8.55	2.986	0.25(0.22)	0.90	0.4	32.50
LONGEST FLOWPATH FROM NODE 32.50 TO NODE 33.00 = 182.50 FEET.							

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.73	8.55	2.986	0.25(0.18)	0.73	1.1	31.00
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 33.00 = 182.50 FEET.							

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.67	8.55	2.986	0.25(0.19)	0.77	1.5	32.50
2	3.67	8.55	2.986	0.25(0.19)	0.77	1.5	31.00
TOTAL AREA(ACRES) = 1.5							

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.67 Tc(MIN.) = 8.547
 EFFECTIVE AREA(ACRES) = 1.46 AREA-AVERAGED Fm(INCH/HR) = 0.19
 AREA-AVERAGED Fp(INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.77

TOTAL AREA (ACRES) = 1.5
 LONGEST FLOWPATH FROM NODE 32.50 TO NODE 33.00 = 182.50 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 40.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<
 =====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.67	8.55	2.986	0.25 (0.19)	0.77	1.5	32.50
2	3.67	8.55	2.986	0.25 (0.19)	0.77	1.5	31.00
LONGEST FLOWPATH FROM NODE 32.50 TO NODE					40.00 =	182.50 FEET.	

** MEMORY BANK # 3 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	8.06	5.48	3.853	0.25 (0.21)	0.84	2.5	25.00
2	7.33	8.70	2.955	0.25 (0.21)	0.85	3.0	10.00
3	7.02	9.54	2.804	0.25 (0.21)	0.85	3.0	21.00
LONGEST FLOWPATH FROM NODE 21.00 TO NODE					40.00 =	240.00 FEET.	

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.14	5.48	3.853	0.25 (0.21)	0.82	3.4	25.00
2	11.03	8.55	2.986	0.25 (0.21)	0.83	4.4	31.00
3	11.03	8.55	2.986	0.25 (0.21)	0.83	4.4	32.50
4	10.96	8.70	2.955	0.25 (0.21)	0.83	4.4	10.00
5	10.45	9.54	2.804	0.25 (0.21)	0.83	4.5	21.00
TOTAL AREA (ACRES) =		4.5					

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 11.14 Tc (MIN.) = 5.478
 EFFECTIVE AREA (ACRES) = 3.39 AREA-AVERAGED Fm (INCH/HR) = 0.21
 AREA-AVERAGED Fp (INCH/HR) = 0.25 AREA-AVERAGED Ap = 0.83
 TOTAL AREA (ACRES) = 4.5

LONGEST FLOWPATH FROM NODE 21.00 TO NODE 40.00 = 240.00 FEET.

=====
END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 4.5 TC (MIN.) = 5.48
EFFECTIVE AREA (ACRES) = 3.39 AREA-AVERAGED F_m (INCH/HR) = 0.21
AREA-AVERAGED F_p (INCH/HR) = 0.25 AREA-AVERAGED A_p = 0.823
PEAK FLOW RATE (CFS) = 11.14

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	T _c (MIN.)	Intensity (INCH/HR)	F _p (F _m) (INCH/HR)	A _p	A _e (ACRES)	HEADWATER NODE
1	11.14	5.48	3.853	0.25 (0.21)	0.82	3.4	25.00
2	11.03	8.55	2.986	0.25 (0.21)	0.83	4.4	31.00
3	11.03	8.55	2.986	0.25 (0.21)	0.83	4.4	32.50
4	10.96	8.70	2.955	0.25 (0.21)	0.83	4.4	10.00
5	10.45	9.54	2.804	0.25 (0.21)	0.83	4.5	21.00

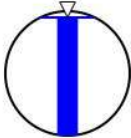
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END OF RATIONAL METHOD ANALYSIS




APPENDIX G- HYDRAULIC CALCULATIONS

Storm Drain Pipe Mainline Capacity Calculation

Existing 18" Mainline			
Outfall Pipe of W1			
Inputs		Results	
Pipe diameter, d_0	18 in	Flow depth, y	17.1000 in
Manning roughness, n	0.013	Flow area, a	249.7127 sq. in.
Pressure slope (possibly ? equal to pipe slope), S_0	0.5 % rise/run	Pipe area, a_0	254.4695 sq. in.
Relative flow depth, y/d_0	.95 fraction	Relative area, a/a_0	0.9813 fraction
		Wetted perimeter, P_w	4.0358 ft
		Hydraulic radius, R_h	0.4297 ft
		Top width, T	0.6538 ft
		Velocity, v	4.6021 ft/sec
		Velocity head, h_v	0.3292 ft H2O
		Froude number, F	0.50
		Average shear stress (tractive force), τ	0.1341 psf
		Flow, Q (See notes)	7.9804 cfs
		Full flow, Q_0	7.4270 cfs
		Ratio to full flow, Q/Q_0	1.0745 fraction



Proposed 24" Mainline			
Watershed E Mainline			
Inputs		Results	
Pipe diameter, d_0	24 in	Flow depth, y	22.8000 in
Manning roughness, n	0.011	Flow area, a	443.9337 sq. in.
Pressure slope (possibly ? equal to pipe slope), S_0	0.3 % rise/run	Pipe area, a_0	452.3902 sq. in.
Relative flow depth, y/d_0	.95 fraction	Relative area, a/a_0	0.9813 fraction
		Wetted perimeter, P_w	5.3811 ft
		Hydraulic radius, R_h	0.5729 ft
		Top width, T	0.8718 ft
		Velocity, v	5.1036 ft/sec
		Velocity head, h_v	0.4048 ft H2O
		Froude number, F	0.48
		Average shear stress (tractive force), τ	0.1073 psf
		Flow, Q (See notes)	15.7333 cfs
		Full flow, Q_0	14.6423 cfs
		Ratio to full flow, Q/Q_0	1.0745 fraction



Proposed 27" Mainline

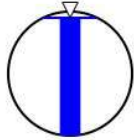
Watershed E Mainline

Inputs

Pipe diameter, d_0	27	in
Manning roughness, n	0.011	
Pressure slope (possibly ? equal to pipe slope), S_0	0.3	% rise/run
Relative flow depth, y/d_0	.95	fraction

Results

Flow depth, y	25.6500	in
Flow area, a	561.8536	sq. in.
Pipe area, a_0	572.5564	sq. in.
Relative area, a/a_0	0.9813	fraction
Wetted perimeter, P_w	6.0537	ft
Hydraulic radius, R_h	0.6445	ft
Top width, T	0.9807	ft
Velocity, v	5.5205	ft/sec
Velocity head, h_v	0.4737	ft H2O
Froude number, F	0.49	
Average shear stress (tractive force), τ	0.1207	psf
Flow, Q (See notes)	21.5391	cfs
Full flow, Q_0	20.0454	cfs
Ratio to full flow, Q/Q_0	1.0745	fraction



Existing 39" Mainline

Outfall Pipe to GBC

Inputs

Pipe diameter, d_0	39	in
Manning roughness, n	0.013	
Pressure slope (possibly ? equal to pipe slope), S_0	0.2	% rise/run
Relative flow depth, y/d_0	.95	fraction

Results

Flow depth, y	37.0500	in
Flow area, a	1172.2624	sq. in.
Pipe area, a_0	1194.5930	sq. in.
Relative area, a/a_0	0.9813	fraction
Wetted perimeter, P_w	8.7443	ft
Hydraulic radius, R_h	0.9310	ft
Top width, T	1.4166	ft
Velocity, v	4.8736	ft/sec
Velocity head, h_v	0.3691	ft H2O
Froude number, F	0.36	
Average shear stress (tractive force), τ	0.1162	psf
Flow, Q (See notes)	39.6734	cfs
Full flow, Q_0	36.9222	cfs
Ratio to full flow, Q/Q_0	1.0745	fraction

