STRUCTURAL GENERAL NOTES – APPLICABLE TO ALL CONSTRUCTION UNLESS OTHERWISE NOTED ON THE PLANS

A. DESIGN SCOPE BY PSE CONSULTING ENGINEERING (PSE):

1. Design Shown on drawings by PSE is for the following items.
   a. Anchor Bolt Analysis For Container Only.
2. Design Shown on PSE drawings does not include: Fabric Truss & any other Roof, Fabric Truss to Container Connection, Container, Foundation, finishes, architectural items, windows, doors, moisture barriers, water proofing, mechanical units, plumbing, or electrical items.

B. GENERAL REQUIREMENT:

1. Furnish all labor, materials, and equipment necessary to complete the work shown or inferred by these drawings.
2. Where construction details are not shown or noted for any part of the work, such details shall be the same as for similar work shown on the drawings.
3. Notes and details on the drawings take precedence over the general notes and typical details in case of conflict.
4. Provide manufacturer's approved product evaluation reports (ICC reports) and a list of all proposed substitutions to the Engineer for review and written approval before fabrication.
5. Pipes, ducts, sleeves, chases, etc. shall not be placed in slabs, beams, or walls unless specifically shown or noted nor shall any structural member be cut for pipe, ducts, etc., unless specifically shown. Obtain prior written approval for installation of any additional holes, ducts, etc.
6. Locate and protect underground or concealed conduit, plumbing or other utilities where new work is being performed.
7. The contract drawings and specifications represent the finished structure and do not indicate methods, procedures or sequence of construction. The contractor shall take necessary precautions to maintain and insure the integrity of the new and any existing structures during construction. The design stresses shall not be exceeded during construction based on the age of each element . Neither the owner nor Architect/Engineer will enforce safety measure regulations. Contractor shall design, construct and maintain all safety devices, including shoring and bracing for the new and any existing structures and shall be solely responsible for conforming to all local, state and federal safety and health standards, laws and regulations. Observation visits to the site by the engineer shall not include inspection of the above items.
8. Obtain prior written approval for any changes to the drawings.
9. The contractor shall review and compare the structural drawings with all other Construction Documents, such as Architectural, Mechanical and Electrical drawings, specifications, etc. Do not scale drawings. The contractor shall verify dimensions, elevations and all information. Report, in writing, any inconsistencies, errors, or omissions to the Architect/Engineer of record before proceeding with the work.
10. All existing constructions shown are schematic only. Contractor is responsible to verify actual conditions and allow for them in his bid. Notify the Architect/Engineer, in writing, in case of any discrepancy between actual conditions and what is shown on the structural drawings before proceeding with the work.
11. See Architectural, Mechanical, Electrical and other drawings for embedded items.
12. Camber shall be provided for all members with 30 feet or more of span. Check beam table and contact the Structural Engineer for the amount of camber.
13. Submit seismic anchorage calculations stamped by a licensed Professional Engineer for all equipment and components weighing more than 400 lb.
14. Submit structural drawings signed and sealed by a professional Engineer licensed in the State where the project is located for any structural member needed for this project that is not designed by P.S.E.
15. Any substitutions for structural members, hardware or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
16. All communication shall be in writing. No verbal communications, decisions, instructions or approvals shall be valid.

C. CODE AND LOADS:

1. All design, material, and construction work for this project shall conform to the CALIFORNIA BUILDING CODE 2022, based on the 2021 International Building Code (IBC).

2. Design parameters: (REFER STRUCTURAL CALCULATION REPORT PAGES FROM DOTEC CORP. FROM OWNER FOR FURTHER DETAILS)

a. Floor Live Load = 312.5 psf.	b. Floor Dead Load = 10 psf.
c. Roof Live Load = 5-12 psf.	d. Roof dead load = 3 psf.
e. Ground Snow Load, Pg = 6 psf.	f. Flat Roof snow load = N/A psf.
g. Snow Exposure Factor, Ce = 1.0	h. Snow Load Importance Factor, Is = 1.0
i. Thermal Factor, Ct = 1.2	j. Basic Wind Speed (3 second gust) = 93 mph
k. Risk Category = II	l. Wind Exposure = C
m. Internal Pressure Coefficient = +/- 0.55	n. Components and Cladding studs = N/A psf
o. Seismic Importance Factor, Ie = 1.0	p. Site Class = D
q. Ss = 0.720	r. S1 = 0.250
s. Sms = 1.050	t. Sm1 = 0.660
u. Sds = 0.700	v. Sd1 = 0.450
w. Seismic Design Category = D	x. Basic Seismic Force Resisting System = STEEL MODULAR CONTAINER
y. Design Base Shear = 0.35 * W	z. Approximate Fundamental Period, T = 0.108
aa. Response Modification Factor, R = 2	bb. Analysis Procedure Used = Equivalent Lateral Force Procedure

D. INSPECTION:

1. All construction shall be inspected by the building officials according to the above Code.
2. It is recommended that the owner or the contractor hire PSE or other Qualified Licensed inspectors to provide inspection during construction.

E. STRUCTURAL STEEL:

1. Unless noted otherwise on plans, all structural steel shall be per Table 2-3 of AISC Manual of Steel Construction, Fourteenth Edition & AISC Specification 360-10, as shown below:

Shape	Steel, Typ	Yield Strength Fy, KSI
W-shape	A 992	50
Angles	A 36	36
Rectangular Tube, HSS	A 500, Gr.C	50
Round Tube, HSS	A 500, Gr.C	46
Pipe	A53, Gr.B	35

2. Fabrication and erection shall be in accordance with the American Institute of Steel Construction (AISC).
3. All beam connections shall be bolted or shop welded as detailed on the drawing or designed by fabricator per AISC Manual of Steel Construction allowable Stress Design, "Framed Beam Connections." Bolts shall be 3/4 inch. diameter ASTM A325, load indicator bolts. All bolts shall be tightened to the minimum tension specified in the specification for structural joints using A325 or A490 bolts.
4. All welding shall conform to the current American Welding Society (AWS) Specifications and be performed by certified welders.
5. Column anchor bolts shall have minimum yield strength of 36 KSI.
6. Metal deck shall be the type as indicated on the drawings. The deck shall be welded to the supporting members per manufacturer's recommendations or as indicated on the drawings whichever is more restricting or stringent.
7. All openings in metal deck to have 4" X 4" X ¼" angle frames set between joists.
8. All structural steel and bar joists shall have one shop coat of rust inhibitor primer paint conforming to specification. Field touch up all unpainted areas and weld areas.
9. Grout for base plates shall be nonmetallic, non-shrinkage cementitious grout having a minimum 3-days compressive strength of 4000 PSI.
10. Reference specifications for additional requirements.

F. SHIPPING / CARGO CONTAINER SPECIFICATION:

1. The shipping / cargo container(s) shall be:
   a) Undamaged (Free of rust, dents, cracks, et cetera that affect the structural integrity of the container).
   b) Made from steel.
   c) Design and tested according to the International Organization for Standardization (ISO) specifications.
   d) Certified for compliance to the Rules for Certification of Cargo Containers and the International Convention for Safe Containers (CSC) for use as shipping containers by the American Bureau of Shipping (ABS) or other approved Certified Inspection and Testing Agency (CITA).
   e) Container shall have a CSC safety approval placard (CSC Plate) and CITA logo prior to any modification.
   f) Walls and roof are continuously welded around its entire periphery and is itself made from sheets of corrugated 14ga. Cor-Ten steel also continuously welded together. This steel, also used for the side and end walls has a minimum yield strength of 50ksi and tensile of 70ksi.
2. Prior to construction/modification, Client / Owner / Contractor shall:
   a) Provide pictures of high/good quality to PSE showing the following of each container:
      - All four sides
      - Roof
      - Under framing
      - Top Rails
      - Bottom Rails
      - Interior
      - CSC Plate
   b) Visually inspect all existing welds for consistency and undamaged.
   c) Confirm existing plywood floor sheathing is:
      - Not damaged, cracked, deformed, delaminated, or showing any other signs that structural integrity has been compromised
      - Free of hazardous materials, liquids, and/or stains, or shall be encapsulated.
      - Fully fastened to container floor joists per original construction.
      - If existing plywood floor sheathing needs to be replaced, inform PSE prior to replacing with equivalent pressure/preservative treated OSB or ¼" steel plate with equivalent or better fastening to container floor joists.
   d) If possible, provide manufacturer drawings of container to PSE.

G. ABBREVIATIONS:

AB	ANCHOR BOLT	EQ	EQUAL	LL	LIVE LOAD	RFT	RAFTERS
ADDL	ADDITIONAL	ES	EACH SIDE	MATL	MATERIAL	SGN	STRUCTURAL GENERAL NOTES
ALT	ALTERNATE	EW	EACH WAY	MAX	MAXIMUM	SEP	SEPARATION
APA	AMERICAN PLYWOOD ASSOCIATION	FA	FRAMING ANCHOR	MB	MACHINE BOLT	SIM	SIMILAR
		FD	FROST DEPTH	MFR	MANUFACTURER	SN	SHEAR NAIL
ARCH	ARCHITECTURAL	FEN	FLOOR EDGE NAILING	MIN.	MINIMUM	SNL	SNOW LOAD
B	BOTTOM	FF	FINISHED FLOOR	MTL	METAL	SPEC	SPECIFICATION
BLKG	BLOCKING	FN	FIELD/INTERMEDIATE NAILING	NO.	NUMBER	STD	STANDARD
BN	BOUNDARY NAIL			NS	NEAR SIDE	STGR	STAGGER
BOF	BOTTOM OF FOOTING	FS	FAR SIDE	NTS	NOT TO SCALE	STIFF	STIFFENERS
IBC	INTERNATIONAL BUILDING CODE	FTG	FOOTING	OC	ON CENTER	T	TOP
		GALV	GALVANIZED	OD	OUTSIDE DIAMETER	TB	TOP & BOTTOM
CJ	CONSTRUCTION JOINT OR CONTROL JOINT	GC	GENERAL CONTRACTOR	ORSC	OREGON RESIDENTIAL SPECIALTY CODE	TD	TYPICAL DETAILS
		GIR	GEOTECHNICAL INVESTIGATION REPORT			TG	TONGUE & GROOVE
CL	CENTER LINE			OH	OPPOSITE HAND	THK	THICKNESS/THICK
CLR	CLEAR	GLB	GLUED LAMINATED BEAM	OSB	ORIENTED STRAND BOARD	TN	TOENAIL
CONN	CONNECTION	GR	GRADE	OSSC	OREGON STRUCTURAL SPECIALTY CODE	TOB	TOP OF BEAM
CONT	CONTINUOUS	HDR	HEADER			TOF	TOP OF FOOTING
DBL	DOUBLE	HGR	HANGER	OSV	ON SITE VERIFY	TOW	TOP OF WALL
DIM	DIMENSION	HORIZ	HORIZONTAL	OTOB	OUT TO OUT OF BEARING	TYP	TYPICAL
DL	DEAD LOAD	HSH	HORIZONTALLY SLOTTED HOLES	PERP	PERPENDICULAR	UON	UNLESS OTHERWISE NOTED
DO	DITTO (REPEAT)	IBC	INTERNATIONAL BUILDING CODE	PL	PLATE	VERT	VERTICAL
DWG	DRAWING			PLF	POUND PER LINEAR FOOT	VSH	VERTICAL SLOTTED HOLES
DWL	DOWEL	ICC	INTERNATIONAL CODE COUNCIL	PSE	PSE CONSULTING ENGINEERS, INC.	WD	WOOD
E	EXISTING	ID	INSIDE DIAMETER			WEN	WALL EDGE NAILING
EA	EACH	INT	INTERIOR	PT	PRESSURE TREATED	WWM	WELDED WIRE MESH
EF	EACH FACE	JT	JOINT	PW	PLATE WASHER	W/	WITH
EL	ELEVATION	LDGR	LEDGER	REF	REFERENCE	W/O	WITHOUT
EMBED	EMBEDMENT	LGST	LIGHT GAUGE STEEL, COLD-FORMED STEEL	REN	ROOF EDGE NAILING		
EN	EDGE NAIL			REINF	REINFORCEMENT		
EOR	ENGINEER OF RECORD						

Fabric Shelters and Containers Engineering is NOT included in PSE's scope of service.

Refer to Cushing Terrell stamped engineering package for Fabric Shelters by Mr. Sushil Shenoy, P.E.

Refer to DOTec Corp. stamped engineering package for Containers engineering by Mr. Ali Elawadi, P.E.

SHEET INDEX:		REV.
S1	GENERAL STRUCTURAL NOTES & DETAILS	
S1.1	SPECIAL INSPECTION NOTES	
S2	ANCHOR LAYOUT PLAN & DETAILS	
S2.1	TYPICAL FRAMING DETAILS	

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PSE CONSULTING ENGINEERS, INC.

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Project:
Container Anchorage

4300 Capital Avenue, West Sacramento, CA 95691.

Owner:
CITY OF WEST SACRAMENTO

Stamp:

4-7-26
REGISTERED PROFESSIONAL ENGINEER
ABHIL FAIZAL
NO. 26002
CIVIL
STATE OF CALIFORNIA
Exp: 12/31/26

MARK	DESCRIPTION	DATE
	Revision Table	

DRAWN BY: AYPN
DESIGNED BY: ISL
CHECKED BY: N.T.
Date: March-17-2026

Project #:
Haynes Studio Arch. 225-001

Title:
GENERAL STRUCTURAL NOTES

Page:
S1

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SPECIAL INSPECTION, MATERIAL TESTING & STRUCTURAL OBSERVATION ITEMS REQUIRED BY CHAPTER 17 OF THE 2021 IBC

Indicate items requiring special inspection, structural testing, or structural observations by checking the appropriate box. All items not requiring inspection/testing should be removed from the form. For items requiring continuous inspection, a special inspector must be present onsite during the performance of that task. In most cases periodic inspections/tests shall be performed prior to commencing the task, intermittently during the task, and at the completion of the task. The Detailed Instructions & Frequency provides a description of the presumed requirements for tasks requiring periodic inspections. The design professional in responsible should revise the requirements as needed on a project-specific basis.

SPECIAL INSPECTION OF FABRICATED ITEMS (IBC 1704.2.5)

☐ Approved Fabricator	Yes	No

☐ Unapproved Fabricator	Yes	No

Fabricators Name:	
Fabricators plant location	

Required In-plant Inspections: ☐ Steel Construction ☐ Concrete Construction ☐ Wood Construction ☐ Cold-formed Construction ☐ Other:_____ ☐ Other:_____

STRUCTURAL STEEL (IBC 1705.2.1, 1705.12.1 & 1705.13.1)

Item			Detailed Instructions and Frequencies
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16):			
Verify welding procedures (WPS) and consumable certificates	☐ Continuous	☒ Periodic	
Material identification	☐ Continuous	☒ Periodic	Verify type and grade of material.
Welder identification	☐ Continuous	☒ Periodic	A system shall be maintained by which a welder who has welded a joint or member can be identified.
Fit- up groove welds	☐ Continuous	☒ Periodic	Verify joint preparation, dimensions, cleanliness, tacking, and backing.
Fit- up of fillet welds	☐ Continuous	☒ Periodic	Verify alignment, gaps at root, cleanliness of steel surfaces, and tack weld quality and location.
DURING WELDING (TABLE N5.4-2, AISC 360-16):			
Use of qualified welders	☐ Continuous	☒ Periodic	Verify that welders are appropriately qualified.
Control and handling of welding consumables	☐ Continuous	☒ Periodic	Verify packaging and exposure control.
Cracked tack welds	☐ Continuous	☒ Periodic	Verify that welding does not occur over cracked tack welds.
Environmental conditions	☐ Continuous	☒ Periodic	Verify win speed is within limits as well as precipitation and temperature.
WPSfollowed	☐ Continuous	☒ Periodic	Verify items such as settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position.
Welding techniques	☐ Continuous	☒ Periodic	Verify interpass and final cleaning, each pass is within profile limitations, and quality of each pass.

AFTER WELDING (TABLE N5.4-3, AISC 360-16):			
Welds cleaned	☐ Continuous	☒ Periodic	Verify that welds have been propyl cleaned.
Size, length, and location of welds	☐ Continuous	☒ Periodic	
Welds meet visual acceptance criteria	☐ Continuous	☒ Periodic	
Arcstrikes	☐ Continuous	☒ Periodic	
k-area	☐ Continuous	☒ Periodic	
Backing & weld tabs removed	☐ Continuous	☒ Periodic	
Repair activities	☐ Continuous	☒ Periodic	
Document acceptance or rejection of welded joint/member	☐ Continuous	☒ Periodic	
NONDESTRUCTIVE TESTING (SECTION N5.5, AISC 360-16):			
CJ P welds (Risk Cat. II)	☐ Continuous	☒ Periodic	Ultrasonic testing shall be performed on 10% of CJP groove welds in butt, T- and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds tested have unacceptable defects.
CJP welds (Risk Cat. III or IV)	☒ Continuous	☐ Periodic	***A reduction in the rate of ultrasonic testing is allowed per Section N5.5e.***
PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-16): ➢ *Not required if only snug-tight joints are specified [per Section N5.6(1) of AISC 360-16].*			
Certifications of fasteners	☐ Continuous	☒ Periodic	
Fasteners marked	☐ Continuous	☒ Periodic	Verify that fasteners have been marked in accordance with ASTM requirements.
Proper fasteners for joint	☐ Continuous	☒ Periodic	Verify grade, type, and bolt lengthif threads are excluded from the shear plane.
Proper bolting procedure	☐ Continuous	☒ Periodic	Verify proper procedure is used for the joint detail.
Connecting elements	☐ Continuous	☒ Periodic	Verify appropriate faying surface condition and hole preparation, if specified, meet requirements.
Pre-installation verification testing	☐ Continuous	☒ Periodic	Observe and document verification testing by installation personnel for fastener assemblies and methods used.
Proper storage	☐ Continuous	☒ Periodic	Verify proper storage of bolts, nuts, washers, and other fastener components.
DURING BOLTING (TABLE N5.6-2, AISC 360-16): ➢ *Not required if only snug-tight joints are specified [per Section N5.6(1) of AISC 360-16].* ➢ *Not required for pretensioned joints using turn-of-the-nut method with match-marking, direct-tension-indicators,or twist-off type tension control method [per Section N5.6(2) of AISC 360-16].*			
Fastener assemblies	☐ Continuous	☒ Periodic	Verify that fastener assemblies are of suitable condition, paced in all holes, and washers are positioned as required.
Snug-tight prior to pretensioning	☐ Continuous	☒ Periodic	Verify that joints are brought to snug-tight condition prior to pretensioning operation.
Fastener component	☐ Continuous	☒ Periodic	Verify that fastener component is not turned by wrench prevented from rotating.
Pretensioned fasteners	☐ Continuous	☒ Periodic	Verify that fasteners are Pretensioned in accordance with RCSC Specification, progressing systematically from the most rigid point toward the free edges.

AFTER BOLTING (TABLE N5.6-3, AISC 360-16):			
Document acceptance or rejection of bolted connections	☐ Continuous	☒ Periodic	
OTHER STEEL INSPECTIONS (SECTION N5.7, AISC 360-16; Tables J8-1 & J10-1, AISC 341-16):			
Structural steel details	☐ Continuous	☒ Periodic	All fabricated steel or steel frames shall be inspected to verify compliance with the details shown in the construction documents, such as braces, stiffeners, member locations, and proper application of joint details at each connection.
Anchor rods and other embedments supporting structural steel	☐ Continuous	☒ Periodic	Shall be on the premises during the placement of anchor rods and other embedments supporting structural steel for compliance with construction documents. Verify the diameter, grade, type, and length of the anchor rod or embedded item, and the extent or depth of embedment prior to placement of concrete.

Special Inspectors Shall:

Be approved by the Building Official prior to performing any duties;
Provide proof of licensure as a special inspector by the State where the project is located for each type of inspection;
Inspection reports are to meet the requirements of IBC 1704.2.4 and State or Municipality standards;
Inspection reports are to be submitted to the code consultant, architect, Engineer of Record, and the State or local Building Official within 48 hours of performing inspections;
A final inspection report shall be submitted following completion of the project documenting the types of special inspections performed and a statement indicating that the structure is in compliance with the approved construction documents and applicable codes (see IBC 1704.2.4).

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Container Anchorage
4300 Capital Avenue, West Sacramento, CA 95691.

Owner:
CITY OF WEST SACRAMENTO

Stamp:

4-7-26
REGISTERED PROFESSIONAL ENGINEER
ABHIL AYYAPPAN PILLAI
NO. C91002
CIVIL
STATE OF CALIFORNIA
Exp: 12/31/26

Revision Table

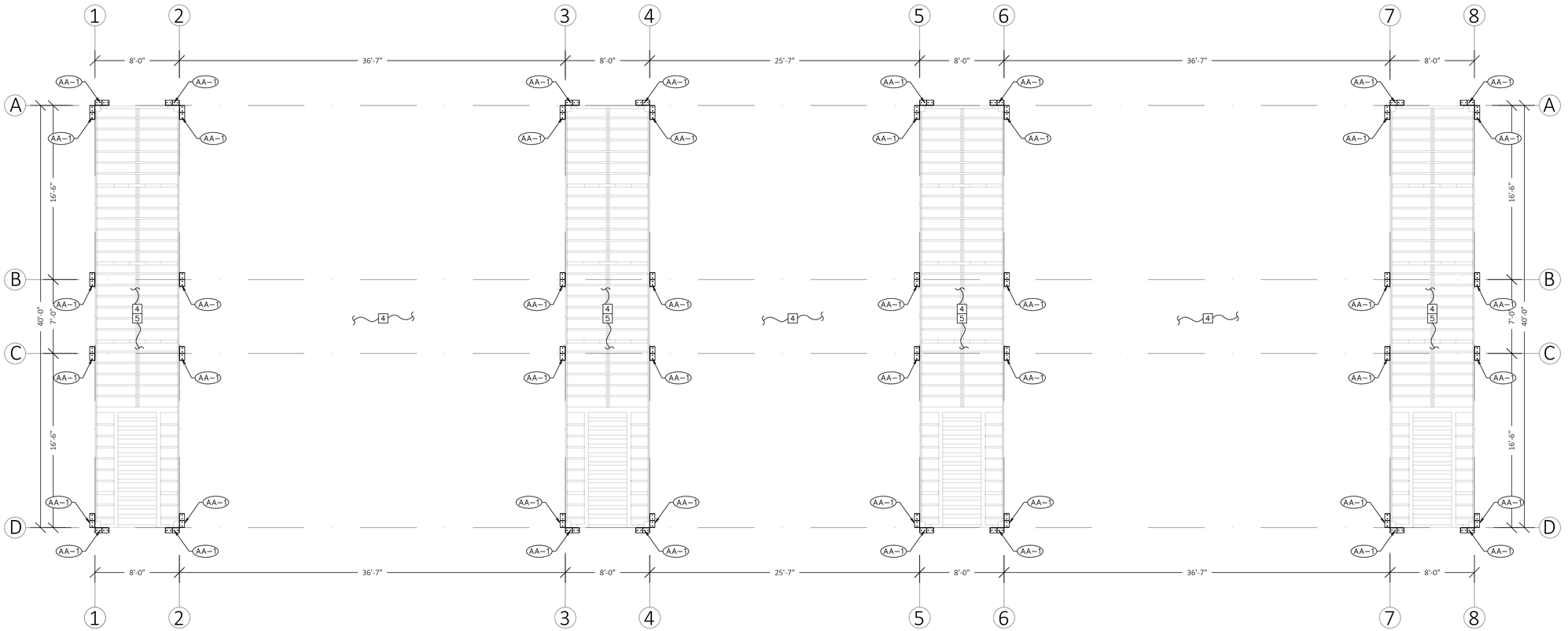
MARK	DESCRIPTION	DATE

DRAWN BY: AYPN
DESIGNED BY: ISL
CHECKED BY: N.T.
Date: March-17-2026

Project #:
Haynes Studio Arch. 225-001

Title:
SPECIAL INSPECTION NOTES

Page:
S1.1



1 ANCHOR LAYOUT PLAN
Approximate Scale: 3/16" = 1'-0"

ANHOR CALLOUT	ANCHOR SPECIFICATION	CONNECTING MEMBER	NOTES
AA-1	(4)-BOLTHOLD SP58 ASPHALT ANCHORS, TYP.	L-ANGLE WITH STIFFENERS AS BASE PLATE & CONTAINER BOTTOM RAIL	SEE 3/S2.1.

2 ANCHOR BOLT SCHEDULE
Scale: N/A

LEGEND:

AA-n INDICATES ANCHOR BOLT MARK, REFER TO ANCHOR BOLT SCHEDULE.

n INDICATES SHEET NOTES.

n NUMERICAL VALUE, 1, 2, 3 ETC.

SHEET NOTES:

1. REFER TO S1 FOR STRUCTURAL GENERAL NOTES AND TO OTHER SHEETS FOR DETAILS.
2. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS. [[[DIMENSIONS PROVIDED MAY BE THE NOMINAL DIMENSIONS OF THE CONTAINER. CONTRACTOR/FABRICATOR, OWNER, AND/OR ARCHITECT TO VERIFY ACTUAL DIMENSION BEFORE CONSTRUCTION]]].
3. BOLTHOLD ASPHALT ANCHORS SHALL BE INSTALLED AS PER MANUFACTURER PROCEDURE
4. 18" DEEP - CEMENT TREATED ASPHALT FOUNDATION BY OTHERS.
5. SCOPE OF SERVICE BY PSE DOES NOT INCLUDE CONTAINER ENGINEERING. AND THE FRAMING DETAILS IN THE CONTAINER WAS NOT SHOWN HERE. REFER OTHER ARCH & STRUCTURAL PLANS FROM OWNER.

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Owner:
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Stamp:
4-7-26
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NABIL TAMI
NO. C60602
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STATE OF CALIFORNIA
Exp: 12/31/26

Revision Table

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DESIGNED BY: ISL
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Title:
ANCHOR LAYOUT PLAN

Page:
S2

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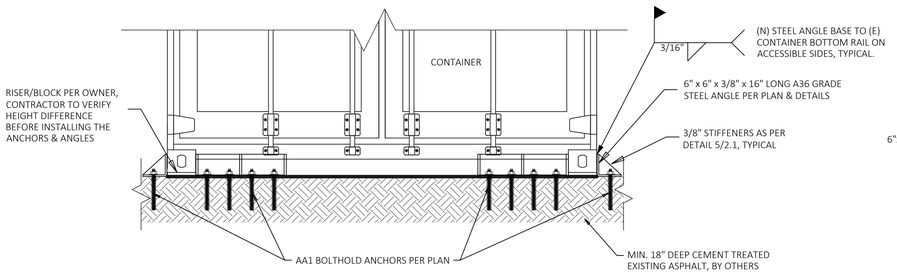
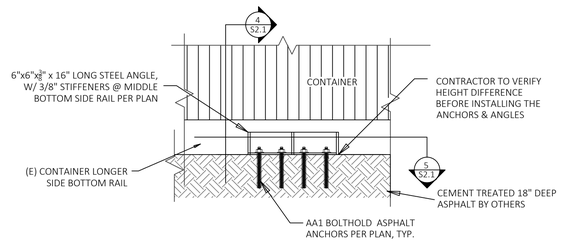
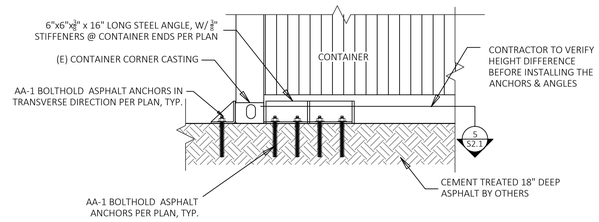


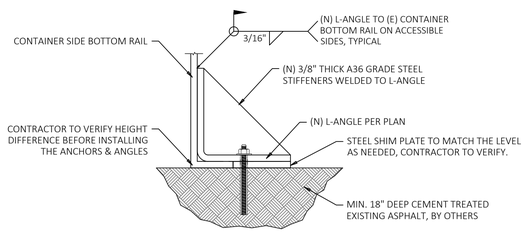
1 CONTAINER BASE ATTACHMENT DETAIL
NTS



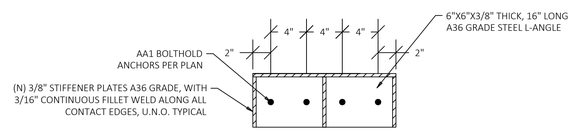
2 CONTAINER TO ANCHORS AT MIDDLE
NTS



3 CONTAINERS TO ANCHORS AT CORNERS
NTS



4 ANCHOR - SECTION, TYPICAL
Scale: NTS



5 ANCHOR - PLAN
Scale: NTS

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Revision Table

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DRAWN BY: AYPN
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Date: March-17-2026

Project #:
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Title:
TYPICAL FRAMING DETAILS

Page:
S2.1

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