

Appendix C

Alternatives Development Technical Memorandum

Shared Use Path Evaluation for Ashley River Bridge



Alternatives Development

1 Introduction

The South Carolina Department of Transportation (SCDOT), in cooperation with the Federal Highway Administration (FHWA), is preparing alternatives for the proposed I-526 Lowcountry Corridor WEST Project (I-526 LCC WEST). This Alternatives Development Technical Memorandum is being prepared according to the provisions of the National Environmental Policy Act (NEPA) and corresponding regulations and guidelines of the FHWA, the lead federal agency (23 Code of Federal Regulations [CFR] 771 and 40 CFR 1500–1508) as well as the requirements of SCDOT, the project sponsor and lead state agency.

The purpose of this technical report is to clearly convey the alternatives development and screening process for the proposed I-526 LCC WEST Environmental Impact Statement (EIS). The boundaries of the study area, shown in Figure 1.1, generally encompass the section of I-26 north and south of the I-26/I-526 interchange and along the I-526 mainline from Paul Cantrell Boulevard to Virginia Avenue. The I-526 LCC WEST project also proposes improvements/ changes to five interchanges along I-526; the I-526 at Paul Cantrell Boulevard interchange; the I-26/I-526 system-to-system interchange; the I-526 at Rivers Avenue interchange; the I-526 at N Rhett Avenue interchange and the I-526 at Virginia Avenue interchange.

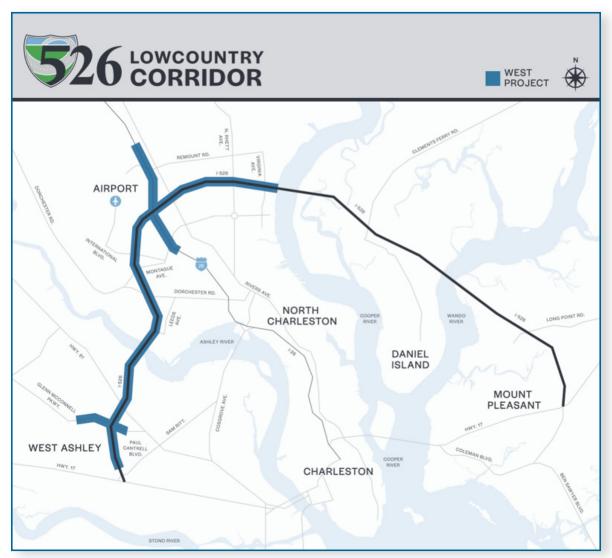


Figure 1.1 I-526 LCC WEST Project Study Area



To address the existing and future congestion and operational issues of the I-526 corridor in Charleston County, SCDOT commissioned a study to develop a long-range plan for the corridor. The Corridor Analysis for I-526 Between North Charleston and West Ashley (2013 Corridor Study) was completed in 2013 and is hereby incorporated by reference. The 2013 Corridor Study documents the travel conditions at the I-526/I-26 system interchange and along I-526 between US 17 at Savannah Highway and US 52 at Rivers Avenue. The purpose of the study was to evaluate potential improvement strategies for the corridor in a holistic manner, and not widening alone. Several strategies to reduce future congestion were studied, including travel demand management, modal improvements for both passengers and freight, traffic operations improvements, and capacity improvements. The 2013 Corridor Study was used to develop alternatives for the I-526 LCC WEST project and are discussed in detail in Section 4.1 of this chapter.

The Berkeley-Charleston-Dorchester Council of Governments (BCDCOG), which serves as the Charleston Area Transportation Study Metropolitan Planning Organization (CHATS MPO), developed the Congestion Management Process (CMP) to assess conditions, identify deficiencies, and make recommendations.² The CMP identifies five strategies for the I-526 WEST corridor that were utilized during the alternative development process. These five strategies from the CMP are outlined below as well as references to where more detailed discussions are located in the report.

- Parallel Pedestrian Facilities/Greenways the creation or enhancement of access for pedestrians and bicyclists, refer to Section 4.1.5.
- Education/Enforcement addresses dangerous traffic behaviors and improved safety behaviors, refer to Section 4.1.6.
- Enhanced Operations includes ramp metering, traffic signal prioritization, and other technology-based improvements, refer to Section 4.1.5.
- Bus on Shoulder/Bus Rapid Transit (BRT) creates a corridor for buses that is separated, has signal prioritization, and fewer stops, refer to Section 4.1.6.
- Congestion Pricing/ Tolling High Occupancy Vehicle/Transit lane to reflect the price of improved mobility on congested roads, refer to Section 4.1.4.



2 What are the Steps of the Alternative Analysis?

Step 1: Preliminary Screening of the Range of Alternatives

Alternatives were developed based on the findings of the 2013 Corridor Study, SCDOT goals and priorities, further evaluation of the corridor, and input from the public and agencies. The alternatives are general in nature and are evaluated based on the ability to satisfy the purpose and need of the I-526 LCC WEST project.

Step 2: Identify Preliminary Alternatives

Alternatives that advance from the preliminary screening are considered Preliminary Alternatives and move on to the next screening.

Step 3: Screening of Preliminary Alternatives

The Preliminary Alternatives are then evaluated by the following screening criteria at a qualitative level:

- Acceptable Level of Service (LOS)
- Compatible with Adjacent Interchange
- Geometric Deficiencies Resolved
- Flexibility with Don Holt Bridge Replacement
- Constructability

If a Preliminary Alternative is unable to meet the criteria above, then it is considered not practicable or feasible. The alternatives that meet the screening criteria are identified as Proposed Reasonable Alternatives.

Step 4: Identify Proposed Reasonable Alternatives

The Preliminary Alternatives that meet the purpose & need of the project are carried forward as the Proposed Reasonable Alternatives.

Step 5: Detailed Impact Evaluation of Proposed Reasonable Alternatives

The Proposed Reasonable Alternatives are being evaluated based on the following evaluation criteria:

- Purpose & Need
 - > Traffic
 - AADT
 - V/C Ratio
 - LOS
- Essential Fish Habitat
- Hazardous Materials
- Cultural Resources
- Noise

- Delineated Wetlands
- Relocations
- Environmental Justice
- Threatened & Endangered Species
- Utilities
- Cost
- Section 4(f) & 6(f)
- Reduce/Eliminate Geometric Deficiencies to Improve Safety
- Hurricane Evacuation Route Compatibility

Step 6: Recommended Preferred Alternative

The Proposed Reasonable Alternative that best balances the potential impacts to the human and natural environment will be recommended as the Preferred Alternative.

AADT = Average Annual Daily Traffic

LOS = Level of Service - Way to describe roadway operating conditions based on speed, travel time, maneuverability, delay and safety

V/C Ratio = Volume to Capacity Ratio - Compares roadway demand (volume) with roadway supply (capacity)



Preliminary Screening of the Range of Alternatives

Identify Preliminary Alternatives

Screening of Preliminary Alternatives

Identify Proposed
Reasonable Alternatives

Detailed Impact Evaluation of Proposed Reasonable Alternatives

Recommended Preferred Alternative

Figure 2.1 Alternatives Development Flowchart



3 How Were the Range of Alternatives Developed?

This technical memorandum outlines the alternative development process for the I-526 LCC WEST and describes the methodology that is being used to determine if an alternative satisfies the purpose and need of the project. The purpose and need of the I-526 LCC WEST was developed with input from the public, Community Advisory Council (CAC), as well as state and federal regulatory agencies. The CAC is a group of residents serving as community liasons from the potentially impacted EJ communities. The council meets regularly to provide input on the project, help guide the process to formulate an EJ Community Mitigation Plan and help inform other residents on how they can get involved and have a voice in the project decision-making process. More detailed information regarding the purpose and need can be found in Chapter 2 of the Draft EIS.

NEPA regulations and guidance from FHWA and the Council on Environmental Quality (CEQ) stipulate three primary reasons why an alternative might be determined to be not reasonable and eliminated from further consideration.

- The alternative does not satisfy the purpose of and need for the project
- The alternative is determined to be not practical or feasible from a technical and/or economic standpoint
- The alternative substantially duplicates another alternative

During the development of the range of alternatives for this DEIS, the October 2013 Corridor Study was used in conjunction with the following goals and priorities provided by SCDOT, input from the public, comments from the agencies, and coordination with SCDOT staff.

- Provide congestion relief by improving I-26/I-526 interchange and I-526 mainline operation
- Reduce/eliminate geometric deficiencies to improve safety
- · Financial constraints of available funding

3.1 October 2013 Corridor Study

In 2013, SCDOT completed a study of I-526 in order to produce a long-range plan for the corridor. The 2013 Corridor Study documented travel conditions along an eight-mile section of I-526 between US 17 (Savannah Highway) and US 52 (Rivers Avenue) including the system-to-system interchange between I-526 and I-26. According to the study, increased congestion is forecasted for the I-526 Corridor. The existing route is a four-lane, divided interstate serving as a freeway around the Charleston area connecting West Ashley to Mount Pleasant and is widely used by commuters and various commercial and industrial operations. The I-526 Corridor has been identified as one of the most congested in the state and has been designated as a "Mega Project" in the State Long-Range Interstate Plan, which indicates construction costs exceed multiple years of the state's interstate program funding. A number of the recommendations from the 2013 Corridor Study were programmed in the State Transportation Improvement Program (STIP).



3.1.1 Public Involvement During the 2013 Corridor Study

Public Involvement in the local Charleston community was crucial in developing the alternatives considered in the 2013 Corridor Study. To engage the community, a project steering committee and a project stakeholder committee were developed as well as a project website, surveys and public information meetings.

The Steering Committee consisted of the following agencies: SCDOT, FHWA, Berkeley Charleston Dorchester Council of Governments (BCDCOG), Charleston Area Regional Transportation Authority (CARTA), Charleston County, City of Charleston, City of North Charleston, South Carolina State Ports Authority, and TriCounty Link. Six steering committee meetings were held. Each agency exhibits a shared interest in addressing the transportation issues within the I-526 corridor.

The project stakeholder committee consisted of individuals representing organizations with interest in the I-526 study corridor due to proximity to the corridor and/or the impact the corridor has on their everyday operations. Three stakeholder committee meetings provided valuable insight from everyday users on the existing deficiencies and potential improvements to address deficiencies in the corridor study area. Refer to Chapter 6: Public Involvement for the stakeholder list.

The first public meeting, regarding the 2013 Corridor Study, was held on September 20, 2011 at North Charleston City Hall council chambers. The meeting allowed the public the opportunity to review graphics showing the study area and existing traffic data along the corridor, as well as a video simulation of existing conditions. Attendees were invited to share their thoughts and comments on reducing traffic congestion within the study area on feedback sheets provided at the meeting. An additional public meeting was held on June 3, 2014 in North Charleston. A formal presentation displayed concepts to reduce traffic congestion along the corridor. These corridor improvement strategies included Transportation Demand Management (TDM), Modal, Traffic Operations and Capacity. As with any design or planning project, public participation was influential in shaping the project to the interests and needs of the community. Different ideas, problems, and solutions were identified by collaborating with the public and gathering feedback.

3.1.2 2013 Corridor Study Improvement Strategies

The purpose of the 2013 Corridor Study was to look at all-inclusive improvement strategies for the corridor which have the best benefit for the travelling public and not only widening. Improvement strategies were organized into four categories: 1) Transportation Demand Management (TDM), 2) Modal (transit/freight), 3) Traffic Operations, and 4) Capacity Improvement. TDM improvements consisted of rideshares, employer-

C-D = Collector-Distributor

Roads that connect the mainline of the interstate to frontage roads/ramps

based incentives, flexible work schedules, and public outreach programs. Modal improvements included new and improved transit routes and facilities as well as public/private partnerships. Traffic Operation strategies focused on a series of improvements to geometric deficiencies along the corridor, upgrades to pavement marking and signing, and intelligent transportation system (ITS) implementation. Capacity Improvement options incorporated both the widening of I-526 to a six-lane section from Paul Cantrell Boulevard to Rivers Avenue and interchange improvements to I-26/I-526 as well as improvements to other interchanges, collector-distributor (C-D) systems, braided ramps, and barrier-separated lanes.



To quantify the existing congestion issues along the I-526 corridor and in the Charleston area, a review of existing travel statistics and operations was conducted and existing traffic information was collected along the I-526 study corridor, including: turning movement counts, intersection signal timing, and geometric data. As part of the analyses for the I-526 corridor, actual origin- destination data was needed to accurately model existing vehicle paths for use in the study VISSIM micro-simulation model.

VISSIM:

a microscopic multi-modal traffic flow simulation software package developed by PTV Planung Transport Verkehr AG in Karlsruhe, Germany. The name is derived from "Verkehr in Städten - SIMulationsmodell" (German for "Traffic in cities - simulation model")

The measures of effectiveness from employing the TDM and Modal strategies was based on overall traffic reduction along I-526 and consequent postponement of the need for large-scale improvements. Based on the traffic reduction calculations, TDM and Modal strategies were expected to have a combined potential reduction of 12.6% in total traffic volumes; if this potential reduction was achieved, capacity improvement strategies could be pushed back five to ten years. However, this reduction in congestion is not substantial enough to meet the purpose and need of the I-526 LCC WEST project which still makes larger infrastructure improvements necessary for these strategies to be successfully implemented in the future. The cost of constructing Bus Rapid Transit (BRT), rail-based transit, and new intermodal facilities would exceed \$300 million. Project grouping strategies within the plan recommended widening I-526 and improving the I-26/I-526 interchange in the year 2020.

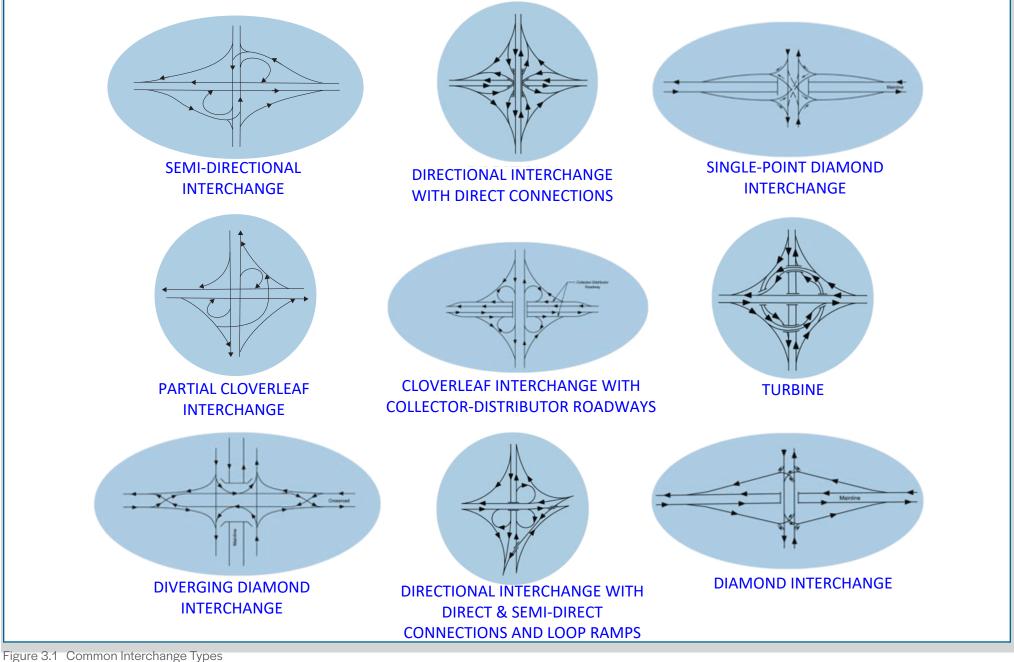
Traffic Operation improvements focused on relieving specific operational concerns within the existing network were also considered for the I-526 corridor and adjacent arterial-street networks. Geometric traffic operation improvements, pavement marking improvements, signing improvements, ITS technologies, and managed lanes strategies were analyzed. Recommended Traffic Operations strategies are discussed in detail in Chapter 7 of the Corridor Analysis for I-526 Between North Charleston and West Ashley.

Capacity Improvement strategies considered for this study include widening the I-526 corridor to a six-lane section, improving C-D systems, interchange improvement alternates, braided entrance/exit ramps, barrier separated lanes, alternate routes, and arterial widening. The 2013 Corridor Study analyses also considered nine interchange operations along the I-526 study corridor and the operation of three adjacent interchanges along I-26. To capture the area of influence for each interchange, the study area was extended to include crossing roads as necessary.

As part of the review of capacity improvements to the I-526 study corridor, several improvement scenarios were considered at the interchanges to address existing and projected congestion issues. These included 1) I-526 & US 17/Sam Rittenberg Boulevard; 2) I-526 at Paul Cantrell Boulevard; 3) Paul Cantrell Boulevard at Magwood Drive; 4) I-526 at Leeds Avenue; 5) I-526 at Dorchester Road/Paramount Drive; 6) I-526 at Montague Avenue/International Boulevard; 7) I-26/I-526 System-to-System; and 8) I-526 at Rivers Avenue. Based on the findings of the 2013 Corridor Study, five of the eight interchanges were recommended for evaluation based on SCDOT goals and priorities previously discussed.

Potential interchange alternatives were developed based on the common interchange types shown in Figure 3.1.







I-526 & Paul Cantrell Boulevard (2013 Corridor Study)

The existing I-526 & Paul Cantrell Boulevard interchange is a partial cloverleaf with loops serving the eastbound and westbound movements from I-526 to Paul Cantrell Boulevard. Partial improvements were identified to address substantial peak hour traffic to/from West Ashley to the west of Paul Cantrell Boulevard. Refer to Figures 3.2 - 3.5.

Alternate 1: Diverging Diamond Interchange (DDI)

- Allows traffic to cross to the opposote side of the roadway to allow for two-phase signalization
- Reduces left-turn conflicts



Figure 3.2 2013 Corridor Study I-526 & Paul Cantrell Boulevard Alternate 1

Alternate 2: Modified Single-Point Urban Interchange (SPUI) with Directional Ramp to I-526 Eastbound

- Directional ramp to I-526 eastbound
- Allows for accommodation of peak hour left turn movements
- Requires bridge structures over Paul Cantrell Boulevard to be replaced and columns from the median to be removed



Figure 3.3 2013 Corridor Study I-526 & Paul Cantrell Boulevard Alternate 2



Alternate 3: Two-Lane Paul Cantrell Eastbound to I-526 Eastbound Loop

 Replaces dual left turns from Paul Cantrell Boulevard eastbound to I-526 eastbound with a two-lane loop ramp



Figure 3.4 2013 Corridor Study I-526 & Paul Cantrell Boulevard Alternate 3

Alternate 4: Triple Left-Turn Lanes

- Maintains existing interchange configuration
- Provides triple left-turn movement from eastbound Paul Cantrell Boulevard to I-526 eastbound
- Requires extension of existing acceleration lanes from I-526 eastbound to the bridge over the Ashley River



Figure 3.5 2013 Corridor Study I-526 & Paul Cantrell Boulevard Alternate 4

The major deficiencies with the I-526 to Paul Cantrell Boulevard interchange include high volume of traffic for eastbound and westbound movements during peak hours. Alternate 4, Triple Left-Turn Lanes, addresses the operations in the AM peak hour. To improve the afternoon peak hour deficiencies, the consideration of improvements to the Paul Cantrell Boulevard at Magwood Drive intersection were identified.



Paul Cantrell Boulevard & Magwood Drive (2013 Corridor Study)

The existing intersection consists of a signalized intersection with a six-lane Paul Cantrell Boulevard cross section with exclusive right-turn lanes and double left-turn lanes from Paul Cantrell Boulevard westbound to Magwood Drive southbound. To improve capacity of the intersection, it was determined grade-separation improvements would be required.

Alternate 1: Tight Urban Diamond

 Replaces existing at-grade intersection with a gradeseparated, compressed diamond interchange



Figure 3.6 2013 Corridor Study Paul Cantrell Boulevard & Magwood Drive Alternate 1

At-grade intersection = where a local road intersects a highway at the same elevation

Grade separation = a method of aligning a junction of two or more roadways at different elevations so that
they will not disrupt the traffic flow on other transit routes when they cross each other

Based upon the review criteria, this alternate is recommended to mitigate the congestion at the Paul Cantrell Boulevard & Magwood Drive Intersection.



I-26/I-526 System-to-System (2013 Corridor Study)

Improvements to the I-26/I-526 system-to-system interchange were also assessed. The I-26/I-526 system-to-system interchange currently consists of a combination of directional and loop ramps providing for all movements from one interstate to another. There is a two-lane directional fly-over ramp from I-526 eastbound to I-26 westbound, loop ramps in the other three quadrants of the interchange, and a C-D road in the two western quadrants of the interchange. During the 2013 Corridor Study, a total of fifteen deficiencies were identified for the I-26/I-526 system-to-system interchange, and seven improvement alternates were developed to address these deficiencies. Using the VISSIM analysis program, the alternates were modeled to simulate potential problem areas with the respective alternates. The seven alternates were then modified for the final concept designs and reviewed to determine a recommended concept for the I-26/I-526 interchange. Figures 3.7 - 3.13 give a description and graphic of the seven alternates developed. This review considered rankings against the following criteria: number of deficiencies addressed, utility impacts, right-of-way impacts, environmental impacts, compatibility with widening to the I-526 median, and cost.

Alternate 1: Semi-Directional Ramps at I-26/I-526 with DDI at Rivers Avenue

- Replaces low speed loops with semi-directional ramps
- Extends I-26 eastbound C-D system from Remount Road to the I-26 & I-526 interchanges
- New westbound I-26 C-D system from Montague Avenue to existing C-D system at Remount Road
- New C-D systems on north and south sides of I-526 between I-26 and Rivers Avenue
- New westbound C-D continuing through International Boulevard

Alt 1

DDI = Diverging Diamond Interchange, C-D = Collector-Distributor

Figure 3.7 2013 Corridor Study I-26/I-526 Alternate 1



Alternate 2: Semi-Directional Ramps at I-26/I-526 with PARCLO at Rivers Avenue

• Similar to Alternate 1 with improved ramp geometry at 1-526 & Rivers Avenue



PARCLO = Partial Cloverleaf Interchange

Figure 3.8 2013 Corridor Study I-26/I-526 Alternate 2

Alternate 3: Semi-Directional Ramps at I-26/I-526 with Relocated PARCLO at Rivers Avenue

• Similar to Alternate 2 with reconfigured Rivers Avenue ramps



PARCLO = Partial Cloverleaf Interchange

Figure 3.9 2013 Corridor Study I-26/I-526 Alternate 3



Alternate 4: Semi-Directional Ramps at I-26/I-526 with one loop retained and ramps removed on west side of Rivers Avenue

- Eliminates westbound I-526 access from Rivers Avenue
- Westbound I-26 to westbound I-526 loop ramp is retained



Figure 3.10 2013 Corridor Study I-26/I-526 Alternate 4

Alternate 5: Directional Ramps at I-26/I-526 with one loop retained with Relocated PARCLO and braided ramps at Rivers Avenue

- Eastbound I-26 directional ramp to the median of eastbound I-526
- Westbound I-26 directional ramp to the outside of westbound I-526



PARCLO = Partial Cloverleaf Interchange

Figure 3.11 2013 Corridor Study I-26/I-526 Alternate 5



Alternate 6: Semi-Directional Flyover Ramps at I-26/I-526 with Relocated PARCLO and braided ramps at Rivers Avenue

- Replaces all loops with semi-directional flyover ramps
- Rivers Avenue ramps similar to Alternate 3



PARCLO = Partial Cloverleaf Interchange

Figure 3.12 2013 Corridor Study I-26/I-526 Alternate 6

Alternate 7: Semi-Directional Turbine Interchange at I-26/I-526 with PARCLO and braided ramps at Rivers Avenue

- Replaces interchange with semi-directional turbine interchange
- Differs from other directional ramp alternates by separating movements and eliminating weaves in interchange



PARCLO = Partial Cloverleaf Interchange

Figure 3.13 2013 Corridor Study I-26/I-526 Alternate 7

Based upon the review criteria, it was determined that Alternate 7 would best mitigate the congestion deficiencies at the I-26/I-526 interchange. Alternate 7 replaces the existing interchange with a semi-directional turbine interchange as shown in Figure 2.3. The key component of this design is that there is no weaving within the interchange. All weaving occurs on the lower-speed C-D systems. The traffic from I-26 westbound to I-526 is placed on a C-D system beginning at Montague Avenue. There are C-D systems on both sides of I-526 between the I-26 and Rivers Avenue interchanges. The traffic from I-26 to I-526 westbound is on a braided- ramp system and the existing C-D systems to and from Remount Road are extended to the new interchange at I-26 & I-526.



I-526 & Rivers Avenue (2013 Corridor Study)

The I-526 & Rivers Avenue interchange is located close to the east of the I-26 & I-526 interchange and is a partial cloverleaf interchange with loops in the southeast and northwest quadrants. Improvements were identified to be consistent with the improvements to the I-26 & I-526 interchange.

Alternate 1: Diverging Diamond Interchange (DDI)

 Replaces existing loops to accommodate considerable left-turn movements from Rivers Avenue



Figure 3.14 2013 Corridor Study I-526 & Rivers Avenue Alternate 1

Alternate 2: Partial Cloverleaf

- Provides an additional loop in the northwest quadrant of the interchange
- Provides more length from traffic to weave between Rivers Avenue and I-26 interchange



Figure 3.15 2013 Corridor Study I-526 & Rivers Avenue Alternate 2



Alternate 3: Partial Interchange

- Provides access to/from I-526 toward Mt. Pleasant only
- Removes movements to/from west on I-526
- Remount Road or Montague Avenue would be used to accommodate traffic to/from west towards I-26



Figure 3.16 2013 Corridor Study I-526 & Rivers Avenue Alternate 3

Alternate 4: Maintain Existing Configuration

- Maintain existing interchange form
- Relocates ramps to accommodate proposed I-526 C-D roads



Figure 3.17 2013 Corridor Study I-526 & Rivers Avenue Alternate 4

Based upon the review criteria, it was determined Alternate 4 is most compatible with the adjacent I-26 & I-526 interchange and would best mitigate the congestion deficiencies present at the I-526 & Rivers Avenue interchange.

Based on the analysis presented in this report, the proposed improvements along the I-526 corridor reduce congestion and improve traffic operations in the area. In addition, the conclusions of the 2013 Corridor Study were based on a 2035 design year. Based on the analysis, travel times in both the eastbound and westbound directions of I-526 increase considerably in the 2035 design year.

The findings of the 2013 Corridor Study were used as a starting point when developing the Range of Alternatives for the I-526 LCC WEST DEIS.



4 What are the Range of Alternatives?

Based on the 2013 Corridor Study, a wide range of alternatives were developed and analyzed to see if they met the primary purpose and need of the project. In an effort to address the existing and future congestion and operational issues identified for the corridor, a range of alternatives were developed to include the following:

- No-Build
- · Improvements to existing local facilities
 - > East Montague Avenue
 - > Remount Road
- · New location alternatives
 - > US 78 to Virginia Avenue
 - > Ashley Phosphate Road to Virginia Avenue
 - > Bees Ferry Road to Dorchester Road
- Managed Lanes
- Transportation System Management (TSM) and Transportation Demand Management (TDM) Strategies
- Mass Transit
- Existing Corridor Improvements

4.1 Preliminary Screening of the Range of Alternatives

The Range of Alternatives are evaluated using the purpose and need of the project. Table 4.1 summarizes the preliminary screening and the details are included in Sections 4.1.1-4.1.7.



Table 4.1 Preliminary Screening of the Range of Alternatives

			nts to Existing Facilities		New Location					F. C. C.	
	No Build	East Montague Ave	Remount Rd	US 78 to Virginia Ave	Ashley Phosphate Rd to Virginia Ave	Bees Ferry Rd to Dorchester Rd	Managed Lanes*	TSM/ TDM*	Mass Transit*	Existing Corridor Improvements	
Satisfies I-526 LCC WEST Purpose & Need	-	×	×	×		×	×	×	×		
Carried Forward as Preliminary Alternatives	⊘	8	×	8		×	8	8	8		

^{*} Eliminated as stand-alone alternatives



4.1.1 No-Build/No Action

Under the provisions of NEPA, the effects of not implementing the proposed action must also be considered. The No-Build Alternative provides a baseline for comparing potential environmental impacts with the other reasonable alternatives. Analysis of the No-Build Alternative must discuss the existing conditions as well as what is reasonably expected to occur in the foreseeable future if the proposed action is not constructed. For example, the No-Build Alternative must include nearby transportation projects that can reasonably be expected to be in place for the design year. Reasonably foreseeable projects typically come from the fiscally constrained list of projects in the State Transportation Improvement Program (STIP) and in the local metropolitan planning organizations long-range plans, as well as other programming documents from the municipalities in which the project occurs. While the No-Build Alternative does not meet the purpose and need of the project, it is carried forward as it provides a foundation for comparing the benefits and environmental impacts of the other alternatives.

4.1.2 Improvements to Existing Local Facilities

SCDOT initiated an evaluation of alternate routes that satisfy the purpose and need of the I-526 LCC WEST project. The study evaluated the enhancement of existing roadway facilities along with the creation of new alignment corridors. The enhancements include the development of alternate alignments which could be used to decrease interstate traffic volumes. The corridors listed do not include any options which provide an alternate route between I-26 and the Cooper River (refer to Figure 4.1).



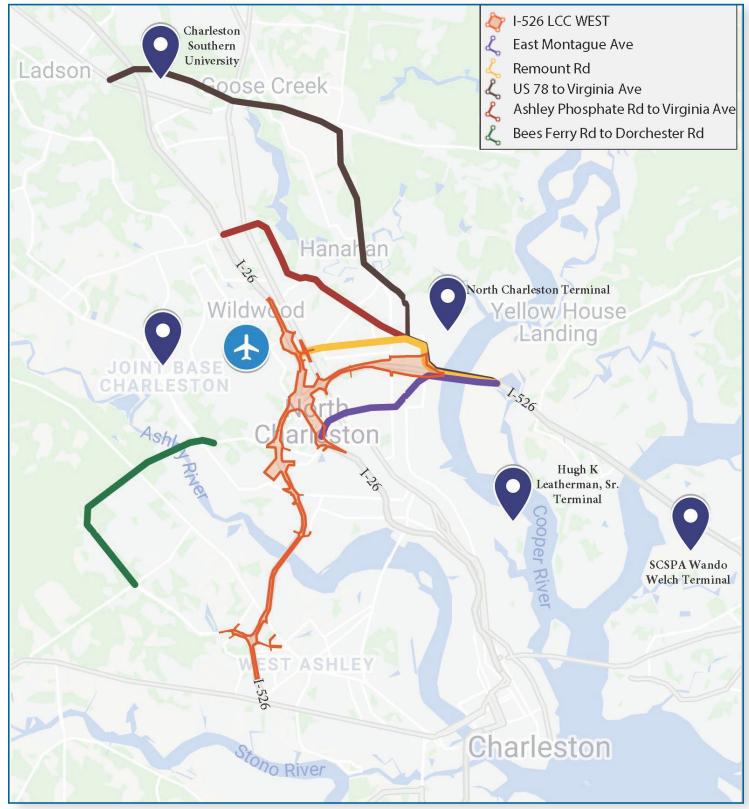


Figure 4.1 Improvements to Existing Local Facilities and New Location Alternatives



Improvements to East Montague Avenue

This existing route runs nearly parallel to I-526 from I-26 to Virginia Avenue, and serves as a minor arterial facility connecting I-26 to the Park Circle area. East Montague Avenue, known as the old "Main Street" weaves through two of the city's most historic neighborhoods. Liberty Hill stands as the oldest surviving neighborhood within North Charleston, while Park Circle represents one of the earliest concepts of a garden community in the United States. Other features along the route include North Charleston High School, North Charleston United Methodist Church, Royal Baptist Family Life and Banquet Center, and the Felix Pinckney Community Center. Residential development dominates along the western segment of the route from North Boulevard to Rivers Avenue, while commercial development is prevalent on the eastern segment from Jenkins Avenue to Virginia Avenue.

Traffic modeling, including the proposed improvements to the existing East Montague Avenue, indicate a 10-24 percent decrease in traffic volumes along the existing I-526 mainline. Although the 24 percent reduction may be substantial enough to meet the purpose and need if it were along the entire corridor, this decrease in traffic volume would only be applicable to approximately 0.5 miles along I-526 from I-26 to Rivers Avenue. As a result, this reduction in congestion would not be substantial enough to meet the purpose and need of the I-526 LCC WEST project, as I-526 would still operate at a level of service (LOS) E/F. Therefore, the improvements to existing East Montague Avenue were eliminated as a potential alternative because it does not meet the purpose and need for the I-526 LCC WEST project.

Improvements to Remount Road

This existing route serves the area just north of the I-526 corridor and connects I-26 to the North Charleston Terminal (NCT) and its associated facilities along the Cooper River. The NCT sits on over 200 acres and handles nearly one-fourth of the Port of Charleston's total container volume, necessitating a large volume of truck traffic along the roadway. Other features along this route include Matilda Dunston Elementary School, Remount Baptist Church, Aldersgate United Methodist Church, Revive Charleston, First Southern Methodist Church, Victory Missionary Baptist Church, and MWV/Kapstone Park. Residential development exists mainly along the south side of the road from Shelton Street to N Rhett Avenue, and commercial development runs along the entire length of the corridor.

Traffic modeling including the proposed improvements to the existing Remount Road indicate a 1-12 percent decrease in traffic volumes along the existing I-526 mainline; this reduction in congestion would not be substantial enough to meet the purpose and need of the I-526 LCC WEST project, as I-526 would still operate at a level of service (LOS) E/F. Therefore, the improvements to existing Remount Road were eliminated as a potential alternative because it does not meet the purpose and need for the I-526 LCC WEST project.



4.1.3 New Location Alternatives

During the process of assessing feasible alternate routes, additional route development is restricted by several regional landmarks and environmental features. Impacts to these points of interest are detrimental to the community as a whole; and any alternate route containing such impacts are deemed unreasonable for improving congestion along I-526.

Charleston International Airport is South Carolina's largest airport. It served nearly 4.5 million travelers in 2018 and is operated under a joint-use agreement with Joint Base Charleston. The combined airport area of civilian facilities and the Charleston Air Force Base extends over 2,000 acres, covering most of the land to the west of the I-26/I-526 interchange between I-26/I-526 and the Ashley River, and extending north to Ashley Phosphate Road. The location and size of the airport prevent alternate route development to the west of I-26 for approximately four miles to the north of the Airport.

The Cooper River defines the easternmost boundary of the North Charleston city limits and remains a vital commercial channel for the region. Currently, the Don Holt Bridge and the Arthur Ravenel Jr. Bridge are the only two structures that provide vehicular access across the river. Any alternate route which involves the construction of a third roadway bridge increases the cost of the project drastically. In addition, many areas east of N Rhett Avenue are comprised of wetlands related to the Cooper River branch that connects to the Goose Creek Reservoir. Alternate routes constructed in this vicinity result in substantial impacts to the surrounding natural environment.

The Goose Creek Reservoir is situated just east of the Rivers Avenue business district near Hanahan and serves as the primary water supply storage for much of the Charleston region. The 600-acre reservoir area is also home to a wide variety of animal species and has become a popular destination for fishers and paddleboaters alike. The reservoir stretches from just northeast of Murray Drive to Goose Creek Road, impeding any new alternate alignment between Rivers Avenue and N Rhett Avenue.

Francis Marion National Forest/Bonneau Ferry Wildlife Management Area prevents new alternate four-lane routes north of I-526 which connect I-26 to US 17. Wildlife management is overseen by the South Carolina Department of Natural Resources.

US 78 to Virginia Avenue

The proposed new alignment is established to connect key points along I-26 and I-526 in the vicinity of the existing Cooper River crossing at the Don Holt Bridge. The US 78 to Virginia Avenue route utilizes portions of Red Bank Road and N Rhett Avenue to create a four-lane, controlled access facility with new interchanges. A new location roadway section running north of Charleston Southern University and North Charleston Wannamaker County Park connects US 78 west of I-26 to the Red Bank Road corridor. Upgrading the existing roadway impacts commercial and residential development along Red Bank Road and potentially impacts the North Charleston Terminal facilities.

Traffic modeling, including the proposed new alignment, indicates a 2 to 10 percent decrease in traffic volumes along the existing I-526 mainline; this reduction in congestion is not substantial enough to meet the purpose and need of the I-526 LCC WEST project, as I-526 still operates at a level of service (LOS) E/F. Therefore, the US 78 to Virginia Avenue route is eliminated as a potential alternative because it does not meet the purpose and need for the I-526 LCC WEST project.



Ashley Phosphate Road to Virginia Avenue

This proposed new alignment is a four-lane, controlled access facility which follows a short section of Ashley Phosphate Road east of I-26, then connects to Railroad Avenue and heads south before traversing on new location to run parallel to Murray Drive along the existing utility easement. A variety of features are impacted by this proposed route, including but not limited to commercial and residential development along Ashley Phosphate Road and Murray Drive, Hanahan Elementary School and Trident Technical College, and the City of Hanahan Recreation Center and its associated park areas. In addition, major utility relocations are required.

Traffic modeling, based on the proposed new alignment, indicates a 7 to 15 percent decrease in traffic volumes along the existing I-526 mainline; this reduction in congestion does not meet the purpose and need of the I-526 LCC WEST project, as I-526 still operates at a level of service (LOS) E/F. Therefore, the Ashley Phosphate Road to Virginia Avenue route is eliminated as a potential alternative because it does not meet the purpose and need for the I-526 LCC WEST project.

Bees Ferry Road to Dorchester Road

A third new alignment route is being evaluated to the west of I-26 which establishes a new connector across the Ashley River. The proposed roadway is four lanes with controlled access but does not include an interchange at Ashley River Road. The proposed Bees Ferry Road to Dorchester Road alignment requires a new bridge over the Ashley River that could potentially impact the existing Shadowmoss Plantation residential development.

Incorporating this alignment into traffic modeling results in an estimated 4 percent decrease in traffic volume along I-526 near the Ashley River, while I-526 volumes to the east of I-26 have negligible reduction. Therefore, the proposed connector is also failing to meet the purpose and need of the I-526 LCC WEST project, as I-526 remains at a level of service (LOS) F. Therefore, the Bees Ferry Road to Dorchester Road new alignment route is eliminated as a potential alternative because it does not meet the purpose and need for the I-526 LCC WEST project.

4.1.4 Managed Lanes

Managed lanes is one of the Transportation Demand Management (TDM) strategies evaluated. The 2013 Corridor Study included an evaluation of managed lanes in the I-526 corridor and predicted the study area is not a long enough corridor to realize the potential of high-occupancy vehicle (HOV) or high-occupancy toll (HOT) lanes, and that a more regional plan including the I-26 corridor should be examined to increase the feasibility of managed lanes.

More recent managed lane studies, (included in the current I-26 Corridor Study) concluded that managed lanes may be feasible on I-526 if they extended westward on I-26 at least as far as the US 52 Connector near Ashley Phosphate Road. A regional managed lane study was conducted as part of the I-26 Corridor Study that included all of I-526 and I-26 from US 17 to Exit 187-Ridgeville. A suggested improvement from the plan is the implementation of HOT managed lanes from Exit 199 (US 17 Alt – Summerville) to I-26 Terminus at US 17 and along I-526 the entire section. There are currently no programmed improvements to I-26 between I-526 and the US 52 Connector; therefore, managed lanes cannot be justified based on a committed improvement ensuring their functionality upon completion of the I-526 LCC WEST Project. Whereas managed lanes alone do not meet the project's purpose and need and therefore not considered a viable stand-alone alternative, the 12-foot shoulders included in the proposed project accommodate future managed lane options on I-26 or potential bus-on-shoulder transfers between the two interstates.



4.1.5 Transportation System Management/Transportation Demand Management

The Transportation System Management/Transportation Demand Management (TSM/TDM) strategies evaluated in the 2013 Corridor Study are listed in Table 4.2. A total reduction of 5.2% of total overall traffic can be expected with the implementation of all 10 of the TDM programs evaluated in the 2013 Corridor Study. TSM includes lower cost improvements to improve efficiency and safety. A few examples of TSM consist of improving signal timing, adding high occupancy vehicle lanes as well as adding turn lanes. TDM focuses on lessening travel demand by reducing the number of vehicle trips and vehicle miles traveled on a roadway or redistributing this demand in space or time to decrease system deficiency. TDM regional strategies may include strategies such as encouraging drivers to carpool or ride the bus, and/or encouraging employers to allow non-standard work hours or telecommuting options for employees.

The following documents were also reviewed to determine if additional TSM/TDM studies provide better estimates of travel demand reduction. These studies did not reference reductions in travel demand related to single occupancy vehicles.

- The Public Transportation element of the Charleston Area Transportation Study (CHATS) Long-Range Transportation Plan (LRTP), January 2019
- Appendix D of the CHATS LRTP, Transit Needs Assessment, January 2019
- Travel Market Analysis element of the BCDCOG Regional Transit Framework Plan, March 2018
- Corridor Alternatives Evaluation & Recommendations element of the BCDCOG Regional Transit Framework Plan, March 2018
- Congestion Management Process report, BCDCOG, January 2019

According to the US Census Bureau American Community Survey, the percentage of commuters driving alone to work has only reduced by 0.4 percent between 2013 and 2019. The percentage of carpoolers and public transit users also declined by an average of less than one percent. This data indicated an increase in telecommuters, but not substantial enough to reduce congestion given the current and future traffic demand for the corridor. I-526 from Mount Pleasant to Savannah Highway was identified in the Regional Transit Framework Plan as a high capacity transit (HCT) corridor. This plan establishes the needs and makes recommendations based on public and stakeholder input, operations, and available funding. However, the plan does not provide forecasts. Based on the American Community Survey data through 2019, and the document review described above, the TSM/TDM recommendations from the 2013 Corridor Study are still applicable.

As a standalone alternative, TSM and TDM improvements do not adequately improve the corridor and meet the purpose and need to increase capacity and reduce congestion given the current and future level of service (LOS). While TSM/TDM strategies alone do not meet the project's purpose and need and therefore are not being considered a viable stand-alone alternative, TSM/TDM alternatives to shift commuter behavior are being considered as future regional projects.



Table 4.2 Transportation Demand Management Strategies

Strategy	Traffic Reduction Potential
Carpools/Rideshare Matching Vanpools	2.0%
Transit Pass Incentives Financial Incentives	1.5%
Telecommuting Compressed Work Week	0.1%
Work Flex Time Staggered Work Hours	0.5%
Bike/Walk Enhancements	0.1%
Education, Promotion	1.0%
Total Reduction Potential	5.2%

Source: Adapted from *I-526 Corridor Analysis Between North Chalreston and West Ashley*, Table ES3 Note: All strategies with the exception of Bike/Walk Enhancements have been funded by FHWA

4.1.6 Mass Transit

The measure of effectiveness for the proposed transit strategies are based on the potential reduction in traffic along the I-526 corridor. Mass transit options are a growing topic of interest in the Charleston area as evidenced by public desire to include mass transit in the project alternatives. In addition to public desire, FHWA also recommends that mass transit alternatives be considered on proposed highway projects in urbanized areas with populations of over 200,000 people (FHWA Technical Advisory 6640.8A). Specific modal strategies studied for the I-526 corridor in the 2013 Corridor Study are listed in Table 4.3. If implemented as a stand-alone alternative, expanding and/or improving mass transit infrastructure does not meet the purpose and need of the project by increasing capacity adequately or improving operations. The total potential reduction of these improvement strategies is estimated to be 7.4% with the implementation of short-term transit and freight improvements. Additionally, the addition of mass transit does not enhance safety, nor improve freight mobility. Because mass transit does not meet the purpose and need as a standalone alternative, it is not carried forward as an alternative for the I-526 LCC WEST Corridor project.

While mass transit is not carried forward as a reasonable alternative based on its ability to meet the purpose and need on its own, Charleston County and BCDCOG are proactively funding a Bus Rapid Transit (BRT) project which will include a bus within a dedicated lane or right of way. The design work for this project is currently being scoped. The BRT corridor crosses the I-526 corridor within the median of Rivers Avenue. Assumptions have been made about the corridor width for purposes of providing adequate clearances with the I-526 improvement alternatives. The I-526 LCC WEST alternatives are developing an assumed clearance envelope for the BRT corridor where it is expected to pass through the I-526 LCC WEST study area. Infrastructure improvements are needed to support adding additional buses. Continued coordination with Charleston County will be required to fully implement as a successful mass transit system. Table 4.4 shows the travel demand alternatives which have been funded and implemented or are under development.



Table 4.3 Modal Strategies

Strategy	Traffic Reduction Potential
Improve Existing Transit Routes	0.3%
New Transit Routes	1.1%
Improve Connectivity to/from Transit	0.3%
Improve Transit Facilities and Equipment	0.3%
Public/Private Partnerships	0.6%
BRT, Commuter Rail, Light Rail	3.4%
Zoning/Transit Oriented Developments	0.0%
Increase Intermodal Split to Rail	3.5%
Expand Port Operating Hours	0.0%
Construct Near-Terminal Staging Areas	0.2%
Peak-Hour Incentives/Disincentives	0.2%
Truck Routes away from I-526	0.9%
Total Modal Reduction Potential	7.4%

Source: Adapted from *I-526 Corridor Analysis Between North Chalreston and West Ashley*, Table ES4 Note: The BRT, Commuter Rail, Light Rail strategy is being funded by Charleston County

Table 4.4 Travel Demand Alternatives Evaluated & Implemented from 2013 Corridor Study

Strategy	Status
Carpools/Rideshare Matching Vanpools	Funded & Implemented
Telecommuting Compressed Work Week	Funded & Implemented
Work Flex Time Staggered Work Hours	Funded & Implemented
Education, Promotion	Funded & Implemented
Bus Rapid Transit	Project Under Development
Signal Improvements & Re-Timing	Funded & Implemented

4.1.7 Existing Corridor Improvements

Improving the existing I-526 LCC WEST mainline from Virginia Avenue to Paul Cantrell Boulevard is proposed to accommodate the current and future vehicular demands, as well as population and employment increases. This alternative could meet the purpose and need by increasing capacity and thereby reducing congestion. Improving the existing corridor is advanced and multiple options are being developed including two widening alternatives as well as five interchanges along I-526; the I-526 at Paul Cantrell Boulevard interchange; the I-26/I-526 System-to-System interchange; the I-526 at Rivers Avenue interchange; the I-526 at N Rhett Avenue interchange and the I-526 at Virginia Avenue interchange.



5 What are the Preliminary Alternatives?

Based on the screening previously described in Section 4, the range of alternatives are evaluated based on their ability to meet the purpose and need of the I-526 LCC WEST project. The following alternatives are identified as Preliminary Alternatives:

- No-Build
- Existing Corridor Improvements
 - > Mainline Interstate Alternatives
 - 6-lane widening
 - 8-lane widening
 - > Interchange Alternatives
 - I-526 at Paul Cantrell Boulevard
 - I-26/I-526 System-to-System
 - I-526 at Rivers Avenue
 - I-526 at N Rhett Avenue and Virginia Avenue (Due to proximity, these interchanges are combined.)

5.1 Screening of the Preliminary Alternatives

The Preliminary Alternatives are evaluated using the purpose and need of the project and the following criteria at a qualitative level:

- Acceptable Level of Service (LOS)
- · Compatible with Adjacent Interchange
- Geometric Deficiencies Resolved
- Flexibility with Don Holt Bridge Replacement
- Constructability

Table 5.1 summarizes the screening of the Preliminary Alternatives and the details are included in Sections 5.1.1-5.1.7.



Table 5.1 Screening of the Preliminary Alternatives

	No Build	Mainline I-526 at Pa					trell Blvd		Paul Cantrell Blvd at Magwood Dr							I-26/	1-526 Sys	stem-to-S	ystem	I-526 at Rivers Ave		I-526 at N Rhett/Virginia A			ia Ave
		6-lane	8-lane	1	2	3	4	5	1	2	3	4	5	6	7	1	2	3	4	1	2	1	2	3	4
Acceptable LOS	×	8	•	8	•	•	⊘	⊘	•	•	•	•	•	8	8	•	•	•	•	•	•	⊘	•	•	•
Compatible with Adjacent Interchange	_	-	_	×	•	8	•	•	8	•	*	×	•	•	8	•	•	•	•	•	•	•	•	×	×
Geometric Deficiencies Resolved	×	-	_	⊘	⊘	•	•	•	-	-	-	_	-	-	-	⊘	⊘	•	•	8	8	•	•	②	•
Flexibility with Don Holt Bridge Replacement	•	⊘	⊘	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	⊘	•	•	•
Constructability	_	⊘		⊘	8	•	8	⊘	⊘	8	•	•	•	⊘	•	②	⊘	8	8	⊘	•	•	•	8	•
Carried Forward as Reasonable Alternatives	•	8	•	8	8	8	8	•	8	8	8	8	•	8	8	⊘	•	8	8	•	•	•	•	8	×



5.1.1 No-Build

The No-Build is carried forward as a Preliminary Alternative. For additional information on the No-Build alternative, refer to section 4.1.1.

5.1.2 Mainline Interstate (I-526) Alternatives

6-Lane Widening

The 2013 Corridor Study recommended adding one lane in each direction on I-526, resulting in a 6-lane cross section through the study area. Subsequently, the CHATS model was updated to reflect higher regional growth, resulting in higher predicted traffic volumes in the corridor. The 6-lane widening alternative (3 lanes in each direction) is determined to be inadequate in providing an acceptable improvement in capacity. Based on traffic analysis, the 6-lane widening does not meet the purpose and need of the project to increase capacity and improve operations. Portions of I-526 would operate at

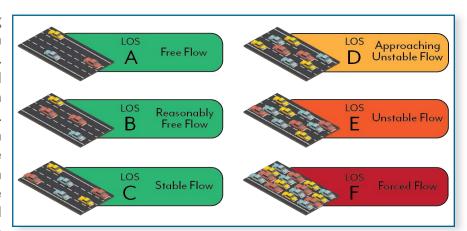


Figure 5.1 Level of Service (LOS)

a Level of Service (LOS) E or F approximately five years after construction. As shown in Table 5.2, traffic analysis is being used to compare the 6-lane and 8-lane alternatives. The 6-lane widening alternative is not carried forward for further evaluation because of a failing LOS. Refer to Figure 5.1 for description of LOS.

Table 5.2 Traffic Analysis of I-526 LCC WEST

0 10 10	0045 4 4 5 7	No Build	100	Build 2050	LOS			
Segment Description	2015 AADT	2050 AADT	LOS	AADT	6-Lane	8-Lane		
Sam Rittenberg to Paul Cantrell Blvd	39,400	59,800	С	68,500	В	В		
Paul Cantrell Blvd to Leeds Ave	79,200	106,900	F	136,900	F	D		
Leeds Ave to Dorchester Rd	78,800	106,400	F	134,000	F	D		
Dorchester Rd to Montague Ave	80,700	108,900	F	127,300	Е	С		
Montague Ave to International Blvd	67,400	91,000	F	109,600	D	С		
International Blvd to I-26	89,000	120,200	F	126,700	Е	С		
I-26 to Rivers Ave	77,200	104,200	F	116,100	D	С		
Rivers Ave to N Rhett Ave	75,600	104,400	F	126,700	Е	С		
N Rhett Ave to Virginia Ave	80,500	122,200	F	148,400	F	D		
East of Virginia Ave	68,900	110,100	F	133,800	F	D		



8-Lane Widening

West of I-26: Paul Cantrell Boulevard (Glenn McConnell Parkway) is a major arterial/expressway facility near the western end of I-526. The I-526 interchange at Paul Cantrell Boulevard is a logical terminus for the I-526 LCC WEST because of the volume of traffic that enters eastbound I-526 and exits westbound I-526 at this point. In the eastbound direction, the mainline widening begins at this location with a two-lane entrance ramp adding the lanes which comprise the four-lane eastbound lanes toward I-26. In the westbound direction, the widened four-lane mainline concept ends at this interchange. A new bridge carries the westbound lanes of Paul Cantrell Boulevard over the intersection with Magwood Drive and touches down on Glenn McConnell Parkway. The westbound exit ramp from I-526 is being widened and uses this new bridge to bypass the Magwood intersection, which currently causes traffic to back up onto I-526. The 8-lane widening of I-526 extends from Paul Cantrell Boulevard to I-26.

At I-26/I-526 System-to-System Interchange: Two of the four eastbound mainline lanes on I-526 serve as the westbound connection to eastbound and westbound I-26. The remaining two lanes extend through onto the existing alignment over I-26 and continue eastbound. In the westbound direction, two lanes are proposed as the ramp lanes from eastbound I-26.

East of I-26: The volume of traffic entering eastbound I-526 from I-26 is similar to the volume of through traffic coming over I-26. Similarly, the westbound I-526 traffic approaching I-26 is well balanced between the volume of traffic that continues west on I-526 and that which is destined for either eastbound or westbound I-26. For these reasons, the extension of collector distributor (C-D) roads from the system-to-system interchange eastward toward the Cooper River works well in reducing the weaving- related congestion that is currently prevalent today on I-526 from I-26, through the Rivers Avenue, N Rhett Avenue and Virginia Avenue interchanges.

I-526 east of Rivers Avenue: The eastern project terminus of Virginia Avenue is being selected based on the closely connected N Rhett Avenue interchange and the extensive traffic that backs onto I-526 from N Rhett Avenue. I-526 east of Rivers Avenue is on an elevated structure until it reaches Daniel Island. The existing structure continues to carry two through lanes for eastbound and westbound traffic on I-526, while the new C-D roads provide the needed additional capacity. The C-D roads also provide critical access after a major seismic event if the existing I-526 structure is not serviceable. The portion of elevated structure between Rivers Avenue and the Don Holt Bridge is not designed to resist seismic forces.

To summarize, the 8-lane widening alternative is being carried forward for a detailed impact evaluation. The additional two lanes in each direction meets the project purpose and need.



5.1.3 Interchange Alternatives

I-526 at Paul Cantrell Boulevard

The interchange at the I-526 and Paul Cantrell Boulevard contributes to the congestion along on I-526 LCC WEST. Figures 5.2 - 5.6 show the five alternatives developed for the interchange of Paul Cantrell Boulevard at I-526.

Alternative 1: Triple Lefts to I-526 eastbound with Improved Loops

- Failed to provide an acceptable LOS
- Not compatible with adjacent interchange
- Not carried forward

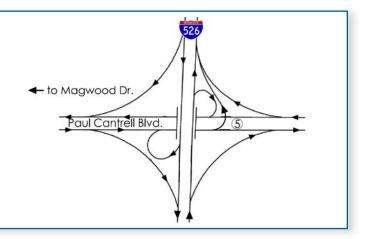


Figure 5.2 I-526 at Paul Cantrell Boulevard Alternative 1

Alternative 2: Semi-Directional Ramp to I-526 eastbound with Improved Loops

- Construtability issues with the westbound off-ramp system and the eastbound directional flyover on-ramp
- · Not carried forward

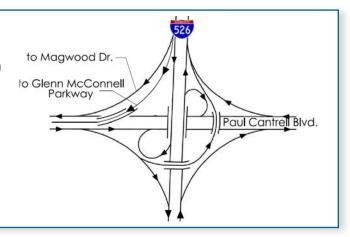


Figure 5.3 I-526 at Paul Cantrell Boulevard Alternative 2



Alternative 3: Diverging Diamond Interchange

- Not compatible with adjacent interchange
- Not carried forward

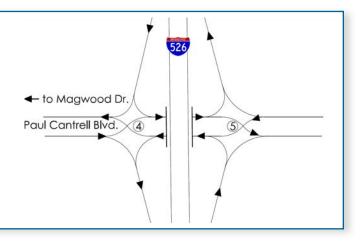


Figure 5.4 I-526 at Paul Cantrell Boulevard Alternative 3

Alternative 4: Single Point Interchange with Semi-Directional Ramp to I-526 eastbound

- Constructability requires extensive redesign and construction
 of the interchange
- Not carried forward

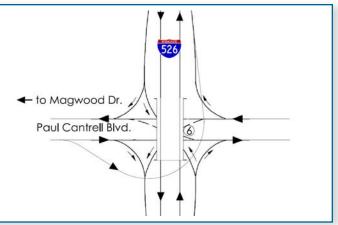


Figure 5.5 I-526 at Paul Cantrell Blvd Alternative 4

Alternative 5: Semi-Directional Ramp to I-526 eastbound with Improved Loop Ramps and Left Turn to I-526 eastbound to Magwood Dr.-

- Acceptable LOS
- Compatible with adjacent interchange
- Carried Forward

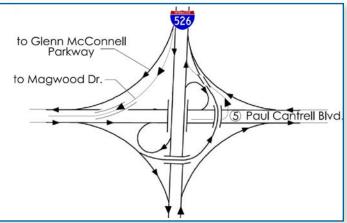


Figure 5.6 I-526 at Paul Cantrell Blvd Alternative 5



Prior to selection of an alternative for I-526 & Paul Cantrell Boulevard based on further analysis, the project goals were refined by SCDOT. These goals prioritized (1) I-526 mainline capacity, (2) improvement of the I-26 & I-526 system-to-system interchange, and (3) ensuring queues from ramp termini and adjacent intersections did not spill back onto the mainline. These priorities led to the selection of a modified Alternative 5, such that the westbound off-ramp system (with two-lanes to the separated overpass and one lane to the Paul Cantrell Boulevard surface street) was retained, but the eastbound directional flyover on-ramp was eliminated. The existing signalized intersection of Paul Cantrell Boulevard & I-526 EB on-ramp was retained. Refer to Figure 5.7 for the modified Alternative 5.

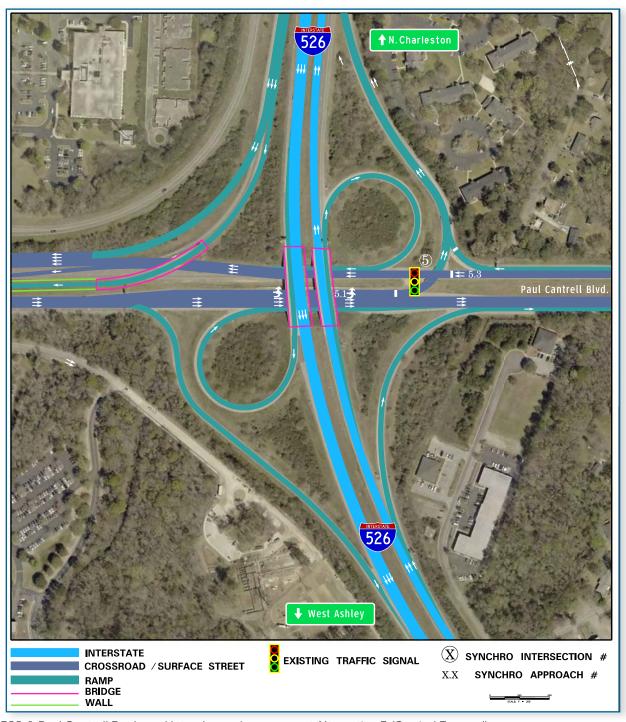


Figure 5.7 I-526 & Paul Cantrell Boulevard Interchange Improvement Alternative 5 (Carried Forward)



Paul Cantrell Boulevard at Magwood Drive

Due to the proximity of Magwood Drive to the I-526 at Paul Cantrell interchange, alternatives were developed and screened to mitigate the existing congestion. Figures 5.8 - 5.14 show the seven alternatives developed for the intersection of Paul Cantrell Boulevard at Magwood Drive.

Alternative 1: Diamond

- Not compatible with adjacent interchange
- · Not carried forward

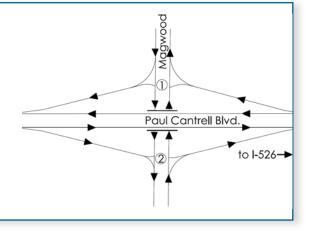


Figure 5.8 Paul Cantrell Boulevard at Magwood Drive Alternative 1

Alternative 2: Diamond with Braided Ramps

- Constuctability issues with the replacement of the westbound overpass bridge to provide a free-flow exit
- Not carried forward

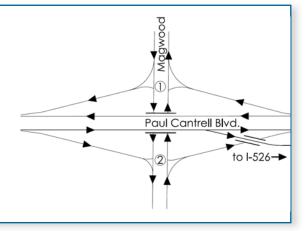


Figure 5.9 Paul Cantrell Boulevard at Magwood Drive Alternative 2

Alternative 3: Single Point Interchange

- · Not compatible with adjacent interchange
- Not carried forward

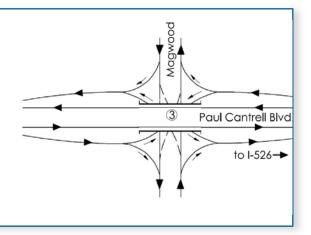


Figure 5.10 Paul Cantrell Boulevard at Magwood Drive Alternative 3



Alternative 4: Compressed Diamond with Phase Overlap

- Not compatible with adjacent interchange
- Not carried forward

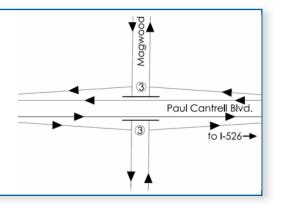


Figure 5.11 Paul Cantrell Boulevard at Magwood Drive Alternative 4

Alternative 5: Interchange with Separated Overpass Bridge

- Acceptable LOS
- Compatible with adjacent interchange
- · Carried forward

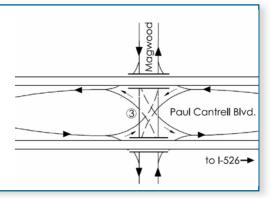


Figure 5.12 Paul Cantrell Boulevard at Magwood Drive Alternative 5

Alternative 6: Maximized At Grade Intersection

- Does not provide acceptable LOS
- Not carried forward

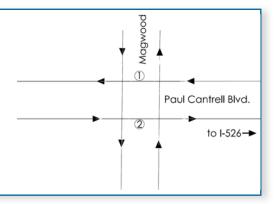


Figure 5.13 Paul Cantrell Boulevard at Magwood Drive Alternative 6

Alternative 7: Continuous Flow Intersection

- Does not provide acceptable LOS
- Not compatible with adjacent interchange
- · Not carried forward

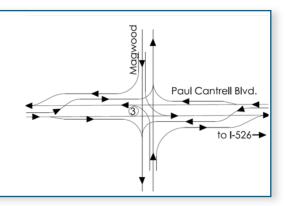


Figure 5.14 Paul Cantrell Boulevard at Magwood Drive Alternative 7



Prior to selection of a preferred alternative for Paul Cantrell Boulevard & Magwood Drive based on further analysis, the project goals were refined by SCDOT. These goals prioritized (1) I-526 mainline capacity, (2) improvement of the I-26 & I-526 system-to-system interchange, and (3) ensuring queues from ramps did not spill back onto the mainline. These priorities altered the alternative screening process for this interchange, leading to the selection of a modified Alternative 5, such that the westbound overpass bridge was retained (to provide free-flow for traffic exiting I-526) but the eastbound overpass bridge was eliminated (as it did not contribute to the three priorities). This geometry is shown in Figure 5.15.

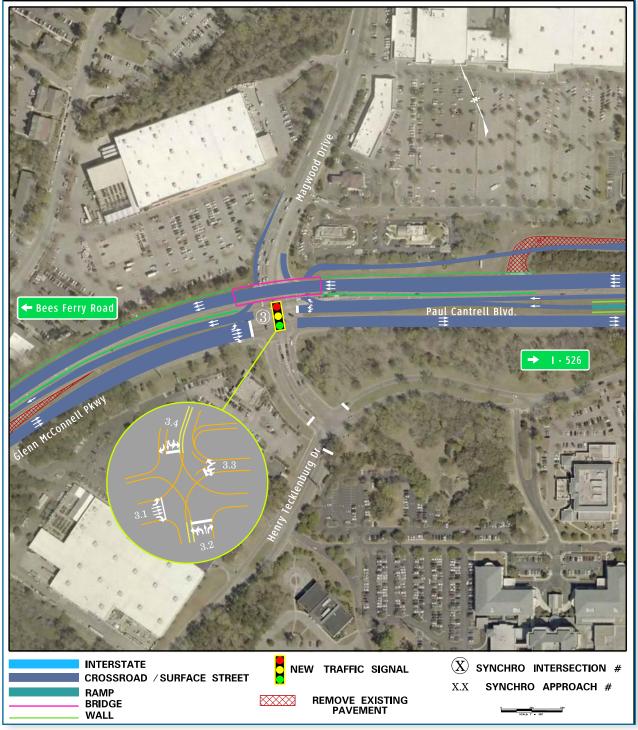


Figure 5.15 Paul Cantrell Boulevard & Magwood Drive Interchange Improvement Alternatives (Carried Forward)



I-26/I-526 System-to-System Interchange

Alternatives were developed based on separating movements that create congestion caused by closely spaced ramps and inadequate lengths of merges and weaves.

Alternative 1: Semi-Directional Interchange

- C-D roads added to north and south side of I-526 through Rivers Avenue interchange and on th N Rhett/Virginia Avenue
- Eastbound I-526 to westbound I-26 directional ramp moved to cross over I-26 north of I-526
- Carried forward as it meets an acceptable LOS, resolves existing geometric deficiencies and is compatible with adjacent interchanges

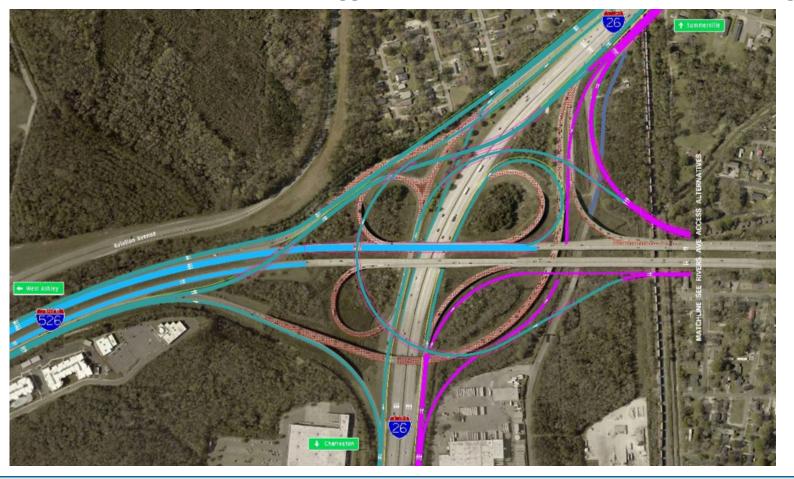


Figure 5.16 I-26/I-526 Alternative 1



Alternative 2: Semi-Directional Interchange with 1 Loop Ramp Retained

- C-D roads added to north and south side of I-526 through Rivers Avenue interchange
- Eastbound I-526 to westbound I-26 uses existing directional ramp
- Carried forward as it has an acceptable LOS, resolves existing geometric deficiencies and is compatible with adjacent interchanges

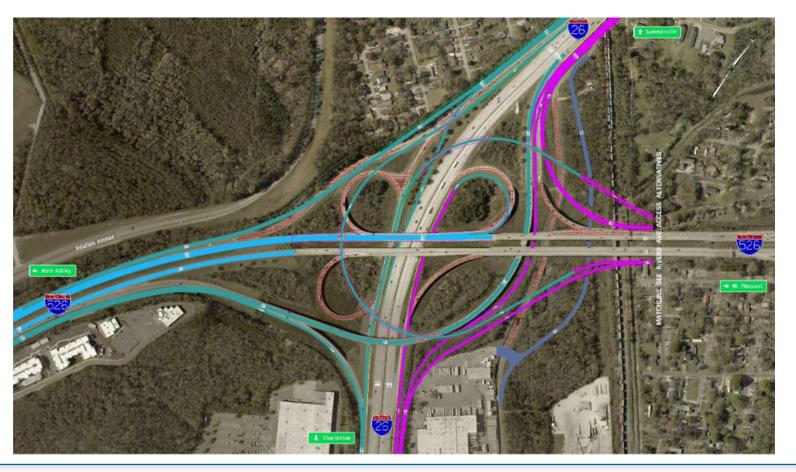


Figure 5.17 I-26/I-526 Alternative 2



Alternative 3: Semi-Directional Turbine Interchange

• Not carried forward due to significantly larger footprint and impacts to federal properties as well as airport flight paths.

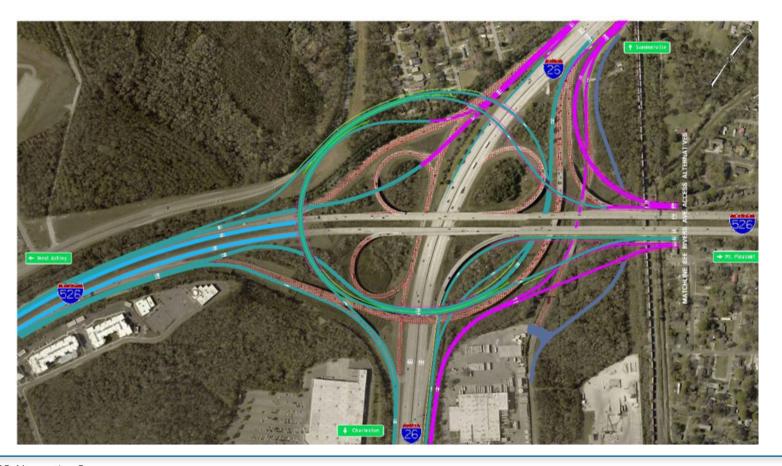


Figure 5.18 I-26/I-526 Alternative 3



Alternative 4: Semi-Directional with 3 Levels of Ramping

- Westbound I-26 to westbound I-526 loop ramp replaced with a directional ramp, creating 3-level-high interchange
- Not carried forward due to complex constructability

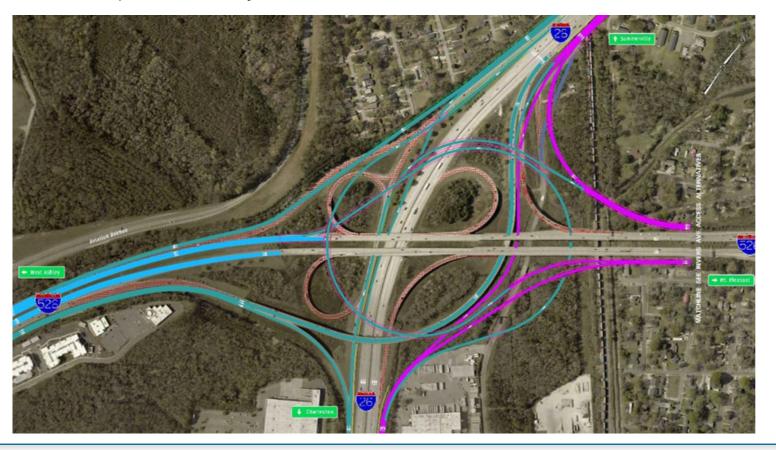


Figure 5.19 I-26/I-526 Alternative 4



I-526 at Rivers Avenue

One interchange alternative was developed from the 2013 Corridor Study, the Partial Cloverleaf Rebuild. This alternative was developed based on separating movements that create congestion caused by closely spaced ramps and inadequate lengths of merges and weaves. The second alternative is a basic build scenario that proposes new C-D roads, but no improvements to the existing interchange.

Rivers Avenue: Basic Build

- New C-D roads constructed over the existing eastbound and westbound Rivers Avenue interchange
- Direct access from Rivers Avenue to I-26 via I-526 is removed; access I-26 from the I-26 at Remount Road interchange to the north or at I-26 at Montague Avenue to the south
- Carried forward as it has acceptable LOS, resolves geometric deficiencies and is compatible with adjacent interchanges

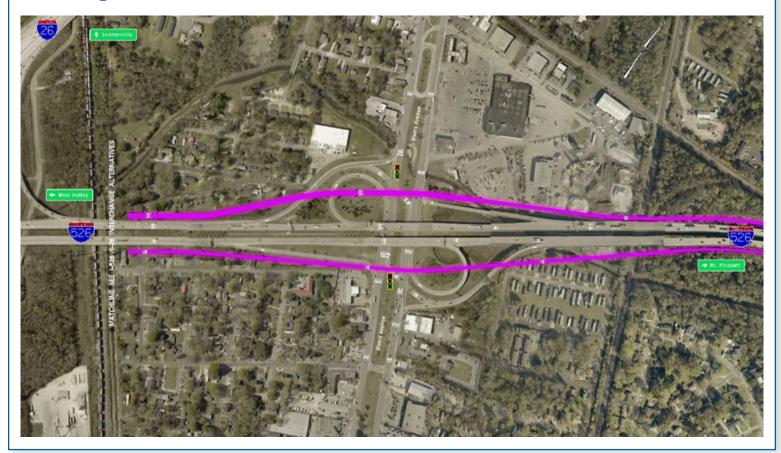


Figure 5.20 I-526 at Rivers Avenue Basic Build



I-526 at Rivers Avenue

Rivers Avenue: Relocated Partial Cloverleaf

- New C-D system constructed over Rivers Avenue
- Additional ramps constructed between Rivers Avenue and C-D system to maintain access to I-26 via I-526 from Rivers Avenue
- Carried forward as it has acceptable LOS, resolves geometric deficiencies and is compatible with adjacent interchanges

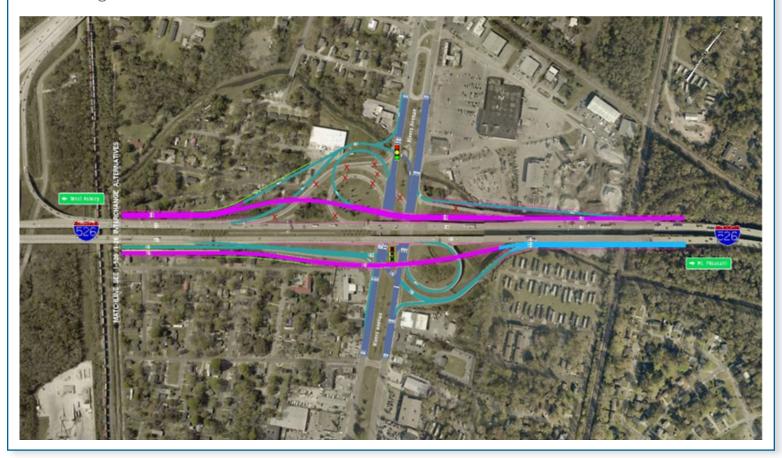


Figure 5.21 I-526 at Rivers Avenue Relocated Partial Cloverleaf



I-526 at N Rhett/Virginia Avenue

Traffic patterns between the Don Holt Bridge and I-26 led the development of the N Rhett/Virginia Avenue interchange alternatives. Traffic projections indicate one-third of westbound traffic exits at N Rhett/Virginia Avenues, one-third exits to I-26 and one-third continues west on I-526 past I-26. Eastbound traffic over the Don Holt Bridge shows similar forecast comparisons with one-third originating west of I-526, one-third coming from I-26, and one-third from N Rhett/Virginia Avenues. Entering and exiting traffic at Rivers Avenue comprises a nominal portion of the traffic in each direction. A key component of the N Rhett/Virginia Avenue interchange concept development is providing connections to the C-D roads in a manner that accommodates forecasted traffic patterns. The forecasted traffic patterns support the use of C-D roads to provide additional capacity between I-26 and the Cooper River.

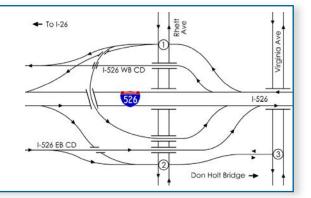
The capacity of the existing N Rhett/Virginia Avenue interchange is limited by geometric deficiencies. The existing loop ramps of the interchange have a 25-mph design speed with very short weave distances in both eastbound and westbound directions. To address these deficiencies and respond to future traffic demand, interchange improvement alternatives were developed for this interchange as part of the I-526 LCC WEST project. The major design constraints considered during alternative development included existing CSX and Norfolk Southern rail lines running adjacent to and underneath the interstate, as well as Filbin Creek, a major tributary to the Cooper River, flowing adjacent to the I-526 mainline and crossing under I-526 just west of N Rhett Avenue.

Four alternatives were developed for the initial screening process to accommodate anticipated traffic demand to a design LOS D or better. Traffic volumes utilizing Virginia Avenue on and off-ramps, particularly trucks accessing the North Charleston Port Terminal and other industrial land uses have expressed the need to retain access. Refer to Figures 5.22 - 5.25 for descriptions of the alternatives.

Alternative 1: On-ramp from N Rhett Avenue to I-526 eastbound and westbound through one intersection along N Rhett Avenue with separate access to Virginia Avenue

- · Compatible with adjacent interchange
- Flexible with Don Holt bridge replacement
- Does not provide direct access between Virginia Avenue and I-526 (traffic must travel through N Rhett Avenue intersection)
- Carried forward

Figure 5.22 I-526 at N Rhett/Virginia Avenue Alternative 1



Alternative 2: Diamond Interchange with access to Virginia Avenue

- · Compatible with adjacent interchange
- Flexible with Don Holt bridge replacement
- Does not provide direct access between Virginia Avenue and I-526 (traffic must travel through N Rhett Avenue intersection)
- Carried forward

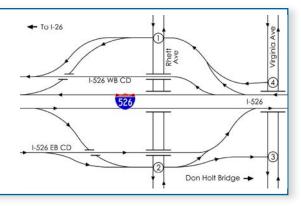


Figure 5.23 I-526 at N Rhett/Virginia Avenue Alternative 2



Alternative 3: Improve existing Loop Ramps

- Not compatible with adjacent interchange
- Does not provide direct access between Virginia Avenue and I-526 (constructability issue due to removing the direct access to/from I-526 and Virginia Avenue, requiring these movements to be made via parallel routes)
- · Not carried forward

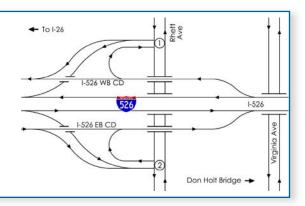


Figure 5.24 I-526 at N Rhett/Virginia Avenue Alternative 3

Alternative 4: Directional ramps from northbound to southbound N Rhett Avenue traffic

- · Not compatible with adjacent interchange
- Does not provide direct access between Virginia Avenue and I-526 (traffic must travel through N Rhett Avenue intersection)
- · Not carried forward

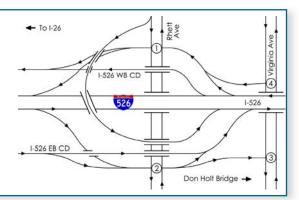


Figure 5.25 I-526 at N Rhett/Virginia Avenue Alternative 4



6 What are the Proposed Reasonable Alternatives Presented at the Public Information Meeting?

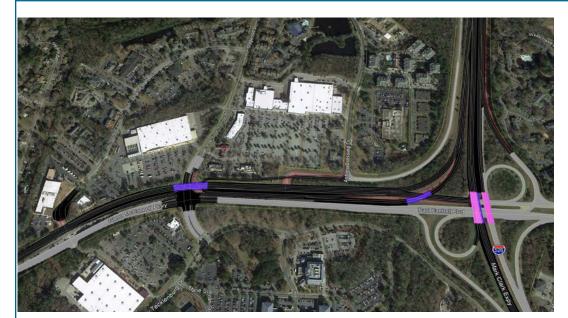
Based on the screening previously described in Section 5, the preliminary alternatives are evaluated based on their ability to meet the purpose and need, as well as additional criteria. Prior to the public meeting the following alternatives were identified as Proposed Reasonable Alternatives:

- No-Build
- Existing Corridor Improvements
 - > Mainline Interstate Alternatives
 - 8-lane widening
 - > Interchange Alternatives (Refer to Figure 6.1)
 - One alternative at I-526 at Paul Cantrell Boulevard that includes the intersection at Magwood Drive
 - Due to proximity of I-526 at I-26 and Rivers
 Avenue, these interchanges are combined. Four
 alternatives are being carried forward as Reasonable Alternatives
 - Two alternatives at I-526 at N Rhett/Virginia Avenue

Preliminary Alternatives Screening Criteria:

- Acceptable LOS
- Compatible with Adjacent Interchange
- Geometric Deficiencies Resolved
- Flexibility with Don Holt Bridge Replacement
- Constructability





I-526 at Paul Cantrell Blvd



I-526 at I-26 and Rivers Ave: Alternative 1



I-526 at I-26 and Rivers Ave: Alternative 1A



I-526 at N Rhett/Virginia Ave: Alternative 1



I-526 at I-26 and Rivers Ave: Alternative 2



I-526 at I-26 and Rivers Ave: Alternative 2A



I-526 at N Rhett/Virginia Ave: Alternative 2



Figure 6.1 I-526 LCC WEST Proposed Reasonable Interchange Alternatives



6.1 Public Input on Proposed Reasonable Alternatives

During the November 21, 2019 Public Information Meeting (PIM) and the Virtual Public Information Meeting (VPIM), the public were encouraged to provide feedback on the proposed reasonable alternatives. For more information about the PIM see Chapter 6, section 6.4.1.3 of the Environmental Impact Statement.

Following the I-526 WEST PIM, feedback was received about the I-526 / North Rhett Avenue and I-526 / Virginia Avenue interchange alternatives. Joint Base Charleston, State Ports Authority, and the City of North Charleston expressed concerns over the removal of direct access between Virginia Avenue and I-526 in the proposed alternatives. In the two alternatives presented at the I-526 WEST PIM access to/from Virginia Avenue from/to I-526 required processing through the ramp terminal intersections on N Rhett Avenue. Alternatives 5 and 6 were developed that incorporated a Texas U-Turn style ramp that traveled from Virginia Avenue, back to the west and under I-526 adjacent to N Rhett Avenue to provide direct access to I-526 eastbound. Access from I-526 westbound would also utilize the U-Turn ramp for direct access to Virginia Avenue. Existing access from Virginia Avenue to I-526 westbound was retained and new ramps will provide access from I-526 eastbound to Virginia Avenue.

In addition to public comments regarding direct access from I-526 and Virginia Avenue, regulatory and commenting agencies also expressed concerns with alternatives avoiding and minimizing impacts to environmentally sensitive areas within the alternative corridors. In an effort to evaluate an alternative that both met the purpose and need of the project and minimized impacts, an alternative that was a combination of reasonable alternatives 6 and 2 was developed. Alternative 2A, Alternative 5, and Alternative 6 were developed as proposed Reasonable Alternatives for the I-526 at N Rhett/Virginia Avenue interchange. Therefore, a total of five alternatives are being carried forward at the N Rhett/Virginia Avenue interchange. Refer to Figure 6.2 through 6.5 for Alternatives 2A, 5, and 6.

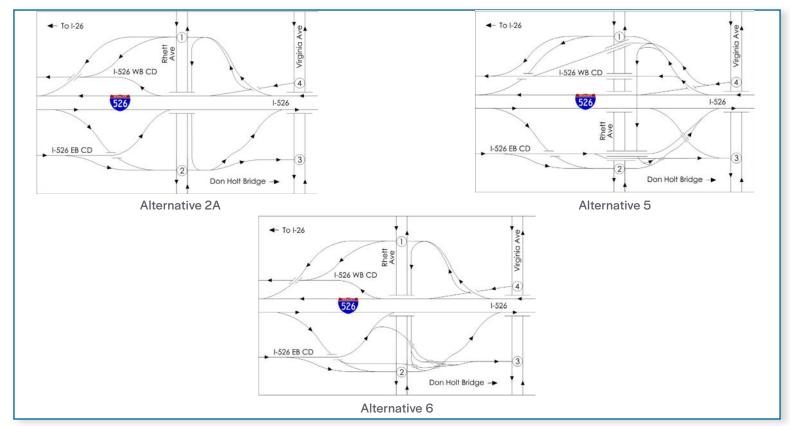


Figure 6.2 I-526 at N Rhett/Virginia Avenue Alternatives 2A, 5, and 6





Figure 6.3 I-526 at N Rhett/Virginia Avenue Alternative 2A



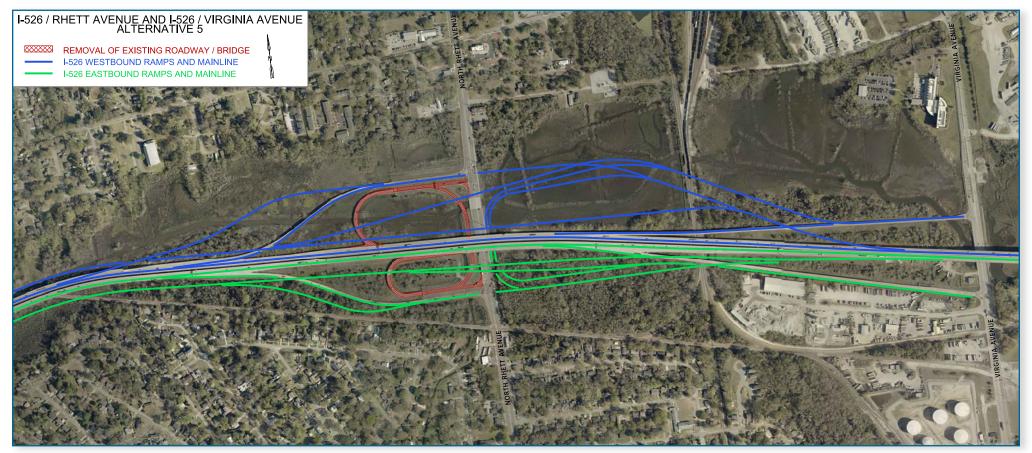


Figure 6.4 I-526 at N Rhett/Virginia Avenue Alternative 5



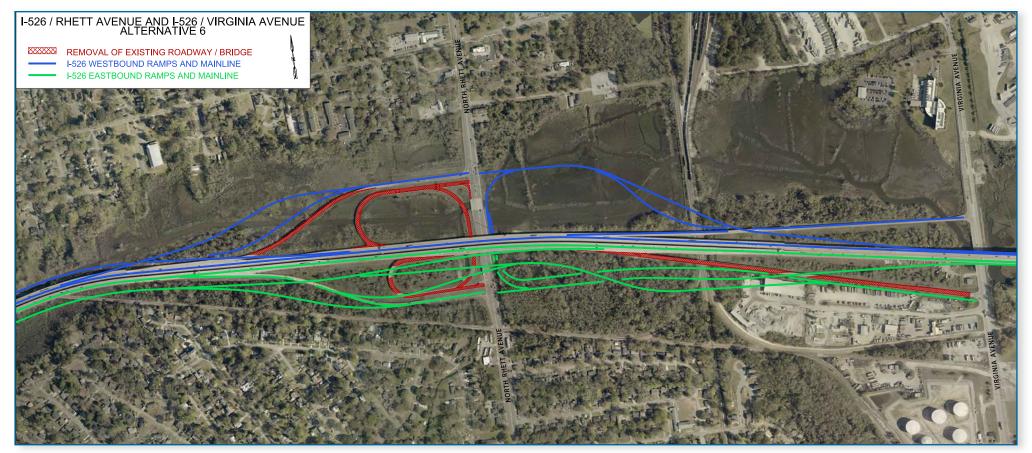


Figure 6.5 I-526 at N Rhett/Virginia Avenue Alternative 6



6.2 Detailed Impact Evaluation of Proposed Reasonable Alternatives

In summary, the Proposed Reasonable Alternatives include the following:

- No-Build
- Mainline Interstate 8-lane widening
- Interchange Alternatives
 - > One alternative at I-526 at Paul Cantrell Boulevard that includes the intersection at Magwood Drive
 - > Four alternatives at I-526 at I-26 and Rivers Avenue
 - Alternative 1
 - Alternative 1A
 - Alternative 2
 - Alternative 2A
 - > Five alternatives at I-526 at N Rhett/Virginia Avenue
 - Alternative 1
 - Alternative 2
 - Alternative 2A
 - Alternative 5
 - Alternative 6

In order to perform a detailed impact evaluation on the above alternatives, the widening of the mainline to 8-lanes was combined with the interchange alternatives into the following three sections and shown in Figure 6.6.

- Paul Cantrell Boulevard to International Boulevard
- International Boulevard to Rivers Avenue
- Rivers Avenue to Virginia Avenue



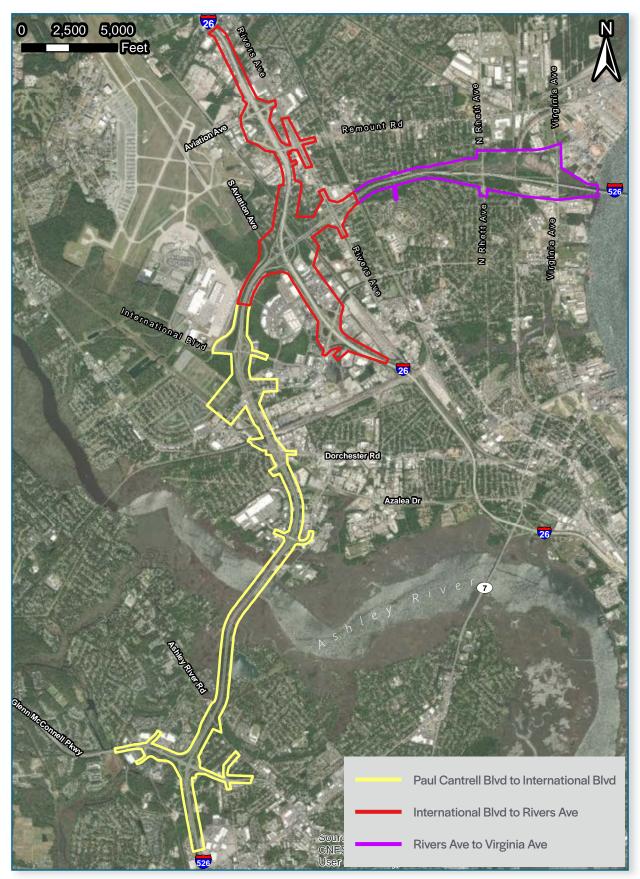


Figure 6.6 Proposed Reasonable Alternatives Sections of I-526 LCC WEST



Each section was then evaluated based on the following criteria:

- Purpose and Need: 2050 Traffic Analysis
 - > Geometric Deficiencies Resolved
 - > Provides Direct Access to/from I-526
 - > Provides Direct Access to/from I-26
 - > Weighted v/c Ratio
 - > Intersection LOS/Delay
 - > Mainline LOS
- Wetlands
 - > Freshwater Impact Based on Right-of-Way
 - > Critical Area Impact Based on Right-of-Way
 - > Critical Area (Ashley River) Bridge Construction Temporary Access Based on Right-of-Way
 - > Pond Impact Based on Right-of-Way
 - > Freshwater Stream Impact Based on Right-of-Way
- Relocations
 - > Residential
 - > Businesses
 - > Churches
 - > Community Facilities
- Environmental Justice
- Threatened & Endangered Species
- Essential Fish Habitat
- Cultural Resources
- Section 4(f) & 6(f)
- Utilities
- Cost

AADT = Average Annual Daily Traffic

LOS = Level of Service - Way to describe roadway operating conditions based on speed, travel time, maneuverability, delay and safety

v/c Ratio = volume to capacity Ratio - Compares roadway demand (volume) with roadway supply (capacity)

Refer to Chapter 2, section 2.1.2.

Weighted v/c Ratio = A way to measure the efficiency of the alternatives for moving traffic within the interchange



6.2.1 Paul Cantrell Boulevard to International Boulevard

This alternative encompasses the interchange at Paul Cantrell Boulevard and I-526, the intersection at Paul Cantrell Boulevard and Magwood Drive, and the widening of I-526 from Paul Cantrell Boulevard to International Boulevard, refer to Figure 6.7.

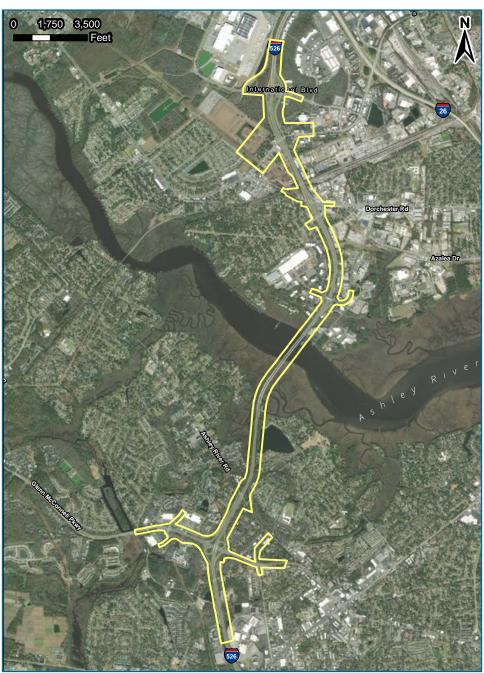


Figure 6.7 I-526 LCC WEST Section from Paul Cantrell Boulevard to International Boulevard

As shown in Table 6.1, the proposed alternative would resolve 15 out of the 16 identified geometric deficiencies as compared to the No-Build Alternative. The proposed alternative would also improve the weighted v/c ratio and the mainline LOS as compared to the No-Build.

Geometric Deficiency is the consideration of the inadequacies of roadway design. For more detials refer to Chapter 2, section 2.1.5.



Table 6.1 Proposed Reasonable Alternatives Screening Matrix: Paul Cantrell Blvd to International Blvd

Table 6.1 Proposed Reasonable Alternative		No-Build	Paul Cantrell Blvd to International Blvd
	Geometric Deficiencies Resolved	0/16	15/16
	Provides Direct Access to/ from I-526 (Yes/No)	Yes	Yes
Purpose & Need: 2050 Traffic Analysis	Provides Direct Access to/ from I-26 (Yes/No)	N/A	N/A
	Weighted v/c Ratio	1.74 2.50 2.90 3.11	0.72 0.75 0.72 0.67
	Intersection Delay/LOS	N/A	N/A
	Mainline LOS	F	D/D/C/C
Freshwater Wetland Impact Based on R/W	(Acres)	0	19.3
Critical Area Impact Based on R/W	(Acres)	0	15.5
Critical Area (Ashley River) Bridge Construction Temporary Access Based on R/W	(Acres)	0	9.1
Pond Impact Based on R/W	(Acres)	0	0.03
Freshwater Stream Impact Based on R/W	(Feet)	0	327.0
	Residential	0	4 Single-Family Homes; 2 Multi-Family Complexes, 6 Units Total
	Businesses	0	8
Relocations	Churches	0	0
	Community Facilitites	0	0
	Total	0	18
Environmental Justice	Yes/No	No	No
Threatened & Endangered Species		0	May Effect, Not Likey to Adversely Affect
Essential Fish Habitat	Yes/No	No	Yes
Cultural Resources	Eligibility for Listing on NRHP	No Effect	No Effect: Indeterminate Eligibility Underwater Resource 006-1
Section 4(f) & 6(f)	Yes/No	No	No
Utilities	\$	\$0	\$12,901,540
Cost	\$	\$0	\$108,600,000
Preferred Alternative	Yes/No	No	Yes

This "weighted v/c ratio" was calculated for the purposes of summarizing and comparing the segment v/c ratio results in a simplified manner to rank each. This method weights each individual v/c ratio according to the volume processed in that movement. The weighted v/c ratio is a way to measure the efficiency of the alternatives for moving traffic within the interchange.



6.2.2 International Boulevard to Rivers Avenue

Table 6.2 shows the detailed evaluation of the four Proposed Reasonable Alternatives from International Boulevard to Rivers Avenue, including the I-526/I-26 interchange and the widening of I-526, refer to Figure 6.8. Alternative 2 is recommended as the preferred alternative between International Boulevard and Rivers Avenue. Although Alternative 1 and 2 would remove access from Rivers Avenue to I-26, using I-526, they would result in lower relocations and potential impact to environmental justice populations than Alternative 1A or 2A. Alternative 1 would require a traffic movement or weave that may result in overcapacity and failing LOS in the segment. The over-congestion of this segment in Alternative 1 may cause upstream backups along I-526 eastbound and I-526 westbound. Alternative 2 does not require this traffic movement or weave, which reduces the number of vehicles which must weave compared to Alternative 1. This results in traffic operations which are under capacity and with acceptable LOS C. Alternative 2 is the recommended preferred alternative between International Boulevard and Rivers Avenue.

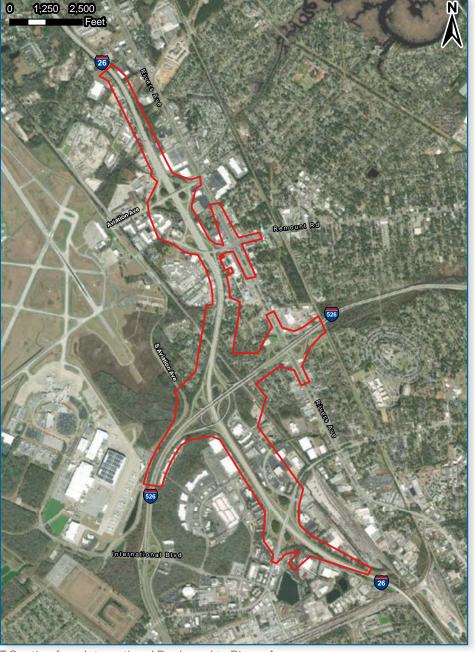


Figure 6.8 I-526 LCC WEST Section from International Boulevard to Rivers Avenue



Table 6.2 Proposed Reasonable Alternatives Screening Matrix: International Blvd to Rivers Ave

Table 6.2 Proposed Reasonable Alternative	8		I-26/I-526 System-to-System & I-526 at Rivers Avenue							
		No-Build	1	2	1A	2A				
	Geometric Deficiencies Resolved	0/11	8/11	8/11	9/11	9/11				
	Provides Direct Access to/ from I-526 (Yes/No)	Yes	Yes	Yes	Yes	Yes				
Purpose & Need: 2050 Traffic Analysis	Provides Direct Access to/ from I-26 (Yes/No)	N/A	No	No	Yes	Yes				
	Weighted v/c Ratio	1.09	0.74	0.71	0.77	0.74				
	Intersection Delay/LOS	N/A	N/A	N/A	N/A	N/A				
	Mainline LOS	F	С	С	С	С				
Freshwater Wetland Impact Based on R/W	(Acres)	0	28.5	28.5	28.5	28.5				
Critical Area Impact Based on R/W	(Acres)	0	0	0	0	0				
Critical Area (Ashley River) Bridge Construction Temporary Access Based on R/W	(Acres)	N/A	N/A	N/A	N/A	N/A				
Pond Impact Based on R/W	(Acres)	0	0	0	0	0				
Freshwater Stream Impact Based on R/W	(Feet)	0	13,327.1	13,327.1	13,327.1	13,327.1				
	Residential	0	35 Single-Family Homes; 15 Mobile Homes; 14 Multi-Family Complexes, 41 Units Total	35 Single-Family Homes; 15 Mobile Homes; 14 Multi-Family Complexes, 41 Units Total	39 Single-Family Homes; 16 Mobile Homes; 20 Multi-Family Complexes, 60 Units Total	39 Single-Family Homes; 16 Mobile Homes; 20 Multi-Family Complexes, 60 Units Total				
	Businesses	0	12	12	13	13				
Relocations	Churches	0	1 - Enoch Chapel Methodist	1 - Enoch Chapel Methodist	2 - Enoch Chapel Methodist, Life Changers Covenant Ministries	2 - Enoch Chapel Methodist, Life Changers Covenant Ministries				
	Community Facilitites	0	2 - Highland Terrace-Liberty Park Community Center, Russelldale Community Center	2 - Highland Terrace-Liberty Park Community Center, Russelldale Community Center	2 - Highland Terrace-Liberty Park Community Center, Russelldale Community Center	2 - Highland Terrace-Liberty Park Community Center, Russelldale Community Center				
	Total	0	106	106	132	132				
Environmental Justice	Yes/No	No	Yes (94)	Yes (94)	Yes (120)	Yes (120)				
Threatened & Endangered Species		No Effect	May Effect, Not Likely to Adversely Affect							
Essential Fish Habitat	Yes/No	No	No	No	No	No				
Cultural Resources	Eligibility for Listing on NRHP	No Effect	No Adverse Effect	No Adverse Effect	No Adverse Effect	No Adverse Effect				
Section 4(f) & 6(f)	Yes/No	No	Yes Highland Terrace-Liberty Park Community Center - 4(f) & 6(f); Russelldale Community Center - 4(f)	Yes Highland Terrace-Liberty Park Community Center - 4(f) & 6(f); Russelldale Community Center - 4(f)	Yes Highland Terrace-Liberty Park Community Center - 4(f) & 6(f); Russelldale Community Center - 4(f)	Yes Highland Terrace-Liberty Park Community Center - 4(f) & 6(f); Russelldale Community Center - 4(f)				
Utilities	\$	\$0	\$37,082,500 (includes Alternative 1 or 2 at N Rhett/Virginia Ave interchange)	\$37,082,500 (includes Alternative 1 or 2 at N Rhett/Virginia Ave interchange)	\$43,582,500 (includes Alternative 1 or 2 at N Rhett/Virginia Ave interchange)	\$43,582,500 (includes Alternative 1 or 2 at N Rhett/Virginia Ave interchange)				
Cost	\$	\$0	\$950,000,000	\$979,000,000	\$1,068,000,000	\$1,066,000,000				
Preferred Alternative	Yes/No	No	No	Yes	No	No				



6.2.3 Rivers Avenue to Virginia Avenue

The five Proposed Reasonable Alternatives from Rivers Avenue to Virginia Avenue, including the I-526 at N Rhett interchange, and the widening of I-526, are shown in Table 6.3 and Figure 6.9. Alternative 2A is estimated to have the lowest potential impact to wetlands, streams, and relocations as compared to the other four alternatives. Alternative 2A is the recommended preferred alternative between Rivers Avenue and Virginia Avenue.

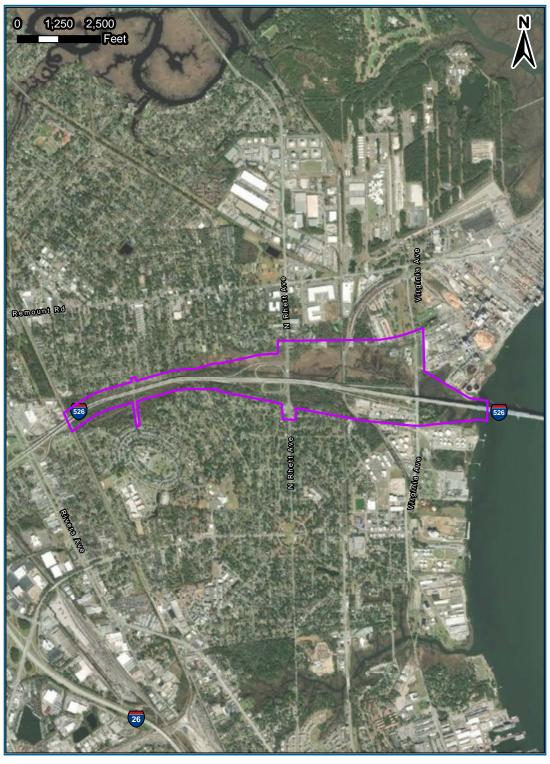


Figure 6.9 I-526 LCC WEST Section from Rivers Avenue to Virginia Avenue



Table 6.3 Proposed Reasonable Alternatives Screening Matrix: Rivers Ave to Virginia Ave

	Neasonable Alternatives			I-526 at N Rhett/Virginia Ave																		
		No-Build	1					;	2				A		5				6			
	Geometric Deficiencies Resolved	0/3	3/3		3/3			3/3			3/3			3/3								
	Provides Direct Access to/from I-526 (Yes/No)	Yes	526 EB to Virginia	526 WB to Virginia	Virginia to 526 EB	Virginia to 526 WB	526 EB to Virginia	526 WB to Virginia	Virginia to 526 EB	Virginia to 526 WB	526 EB to Virginia	526 WB to Virginia	Virginia to 526 EB	Virginia to 526 WB	526 EB to Virginia	526 WB to Virginia	Virginia to 526 EB	Virginia to 526 WB	526 EB to Virginia	526 WB to Virginia	Virginia to 526 EB	Virginia to 526 WB
			No	No	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Purpose & Need: 2050 Traffic Analysis	Provides Direct Access to/from I-26 (Yes/No)	N/A		N/	'A			N	/A			N	/A			N/	/A		N/A			
	Weighted v/c Ratio	1.14		1.0	00			0.	99			0.	91			0.0	36		0.91			
			AM Pea	ak Hour	PM Pe	ak Hour	AM Pea	ak Hour	PM Pea	ak Hour	AM Pea	ık Hour	PM Pe	ak Hour	AM Pea	ak Hour	PM Pea	k Hour	AM Pe	ak Hour	PM Pea	ak Hour
	Intersection Delay/LOS	N/A	EB C/22.7	WB F/155.8	EB D/37.3	WB F/195.3	EB F/102.9	WB D/43.8	EB E/67.1	WB D/37.8	EB C/30.1	WB B/18.6	EB C/30.7	WB B/11.6	EB C/30.3	WB B/18.4	EB C/31.1	WB B/11.6	EB C/30.3	WB B/18.4	EB C/31.1	WB B/11.6
	Mainline LOS	F		C/	D			C	/D			C	/D			C/	′D			С	/D	
Freshwater Wetland Impact Based on R/W	(Acres)	0		54	.5			5′	1.3			49).9			57	7.3		50.8			
Critical Area Impact Based on R/W	(Acres)	0	2.3			2	.3			2	.4		2.8			2.7						
Critical Area (Ashley River) Bridge Construction Temporary Access Based on R/W	(Acres)	N/A	N/A		N/A			N/A			N/A			N/A								
Pond Impact Based on R/W	(Acres)	0	0		0				(0 0				0								
Freshwater Stream Impact Based on R/W	(Feet)	0		5,15	9.6		5,169.1			4,977.6 5,197.4				5,205.9								
	Residential	0		1 Single-Fa	mily Home		1 Single-Family Home			1 Single-Family Home			1 Single-Family Home			1 Single-Family Home						
	Businesses	0		3	3		3			0			3			3						
Relocations	Churches	0		C)		0			0			0			0						
	Community Facilitites	0		C)			()		0			0				0				
	Total	0		4	+		4			1			4			4						
Environmental Justice	Yes/No	No		Yes	(1)		Yes (1)			Yes (1)			Yes (1)			Yes (1)						
Threatened & Endangered Species		No Effect	May Effe	ect, Not Likey	to Adverse	ely Affect	May Effe	ect, Not Like	y to Adverse	y Affect	May Effect, Not Likey to Adversely Affect			ly Affect	May Effect, Not Likey to Adversely Affect			May Eff	ect, Not Like	y to Adverse	ly Affect	
Essential Fish Habitat	Yes/No	No	Yes			Y	es		Yes			Yes		Yes								
Cultural Resources	Eligibility for Listing on NRHP	No Effect	No Effect: No Potentially Eligible Resources		No Effect: No Potentially Eligible Resources			No Effect: No Potentially Eligible Resources			No Effect: No Potentially Eligible Resources			No Effect: No Potentially Eligible Resources								
Section 4(f) & 6(f)	Yes/No	No	No		No			No			No			No								
Utilities	\$	\$0	See Utility Costs Under I-526/I		'I-26/Rivers Avenue Alternatives			+ Approx. \$3.5 Million in Transmission Line Relocations			+ Approx. \$3.5 Million in Transmission Line Relocations			+ Approx. \$3.5 Million in Transmission Line Relocations								
Cost	\$	\$0		\$336,00	00,000			\$338,0	00,000			\$341,0	00,000			\$473,00	00,000			\$461,0	00,000	
Preferred Alternative	Yes/No	No		N	0			N	lo			Y	es			N	0			N	lo	



6.3 Recommended Preferred Alternative

Table 6.4 shows the recommended preferred alternative by the previously discussed sections, while Table 6.5 provides a summary of all the combined potential impacts.

Table 6.4 Proposed Reasonable Alternatives S	ble 6.4 Proposed Reasonable Alternatives Screening Matrix: Recommended Preferred Alternative								
		No-Build	Paul Cantrell Blvd to International Blvd	I-526 at I-26 including Rivers Ave: Alternative 2	I-526 at N Rhett/Virginia Ave: Alternative 2A				
	Geometric Deficiencies Resolved	0/30	15/16	8/11	3/3				
	Provides Direct Access to/from I-526 (Yes/No)	Yes	Yes	Yes	526 EB to Virginia Virginia to Virginia to Virginia to S26 WB No Yes Yes Yes Yes				
Purpose & Need: 2050 Traffic Analysis	Provides Direct Access to/from I-26 (Yes/No)	N/A	N/A	No	N/A				
	Weighted v/c Ratio	> 1.00	0.72 0.75 0.72 0.67	0.71	0.91				
	Intersection Delay/LOS	N/A	N/A	N/A	AM Peak Hour PM Peak Hour EB WB EB WB C/30.1 B/18.6 C/30.7 B/11.6				
	Mainline LOS	F	D/D/C/C	С	C/D				
Freshwater Wetland Impact Based on R/W	(Acres)	0	19.3	28.5	49.9				
Critical Area Impact Based on R/W	(Acres)	0	15.5	0	2.4				
Critical Area (Ashley River) Bridge Construction Temporary Access Based on R/W	(Acres)	0	9.1	N/A	N/A				
Pond Impact Based on R/W	(Acres)	0	0.03	0	0				
Freshwater Stream Impact Based on R/W	(Feet)	0	327.0	13,327.1	4,977.6				
	Residential	0	4 single-family homes; 2 multi-family complexes, 6 units total	35 Single-Family Homes; 15 Mobile Homes; 14 Multi-Family Complexes, 41 Units Total	1 Single-Family Home				
	Businesses	0	8	12	0				
Relocations	Churches	0	0	1 - Enoch Chapel Methodist	0				
	Community Facilitites	0	0	2 - Highland Terrace-Liberty Park Community Center, Russelldale Community Center	0				
	Total	0	18	106	1				
Environmental Justice	Yes/No	No	No	Yes (94)	Yes (1)				
Threatened & Endangered Species		0	May Effect, Not Likey to Adversely Affect	May Effect, Not Likey to Adversely Affect	May Effect, Not Likey to Adversely Affect				
Essential Fish Habitat	Yes/No	No	Yes	No	Yes				
Cultural Resources	Eligibility for Listing on NRHP	No Effect	No Effect: Potentially Eligible Underwater Resource 006-1	No Adverse Effect	No Effect: No Potentially Eligible Resources				
Section 4(f) & 6(f)	Yes/No	No	No	Yes Highland Terrace-Liberty Park Community Center - 4(f) & 6(f); Russelldale Community Center - 4(f)	No				
Utilities	\$	\$0	\$12,901,540	\$37,082,500 (includes Alternative 1 or 2 at N Rhett/Virginia Ave)	+ Approx. \$3.5 Million in Transmission Line Relocation				
Cost	\$	\$0	\$108,600,000	\$979,000,000	\$341,000,000				
Preferred Alternative	Yes/No	No	Yes	Yes	Yes				



Table 6.5 Proposed Reasona	able Alternatives Screening		
		No-Build	Preferred Alternative
	Geometric Deficiencies Resolved	0/30	26/30
	Provides Direct Access to/from I-526 (Yes/No)	Yes	Yes
Purpose & Need: 2050 Traffic Analysis	Provides Direct Access to/from I-26 (Yes/No)	Yes	No
	Weighted v/c Ratio	> 1.00	<1.00
	Intersection Delay/LOS	N/A	N Rhett/Virginia Ave, Refer to Table 6.4
	Mainline LOS	F	D/D/C/C/C/C/D
Freshwater Wetland Impact Based on R/W	(Acres)	0	97.7
Critical Area Impact Based on R/W	(Acres)	0	17.9
Critical Area (Ashley River) Bridge Construction Temporary Access Based on R/W	(Acres)	0	9.1
Pond Impact Based on R/W	(Acres)	0	0.03
Freshwater Stream Impact Based on R/W	(Feet)	0	18,631.7
Floodplains	(Acres)	0	950
	Residential	0	104
	Businesses	0	31
Relocations	Churches	0	1
	Community Facilitites	0	2
	Total	0	138
Environmental Justice	Yes/No	No	Yes (95)
Threatened & Endangered Species		0	May Effect, Not Likey to Adversely Affect
Essential Fish Habitat	Yes/No	No	Yes
Cultural Resources	Eligibility for Listing on NRHP	No Effect	No Effect
Section 4(f) & 6(f)	Yes/No	No	Yes
Utilities	\$	\$0	\$49.5 M
Cost	\$	\$0	\$1.43 B



References Cited

- 1 Corridor Analysis for I-526 Between North Charleston and West Ashley, SCDOT, October 2013.
- Congestion Management Process, Berkeley-Charleston-Dorchester Council of Governments, January
 2019



SHARED USE PATH EVALUATION FOR ASHLEY RIVER BRIDGE FINAL REPORT

Prepared for:



Prepared by:



October 5, 2020

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TABLE OF CONTENTS

1.0 Executive Summary	1-1
2.0 IDENTIFICATION of Alternatives	2-1
3.0 Selection of Evaluation Criteria	3-1
4.0 Alternatives Analysis	4-1
5.0 Evaluation Matrix for Alternatives	5-1

LIST OF TABLES

TABLE 5.1 Evaluation Matrix for Alternatives - Costs

TABLE 5.2 Evaluation Matrix for Alternatives – Other Evaluation Factors

LIST OF APPENDICES

APPENDIX A – Location Alternatives for the SUP on the Bridge Structures

APPENDIX B – Cross-Sections and Plan Views for SUP Location on Roadway Approaches

APPENDIX C – Support Documentation for Cost Summary



1.0 EXECUTIVE SUMMARY

The purpose of this study is to evaluate alternatives for incorporating a shared used path (SUP) into the planned widening of the I-526 Bridge over the Ashley River. This report supplements the study to evaluate the roadway widening alternatives for the I-526 Bridge Widening over the Ashley River.

The Ashley River Bridge is a dual structure with a separate bridge carrying eastbound (EBL) and westbound (WBL) traffic. The roadway design for the I-526 Low Country Corridor Project (LCC) involves adding two additional lanes in each direction to the bridge over the Ashley River. Public input resulting from the I-526 LCC Public Information Meeting held on November 21, 2019 and local stakeholder coordination has resulted in the need to add a 14-ft wide SUP for pedestrian and bicycle traffic to the corridor crossing the river. Stantec evaluated the roadway widening alternatives to determine which were suitable for the addition of an SUP, resulting in data being developed for seven viable alternatives to add an SUP to the Ashley River Bridge.

Impacts and costs are quantified in this report for each of the alternatives providing a framework for identifying Option 2A as the recommended alternative for inclusion in the Draft Environmental Impact Statement.



2.0 IDENTIFICATION of ALTERNATIVES

The identification of the alternatives for the SUP study utilizes alternatives developed during the roadway widening study. Alternatives considered for incorporating the SUP in an alternatives analysis are described below:

Alternatives Considered but Eliminated

- Widening each of the bridges toward the median only
 - This alternative would provide insufficient room to construct even the required roadway
 widening to the median, which would not meet the purpose and need of the project.
 Therefore, this alternative was not reasonable or feasible and was eliminated and is not
 considered for further SUP study.
- Widening each of the bridges in both directions
 - This alternative proved to be the costliest roadway alternative and resulted in greater environmental impacts. Therefore, this alternative was deemed not reasonable or feasible and eliminated and not considered for further SUP study.
- Widening each of the bridges to the outside only
 - This alternative results in the logical location for the SUP to be in the median between EBL and WBL traffic. Providing pedestrian access to the median on each end of the bridge would be difficult, involving switch-back ramping systems passing beneath one of the bridges. Locating pedestrians in the center of high-speed traffic raises safety concern about pedestrians being in the center of high-speed traffic, and their exposure to potentially high noise levels. It also results in the least aesthetically pleasing configuration for the SUP. The recent Ravenel Bridge in Charleston has an SUP located on the downstream side of the bridge, as public comment expressed a preference for the path to be located on the exterior of the bridge for unobscured views of the scenic vistas. For these reasons, this alternative was deemed not reasonable or feasible and eliminated and not considered for further SUP study.

Alternatives Considered for Further Study

Widening each of the bridges in upstream direction only

- This alternative allows for an SUP to be located on either the exterior (upstream) side of the WBL bridge or the exterior (downstream) side of the EBL bridge. This alternative was deemed reasonable and feasible and carried forward for further analysis..
- Widening each of the bridges in the downstream direction only
 - This alternative also allows for an SUP to be located on either the exterior (upstream) side of the WBL bridge or the exterior (downstream) side of the EBL bridge. This roadway alternative was deemed reasonable and feasible and carried forward for further analysis..

Alternatives Development

The two selected roadway alternatives result in the four alternatives for locating the SUP on the bridges, detailed in Appendix A:

- Option 1: Roadway widening performed on the downstream side of the EBL and WBL bridges with the SUP added to the upstream side of the WBL bridge.
- Option 2: Roadway widening performed on the upstream side of the EBL and WBL bridges with the SUP added to the upstream side of the WBL bridge.
- Option 3: Roadway widening performed on the downstream side of the EBL and WBL bridges with the SUP added to the downstream side of the EBL bridge.
- Option 4: Roadway widening performed on the upstream side of the EBL and WBL bridges with the SUP added to the downstream side of the EBL bridge.

Consideration of the location of the SUP on the roadway approaches expands each of these four options to two, resulting in a total of eight alternatives to advance the study and develop the evaluation matrix. For this study, the southern approach is the one on the Ashley Harbor side of the river and the northern approach is the one on the Marina side of the river:

- Option 1A: Option 1 bridge configuration with SUP approaches on the upstream (WBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.
- Option 1B: Option 1 bridge configuration with SUP approaches on the downstream (EBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.
- Option 2A: Option 2 bridge configuration with SUP approaches on the upstream (WBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.

- Option 2B: Option 2 bridge configuration with SUP approaches on the downstream (EBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.
- Option 3A: Option 3 bridge configuration with SUP approaches on the upstream (WBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.
- Option 3B: Option 3 bridge configuration with SUP approaches on the downstream (EBL) side of I-526 on the Ashley Harbor approach and upstream (WBL) side of the I-526 on the Marina approach.
- Option 4A: Option 4 bridge configuration with SUP approaches on the upstream (WBL) side of I-526 on the Ashley Harbor approach and downstream (EBL) side of the I-526 on the Marina approach.
- Option 4B: Option 4 bridge configuration with SUP approaches on the downstream (EBL) side of I-526 on the Ashley Harbor approach and downstream (EBL) side of the I-526 on the Marina approach.



3.0 SELECTION OF EVALUATION CRITERIA

The planned SUP crossing of the Ashley River is being accommodated as part of the I-526 Low Country Corridor Project within limits critical to SCDOT construction and maintenance operations. Outside of those limits, the SUP is being planned and built by other local governmental agencies as part of a more extensive SUP system within the Charleston area. The extent of SCDOT involvement is dictated by the need to provided ingress and egress points for a specific SCDOT inspection vehicle to enter the path and exit from it during bridge inspection operations. Access to this path is required for the inspection vehicle to enable bridge inspections to be conducted as needed. The width of the bridge, combined with the limitations of the vehicle, makes inspection without SUP access not feasible. For these reasons, the alternatives analysis has been developed using parameters within these SCDOT SUP construction limits. Any portion of the SUP outside of those limits will not have an undue influence on the selection of a preferred alternative, as impacts and construction costs for the SUP connecting to Ashley River Road or Leeds Avenue will be an order of magnitude less than those for the SCDOT section of the SUP and should be relatively the same for all alternatives.

The seven selected alternatives have been evaluated for the following design considerations:

- Critical Area Wetland Impacts
 - Each alternative impacts a different amount of wetland acreage within the evaluation area. The acreage of critical area impacts is quantified in the matrix.
- Rights-of-way impacts
 - In some cases, an SUP alternative requires additional right of way. These additional ROW areas are included in the matrix and a cost is shown calculated at \$325,000 per acre.
- Construction costs

 Bridge and roadway costs are included using unit prices consistent with those used for the LCC opinion of probable costs. While the amount of construction in terms of area of bridge and roadway is similar for all alternatives, some alternatives require three stages of construction rather than two, thus adding time and the amount of needed temporary

¹ WALK BIKE BCD, 2017 https://www.walkbikebcd.com/documents.html

trestle. Option 3B requires the construction of the free-standing SUP structure on the marina side of the river, adding cost.

Stakeholder concerns

o As access points vary between alternatives, there may be different levels of convenience for the public depending on the alternative chosen.

Maintenance concerns

Each alternative provides for maintenance access, but some alternatives involve more constraints than others.

Construction concerns

o Factors beyond just cost are noted for consideration in the evaluation

Analysis of these alternatives is provided inn Section 4 and the evaluation matrix is provided in Tables 5.1 and 5.2 in Section 5 of this report, populated with numerical values where applicable.



4.0 ALTERNATIVES ANALYSIS

Options 2B, and 4B locate the Ashley Harbor SUP approach on the downstream side of I-526. This places the SUP immediately adjacent to the Ashley Harbor community and would require additional right of way (ROW). The risk involved in obtaining ROW from Ashley Harbor property is significant. The cost could easily exceed the average ROW costs in Table 5.1 and cause significant delay to the project. In addition, other alternatives are equal or better across all other evaluation criteria. Therefore, neither of these alternatives were considered as the recommended alternative.

To enable an SUP configuration with an upstream approach on the Ashley Harbor side of the river and a downstream approach on the Marina side of the river (or conversely, a downstream approach on the Ashley Harbor side of the river and an upstream approach on the Marina side of the river), crossover points must be provided as part of the SUP alignment. For Options 1B and 4A an at-grade path beneath the second span of the bridge provides a crossover point parallel to Bull Creek. The elevation of the path beneath the bridge provides 8 feet of headroom and stays above the 100-year flood elevation of approximately Elev. 11.0 (NAVD88). However, these alternatives are two of the highest cost alternatives and remaining alternatives are equal or better across all other evaluation criteria. Option 4A has the highest critical area wetland impacts. Therefore, neither of these alternatives were considered as the recommended alternative.

Option 3B is a special case. Having the roadway widening on the downstream side of the EBL bridge, the limits of construction encroach within approximately 10 feet of the Marina property line. A retaining wall is necessary adjacent to the building already to maintain the 10-foot offset, considered the minimum for periodic maintenance access. There is no room to accommodate an additional 14 feet wide SUP exiting the bridge on that side. For this reason, a free-standing SUP structure is needed to direct the path users beneath the bridge before they infringe on the marina or adjacent boat slips so that they can exit on the upstream side on the marina side of the river. Appendix A contains details of this free-standing SUP structure. This option severely limits bridge inspection and maintenance operations, as the required inspection vehicle will not be able to traverse the full length of the bridge being unable to navigate the free-standing section of the SUP. It will have to reverse back along the SUP to exit. Also, the SUP section located beneath the bridge may impose additional restrictions on access

to the boat slips at the Marina. Therefore, this alternative was deemed not reasonable and was eliminated from additional analysis.

Option 3A is essentially the same as Option 1A, except the SUP passes underneath the bridge twice merely to accommodate the SUP on the downstream side of the EBL bridge. As the SUP cannot exit the bridge on the downstream side on the marina side of the river when the roadway widening occurs in that direction, Option 3A offers no advantage over Option 1A. Therefore, this alternative was deemed not reasonable and was eliminated from additional analysis.

Option 1A has the bridge widening for the roadway section on the downstream side of each of the bridges. This location brings the construction close to the Marina facility and to the Ashley Harbor community. A retaining wall is required adjacent to the Marina's boat storage building that will require future maintenance, which is a concern. Construction is more difficult and costly for Option 1A, as it requires three stages of construction, compared to only two stages for Option 2A. More ROW will be acquired from the Ashley Harbor community for Option 1A than for the remaining alternative, Option 2A. Option 2A does impact more critical area wetlands that Option 4A, but it is not the highest of all the alternatives and there are opportunities to explore minimization and avoidance strategies, if required.

From this analysis, Option 2A is the recommended alternative to adopt for the project.



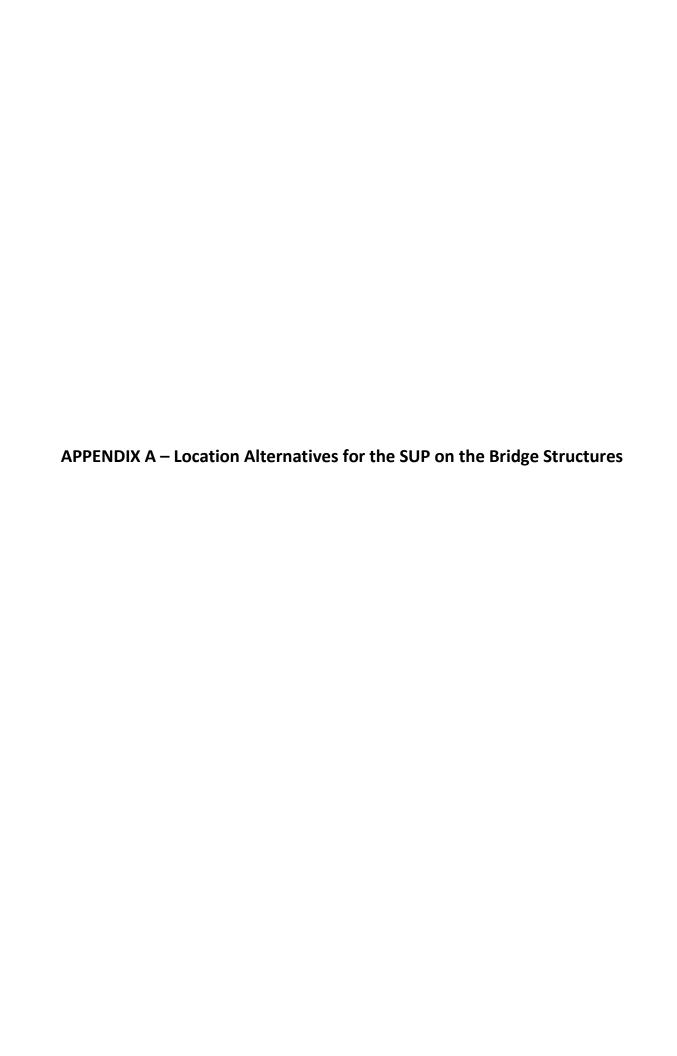
5.0 EVALUATION MATRIX FOR ALTERNATIVES

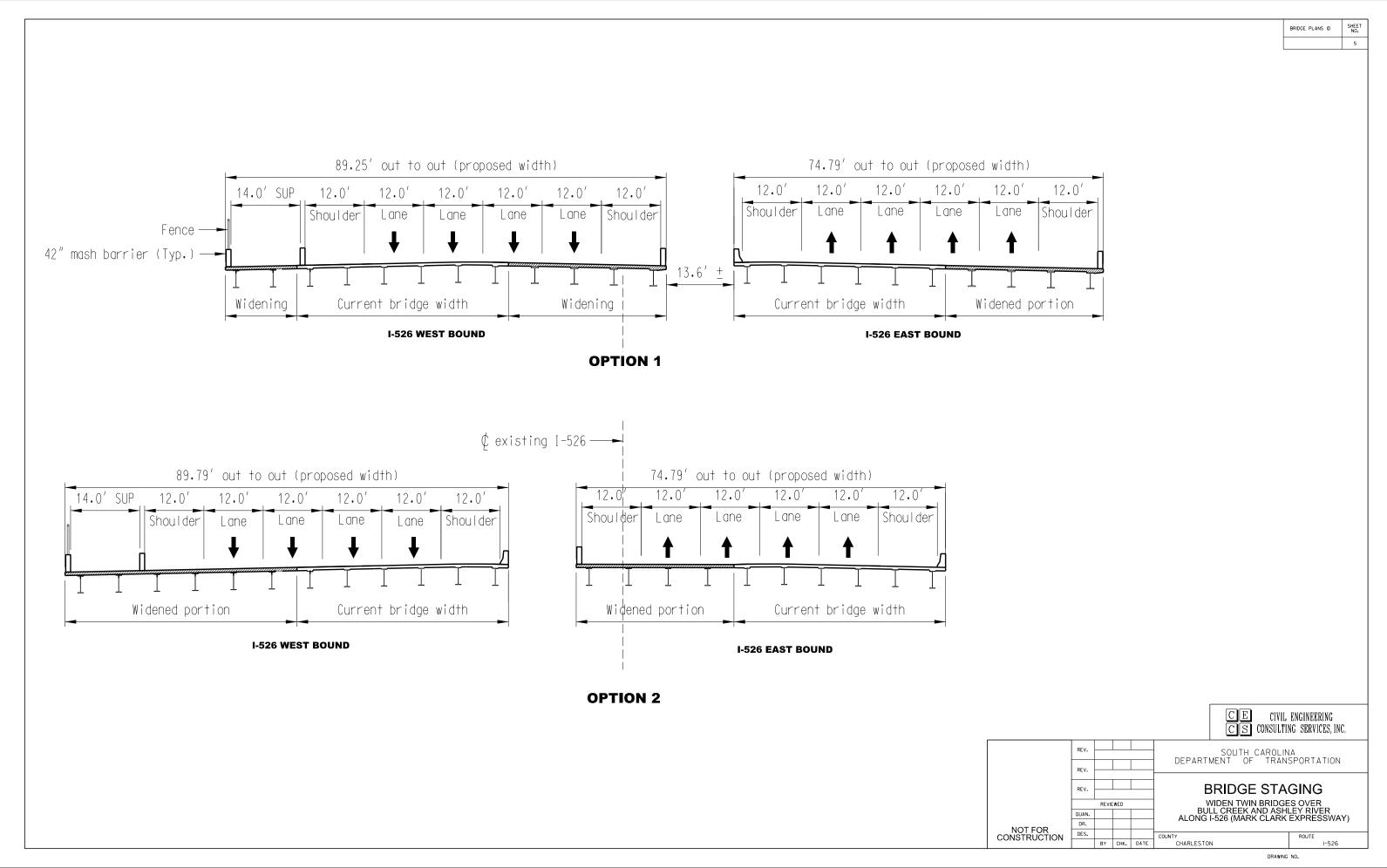
		I-526 ASHLEY RIVER B	RIDGE WIDENING ANI	D SUP OPTIONS PREL	IMINARY COST ESTIN	MATES			
Bridge Configuration	ОРТ	TION 1	ОРТ	TION 2	OPTIO	ON 3 *	OPTION 4		
Bridge Configuration	WIDEN DOWNSTRE	AM / SUP UPSTREAM	WIDEN UPSTREAM	WIDEN UPSTREAM / SUP UPSTREAM		M / SUP DOWNSTREAM	WIDEN UPSTREAM / SUP DOWNSTREAM		
Alternative	OPTION 1A	OPTION 1B	OPTION 2A	OPTION 2B	OPTION 3A	OPTION 3B	OPTION 4A	OPTION 4B	
Ashley Harbor Side Approach Path	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	
Marina Side Approach Path	UPSTREAM	UPSTREAM	UPSTREAM	UPSTREAM	UPSTREAM	UPSTREAM	DOWNSTREAM	DOWNSTREAM	
Needs Bull Creek Path?	No	Yes	No	Yes	Yes	No	Yes	No	
Needs Independent SUP Structure Near Marina?	No	No	No	No	Yes Yes		No	No	
Stages of Bridge Construction /Trestles	3	3	2	2	2 2		3	3	
Critical Area Wetland Impact (acre)	1.24	1.70	2.06	2.52	/		2.56	1.76	
ength of Wall to Mitigate Property Impacts (feet)	475	475	0	0			0	0	
Increased Anomoly Concern	No	No	No	No			No	No	
Regular ROW Takes (Acres)	12.0	12.5	12.7	13.2			13.8	13.4	
ROADWAY COSTS (INCLUDING BULL CREEK RAMP/PATH, WHEN NEEDED)	\$466,000.00	\$523,000.00	\$477,000.00	\$536,000.00			\$756,000.00	\$642,000.00	
BRIDGE CONSTRUCTION COSTS (WITHOUT SUP)	\$63,450,000.00	\$63,450,000.00	\$63,347,000.00	\$63,347,000.00			\$63,347,000.00	\$63,347,000.00	
BRIDGE CONSTRUCTION COSTS (INCLUDING SUP STRUCTURE, WHEN NEEDED)	\$79,995,000.00	\$79,995,000.00	\$72,908,000.00	\$72,908,000.00			\$79,893,000.00	\$79,893,000.00	
ENVIRONMENTAL MITIGATION COSTS	\$1,227,000.00	\$1,692,000.00	\$2,122,000.00	\$2,580,000.00			\$2,615,000.00	\$1,789,000.00	
RIGHT OF WAY COSTS	\$3,887,000.00	\$4,059,000.00	\$4,134,000.00	\$4,284,000.00			\$4,495,000.00	\$4,355,000.00	
TOTAL COST*	\$85,575,000.00	\$86,269,000.00	\$79,641,000.00	\$80,308,000.00	= /		\$87,759,000.00	\$86,679,000.00	

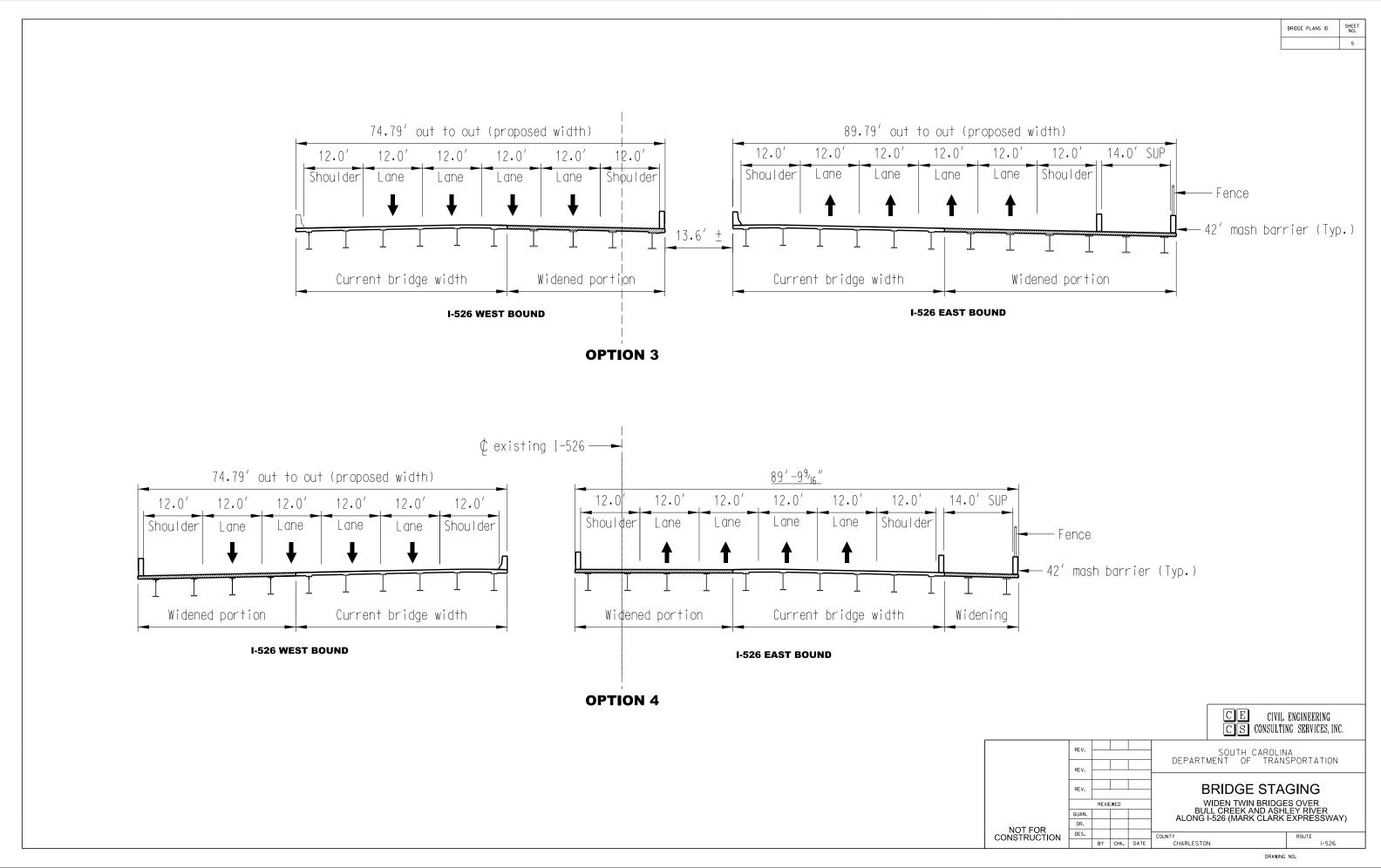
^{*} Option 3 eliminated from consideration. See discussion on Page 3-2

	Table 4.2 - I-526 ASHLEY RIVER BRIDGE WIDENING AND SUP OPTIONS EVALUATION FACTORS									
Bridge Configuration	OPTION 1 WIDEN DOWNSTREAM / SUP UPSTREAM		ОРТІО	DN 2	OPTI	ON 3	ОРТІС	N 4		
bridge configuration			WIDEN UPSTREAM / SUP UPSTREAM		WIDEN DOWNSTREAM	/ SUP DOWNSTREAM	WIDEN UPSTREAM / SUP DOWNSTREAM			
Alternative	OPTION 1A	OPTION 1B	OPTION 2A	OPTION 2B	OPTION 3A OPTION 3B		OPTION 4A	OPTION 4B		
STAKEHOLDER CONCERNS	Access advantage to stakeholders upstream from bridge	Access advantage to stakeholders downstream of bridge SUP located adjacent to Ashley Harbor community	Access advantage to stakeholders upstream from bridge	Access advantage to stakeholders downstream of bridge SUP located adjacent to Ashley Harbor community			Access advantage to stakeholders downstream of bridge SUP located adjacent to Ashley Harbor community	Access advantage to stakeholders downstream of bridge SUP located adjacent to Ashley Harbor community		
MAINTENANCE CONCERNS	Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Ensure access is provided for entrance and egress for inspection vehicle onto path	Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Ensure access is provided for entrance and egress for inspection vehicle onto path	Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Ensure access is provided for entrance and egress for inspection vehicle onto path	Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Access is provided for entrance and egress for inspection vehicle onto path			Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Ensure access is provided for entrance and egress for inspection vehicle onto path	Ensure walkway is designed to carry inspection vehicle Ensure exterior fence is 8 feet high, max. Ensure access is provided for entrance and egress for inspection vehicle onto path		
CONSTRUCTION CONCERNS	Requires three stages of construction	Requires three stages of construction Requires Bull Creek path		Requires Bull Creek path			Requires three stages of construction Requires Bull Creek path	Requires three stages of construction		

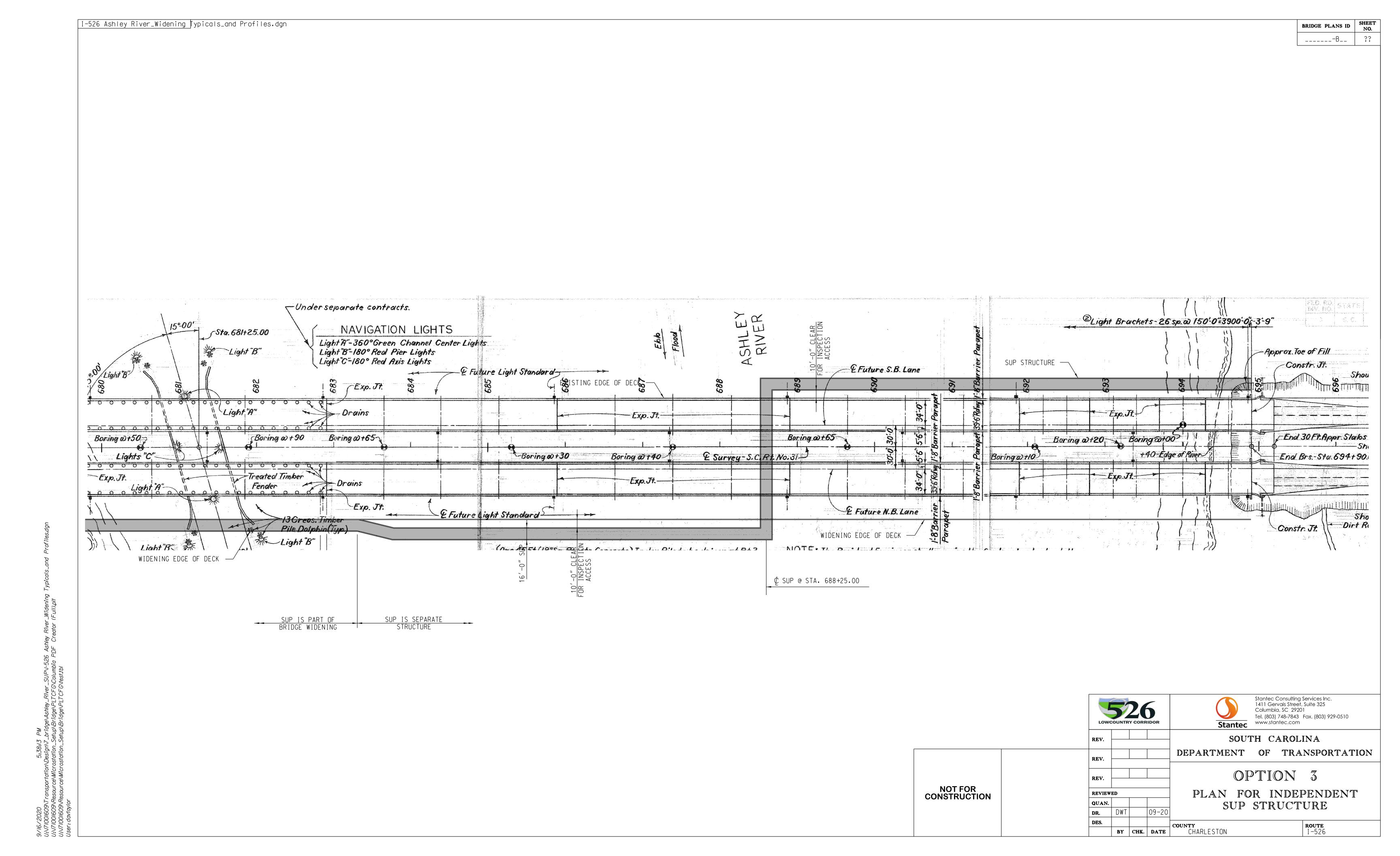
^{*} Option 3 eliminated from consideration. See discussion on Page 3-2







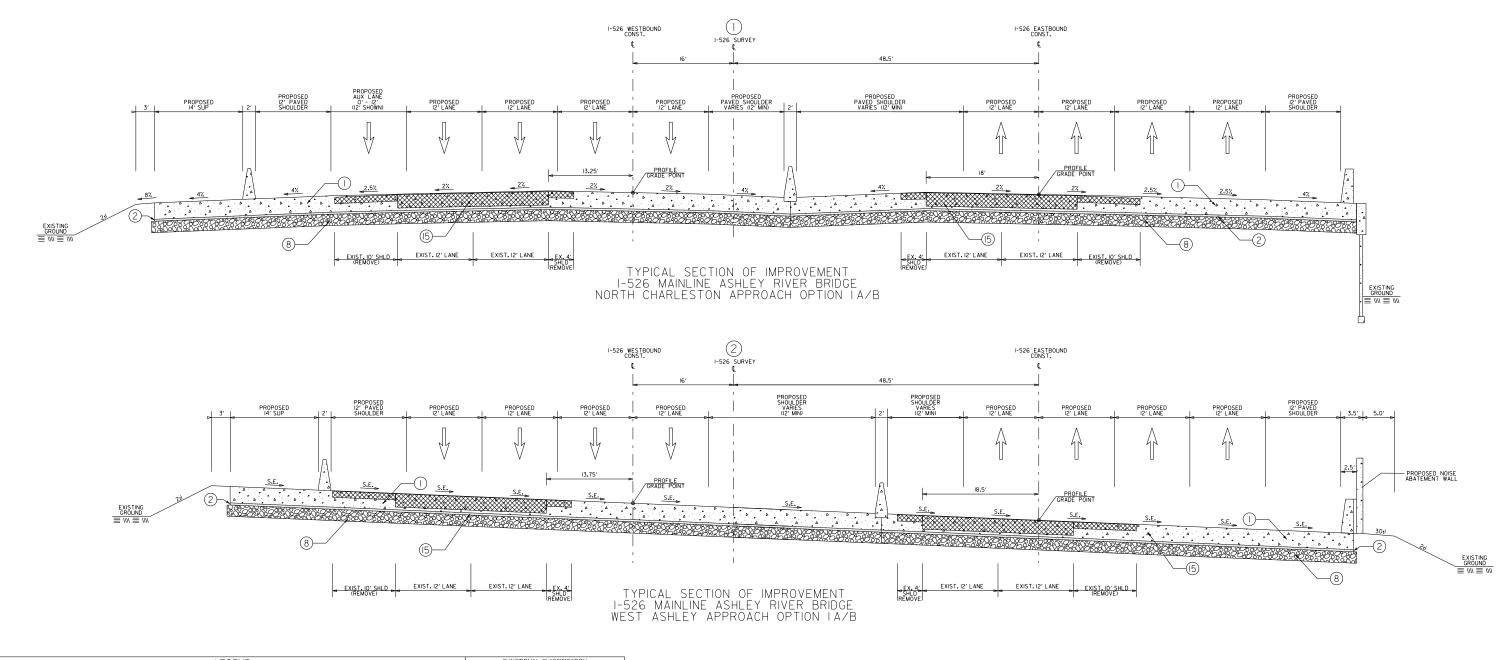
I-526 Ashley River_Widening Typicals_and Profiles.dgn _____B__ Fin.Grade F.G.EI _P.T.Sta.688+90.00 EP. C-Sta. 691+15.00 /-P.I.-Stal 69 F.G.El.31.395 FG.E1.23.520 3.50% E1.12.058(Fc $'
\leftarrow R.C.Pier \longrightarrow$ /Fin.Grade 35' VERTICAL CLEARAN SE 60 Hor. CI. 12 Channel 655 V.C APPRON. ELEVATION (NAVD88) Top of Fenders-El.+12.75 H.Tide-E1.+2.75-Approx. 0.6% 682+00 Timber -20'Sq.Prestr.Concrete Piles i Stantec Consulting Services Inc. 1411 Gervais Street, Suite 325 Columbia, SC 29201 Tel. (803) 748-7843 Fax. (803) 929-0510 www.stantec.com SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION OPTION 3 NOT FOR CONSTRUCTION ELEVATION FOR INDEPENDENT REVIEWED QUAN. SUP STRUCTURE 09-20 DR. DWT COUNTY CHARLESTON **ROUTE** 1-526 BY CHK. DATE



APPENDIX B – Cross-Sections for SUP Location on Roadway Approaches

TYPICAL SECTION OF IMPROVEMENT SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION COLUMBIA, S.C.

3 S.C. CHARLESTON P027507



FUNCTIONAL CLASSIFICATION MILL 1.5" TO REMOVE OGFC 12" PCC PAVEMENT HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "C" (175 lbs/sy) (I) (II) MILL 2" HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "B" (200 lbs/sy) 8% FULL-DEPTH PATCHING, DIAMOND GRINDING, AND JOINT SEALING LEVELING - HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "A" (VAR) (13) RETAIN EXISTING PAVEMENT NEW IO' WIDE 12" ASPHALT SHOULDER FOR TRAFFIC CONTROL, TO REPLACE 8" GABC HOT MIX ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE "B" (200lbs/sy) DESIGN SPEED ROUTE MPH 6 HOT MIX ASPHALT CONCRETE BASE COURSE, TYPE "A" (450 lbs/sy) (15) EXISTING PAVEMENT REMOVAL HOT MIX ASPHALT CONCRETE BASE COURSE, TYPE "A" (900 lbs/sy) (8) NEW 8" GABC BASE COURSE. EXCEPTIONS TO DESIGN SPEED NEW 10" GABC BASE COURSE.

PLANS PREPARED BY:) Stantec Stantec Consulting Services 4969 Centre Pointe Drive, Suite 200

North Charleston, SC 29418 Tel: 843.740.7700 Fax: 843.740.7707 www.stantec.com

3 2 REV. NO. DATE DESCRIPTION OF REVISION TOPO. DWG. DATE GROUP _

SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

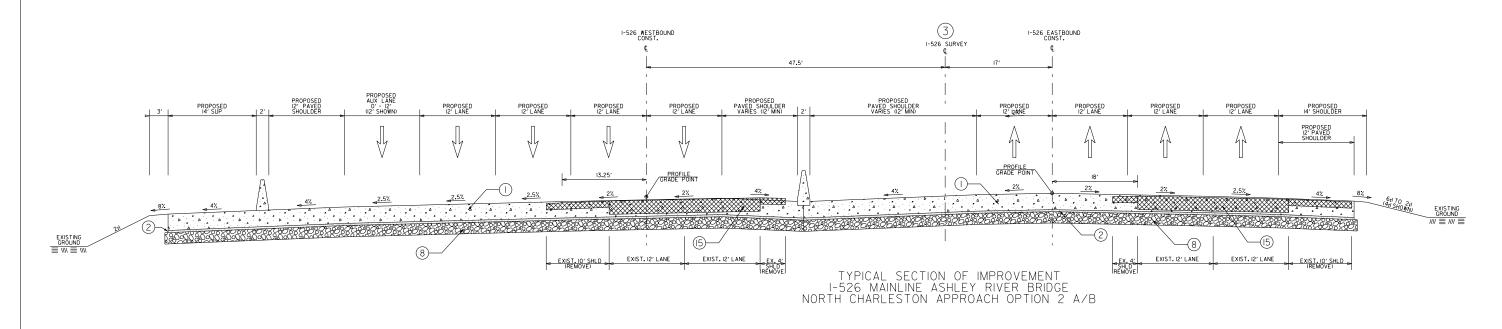
I-526 LOWCOUNTRY CORRIDOR WEST

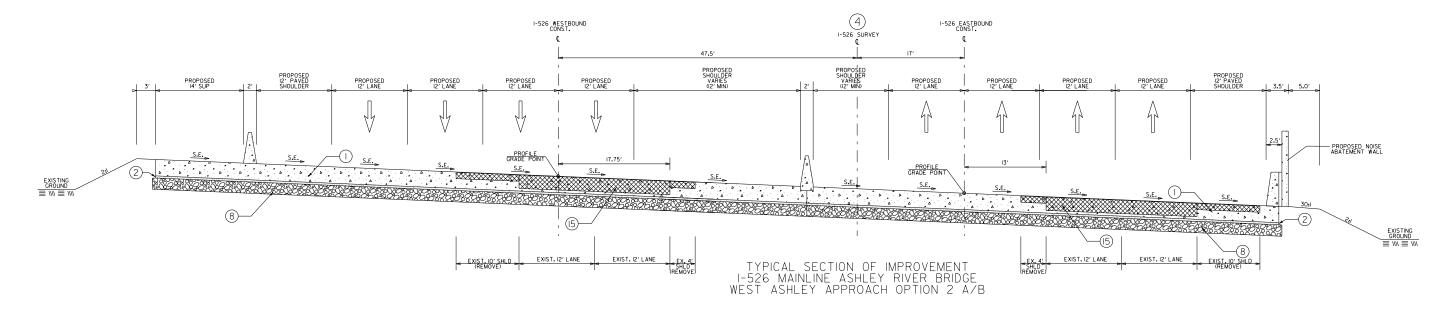
TYPICAL SECTIONS

SCALE I"V=

TYPICAL SECTION OF IMPROVEMENT SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION COLUMBIA, S.C.

3 S.C. CHARLESTON P027507





		LEG	END		FUNCTIONA	L CLASSIFICAT	TION	
		12" PCC PAVEMENT		MILL 1.5" TO REMOVE OGFC				
2	\bigcirc	HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "C" (175 lbs/sy)		MILL 2"				
3		HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "B" (200 lbs/sy)		8% FULL-DEPTH PATCHING, DIAMOND GRINDING, AND JOINT SEALING				
4		LEVELING - HOT MIX ASPHALT CONCRETE SURFACE COURSE, TYPE "A" (VAR)	③ ∅	RETAIN EXISTING PAVEMENT				
(5)	\bigcirc	HOT MIX ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE "B" (2001bs/sy)	(4) (NEW 10' WIDE 12'' ASPHALT SHOULDER FOR TRAFFIC CONTROL, TO REPLACE 8'' GABC		IGN SPEED		PLANS PREPARED E
6		HOT MIX ASPHALT CONCRETE BASE COURSE, TYPE "A" (450 lbs/sy)	(5)		ROUTE		MPH	() Stante
7		HOT MIX ASPHALT CONCRETE BASE COURSE, TYPE "A" (900 lbs/sy)						Stantec Consultin 4969 Centre Poin
8		NEW 8" GABC BASE COURSE,			EXCEPTIONS	TO DESIGN S	SPEED	North Charleston, Tel: 843.740.7700
(9)		NEW 10" GABC BASE COURSE.						Fax: 843.740.7707

PREPARED BY: N Stantec antec Consulting Services 69 Centre Pointe Drive, Suite 200 orth Charleston, SC 29418 l: 843.740.7700 www.stantec.com



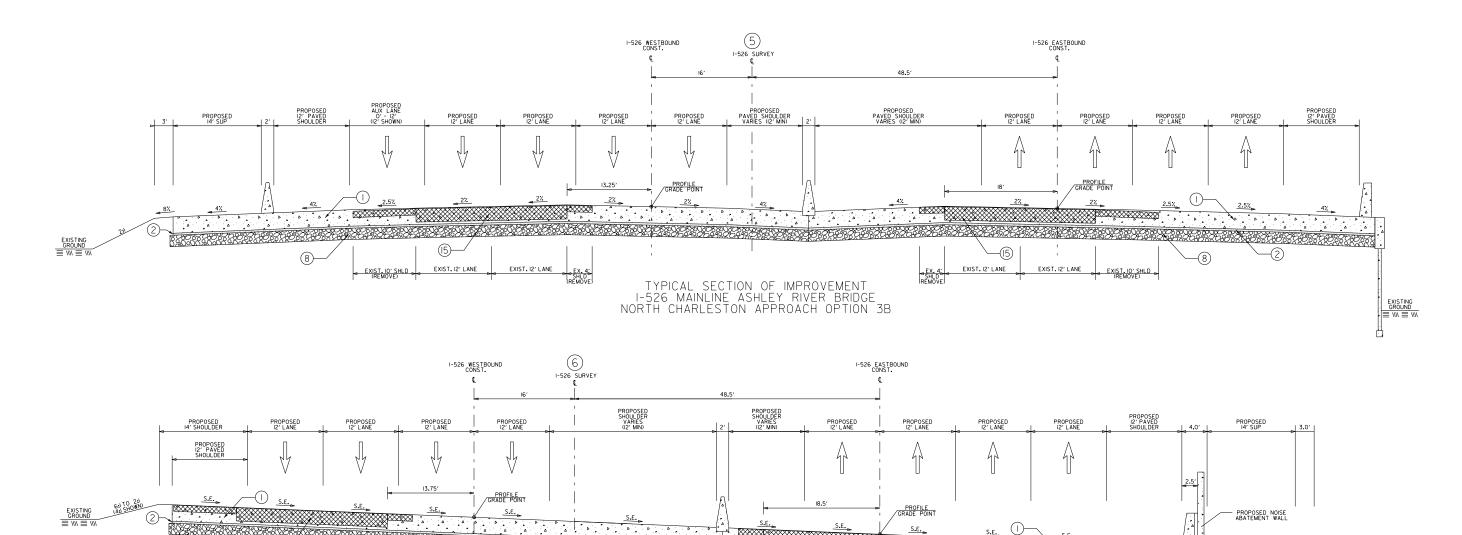
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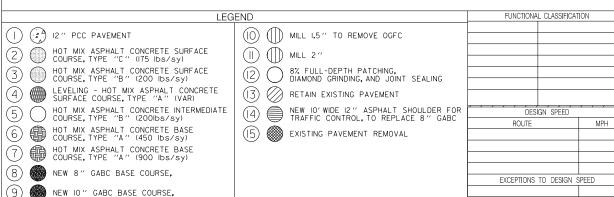
SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-526 LOWCOUNTRY CORRIDOR WEST

TYPICAL SECTIONS

TYPICAL SECTION OF IMPROVEMENT SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION COLUMBIA, S.C.





EXIST. 12' LANE

EXIST. 12' LANE

TYPICAL SECTION OF IMPROVEMENT I-526 MAINLINE ASHLEY RIVER BRIDGE WEST ASHLEY APPROACH OPTION 3B

> PLANS PREPARED BY:) Stantec

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EXIST. 12' LANE

EXIST. 12' LANE

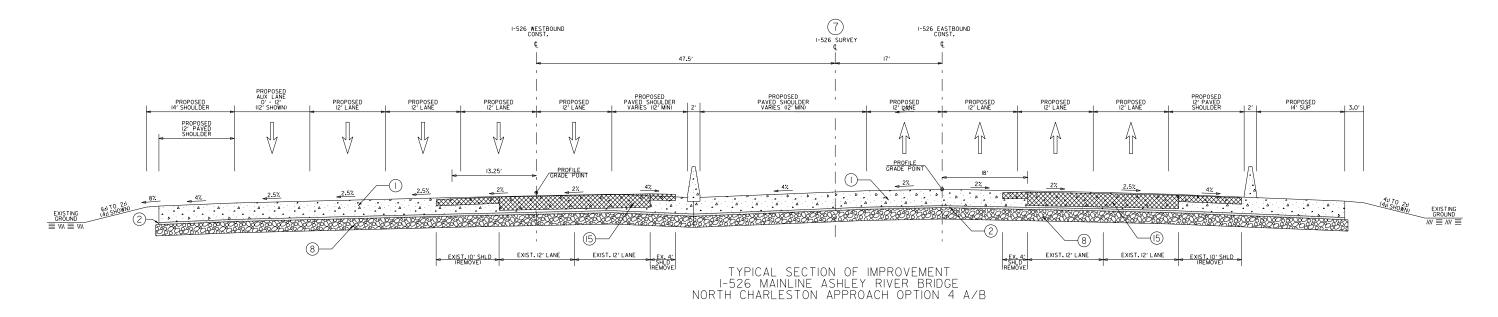
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DWG	DATE		GROUP
D ///	0.175		

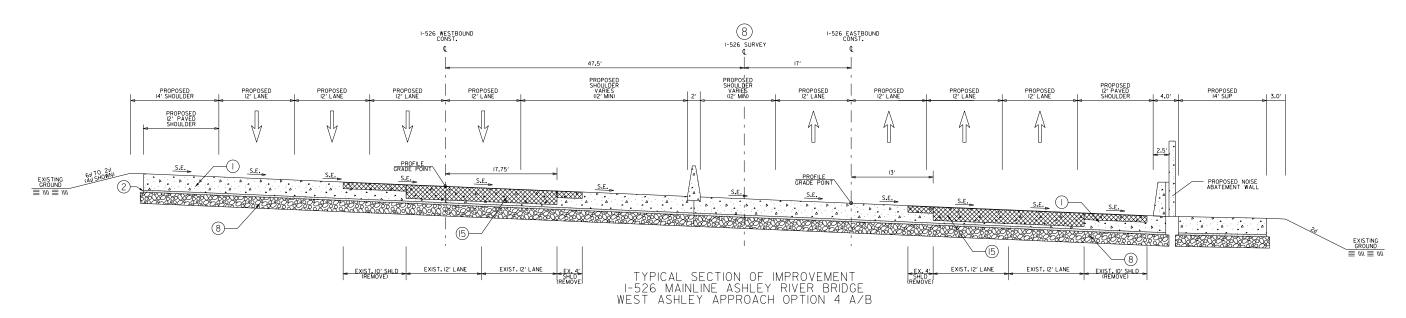
SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-526 LOWCOUNTRY CORRIDOR WEST

TYPICAL SECTIONS

TYPICAL SECTION OF IMPROVEMENT SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION COLUMBIA, S.C.





PLANS PREPARED BY:

Tel: 843.740.7700

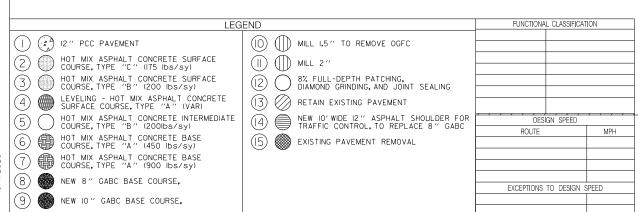
Fax: 843.740.7707

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) Stantec

Stantec Consulting Services 4969 Centre Pointe Drive, Suite 200

North Charleston, SC 29418



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SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

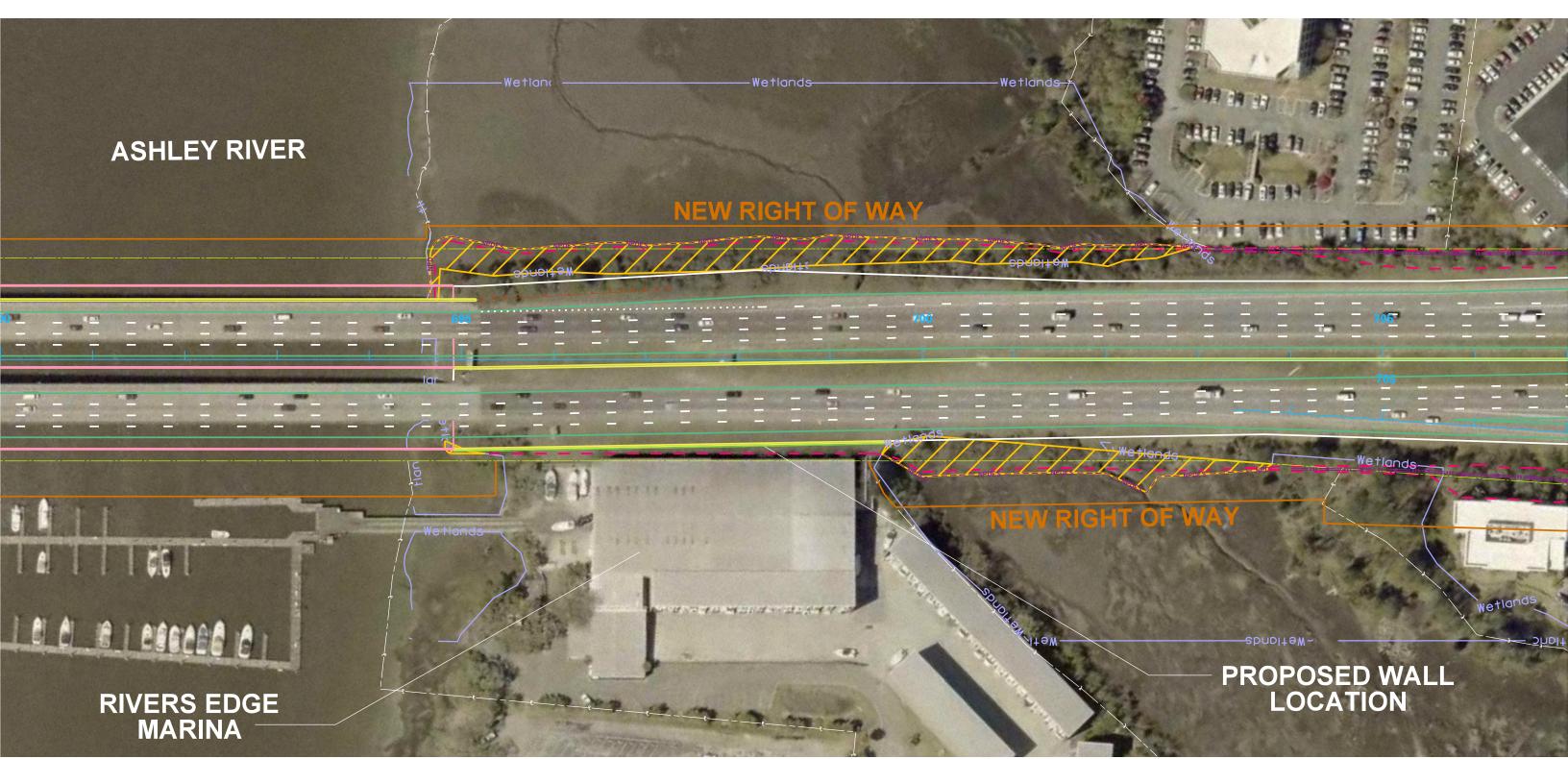
I-526 LOWCOUNTRY CORRIDOR WEST

TYPICAL SECTIONS

SCALE I'V= SCALE I'H= RTE./RD.

I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 1A

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
NORTH CHARLESTON APPROACH

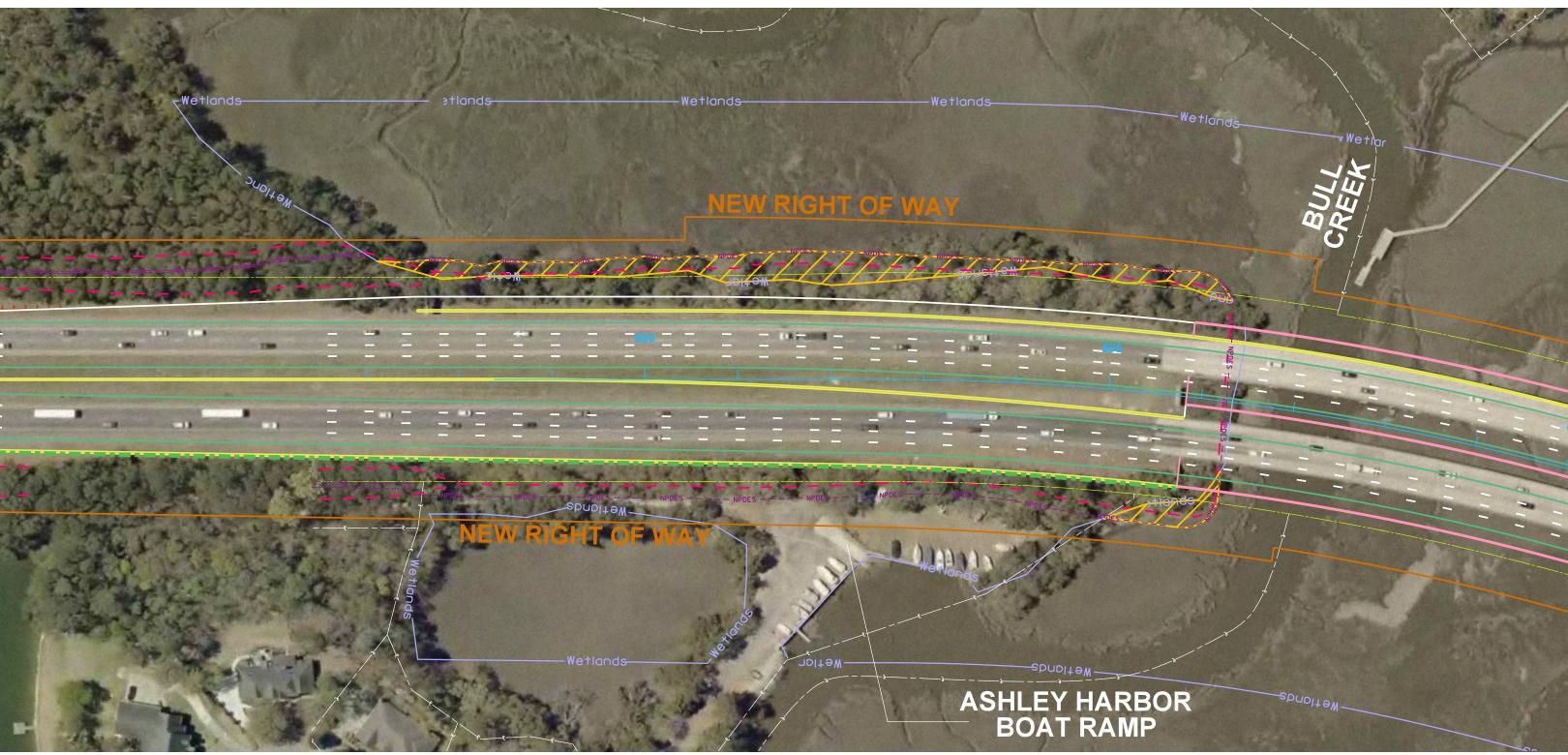






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 1A

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST) WEST ASHLEY APPROACH

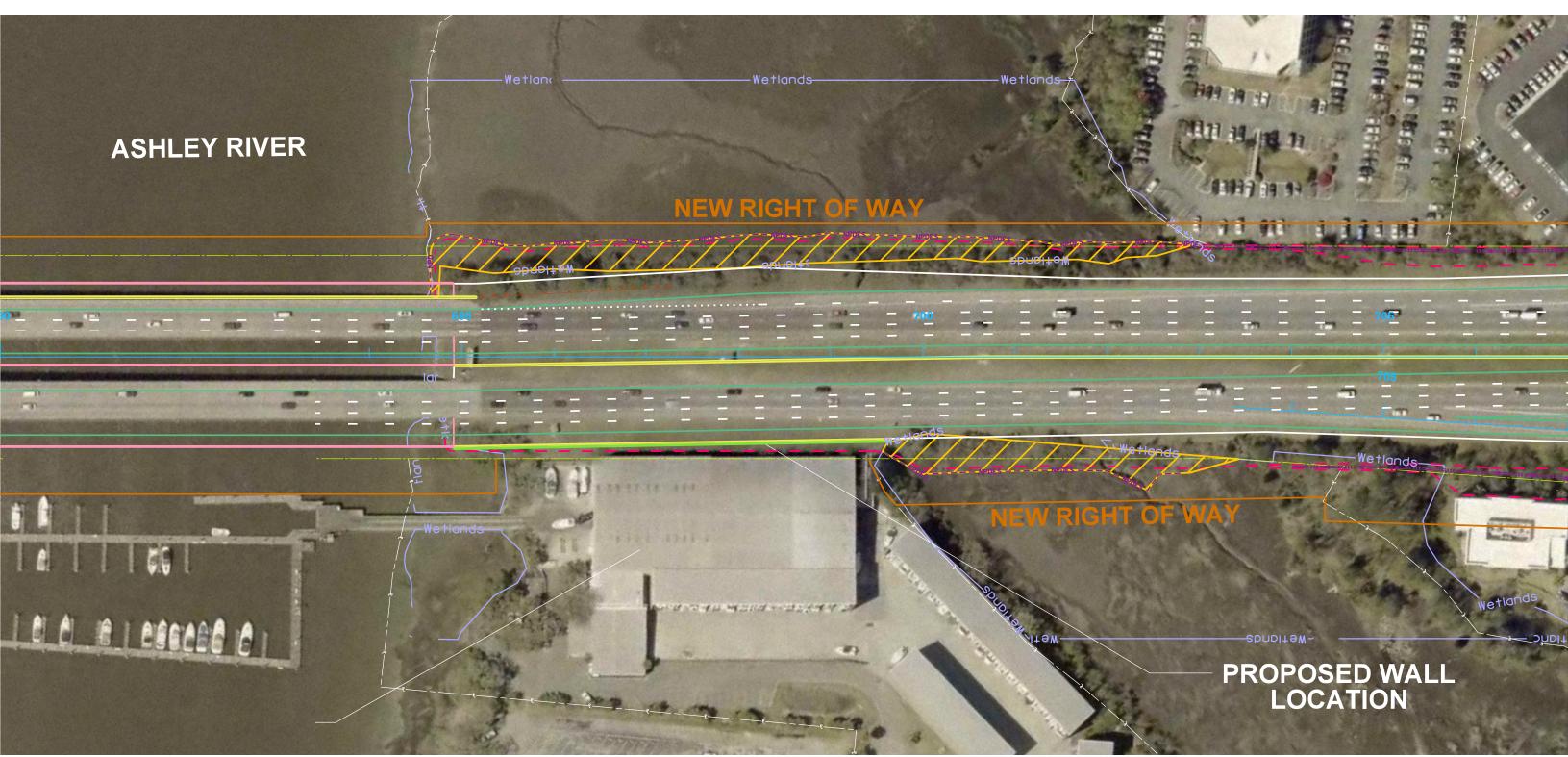






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 1B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
NORTH CHARLESTON APPROACH

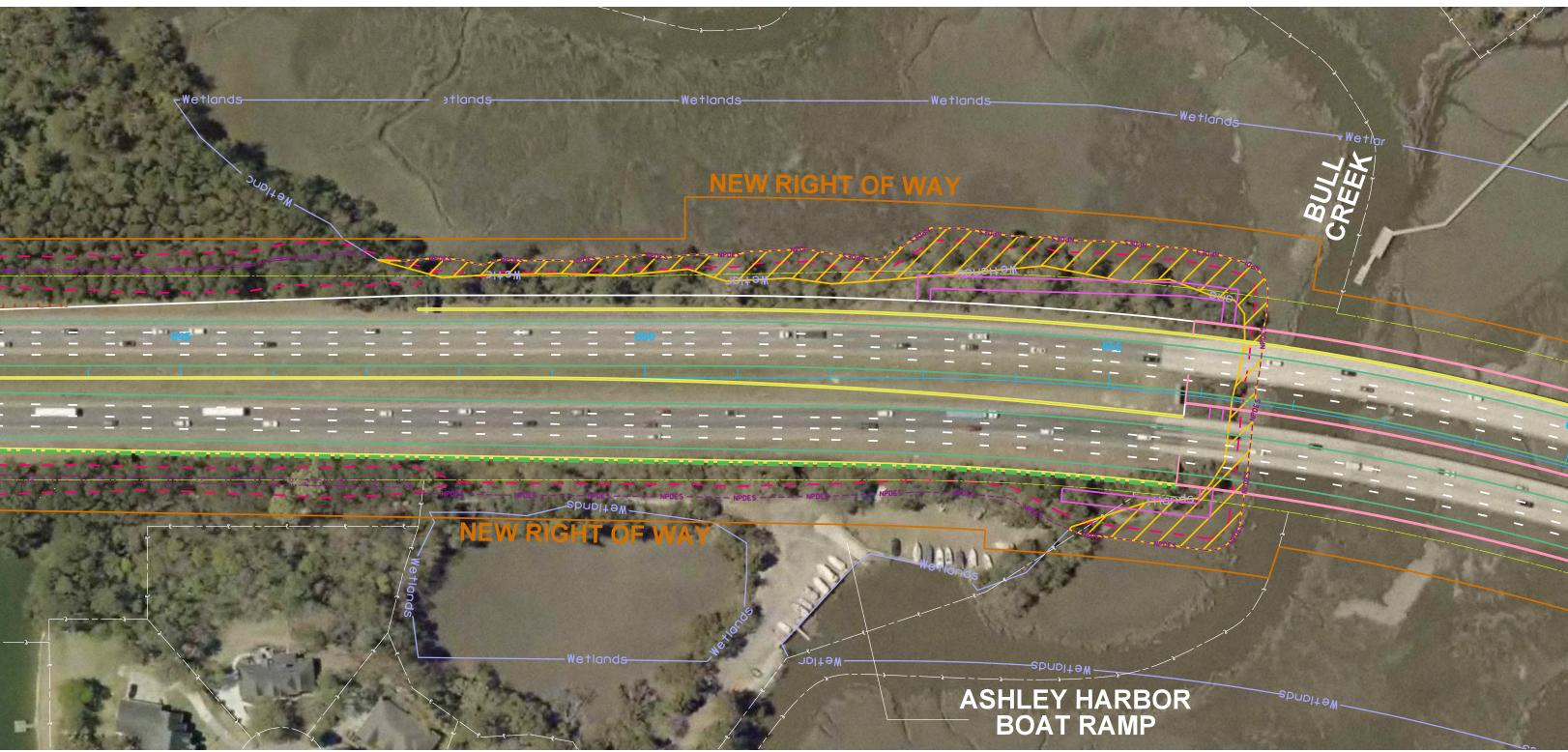






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 1B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
WEST ASHLEY APPROACH

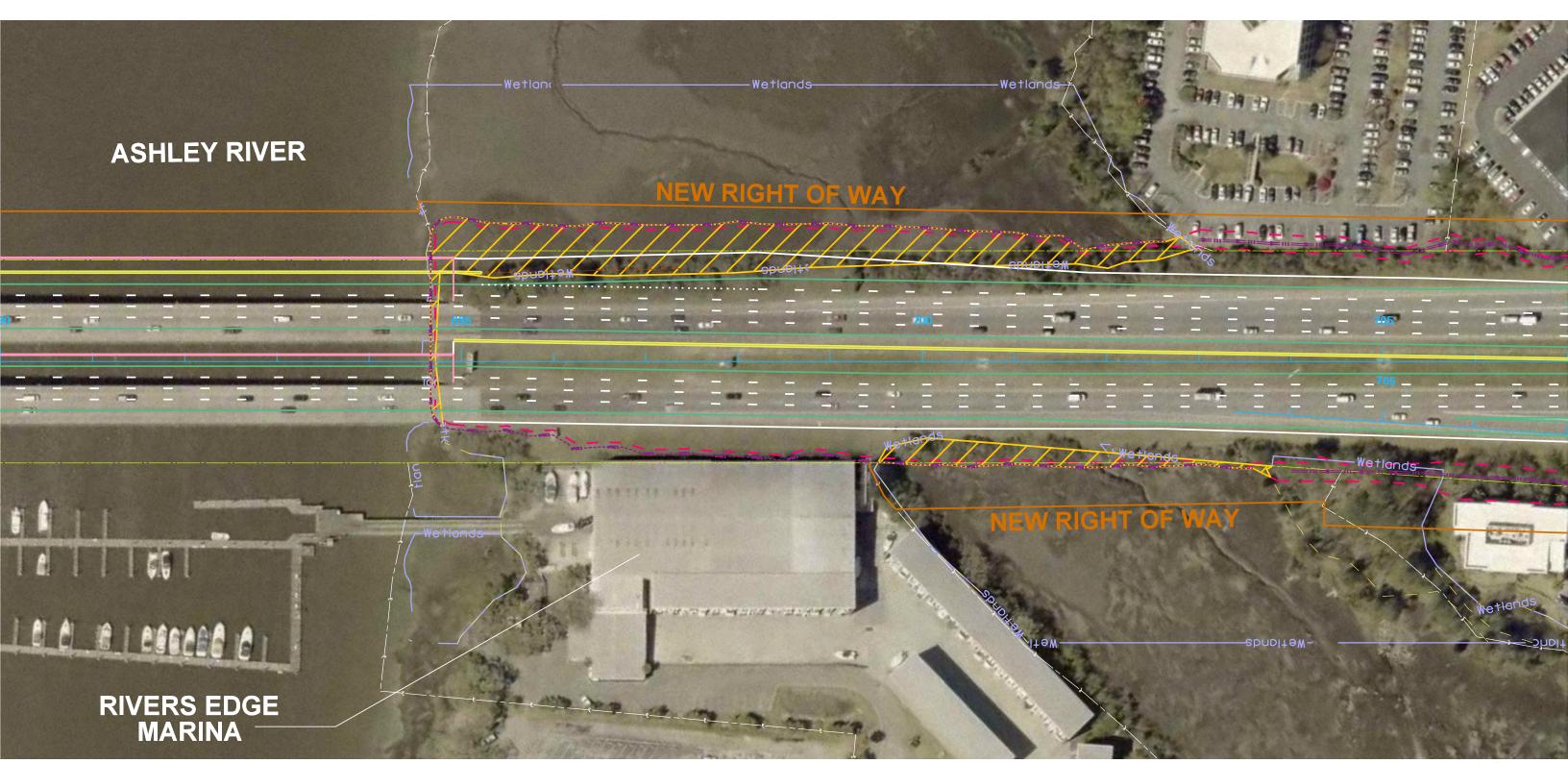






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 2A

WIDEN BRIDGES UPSTREAM (WEST) / SUP UPSTREAM (WEST) NORTH CHARLESTON APPROACH

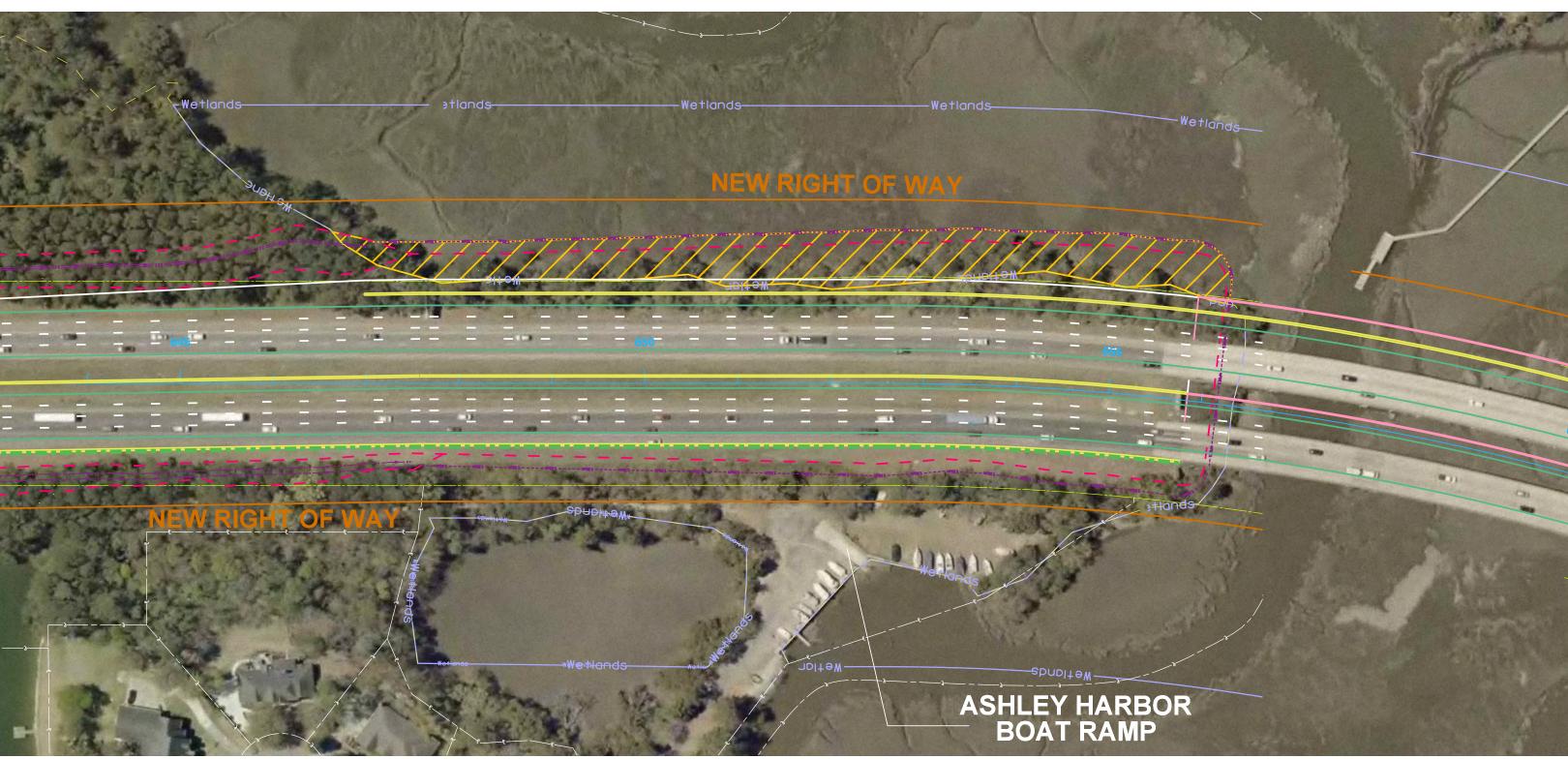






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 2A

WIDEN BRIDGES UPSTREAM (WEST) / SUP UPSTREAM (WEST) WEST ASHLEY APPROACH

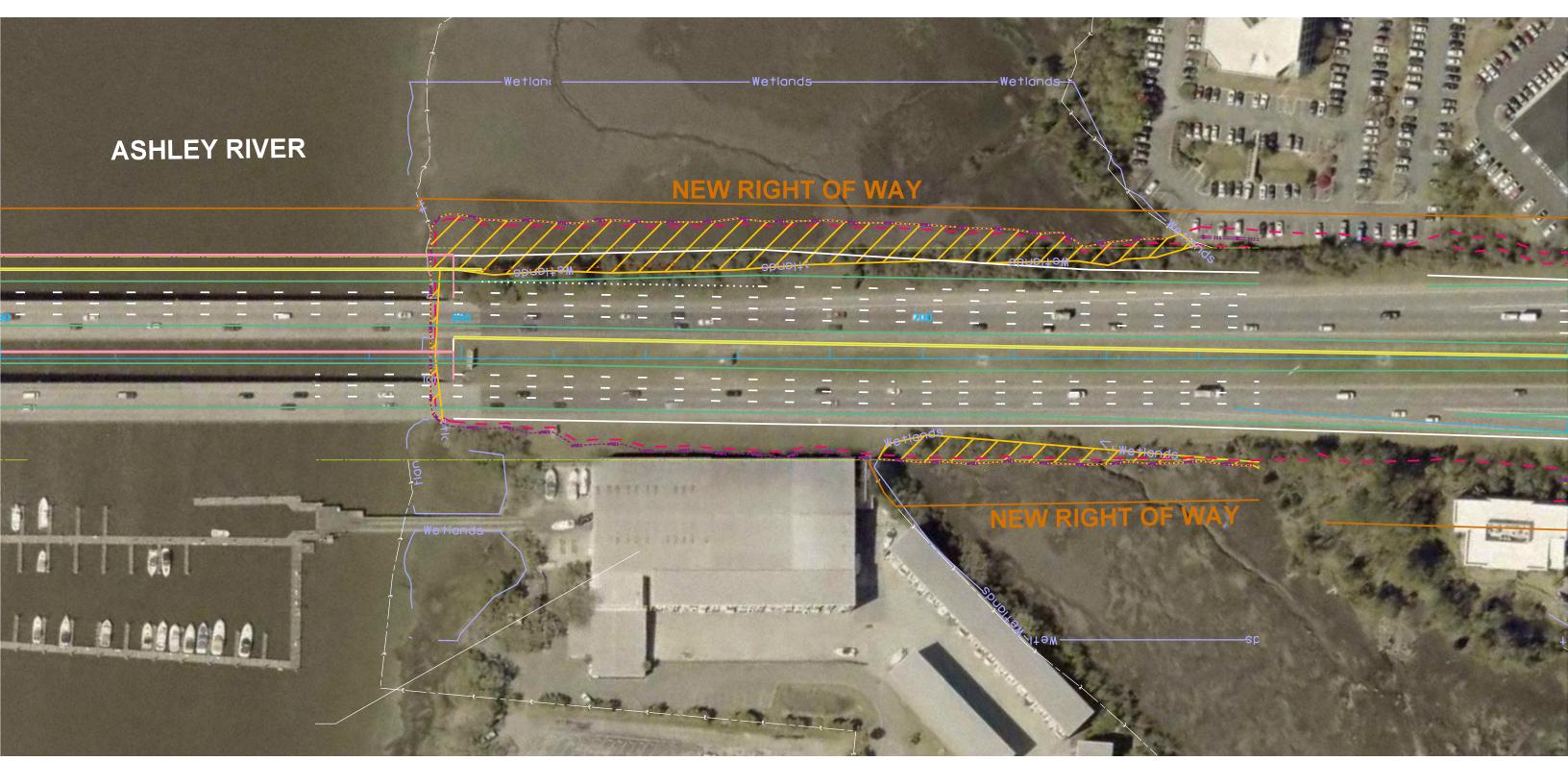






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 2B

WIDEN BRIDGES UPSTREAM (WEST) / SUP UPSTREAM (WEST) NORTH CHARLESTON APPROACH

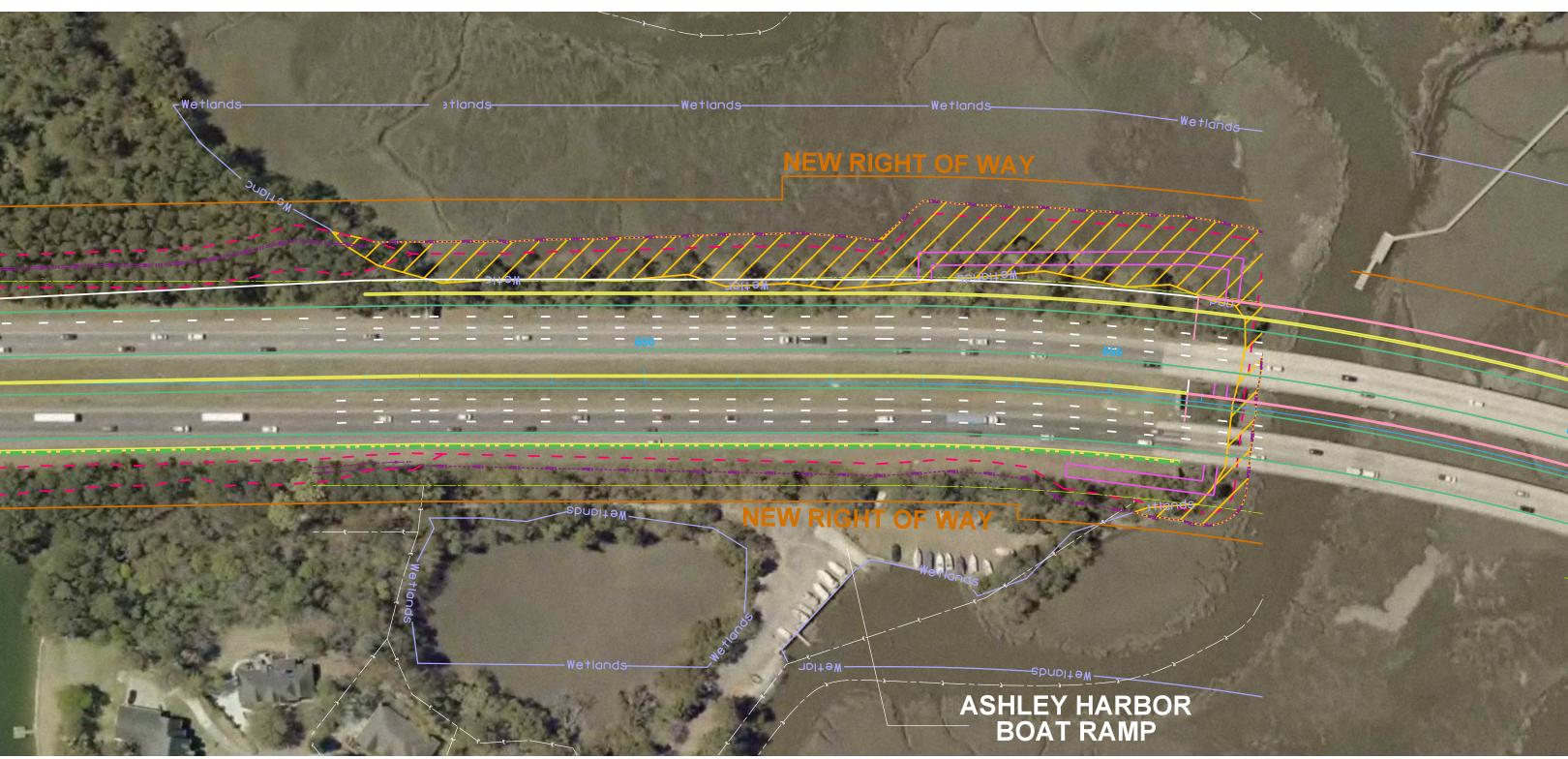






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 2B

WIDEN BRIDGES UPSTREAM (WEST) / SUP UPSTREAM (WEST) WEST ASHLEY APPROACH

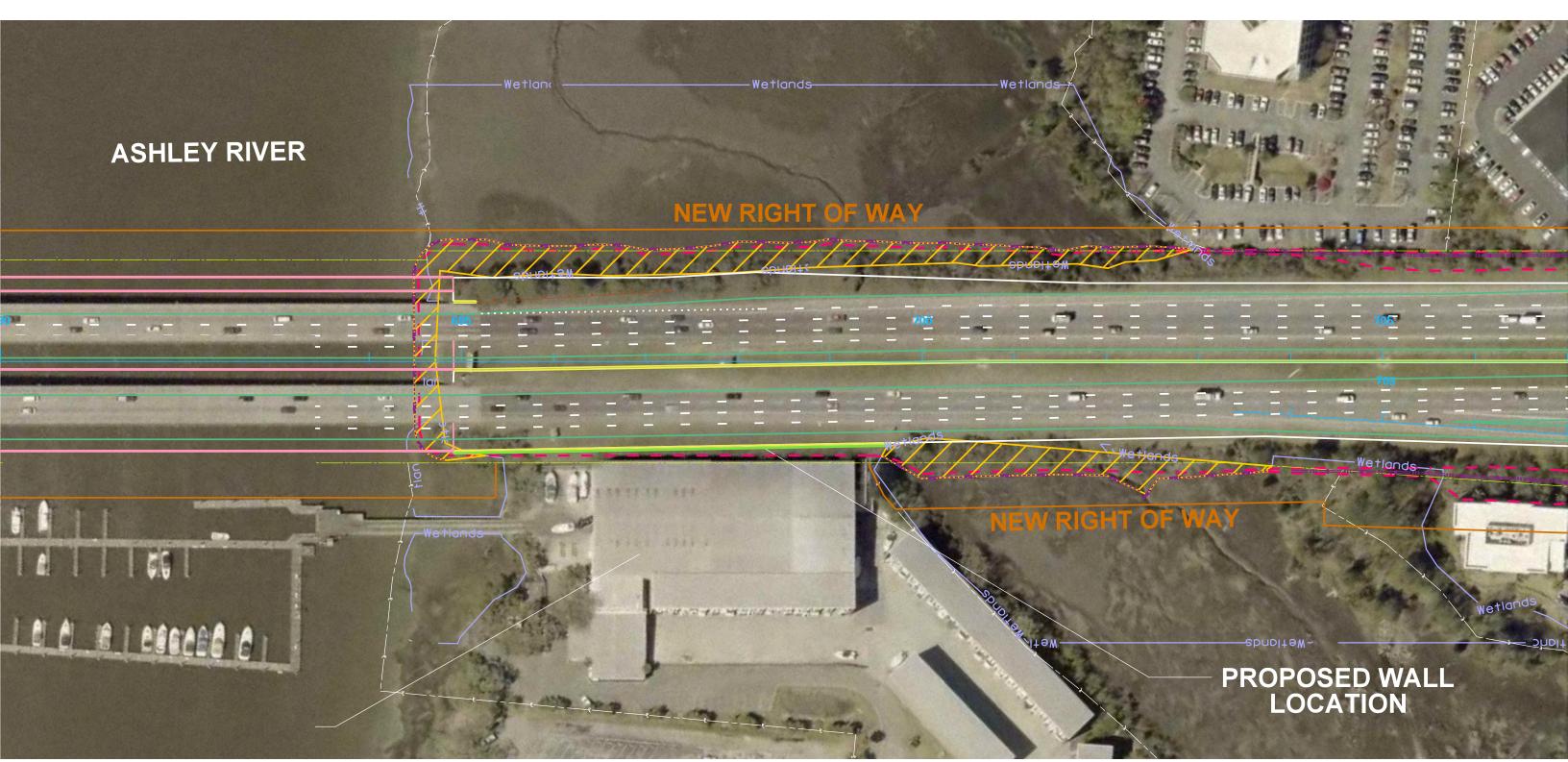






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 3B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP DOWNSTREAM (EAST) NORTH CHARLESTON APPROACH

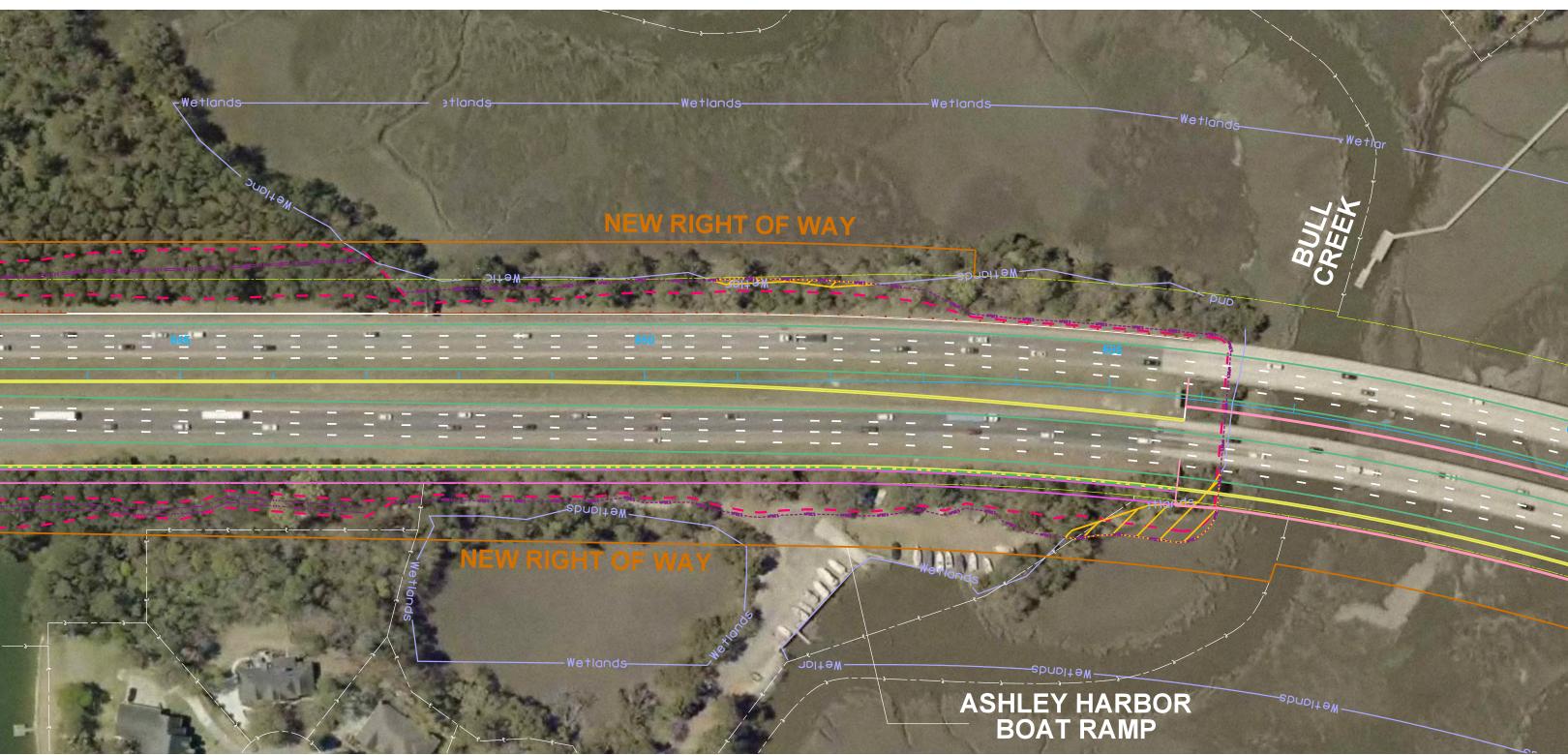






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 3B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP DOWNSTREAM (EAST) WEST ASHLEY APPROACH

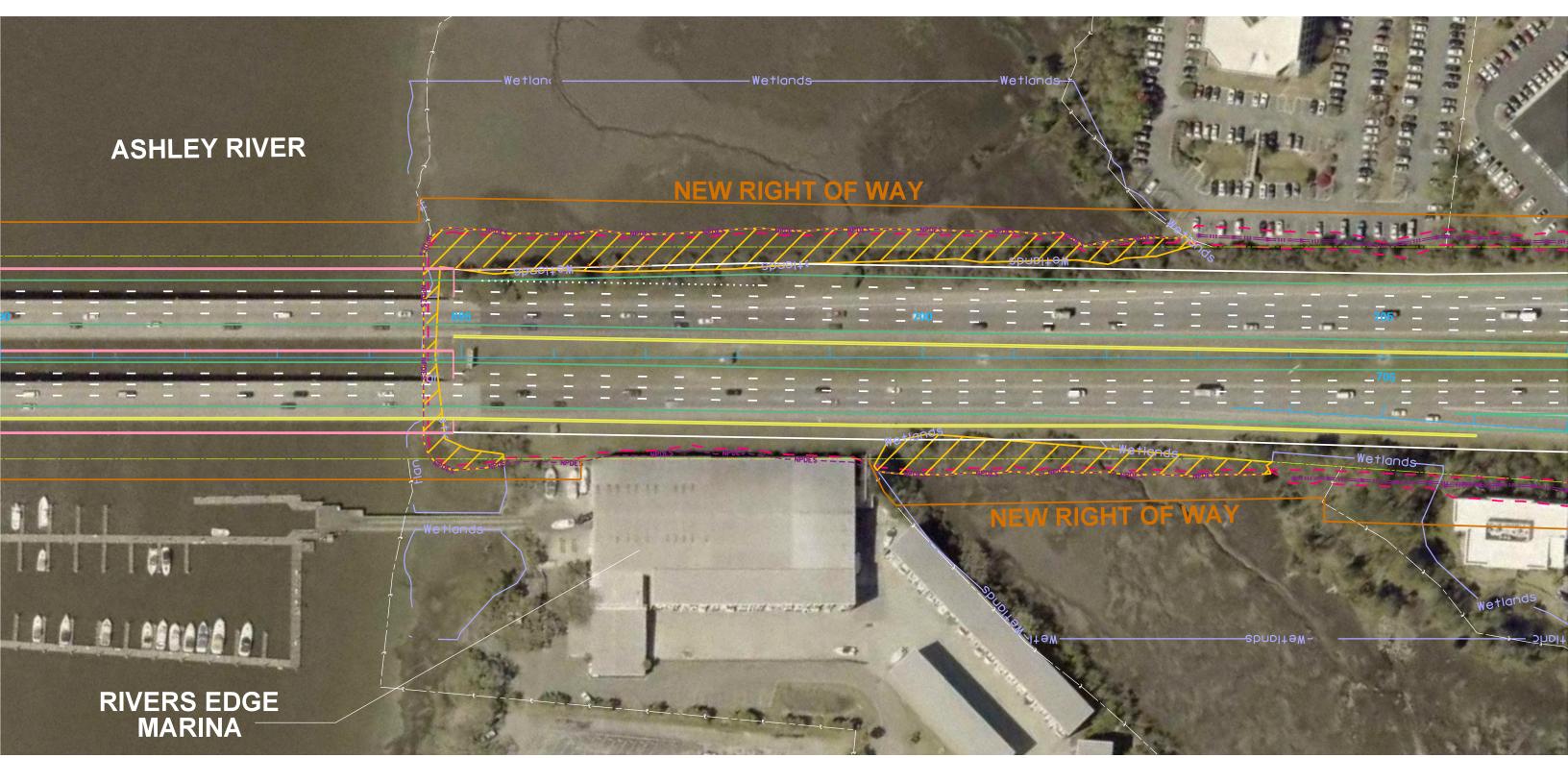






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 4A

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
NORTH CHARLESTON APPROACH

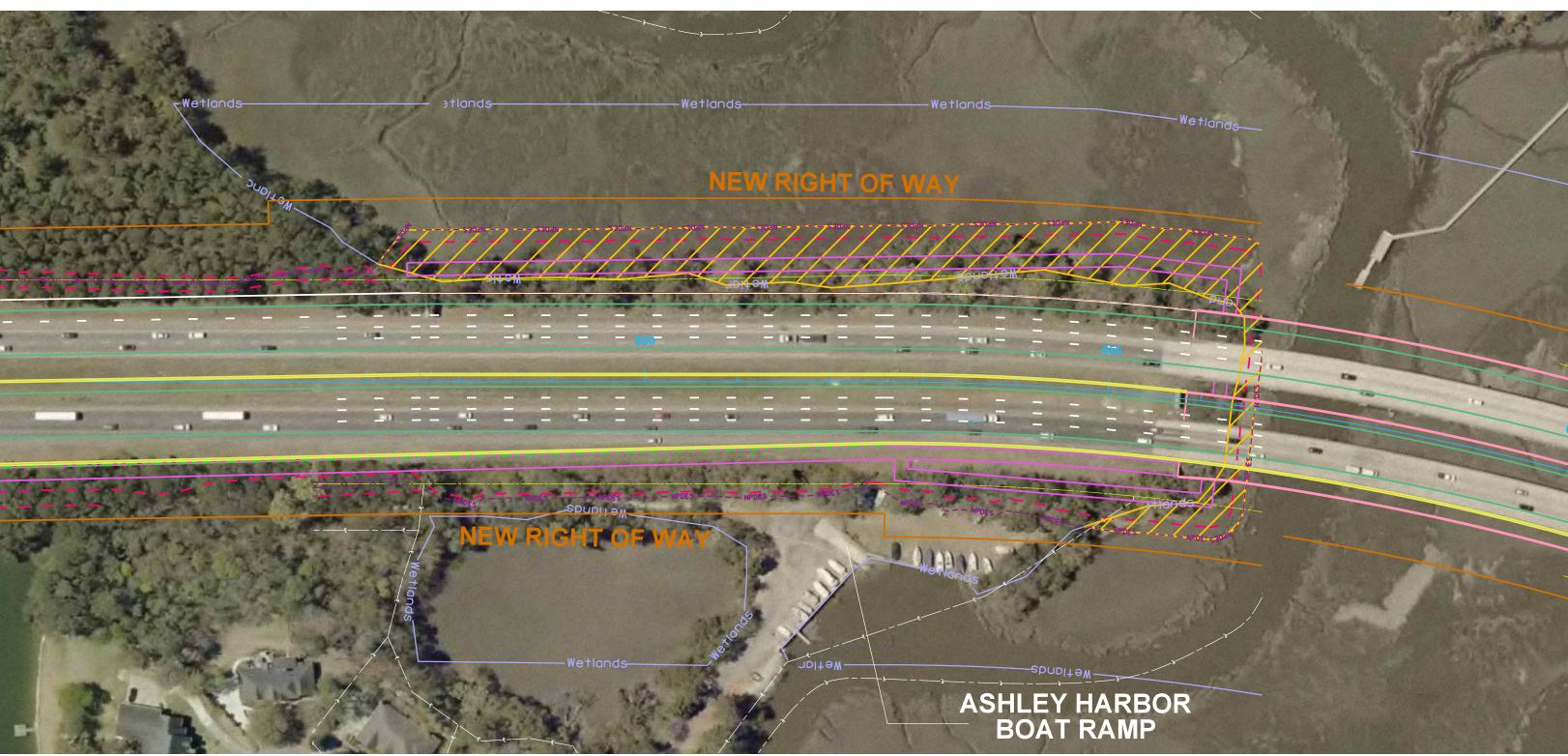






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 4A

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST) WEST ASHLEY APPROACH

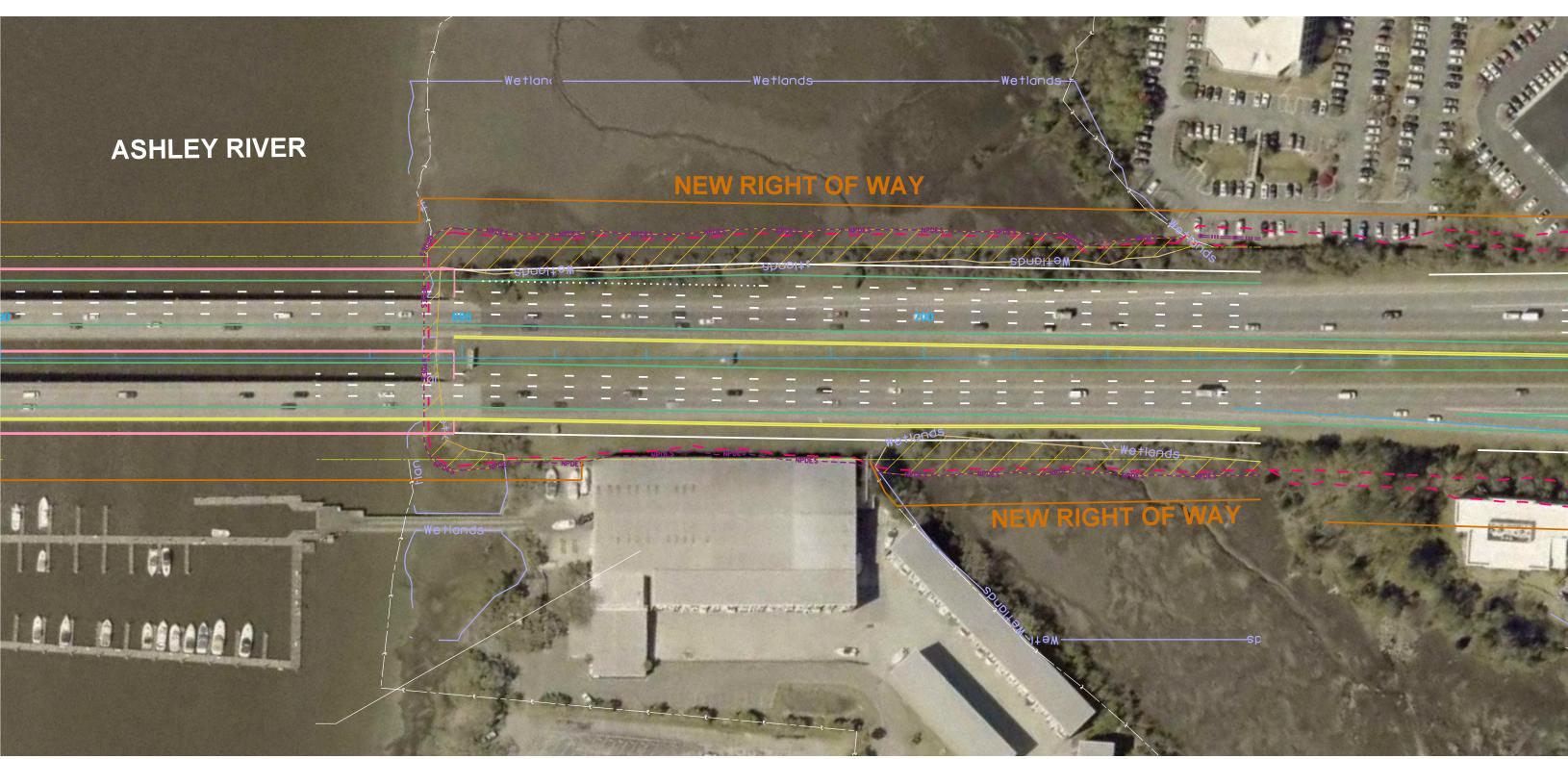






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 4B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
NORTH CHARLESTON APPROACH

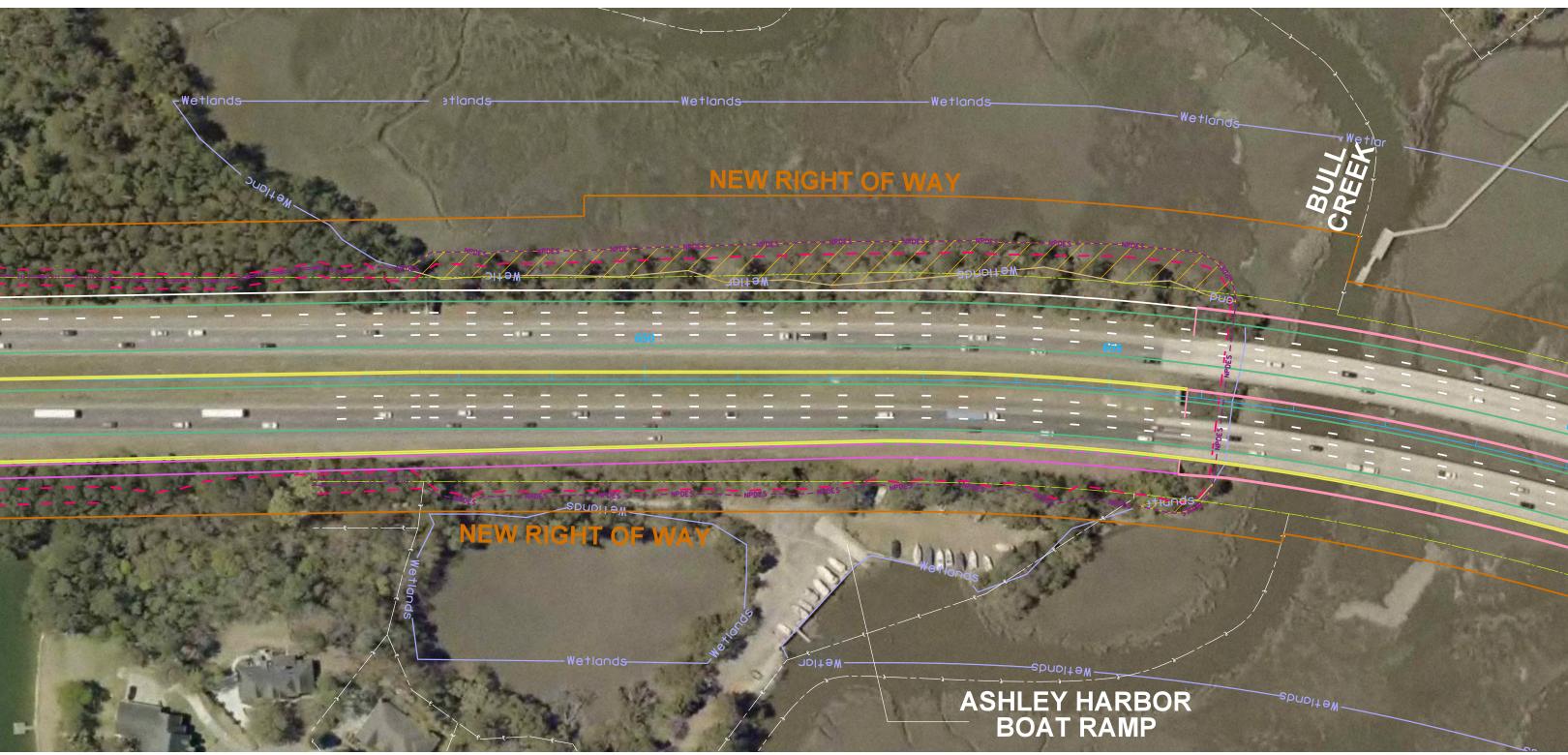






I-526 ASHLEY RIVER BRIDGE SUP WIDENING - OPTION 4B

WIDEN BRIDGES DOWNSTREAM (EAST) / SUP UPSTREAM (WEST)
WEST ASHLEY APPROACH









I-526 ASHLEY RIVER BRIDGE WIDENING AND SUP OPTIONS PRELIMINARY COST ESTIMATES

ESTIMATED COSTS	OPTION 1A WIDEN DOWNSTREAM / SUP UPSTREAM	OPTION 2A WIDEN UPSTREAM / SUP UPSTREAM	OPTION 3A WIDEN DOWNSTREAM / SUP DOWNSTREAM	OPTION 4A WIDEN UPSTREAM / SUP DOWNSTREAM
CONSTRUCTION COSTS	\$80,460,493.55	\$73,385,361.89	N/A	\$80,650,421.43
ENVIRONMENTAL MITIGATION COSTS	\$1,227,077.31	\$2,121,677.84	N/A	\$2,614,908.24
RIGHT OF WAY COSTS	\$3,885,700.00	\$4,135,365.00	N/A	\$4,495,530.00
TOTAL COST*	\$85,573,270.86	\$79,642,404.73	\$0.00	\$87,760,859.67

ESTIMATED COSTS	OPTION 1B WIDEN DOWNSTREAM / SUP UPSTREAM	OPTION 2B WIDEN UPSTREAM / SUP UPSTREAM	OPTION 3B WIDEN DOWNSTREAM / SUP DOWNSTREAM	OPTION 4B WIDEN UPSTREAM / SUP DOWNSTREAM
CONSTRUCTION COSTS	\$80,518,032.11	\$73,444,050.78	\$75,467,677.22	\$80,535,120.60
ENVIRONMENTAL MITIGATION COSTS	\$1,692,302.63	\$2,580,018.68	\$1,036,528.35	\$1,789,137.51
RIGHT OF WAY COSTS	\$4,058,047.50	\$4,284,605.00	\$3,907,020.00	\$4,354,610.00
TOTAL COST*	\$86,268,382.24	\$80,308,674.46	\$80,411,225.57	\$86,678,868.11

^{*}utility costs would be constant across all options and predominantly consist of SCDOT owned ITS fiber in median

I-526 ASHLEY RIVER WIDENING OPTIONS PRELIMINARY BRIDGE AND WALL CONSTRUCTION COSTS⁺

BRIDGE, WALL AND ROADWAY ITEMS	OPTION 1A WIDEN DOWNSTREAM / SUP UPSTREAM		OPTION 2A WIDEN UPSTREAM / SUP UPSTREAM		OPTION 3A WIDEN DOWNSTREAM / SUP DOWNSTREAM	OPTION 4A WIDEN UPSTREAM / SUP DOWNSTREAM	
NEW BRIDGE DECK (\$175/s.f.)	\$	65,093,000.00	\$	65,093,000.00	N/A	\$	65,093,000.00
BRIDGE WIDENING PREP. 3908' x \$1000.00/l.f. = \$4,000,000	\$	11,723,000.00	\$	7,815,000.00	N/A	\$	11,723,000.00
WALL COSTS (\$43.00/s.f.) Average 5' height	\$	102,125.00	\$	-	N/A	\$	-
Addl Const. Costs*	\$	3,076,725.00	\$	-	N/A	\$	3,076,725.00
SUP CONCRETE PAVEMENT (\$5.55/s.f.)	\$	129,842.21	\$	135,924.71	N/A	\$	408,538.74
SUP CONCRETE MEDIAN (\$70/I.f.)	\$	335,801.34	\$	341,437.18	N/A	\$	349,157.69
TOTAL	\$	80,460,493.55	\$	73,385,361.89	\$ -	\$	80,650,421.43

^{*}Cost per s.f new deck increased to \$210/s.f. = 20% increase. Costs attributed to 1) additional construction phase 2) added trestle each side 3) 6 months longer construction period due to additional phase.

I-526 ASHLEY RIVER BRIDGE WIDENING OPTIONS PRELIMINARY ENVIRONMENTAL MITIGATION COSTS

_		am, SUP Upstream	ownstre	Bridge D	en Each	OPTION 1A - Wid
15% increase*	Total Cost	Unit Cost (current rate)	Est Credit Need	Mitigation Factor	Total Impacts	Wetlands
\$869,809	\$756,356.25	\$65,000.00	11.63625	14.5	0.80	Critical Area Wetland Fill (acre)
\$357,267.	\$310,667.50	\$65,000.00	4.7795	11	0.43	Critical Area Wetland Clearing (acre)
\$1,227,077.	\$1,067,023.75	_				
		am, SUP Upstream	ownstre	Bridge D	en Each	OPTION 1B - Wid
15% increase*	Total Cost	Unit Cost (current rate)	Est Credit Need	Mitigation Factor	Total Impacts	Wetlands
<u>I</u> \$1,230,198	\$1,069,737.50	\$65,000.00	16.4575	14.5	1.14	Critical Area Wetland Fill (acre)
\$1,230,198 \$462,104.	\$401,830.00	\$65,000.00	6.182	11	0.56	Critical Area Wetland Clearing (acre)
\$1,692,302.	\$1,471,567.50		0.102	11	0.50	childa Area Welland Cicaring (acre)
		m, SUP Upstream	linstrea	h Bridge	den Fac	OPTION 2A - Wi
Ī		in, sor opstream		ii biiage	l Luc	OI HOIL EA WI
15% increase*	Total Cost	Unit Cost (current rate)	Est Credit Need	Mitigation Factor	Total Impacts	Wetlands
\$1,764,656	\$1,534,484.25	\$65,000.00	23.60745	14.5	1.63	Critical Area Wetland Fill (acre)
\$357,020	\$310,453.00	\$65,000.00	4.7762	11	0.43	Critical Area Wetland Clearing (acre)
		m, SUP Upstream	Upstrea	h Bridge	den Eac	OPTION 2B - Wi
			Est Credit	Mitigation	Total	
15% increase*	Total Cost	Unit Cost (current rate)	Need	Factor	Impacts	Wetlands
15% increase* \$2,115,940	Total Cost \$1,839,948.50	Unit Cost (current rate) \$65,000.00		_	Impacts 1.95	
l		, ,	Need	Factor	·	Critical Area Wetland Fill (acre)
[\$2,115,940	\$1,839,948.50	\$65,000.00	Need 28.3069	Factor 14.5	1.95	Critical Area Wetland Fill (acre)
[\$2,115,940 \$464,077	\$1,839,948.50 \$403,546.00 \$2,243,494.50	\$65,000.00	Need 28.3069 6.2084	14.5 11	1.95 0.56	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre)
[\$2,115,940 \$464,077	\$1,839,948.50 \$403,546.00 \$2,243,494.50	\$65,000.00 \$65,000.00	Need 28.3069 6.2084	14.5 11	1.95 0.56	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre)
\$2,115,940 \$464,077 \$2,580,018 .	\$1,839,948.50 \$403,546.00 \$2,243,494.50	\$65,000.00 \$65,000.00 _	Need 28.3069 6.2084 /nstrean Est Credit	14.5 11 idge Dow	1.95 0.56 Each Br	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands
\$2,115,940 \$464,077 \$2,580,018.	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost	\$65,000.00 \$65,000.00	28.3069 6.2084 /nstream Est Credit Need	Factor 14.5 11 idge Dow Mitigation Factor	1.95 0.56 Each Br Total Impacts	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre)
\$2,115,940 \$464,077 \$2,580,018. 15% increase* \$839,352	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost \$729,872.00	\$65,000.00 \$65,000.00 n, SUP Downstream Unit Cost (current rate) \$65,000.00	28.3069 6.2084 //nstream Est Credit Need 11.2288	Factor 14.5 11 idge Dow Mitigation Factor 14.5	1.95 0.56 Each Br Total Impacts	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre)
\$2,115,940 \$464,077 \$2,580,018. 15% increase* \$839,352 \$197,175.	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost \$729,872.00 \$171,457.00	\$65,000.00 \$65,000.00 n, SUP Downstream Unit Cost (current rate) \$65,000.00	28.3069 6.2084 /nstrean Est Credit Need 11.2288 2.6378	idge Dov Mitigation Factor	1.95 0.56 Each Br Total Impacts 0.77 0.24	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre)
\$2,115,940 \$464,077 \$2,580,018. 15% increase* \$839,352 \$197,175.	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost \$729,872.00 \$171,457.00	\$65,000.00 \$65,000.00	28.3069 6.2084 /nstrean Est Credit Need 11.2288 2.6378	idge Dov Mitigation Factor	1.95 0.56 Each Br Total Impacts 0.77 0.24	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre)
\$2,115,940 \$464,077 \$2,580,018. 15% increase* \$839,352 \$197,175. \$1,036,528.	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost \$729,872.00 \$171,457.00 \$901,329.00	\$65,000.00 \$65,000.00	Need 28.3069 6.2084 /nstrean Est Credit Need 11.2288 2.6378 pstream Est Credit	Factor 14.5 11 idge Dow Mitigation Factor 14.5 11 Bridge U Mitigation	1.95 0.56 Each Br Total Impacts 0.77 0.24 en Each Total	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 4A - Wid Wetlands
\$2,115,940 \$464,077 \$2,580,018. 15% increase* \$839,352 \$197,175. \$1,036,528.	\$1,839,948.50 \$403,546.00 \$2,243,494.50 Total Cost \$729,872.00 \$171,457.00 \$901,329.00	\$65,000.00 \$65,000.00	28.3069 6.2084 /nstrean Est Credit Need 11.2288 2.6378 pstream Est Credit Need	Factor 14.5 11 idge Dov Mitigation Factor 14.5 11 Bridge U Mitigation Factor	1.95 0.56 Each Br Total Impacts 0.77 0.24 en Each Total Impacts	Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 3 - Widen Wetlands Critical Area Wetland Fill (acre) Critical Area Wetland Clearing (acre) OPTION 4A - Wid

^{*}Shown to account for possibility of rise in mitigation credit costs from time of estimation to time of purchase.

I-526 ASHLEY RIVER BRIDGE WIDENING OPTIONS PRELIMINARY RIGHT OF WAY IMPACT COSTS AT BRIDGE APPROACHES

OPTION 1A - Widen Each Bridge Downstream, SUP Upstream								
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	14	11.96	\$ 325,000.00	\$3,885,700.00				
OPTION 1B - Widen Each Bridge Downstream, SUP Upstream								
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	15	12.49	\$ 325,000.00	\$4,058,047.50				
ОРТІС	OPTION 2A - Widen Each Bridge Upstream, SUP Upstream							
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	12	12.72	\$ 325,000.00	\$4,135,365.00				
OPTION 2B - Widen Each Bridge Upstream, SUP Upstream								
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	12	13.18	\$ 325,000.00	\$4,284,605.00				
OPTION	3B - Widen Each Brid	lge Downstr	eam SLIP Dow	ınstroam				
01 11014	JD - Wideli Lacii Diic	ige Downsti		iisti caiii				
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	15	12.02	\$ 325,000.00	\$3,907,020.00				
OPTIO	OPTION 4A - Widen Each Bridge Upstream, SUP Downstream							
OFFICE	V 4A - Wideli Lacii Bi	iuge Opstre	Jili, 30F DOWII	stream				
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	15	13.83	\$ 325,000.00	\$4,495,530.00				
OPTIOI	OPTION 4B - Widen Each Bridge Upstream, SUP Downstream							
Right of Way	Number of Properties	ACRES	UNIT COST	TOTAL COST				
Regular Take	15	13.40	\$ 325,000.00	\$4,354,610.00				